# CITY OF ALBUQUERQUE

*Planning Department* Brennon Williams, Director



Mayor Timothy M. Keller

December 17, 2019

J. Graeme Means, P.E. High Mesa Consulting Group 6010 B Midway Park Blvd NE Albuquerque, NM 87109

#### RE: International District Library 7667 Central NE Grading and Drainage Plan Stamp Date: 12/11/19 Hydrology File: K19D005

Dear Mr. Means,

Based on the submittal received on 12/12/19, this project is approved for Building Permit.

PO Box 1293 Prior to Certificate of Occupancy (For Information):

Albuquerque

NM 87103

1. Engineer's Certification, per the DPM Chapter 22.7: *Engineer's Certification Checklist For Non-Subdivision* is required.

2. The public work order will need to be closed out and accepted by the City, unless a financial guarantee has been posted.

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

www.cabq.gov

Sincerely,

Dana Peterson, P.E. Senior Engineer, Planning Dept. Development Review Services

# GENERAL NOTES

- 1. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR APPROVED FOR HEREON. BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS-PUBLIC WORKS CONSTRUCTION-1986-UPDATE NO. 9.
- 2. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM, 811, FOR DESIGNATION (LINE-SPOTTING) OF EXISTING PUBLIC UTILITIES.
- 3. UTILITY INFORMATION SHOWN HEREON IS BASED UPON ONSITE SURFACE EVIDENCE, REVIEW OF AVAILABLE ABCWUA AND CITY OF ALBUQUERQUE RECORD DRAWINGS AND DISTRIBUTION MAPS AND UTILITY LINE-SPOTS PROVIDED BY HIGH MESA CONSULTING GROUP (2018.017.1). IN ADDITION, UTILITY LINE-SPOTS WERE REQUESTED VIA THE NEW MEXICO ON CALL SERVICE (TICKET NO. 18MA160146). UTILITY LINES THAT APPEAR ON THESE DRAWINGS ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION. DEPTH. SIZE, OR TYPE OF EXISTING UTILITY LINES. PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- 4. SHOULD A CONFLICT EXIST BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY FOR ALL PARTIES.
- 5. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ADJACENT PROPERTIES DURING CONSTRUCTION.
- LAWS, RULES AND REGULATIONS CONCERNING SAFETY AND HEALTH.
- 7. ALL UTILITIES AND UTILITY SERVICE LINES SHALL BE INSTALLED PRIOR TO PAVING. 8. BACKFILL COMPACTION SHALL BE ACCORDING TO SPECIFIED STREET USE OR PER COA STANDARD DRAWING 2465,
- WHICHEVER IS MORE STRINGENT. 9. TACK COAT REQUIREMENTS SHALL BE DETERMINED DURING CONSTRUCTION BY THE PROJECT ENGINEER.
- 10. SIDEWALKS AND WHEELCHAIR RAMPS WITHIN THE CURB RETURNS SHALL BE CONSTRUCTED WHEREVER A NEW CURB RETURN IS CONSTRUCTED
- 11. IF CURB IS DEPRESSED FOR A DRIVEPAD OR A HANDICAP RAMP, THE DRIVEPAD OR RAMP SHALL BE CONSTRUCTED PRIOR TO ACCEPTANCE OF THE CURB AND GUTTER.
- 12. ALL STORM DRAINAGE FACILITIES SHALL BE COMPLETED PRIOR TO FINAL ACCEPTANCE. 13. THE CONTRACTOR SHALL COORDINATE WITH THE WATER AUTHORITY SEVEN (7) DAYS IN ADVANCE OF PERFORMING WORK THAT WILL AFFECT THE PUBLIC WATER OR SANITARY SEWER INFRASTRUCTURE. WORK REQUIRING SHUTOFF OF FACILITIES DESIGNATED AS MASTER PLAN FACILITIES MUST BE COORDINATED WITH THE WATER AUTHORITY 14 DAYS IN ADVANCE OF PERFORMING SUCH WORK. ONLY WATER AUTHORITY CREWS ARE AUTHORIZED TO OPERATE PUBLIC VALVES. SHUTOFF
- REQUESTS MUST BE MADE ONLINE AT http://www.abcwua.org/Water\_Shut\_off\_and\_Turn\_on\_Procedures.aspx. 14. CONTRACTOR SHALL NOTIFY THE CITY SURVEYOR NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT THE CITY SURVEYOR MAY TAKE NECESSARY MEASURES TO INSURE THÉ PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT THE CONSENT OF THE CITY SURVEYOR AND SHALL NOTIFY THE CITY SURVEYOR AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED WITHOUT PERMISSION. REPLACEMENT SHALL BE DONE ONLY BY THE CITY SURVEYOR. WHEN A CHANGE IS MADE IN THE FINISHED ELEVATION OF THE PAVEMENT OF ANY ROADWAY IN WHICH A PERMANENT SURVEY MONUMENT IS LOCATED CONTRACTOR SHALL, AT HIS OWN EXPENSE, ADJUST THE MONUMENT COVER TO THE NEW GRADE UNLESS OTHERWISE SPECIFIED. REFER TO SECTION 4.4 OF THE SPECIFICATIONS.
- 15. SEVEN (7) WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION THE CONTRACTOR SHALL SUBMIT TO THE CONSTRUCTION COORDINATION DIVISION A DETAILED CONSTRUCTION SCHEDULE. TWO (2) WORKING DAYS PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL OBTAIN A BARRICADING PERMIT FROM THE CONSTRUCTION COORDINATION DIVISION. CONTRACTOR SHALL NOTIFY BARRICADE ENGINEER (924-3400) PRIOR TO OCCUPYING AN INTERSECTION. CONTRACTOR MUST REFER TO
- SECTION 19 OF THE STANDARD SPECIFICATION FOR TRAFFIC CONTROL. 16. TWO WEEKS PRIOR TO CONSTRUCTION, THE CONTRACTOR SHOULD NOTIFY THE TRANSIT DEPARTMENT OF ANY IMPACT THE PROPOSED PROJECT WILL HAVE ON THE TRANSIT SYSTEM SUCH AS CAUSING DETOUR, OR CAUSE THE CLOSING OR RELOCATION OF A BUS STOP. THE CONTACT PERSON IS DOUGLAS GOFF, OFFICE PHONE 505-724-3137, CELL PHONE 505-206-0151, AND EMAIL DGOFF@CABQ.GOV.
- 17. ALL STREET STRIPING ALTERED OR DESTROYED SHALL BE REPLACED WITH PLASTIC REFLECTORIZED STRIPING BY CONTRACTOR TO EXISTING LOCATION OR AS INDICATED BY THIS PLAN SET. 18. CAUTION: THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY WHICH SHALL
- CARRIED-OUT IN ACCORDANCE WITH OSHA 29 CFR 1926, SUBPART P-EXCAVATIONS 19. ANY WORK OCCURRING WITHIN AN ARTERIAL ROADWAY MAY REQUIRE TWENTY-FOUR HOUR CONSTRUCTION.
- GRAFFITI FROM EQUIPMENT, WHETHER PERMANENT OR TEMPORARY. 21. WHEN APPLICABLE, CONTRACTOR SHALL, ON BEHALF OF THE OWNER AND OPERATORS, SECURE "TOPSOIL DISTURBANCE
- PERMIT" FROM THE CITY AND/OR FILE A NOTICE OF INTENT (N.O.I.) WITH THE EPA PRIOR TO BEGINNING CONSTRUCTION. 22. CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING AND MAINTAINING ALL CONSTRUCTION SIGNING UNTIL THE PROJECT HAS BEEN ACCEPTED BY THE CITY OF ALBUQUERQUE.
- 23. ALL FILL SHALL BE CLEAN, FREE FROM VEGETATION, DEBRIS, AND OTHER DELETERIOUS MATERIALS, AND SHALL NOT BE
- CONTAMINATED WITH HYDROCARBONS OR OTHER CHÉMICAL CONTAMINANTS. 24. CONTRACTOR SHALL REFER TO GEOTECHNICAL REPORT FOR EARTHWORK REQUIREMENTS, AS APPLICABLE.
- 25. CONTRACTOR SHALL TEST SUBGRADE R-VALUE PRIOR TO CONSTRUCTION. IN THE EVENT THE R-VALUE IS LESS THAN 50, CONTRACTOR SHALL REMOVE 2 FT. OF SUBGRADE MATERIAL AND IMPORT SUITABLE MATERIAL WITH R-VALUE 50 OR GRFATER

### CONSTRUCTION NOTES

- 1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM, 811, FOR DESIGNATION (LINE-SPOTTING) OF EXISTING PUBLIC UTILITIES.
- 2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL INTERPRETATIONS IT MAKES WITHOUT FIRST CONTACTING THE ENGINEER AS REQUIRED ABOVE
- 3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH. 4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF
- ALBUQUERQUE STANDARDS AND PROCEDURES. 5. UTILITY INFORMATION SHOWN HEREON IS BASED UPON ONSITE SURFACE EVIDENCE, REVIEW OF AVAILABLE ABCWUA AND CITY OF ALBUQUERQUE RECORD DRAWINGS AND DISTRIBUTION MAPS AND UTILITY LINE-SPOTS PROVIDED BY HIGH MESA CONSULTING GROUP (2018.017.1). IN ADDITION, UTILITY LINE-SPOTS WERE REQUESTED VIA THE NEW MEXICO ON CALL SERVICE (TICKET NO. 18MA160146). UTILITY LINES THAT APPEAR ON THESE DRAWINGS ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE. PIPELINE. OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES. PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- 6. THE DESIGN OF PLANTERS AND LANDSCAPED AREAS IS NOT PART OF THIS PLAN. ALL PLANTERS AND LANDSCAPED AREAS ADJACENT TO THE BUILDING(S) SHALL BE PROVIDED WITH POSITIVE DRAINAGE TO AVOID ANY PONDING ADJACENT TO THE STRUCTURE. FOR CONSTRUCTIÓN DETAILS, REFER TO LANDSCAPING PLAN.
- ON THE PLANS INCLUDING, BUT NOT LIMITED TO, SURFACE DRAINAGE STRUCTURES, PAVING AND LANDSCAPING SURFACING.
- 9. BACKFILL COMPACTION SHALL BE ACCORDING TO ARTERIAL STREET USE.
- APPLICABLE 11. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.

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6. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL

REMAIN THE RESPONSIBILITY OF THE CONTRACTOR. ALL EXCAVATION, TRENCHING AND SHORING ACTIVITIES MUST BE

20. CONTRACTOR SHALL MAINTAIN A GRAFFITI-FREE WORK SITE. CONTRACTOR SHALL PROMPTLY REMOVE ANY AND ALL

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7. THE GRADES INDICATED ON THIS PLAN ARE FINISHED GRADES UNLESS OTHERWISE INDICATED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LEAVING SUBGRADE AT ELEVATIONS THAT SHALL ACCOMMODATE PROPOSED IMPROVEMENTS AS INDICATED 8. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY. AN APPROVED COPY OF THESE PLANS MUST BE SUBMITTED AT THE TIME OF APPLICATION FOR THIS PERMIT.

10. CONTRACTOR SHALL REFER TO GEOTECHNICAL REPORT AND/OR STRUCTURAL FOR EARTHWORK REQUIREMENTS, AS

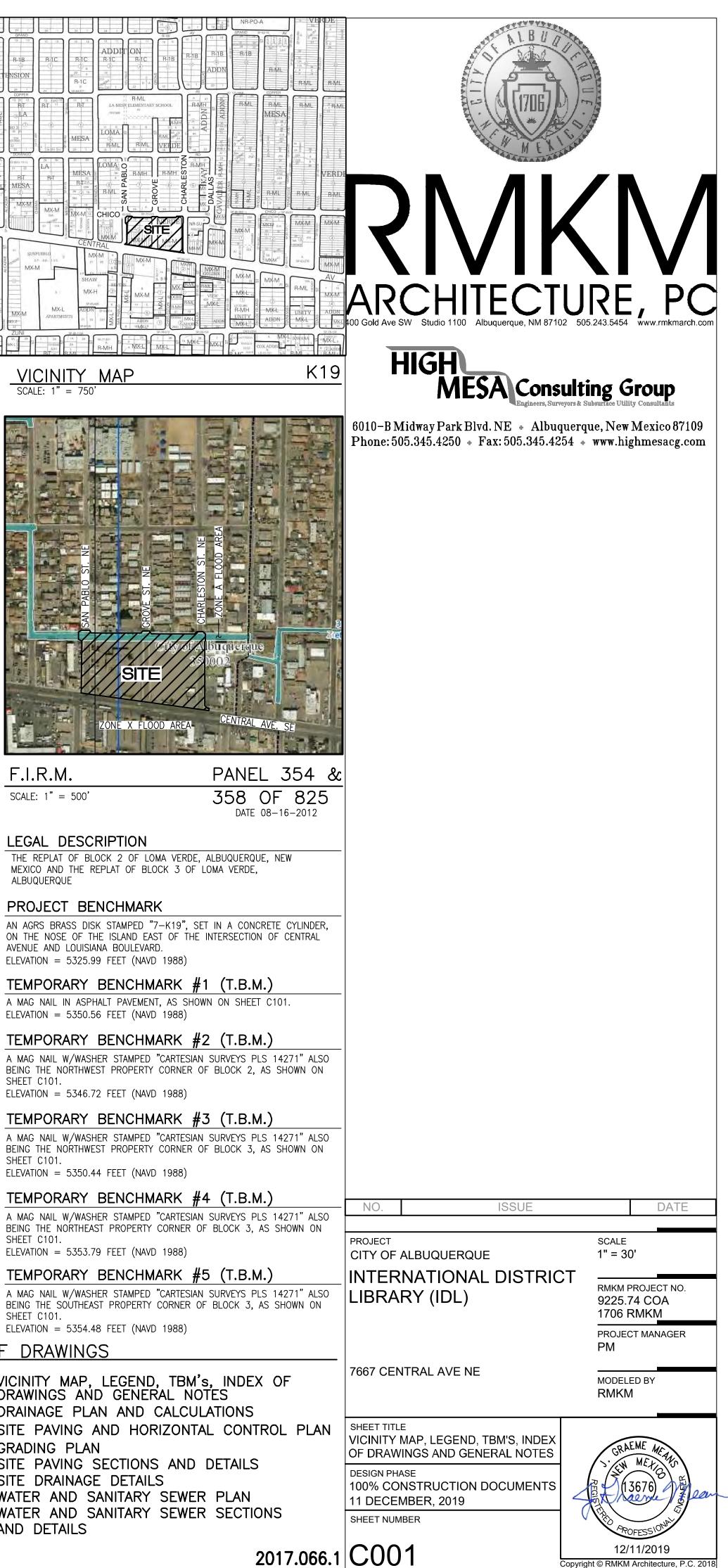
| RV             | WATER AIR RELEASE VALVE                         | Ô                   |
|----------------|---|---------------------|
| SPH            |   |                     |
| ЮН             | BUILDING OVERHANG<br>COMMUNICATION              | igodol              |
| &G             | CURB AND GUTTER                                 | σ                   |
| /PM            | COMMUNICATION BY PAINT MARK                     | <b>\</b>            |
| C              | CONCRETE CURB                                   |                     |
| CND<br>CP      | COMMUNICATION CONDUIT<br>CONCRETE CYLINDER PIPE | $\bigcirc$          |
| :DP            | CONCRETE CTLINDER PIPE                          | •                   |
| :              | CONCRETE ISLAND                                 | _                   |
| HC             | CONCRETE HEADER CURB                            | $\bowtie$           |
| SLF<br>SONC    | CHAINLINK FENCE<br>CONCRETE                     |                     |
| SW             | CONCRETE SIDEWALK                               | $\sim$              |
| /PM            | ELECTRIC BY PAINT MARK                          | 00                  |
| A              | EDGE OF ASPHALT                                 |                     |
| CND<br>H       | ELECTRIC CONDUIT<br>FIRE HYDRANT                | <u></u> -           |
| Ľ              | FLOWLINE  | -                   |
| /PM            | GAS BY PAINT MARK                               |                     |
| Â              | GATE  | 0-                  |
| iRV<br>W       | GRAVEL<br>GUY WIRE                              | _                   |
| IC             | HANDICAP PARKING SPACE                          | 0-                  |
| ICS            | HANDICAP SIGN                                   | SD·                 |
| ₩<br>SD        |   |                     |
| SD<br>ICG      | LANDSCAPE DIVIDER<br>MEDIAN CURB AND GUTTER     |                     |
| IED            | MEDIAN  | W-                  |
| IH             | MANHOLE   | W-                  |
| ILP            | METAL LIGHT POLE                                |                     |
| IS<br>ITC      | METAL SIGN<br>METAL TRASH CAN                   | SAS                 |
| HC(2)          | OVERHEAD COMMUNICATION (# OF LINES)             | ——SAS               |
| )HE(2)         | OVERHEAD ELECTRIC (# OF LINES)                  | _                   |
| 4              | PAINTED ISLAND                                  |                     |
| rS<br>rVC      | PARKING STRIPE<br>POLYVINYL CHLORIDE            | F                   |
| SAS            | SANITARY SEWER                                  |                     |
| GP             | STEEL GUARD POST                                |                     |
| P              | STEEL POLE                                      |                     |
| STB<br>SW      | STOP BAR<br>SIDEWALK                            | INV                 |
| WC             | SIDEWALK CULVERT                                | TA                  |
| A              | TOP OF ASPHALT                                  |                     |
| С<br>ГС        | TOP OF CURB                                     | TC                  |
| CO<br>RC       | TOP OF CONCRETE<br>TRASH CAN                    | TG                  |
| S              | TRAFFIC SIGN                                    | + 202               |
| YP             | TYPICAL   | • 02.               |
| 'G<br>'CP      | VALLEY GUTTER<br>VITRIFIED CLAY PIPE            | 1                   |
| 'RT            | VERTICAL RAILROAD TIES                          |                     |
| /CR            | WHEELCHAIR RAMP                                 |                     |
| /L             | WATERLINE                                       |                     |
| /L/PM          | WATERLINE BY PAINT MARK                         | 90                  |
| /L/RCD<br>/MB  | WATERLINE BY RECORD DRAWING<br>WATER METER BOX  | $\triangleleft$     |
| /PP            | WOOD POWER POLE                                 |                     |
| /PP/C          | WOOD POWER POLE WITH CONDUIT                    |                     |
| /PP/L          | WOOD POWER POLE WITH STREET LIGHT               |                     |
| IVB<br>IS      | WATER VALVE BOX<br>WHEEL STOP                   |                     |
|                |   |                     |
| *              | PAINTED UTILITY MARKER                          | $\downarrow$        |
|                |   |                     |
| .2'ø           | DIAMETER OF TREE                                |                     |
|                |   |                     |
| VL             |   | 0.0.000.000.000.000 |
| K              | DECIDUOUS TREE                                  |                     |
|                |   |                     |
| ¥              | SMALL DECIDUOUS TREE                            |                     |
| 71             |   |                     |
| M2             |   |                     |
| · <del>`</del> | CONIFEROUS TREE                                 |                     |
| 'V N           |   |                     |
| WL.            |   |                     |
|                | SMALL CONIFEROUS TREE                           |                     |
| 524            |   |                     |
| کر             | SHRUB   |                     |
| <b>*</b>       | YUCCA   |                     |
| ~              |   |                     |
| $\supset$      | LANDSCAPE ROCK/BOULDER                          |                     |
|                |   |                     |

| $\odot$         | EXISTING STORM DRAIN MANHOLE           |
|-----------------|--|
| $oldsymbol{O}$  | PROPOSED STORM DRAIN MANHOLE           |
| σ               | EXISTING FIRE HYDRANT                  |
| <b>¥</b>        | PROPOSED FIRE HYDRANT                  |
| $\bigcirc$      | EXISTING SANITARY SEWER MAN HOLE       |
| •               | SANITARY SEWER MAN HOLE                |
| $\bowtie$       | EXISTING VALVE BOX                     |
|                 | PROPOSED VALVE BOX                     |
|                 | EXISTING DOUBLE CLEANOUT               |
|                 | PROPOSED DOUBLE CLEANOUT               |
|                 | EXISTING SINGLE CLEANOUT               |
| <b>●</b>        | PROPOSED SINGLE CLEANOUT               |
| 0               | EXISTING WATER SERVICE                 |
|                 | PROPOSED WATER SERVICE                 |
| SD              | EXISTING STORM DRAIN                   |
|                 | PROPOSED STORM DRAIN                   |
| W               | EXISTING WATER LINE                    |
|                 | PROPOSED WATER LINE                    |
|                 | EXISTING SANITARY SEWER LINE           |
|                 |  |
| SAS             | PROPOSED SANITARY SEWER LINE           |
|                 | EXISTING FIRE LINE                     |
|                 | PROPOSED FIRE LINE                     |
| +×4             | EXISTING POST INDICATOR VALVE          |
|                 | PROPOSED POST INDICATOR VALVE          |
| INV             |  |
| TA<br>TC        | TOP OF ASPHALT PAVEMENT<br>TOP OF CURB |
| TG              | TOP OF GRATE                           |
| + 202.11        | EXISTING SPOT ELEVATION                |
| • 02.00         | PROPOSED SPOT ELEVATION                |
| <u> </u>        | EXISTING FLOWLINE                      |
| <b>_</b>        | PROPOSED FLOWLINE                      |
|                 | EXISTING CONTOUR                       |
| <b>——90</b> ——  | PROPOSED CONTOUR                       |
| $\triangleleft$ | EXISTING DIRECTION OF FLOW             |
| ←──             | PROPOSED DIRECTION OF FLOW             |
|                 | RIGHT OF WAY LINE                      |
|                 | PUBLIC EASEMENT LINE                   |
|                 | high point / divide                    |
|                 | PROPOSED CONCRETE PAVING               |
|                 | PROPOSED ASPHALT PAVING                |
|                 |  |
|                 |  |
|                 |  |

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| INDEX | OF   |
|-------|------|
| C001  | VICI |
| C002  | DRA  |
| C100  | SITE |
| C101  | GRA  |
| C102  | SITE |
| C103  | SITE |
| C104  | WAT  |
| C105  | WAT  |
|       | AND  |
|       |      |

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#### I. INTRODUCTION AND EXECUTIVE SUMMARY

THIS SITE IS LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF CENTRAL AVENUE NE AND SAN PABLO STREET NE IN ALBUQUERQUE, NEW MEXICO. THIS PROJECT REPRESENTS A REDEVELOPMENT OF A PREVIOUSLY DEVELOPED SITE WITHIN AN INFILL AREA. THE EXISTING SITE 'CARAVAN CLUB' BUILDING IMPROVEMENTS HAVE BEEN COMPLETELY DEMOLISHED AND REMOVED (BY SEPARATE 2018 PROJECT) RESULTING IN A GENERALLY OPEN SITE WITH PAVED AND UNPAVED SURFACES AND MINIMAL LANDSCAPING. THE PROPOSED DEVELOPMENT FOR THE SITE IS TO CONSTRUCT A NEW PUBLIC LIBRARY WITH ASSOCIATED PAVED PARKING AND COURTYARD IMPROVEMENTS, AS WELL AS ASSOCIATED NEW LANDSCAPING WITHIN THE WESTERN PORTION OF THE SITE. THE EASTERN PORTION OF THE SITE WILL REMAIN UNDEVELOPED AT THIS TIME AND THERE ARE TENTATIVE PLANS FOR THE CITY AND AMAFCA TO DEVELOP IT AS A PUBLIC DRAINAGE FACILITY TO HELP ALLEVIATE DOWNSTREAM FLOODING.

THIS DRAINAGE PLAN ADDRESSES THE DRAINAGE CONCEPTS AND IMPROVEMENTS PROPOSED FOR DEVELOPMENT OF THE NEW LIBRARY SITE, AS WELL AS THE PROPOSED ONSITE STORMWATER WATER HARVESTING / FIRST FLUSH CAPTURE REQUIRED TO PARTIALLY MEET THE DEVELOPED 100-YEAR STORM EVENT DISCHARGE AND FIRST FLUSH REQUIREMENTS BY THE CITY OF ALBUQUERQUE. THIS PLAN IS SUBMITTED IN SUPPORT OF BUILDING PERMIT APPROVAL BY THE CITY OF ALBUQUERQUE. A PREVIOUS SUBMITTAL ADDRESSED THE VACATION, REPLAT, AND WORK ORDER ASPECTS OF THE PROJECT.

#### II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP, THE SITE IS BOUNDED BY CENTRAL AVENUE TO THE SOUTH, SAN PABLO STREET NE TO THE WEST, CHICO ROAD NE TO THE NORTH, AND CHARLESTON STREET NE TO THE EAST. THE SITE CONSISTS OF TWO LOTS (BLOCKS 2 AND 3, LOMA VERDE SUBDIVISION), AND GROVE STREET NE, WHICH CURRENTLY BISECTS THE SITE. A SEPARATE, CONCURRENT PLATTING ACTION IS IN PROGRESS TO VACATE GROVE STREET RIGHT-OF-WAY AND COMBINE THE SITE INTO ONE LOT. ALL PERIMETER STREETS REFERENCED ABOVE ARE FULLY DEVELOPED PUBLIC STREETS WITH CURB AND GUTTER AND SIDEWALKS. THIS SITE IS OWNED AND OPERATED BY THE CITY OF ALBUQUERQUE. AS INDICATED BY PANELS 354 AND 358 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, DATED AUGUST 16, 2012, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE. THE SITE DOES LIE IMMEDIATELY ADJACENT TO A ZONE 'A; DESIGNATED FLOOD HAZARD ZONE ASSOCIATED WITH THE CHICO ROAD NE PUBLIC RIGHT OF WAY, WHERE THE 1% ANNUAL CHANCE FLOOD DISCHARGE IS CONTAINED WITHIN THE PUBLIC RIGHT OF WAY. THE SITE IS CURRENTLY PERMITTED FREE DISCHARGE TO THE ADJACENT PUBLIC STREETS BASED ON HISTORIC CONDITIONS AND PREVIOUSLY APPROVED PLANS.

#### III. BACKGROUND DOCUMENTS & RESEARCH

THE PREPARATION OF THIS PLAN RELIED UPON THE FOLLOWING DOCUMENTS:

- TOPOGRAPHIC AND UTILITY SURVEY PREPARED BY HIGH MESA CONSULTING GROUP (NMPS 11184) DATED 04/30/2018. THIS REFERENCED SURVEY PROVIDES THE BASIS FOR THE EXISTING CONDITIONS OF THE SITE. • SITE DRAINAGE PLAN PREPARED BY CONSULTANTS, INC. NMPE 6653, DATED OCTOBER, 1979. THIS REFERENCED DRAINAGE PLAN CONTINUED THE APPROVED
- EXISTING DRAINAGE CONCEPT OF DISCHARGE OF EXISTING RUNOFF FROM THE SITE TO THE ADJACENT PUBLIC STREETS. • SITE DEMOLITION PLAN PREPARED BY CHERRY SEE REAMES ARCHITECTURE AND TETSUDO ENGINEERING (NMPE 23914), DATED JUNE 19, 2017. THIS
- REFERENCED PLAN DEMOLISHED ALL EXISTING BUILDINGS ON THE TWO PROJECT LOTS AND REMOVAL OF ALL EXISTING UTILITY SERVICES TO THE EXISTING PROPERTY BOUNDARIES. PORTIONS OF THE PAVED PARKING LOT WERE REMOVED DURING THE DEMOLITION; HOWEVER, THE MAJORITY OF THE PAVED PARKING STILL REMAINS BASED ON THE 2018 SURVEY REFERENCED ABOVE.

#### IV. EXISTING CONDITIONS

THE PROJECT SITE CONSISTS OF TWO EXISTING LOTS (BLOCKS 2 AND 3, LOMA VERDE) SEPARATED BY GROVE STREET. THE WEST LOT IS MOSTLY PAVED PARKING LOT, WITH UNPAVED DIRT AND GRAVEL PORTIONS WHERE THE PREVIOUS 'CARAVAN CLUB' BUILDING AND ASSOCIATED UTILITIES WERE DEMOLISHED IN 2018. MINIMAL LANDSCAPED AREAS ARE LOCATED ON THE SOUTH SIDE OF THE SITE. THE EASTERN PORTION IS SIMILAR, WITH A SMALL UNPAVED PATCH AT THE LOCATION A PREVIOUS BUILDING WAS DEMOLISHED IN 2018. BOTH LOTS GENERALLY SHEET FLOW FROM EAST TO WEST, WITH A SLIGHT CROWN IN THE CENTER OF BOTH LOTS. THE WEST LOT PRIMARILY FREE DISCHARGES STORMWATER RUNOFF OVER THE PUBLIC SIDEWALKS OR THROUGH EXISTING DRIVEPADS INTO SAN PABLO STREET NE TO THE WEST, WITH PORTIONS DISCHARGING TO CHICO ROAD NE TO THE NORTH AND CENTRAL AVE TO THE SOUTH. THE EAST LOT PRIMARILY FREE DISCHARGES STORMWATER RUNOFF OVER THE PUBLIC SIDEWALKS OR THROUGH EXISTING DRIVEPADS INTO GROVE STREET NE, WITH PORTIONS DISCHARGING TO CHICO ROAD NE TO THE NORTH AND CENTRAL AVE TO THE SOUTH. THERE ARE NO ONSITE PONDING IMPROVEMENTS ON EITHER LOT.

GROVE STREET IS A FULLY DEVELOPED PUBLIC STREET WITH CURB AND GUTTER AND SIDEWALK ON BOTH SIDES. THE EXISTING STREET EXTENDS NORTH AND SOUTH, CONNECTING CHICO ROAD NE TO THE NORTH TO CENTRAL AVE TO THE SOUTH. GROVE STREET ACCEPTS FREE DISCHARGE OF STORMWATER RUNOFF FROM THE EAST LOT (BLOCK 3. LOMA VERDE). A HIGH POINT IN THE STREET IS LOCATED APPROXIMATELY HALFWAY BETWEEN THE TWO ROADS. RESULTING IN A SPLIT OF THE RUNOFF WITHIN GROVE STREET TO BOTH CHICO ROAD NE AND CENTRAL AVE.

THERE ARE NO APPARENT OFFSITE FLOWS IMPACTING THE EXISTING SITE. OFFSITE FLOWS ARE CONTAINED AND CONVEYED AROUND THE SITE TO THE EAST. NORTH, AND SOUTH WITHIN THE ADJACENT, FULLY DEVELOPED PUBLIC RIGHTS-OF-WAY OF CHICO ROAD NE, CHARLESTON ROAD NE, AND CENTRAL AVE, SAN PABLO STREET TO THE WEST IS TOPOGRAPHICALLY LOWER THAN THE SITE.

#### V. DEVELOPED CONDITIONS

THE PROPOSED DEVELOPED CONDITIONS FOR THIS SITE CONSIST A NEW LIBRARY BUILDING, PAVED PARKING LOT, AND ASSOCIATED PAVED PEDESTRIAN AND LANDSCAPING IMPROVEMENTS IN THE WESTERN PORTION OF THE SITE. THE EXISTING STREET IS PROPOSED TO BE VACATED BY CONCURRENT VACATION AND PLATTING ACTION, WHICH WILL FACILITATE THE COMPLETE REMOVAL OF THE EXISTING STREET IMPROVEMENTS. DEVELOPMENT OF THE EASTERN PORTION OF THE SITE (BLOCK 3, LOMA VERDE) IS UNDER DISCUSSION AND COORDINATION BETWEEN THE CITY OF ALBUQUERQUE AND AMAFCA; THIS PORTION OF THE SITE IS ANTICIPATED TO BE DEVELOPED AT A LATER DATE PENDING RESOLUTION OF THOSE COORDINATION EFFORTS.

THE PROPOSED LIBRARY DEVELOPMENT IMPROVEMENTS INCLUDE DEPRESSED PARKING ISLANDS AND PONDING AREAS INTENDED FOR WATER HARVESTING AND CAPTURE OF SITE RUNOFF. THESE LANDSCAPED AREAS WILL RESULT IN AN OVERALL DECREASE IN IMPERVIOUS AREA FROM THE EXISTING CONDITION, AND CALCULATIONS INCLUDED HEREWITH DEMONSTRATE AN OVERALL DECREASE IN DEVELOPED RUNOFF GENERATED DURING A 100-YEAR, 6 HOUR STORM EVENT.

THE SITE IS DIVIDED INTO TWO SEPARATE DRAINAGE BASINS, BASIN A IS THE WESTERN PORTION TO BE DEVELOPED BY THIS PLAN, AND BASIN B IS THE EASTERN PORTION TO BE DEVELOPED AT A FUTURE DATE. BASIN A IS FURTHER SUBDIVIDED INTO BASIN A-1 AND BASIN A-2. BASIN A-1 CONSISTS OF THE PARKING LOT AND PORTIONS OF THE BUILDING THAT DISCHARGE THERETO, AND BASIN A-2 CONSISTS OF THE REMAINDER OF THE BUILDING, THE FRONT ENTRY COURTYARD, AND THE NEW ACCESS ROAD TO CENTRAL AVENUE.

BASIN A-1 DRAINS FROM EAST TO WEST ACROSS THE NEW PARKING LOT, AND DISCHARGES TO SEVERAL PONDING AREAS WITHIN THE PARKING LOT ISLANDS AND AT THE NORTHWEST CORNER OF THE SITE DESIGNED TO CAPTURE THE MINOR INCREASE IN DEVELOPED RUNOFF ( $\Delta V100 = 440$  CF: VPOND = 2,016 CF) FROM THE NEW PARKING LOT AND NORTH PORTION OF THE BUILDING. THESE PONDING AREAS ARE ALSO SIZED TO CAPTURE AND TREAT THE FIRST FLUSH OF RUNOFF (VFF = 1,380 CF; VPOND = 2,016 CF) GENERATED BY THIS SUB-BASIN.

BASIN A-2 CONSISTS OF THE BUILDING, ENTRY COURTYARD, AND CENTRAL AVENUE ACCESS ROAD. THIS SUB-BASIN DISCHARGES TO THE SOUTH AND WEST (CENTRAL AVE AND SAN PABLO STREET). THIS SUB-BASIN INCLUDES SEVERAL DEPRESSED AREAS OF LANDSCAPING, BUT THEY ARE NEGLIGIBLE IN SIZE AND WILL NOT COLLECT SIGNIFICANT AMOUNTS OF RUNOFF FROM THIS SUB-BASIN. A STORM WATER FEATURE / CISTERN LOCATED NEAR THE SOUTHWEST CORNER OF THE NEW BUILDING IS PROPOSED TO COLLECT A PORTION OF THE ROOF RUNOFF, BUT AS THIS FEATURE IS AN ALTERNATE TO THE BASE BID, THE CAPACITY OF THIS STORMWATER COLLECTION FEATURE IS NOT INCLUDED IN THE CALCULATIONS FOR THIS SITE. CALCULATIONS REFERENCED HEREWITH DEMONSTRATE THAT THE DEVELOPED CONDITION FOR BASIN A-2 WILL RESULT IN A MINOR DECREASE ( $\Delta V100 = 690$  CF) OF STORMWATER RUNOFF VOLUME GENERATED, THEREFORE PONDING IS NOT REQUIRED FOR THE 100 YEAR STORM EVENT. ON-SITE WATER HARVESTING / PONDING OPPORTUNITIES IN BASIN A-2 ARE VERY LIMITED RESULTING FROM THE BUILDING PROXIMITY TO THE ADJACENT PUBLIC STREETS; THEREFORE THE FIRST FLUSH GENERATED BY BASIN A-2 (VFF = 1,270 CF) WILL FREE DISCHARGE DIRECTLY TO THE PUBLIC RIGHT OF WAYS ALONG CENTRAL AVENUE AND SAN PABLO STREET.

BASIN B WILL NOT BE DEVELOPED AS PART OF THIS PLAN, PENDING FUTURE SITE DEVELOPMENT. THIS PLAN PROPOSES TO CONSTRUCT A TEMPORARY RETENTION POND AT THE WEST END OF BASIN B IN ORDER TO CAPTURE THE RUNOFF GENERATED BY BASIN B AND PREVENT IT FROM DRAINING ONTO THE NEWLY DEVELOPED BASIN A LIBRARY IMPROVEMENTS. CALCULATIONS INCLUDED HEREWITH DEMONSTRATE THAT THE POND CAPACITY IS SIZED TO CAPTURE THE 100 YEAR STORM EVENT GENERATED BY BASIN B.

AS NOTED ABOVE, THERE ARE NO APPARENT OFFSITE FLOWS IMPACTING THE SITE.

1

#### VI. CALCULATIONS

CALCULATIONS ANALYZING THE EXISTING AND PROPOSED DEVELOPED CONDITIONS FOR THE 100 YEAR, 6-HOUR RAINFALL EVENT HAVE BEEN PREPARED FOR EACH DRAINAGE BASIN. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. AS DEMONSTRATED BY THESE CALCULATIONS, THE PROPOSED DEVELOPMENT WILL RESULT IN AN OVERALL DECREASE IN PEAK RATE AND VOLUME OF RUNOFF GENERATED BY THE SITE (1.0 CFS AND 2,060 CF REDUCTION). CALCULATIONS FOR THE PROPOSED ONSITE DETENTION PONDS, PREPARED USING THE AVERAGE END-AREA METHOD, DEMONSTRATE THAT THE ONSITE PONDING CAPACITY IS SIZED TO CONTAIN THE FIRST FLUSH OF RUNOFF IN BASIN A-1, AND THE 100 YEAR STORM RUNOFF GENERATED BY BASIN B.

#### VIII. CONCLUSIONS

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVALUATIONS CONTAINED HEREIN:

- 1. THIS DRAINAGE PLAN ADDRESSES THE LIBRARY IMPROVEMENTS FOR THE WESTERN PORTION OF THE SITE; FUTURE DEVELOPMENT FOR THE EASTERN PORTION OF THE SITE WILL REQUIRE SEPARATE SUBMITTAL.
- 2. THE PROPOSED IMPROVEMENTS WILL CONTINUE TO DRAIN TO THE ADJACENT PUBLIC STREETS, HOWEVER THE DEVELOPED WESTERN PORTION OF THE SITE WILL BE PRIMARILY TO SAN PABLO STREET NE AND CENTRAL AVE; THE DISCHARGE OF DEVELOPED RUNOFF TO CHICO ROAD NE WILL BE NEGLIGIBLE.
- 3. THE PROPOSED IMPROVEMENTS WILL MAINTAIN THE STATUS QUO FOR THE SITE OF FREE DISCHARGE AFTER THE ON-SITE STORMWATER RETENTION/CAPTURE AREAS ARE INITIALLY FILLED TO THEIR CAPACITY. THE PROPOSED IMPROVEMENTS WILL RESULT IN A DECREASE IN THE PEAK RATE OF RUNOFF AND VOLUME DRAINING TO THE PUBLIC RIGHT-OF-WAY DUE TO THE DECREASE IN IMPERVIOUS LAND TREATMENT AND THE ADDITION OF SEVERAL RETENTION ARFAS.
- 4. THE PROPOSED DEVELOPMENT WILL NOT ADVERSELY IMPACT DOWNSTREAM PROPERTIES OR DOWNSTREAM DRAINAGE CONDITIONS.
- 5. WHILE WATER HARVESTING / POND AREAS WILL CAPTURE AND TREAT THE FIRST FLUSH FROM BASIN A-1. THE FIRST FLUSH FROM BASIN A-2 WILL NOT BE CAPTURED ONSITE DUE TO SITE CONSTRAINTS PROHIBITING ADEQUATE WATER HARVESTING OPPORTUNITIES. THEREFORE 1.270 CF OF FIRST FLUSH WILL FREE DISCHARGE TO THE CITY RIGHT OF WAY. IT SHOULD BE NOTED THAT IF A CISTERN PROPOSED IN BASIN A-2 AS AN ALTERNATE IS SELECTED BY THE CITY FOR CONSTRUCTION, THIS WILL BE USED TO CAPTURE THE FIRST FLUSH RUNOFF FROM THE NEW BUILDING TO THE MAXIMUM EXTENT PRACTICABLE.

2

### CALCULATIONS

I. SITE CHARACTERISTICS

PRECIPITATION ZONE =  $P_{100, 6 HR} = P_{360} =$ 

208,826|SF TOTAL PROJECT AREA  $(A_{\tau}) = -$ 

2.60

IN

D. LAI

| 4.79         |   |   |   |  |  |  |
|--------------|---|---|---|--|--|--|
| ND TREATMENT |   |   | DEVELOPED   | LAND TREATMENT   |  |  |
| 59,120       | 59,120 <b>SF</b>  |   |   | 59, 120 <b>SF</b>  |  |  |
| 1.36 AC      |   |   | BASIN A-1   | 1.36 AC  |  |  |
| AREA (SF/A   | (C)   | %   | LAND TREATMENT  | AREA (SF/AC)   | %  |  |
|              |   | -   | A   |  | _  |  |
|              |   | -   | В   |  | -  |  |
| 15,470       | SF  | 0.001   | 2   | 10,440 SF  | - 18%  |  |
| 0.36         | AC  | - 26%   | C   | 0.24 AC  |  |  |
| 43,650       | SF  | 740/  |   | 48,680 SF  | 0.00/  |  |
|              |   | 74%   | U   | 1.12 AC  | 82%  |  |
|              |   |   |   |  |  |  |
| 1.36 AC      |   |   | 1.36 AC   |  |  |  |
| AREA (SF/A   | (C)   | %   | LAND TREATMENT  | AREA (SF/AC)   | %  |  |
|              |   | -   | A   |  | -  |  |
|              |   | -   | В   |  |  |  |
| 6,535        | SF  | 44.07   | 0   | 14,450 SF  | 24%  |  |
| 0.15         | AC  | - 11%   | C   | 0.33 AC  |  |  |
|              |   | 000/  | D   | 44,725 SF  | 760  |  |
|              |   | - 09%   | В   |  | - 76%  |  |
|              |   |   |   |  |  |  |
|              |   |   |   |  |  |  |
| AREA (SF/A   | (C)   | %   | LAND TREATMENT  | AREA (SF/AC)   | %  |  |
|              |   | -   | A   |  | -  |  |
|              |   | -   | В   |  | -  |  |
|              |   | - 8%  | С   | 28,160 SF<br><b>0.65 AC</b>  | 319  |  |
| 82,995       | SF  | 92%   | D   | 62,295 SF  | - 699  |  |
|              | 1.36<br>AREA (SF/A<br>15,470<br>0.36<br>43,650<br>1.00<br>59,175<br>1.36<br>AREA (SF/A<br>6,535<br>0.15<br>52,640<br>1.21<br>90,455<br>2.08<br>AREA (SF/A<br>90,455<br>2.08<br>AREA (SF/A | 59,120 SF<br>1.36 AC<br>AREA (SF/AC)<br>15,470 SF<br>0.36 AC<br>43,650 SF<br>1.00 AC<br>59,175 SF | 59,120 SF   1.36 AC   AREA (SF/AC) %   15,470 SF   15,470 SF   26%   43,650 SF   74%   59,175 SF   1.00 AC   59,175 SF   1.36 AC   AREA (SF/AC) %   6,535 SF   1.36 AC   AREA (SF/AC) %   6,535 SF   1.36 AC   AREA (SF/AC) %   6,535 SF   90,455 SF   2.08 AC   90,455 SF   2.08 AC   AREA (SF/AC) %   90,455 SF   2.08 AC   AREA (SF/AC) %   90,455 SF   2.08 AC   AREA (SF/AC) %   7,460 SF   8% 8%   6,7,460 SF   8% 32,995 | 59,120 SF BASIN A-1   1.36 AC A   AREA (SF/AC) % LAND TREATMENT   A A B   15,470 SF 26% C   0.36 AC 26% C   43,650 SF 74% D   59,175 SF BASIN A-2   1.00 AC 74% D   59,175 SF BASIN A-2   1.36 AC A   59,175 SF BASIN A-2   AREA (SF/AC) % LAND TREATMENT   AREA (SF/AC) % LAND TREATMENT   6,535 SF 11% C   52,640 SF 89% D   90,455 SF BASIN B   2.08 AC A   90,455 SF BASIN B   AREA (SF/AC) % LAND TREATMENT   90,455 SF BASIN B   AREA (SF/AC) % A   A A B   37,460 SF 8% | 59,120   SF   BASIN A-1   59,120   SF     1.36   AC    1.36   AC   1.36   AC     AREA (SF/AC)   %   LAND TREATMENT   AREA (SF/AC)   A    A     15,470   SF   26%   C   10,440   SF     0.36   AC   26%   C   0.24   AC     48,680   SF   74%   D   48,680   SF     1.00   AC   74%   D   1.12   AC     59,175   SF   BASIN A-2   1.36   AC     1.36   AC   74%   D   1.12   AC     1.36   AC   74%   D   1.12   AC     1.36   AC   B   1.36   AC     1.36   AC   %   LAND TREATMENT   AREA (SF/AC)     A   B   1.36   AC   0.33   AC     52,640   SF   89%   D   1.0.31   AC |  |

|    | С   | 0.17   |            | 8%           |           | С           |                       |     |
|----|---|--|------------|--------------|-----------|-------------|-----------------------|-----|
|    | D   | 82,995<br><b>1.91</b>  |            | 92%          |           | D           |                       |     |
|    |   |  |            |              |           |             |                       |     |
|    | ISTING CONDITION 100 YEAR   | • A <sub>c</sub> + E <sub>D</sub> • A <sub>D</sub> )/A <sub>T</sub><br>• 0.00) + (1.29 • 0.3 |            |              |           | _           | 2.08                  |     |
|    | <u>b. PEAK DISCHARGE 100-YR</u><br>$Q_{100} = Q_A \cdot A_A + Q_B \cdot A_B + Q_C$  |  |            |              |           | _           | · ·                   | CFS |
| 2. | $\begin{array}{l} \underline{\textbf{BASIN A-2}} \\ \underline{a. \ VOLUME \ 100-YR, \ 6-HR} \\ WT_E = (E_A \cdot A_A + E_B \cdot A_B + E_C \\ \Rightarrow (0.66 \cdot 0.00) + (0.92 \\ V_{100, 6HR} = (E_W/12) \cdot A_T \\ \\ \underline{b. \ PEAK \ DISCHARGE \ 100-YR \\ Q_{100} = Q_A \cdot A_A + Q_B \cdot A_B + Q_C \\ \Rightarrow (1.87 \cdot 0.00) \\ \end{array}$ | • 0.00) + (1.29 • 0.)<br>⇒ (2.24/12)   | • 1.36 =   | 0.2536       | AC-FT =   |             | 2.24<br>11,050<br>6.6 |     |
| 4. | <b>BASIN B</b><br><u>a. VOLUME 100-YR, 6-HR</u><br>WT <sub>E</sub> = (E <sub>A</sub> • A <sub>A</sub> + E <sub>B</sub> • A <sub>B</sub> + E <sub>C</sub><br>⇒ (0.66 • 0.00) + (0.92)<br>V <sub>100,6 HR</sub> = (E <sub>W</sub> /12) • A <sub>T</sub>   | • A <sub>C</sub> + E <sub>D</sub> • A <sub>D</sub> )/A <sub>T</sub>                          | 17) + (2.3 | 6•1.91)      | /2.08 =   | -           | 2.27<br>17,110        | IN  |
|    |   | + (2.60 • 0.00) + (3.  | 45 • 0.17) | + (5.02      | • 1.91) = | _           | 10.2                  | CFS |
|    | <b>EVELOPED CONDITION 100 YEA</b><br><b>BASIN A-1</b><br>a. VOLUME 100-YR, 6-HR<br>WT <sub>E</sub> = (E <sub>A</sub> • A <sub>A</sub> + E <sub>B</sub> • A <sub>B</sub> + E <sub>C</sub><br>⇒ (0.66 • 0.00) + (0.92<br>V <sub>100,6 HR</sub> = (E <sub>W</sub> /12) • A <sub>T</sub><br>b. FIRST FLUSH VOLUME   | • A <sub>c</sub> + E <sub>D</sub> • A <sub>D</sub> )/A <sub>T</sub><br>• 0.00) + (1.29 • 0.2 | <i>,</i> , | ,            |           | -           | 2.17<br>10,690        |     |
|    | V <sub>FF</sub> = ((P <sub>FF</sub> - IA <sub>D</sub> )/12) • A <sub>D</sub><br>⇒ ((0.44 -<br>c. PEAK DISCHARGE 100-YR  | 0.10)/12) • (1.12) =   |            | 0.0317       | AC-FT =   | _           | 1,380                 | CF  |
|    | $\overline{\mathbf{Q}_{100} = \mathbf{Q}_{A} \cdot \mathbf{A}_{A} + \mathbf{Q}_{B} \cdot \mathbf{A}_{B} + \mathbf{Q}_{C}}$  | • A <sub>C</sub> + Q <sub>D</sub> • A <sub>D</sub><br>+ (2.60 • 0.00) + (3.                  | 45 • 0.24) | + (5.02      | • 1.12) = | _           | 6.4                   | CFS |
|    | $\begin{array}{r} \hline \begin{array}{c} \text{POND} \ \text{VOLOME} \ \text{O}, \text{If } \text{VOLOME} \end{array} \\ \hline \\ \text{POND}_1 + \text{POND}_2 + \text{PONI} \\ \hline \\ \Rightarrow 1150 \text{ CF} + 626 \text{ CF} + 80 \\ \hline \\ \Rightarrow \textbf{V}_{\text{POND} \ \text{TOTAL}} = 2,016 \text{ CF} \end{array}$                             | CF + 80 CF + 80 C  | -          |              |           | _           | 2,016                 | CF  |
| 2. | $\begin{array}{l} \underline{\textbf{BASIN A-2}} \\ \underline{a. \ VOLUME \ 100-YR, \ 6-HR} \\ WT_{E} = (E_{A} \bullet A_{A} + E_{B} \bullet A_{B} + E_{C} \\ \Rightarrow (0.66 \bullet 0.00) + (0.92 \\ V_{100, 6 \ HR} = (E_{W}/12) \bullet A_{T} \end{array}$   | • 0.00) + (1.29 • 0.3  |            |              |           | -           | 2.10<br>10,360        |     |
|    | b. FIRST FLUSH VOLUME<br>$V_{FF} = ((P_{FF} - IA_D)/12) \cdot A_D$<br>⇒ ((0.44 -<br>- NEGLIGIBLE WATER HAR)<br>1,270 CF OF FIRST FLUSH D  |  | LUSH DI    | SHARG        | E CAPTUR  |             | 1,270<br>SIN A-2;     |     |
|    | <u>c. PEAK DISCHARGE 100-YR</u><br>$Q_{100} = Q_A \cdot A_A + Q_B \cdot A_B + Q_C$  |  |            |              |           | _           | 6.3                   | CFS |
|    | d. STORM DRAIN PIPE FLOW<br>STORM DRAIN<br>A (SE COURTYARD SD)  | Q <sub>100 DEV</sub> DIA.  | HGL/EG     | <u>6 5</u> " | VEL.      | EGL<br>8 5" |                       |     |

| A (SE COURTYARD SD) | 1 CFS   | 8" | 0.7% | 6.5" | 3.3 |
|---------------------|---------|----|------|------|-----|
| B (S BLDG SD)       | 1.2 CFS | 8" | 4%   | 4"   | 6.9 |
| C (SW BLDG SD)      | 1.0 CFS | 6" | 3%   | 3.5" | 5.1 |
| D (NW BLDG SD)      | 0.8 CFS | 8" | 5%   | 3"   | 6.7 |
|                     |         |    |      |      |     |

8.5"

12.9" 8.3"

11.4"

4

#### e. 12" PUBLIC SIDEWALK CULVERT CAPACITY (8" DEPTH)

Q = 1.486/n \* A \* R<sup>2/3</sup> \* S<sup>1/2</sup> n = 0.013

A = 0.67 SF

R = 0.29 FT

- S = 0.015 FT/FT Q = 4.1 CFS

## 4. BASIN B

a. VOLUME 100-YR, 6-HR  $WT_{E} = (E_{A} \cdot A_{A} + E_{B} \cdot A_{B} + E_{C} \cdot A_{C} +$  $\Rightarrow$  (0.66 • 0.00) + (0.92 • 0.00  $V_{100.6 \text{ HR}} = (E_W/12) \cdot A_T$ 

b. FIRST FLUSH VOLUME  $V_{FF} = ((P_{FF} - IA_D)/12) \cdot A_D$  $\Rightarrow$  ((0.44 - 0.10)/

c. PEAK DISCHARGE 100-YR  $Q_{100} = Q_A \bullet A_A + Q_B \bullet A_B + Q_C \bullet A_C +$  $\Rightarrow$  (1.87 • 0.00) + (2.60

d. POND VOLUME CAPACITY POND<sub>6</sub> = 18,150 CF > V<sub>100</sub> = 1

C. COMPARISON 100 YEAR STORM BASIN A-1 a. VOLUME 100-YR, 6-HR

> ΔV100, 6 H b. PEAK DISCHARGE 100-YR

ΔQ 2. BASIN A-2 a. VOLUME 100-YR, 6-HR

∆V100, 6 H b. PEAK DISCHARGE 100-YR ΔQ

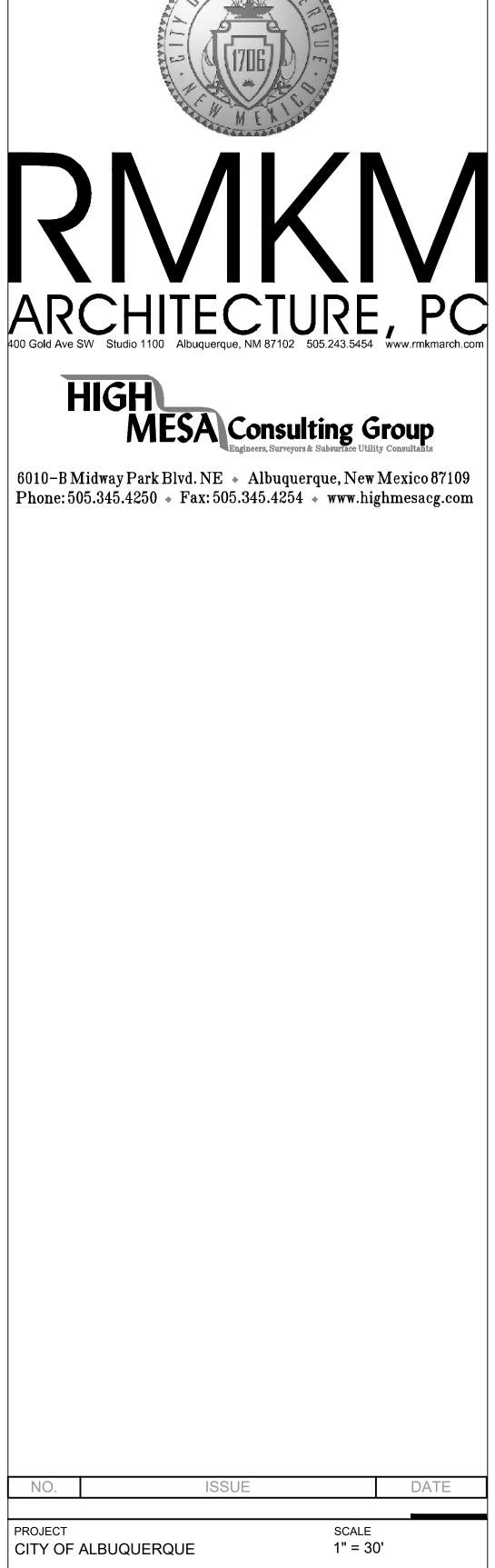
4. BASIN B a. VOLUME 100-YR, 6-HR ΔV<sub>100, 6</sub>н b. PEAK DISCHARGE 100-YR

ΔQ 5. OVERALL SITE a. VOLUME 100-YR, 6-HR

ΔV100, 6 H b. PEAK DISCHARGE 100-YR  $\Delta Q_{100} = 22.1 - 23.1$ 

| + E <sub>D</sub> • A <sub>D</sub> )/A <sub>T</sub><br>0) + (1.29 • 0.65) + (2.36 • 1.43)<br>⇒ (2.03/12) • 2.08 = <b>0.3513</b> |           | 2.03 IN<br>15,300 CF |
|--|-----------|----------------------|
| /12) • (1.43) = <b>0.0405</b>  | AC-FT =   | 1,770 CF             |
| + Q <sub>D</sub> • A <sub>D</sub><br>60 • 0.00) + (3.45 • 0.65) + (5.02  | • 1.43) = | 9.4 CFS              |
| 15,300 CF  |           |                      |
|  |           |                      |
| <sub>HR</sub> = 10690 - 10250 =  | 440 CF    | (INCREASE)           |
| 100 = 6.4 - 6.3 =  | 0.1 CFS   | (INCREASE)           |
| <sub>HR</sub> = 10360 - 11050 =  | -690 CF   | (DECREASE)           |
| <sub>100</sub> = 6.3 - 6.6 =   | -0.3 CFS  | (DECREASE)           |
| <sub>HR</sub> = 15300 - 17110 =  | -1,810 CF | (DECREASE)           |
| <sub>100</sub> = 9.4 - 10.2 =  | -0.8 CFS  | (DECREASE)           |
|  |           |                      |
| <sub>HR</sub> = 36,350 - 38,410  | -2060 CF  | (DECREASE)           |

-1.0 CFS (DECREASE)



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SHEET TITLE DRAINAGE PLAN AND CALCULATIONS DESIGN PHASE 100% CONSTRUCTION DC 11 DECEMBER, 2019

|          | GRAEME MEAN    |
|----------|----------------|
| OCUMENTS | REAL TRANSPORT |
|          | PROFESSIONAL   |
|          | 12/11/2019     |

SHEET NUMBER

