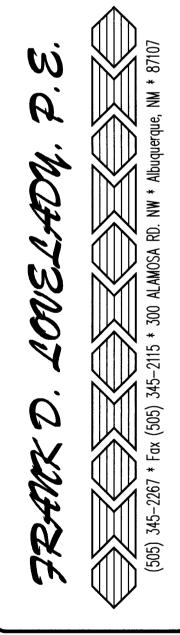




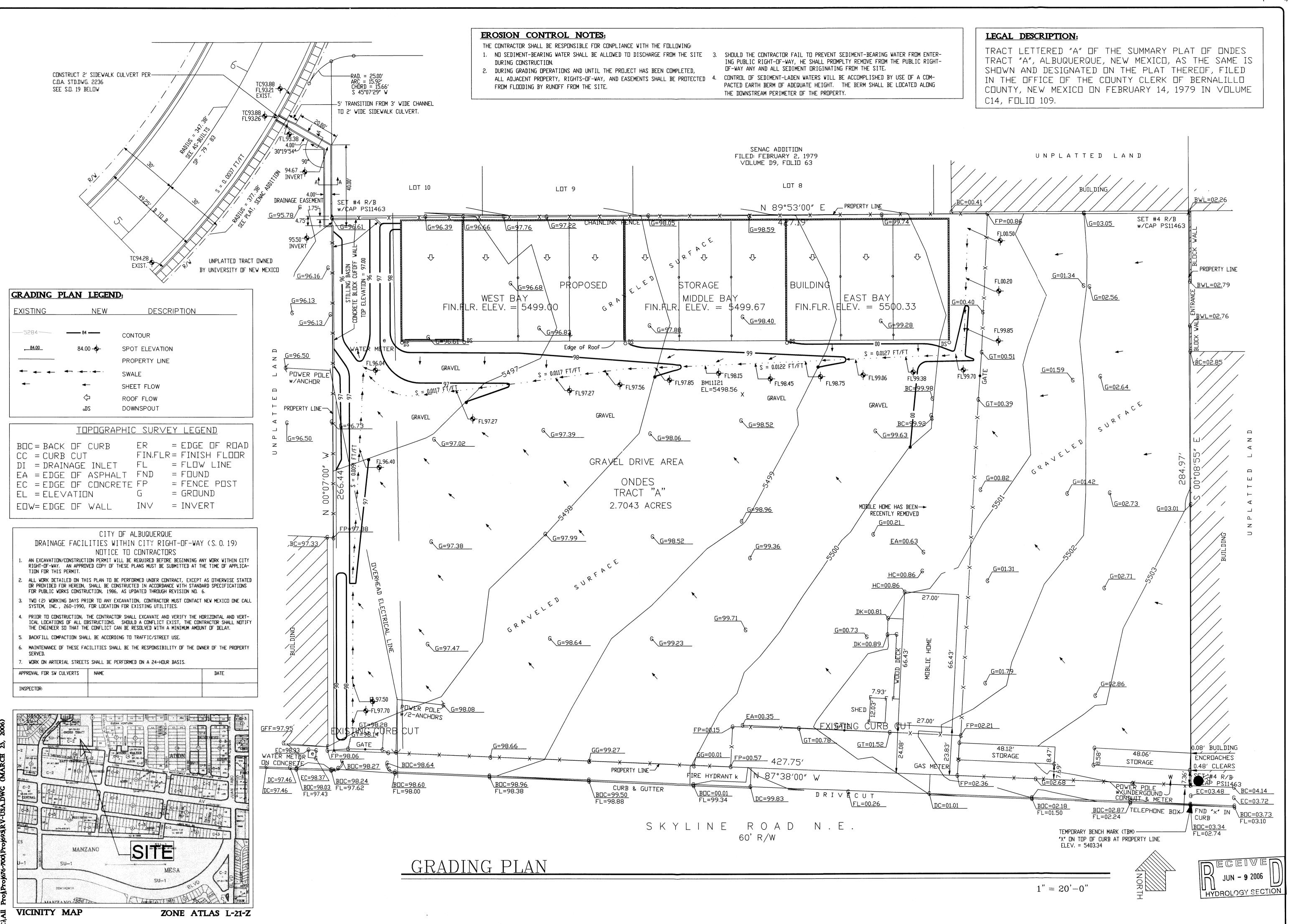
CERTIFIED GRADING AND DRAINAGE PLAN
RV-USA
11117 SKYLINE ROAD NE



DATE: September 28, 2006

REVISIONS





FEBRUARY 22 , 2006 Revised 03/23/06

JOB NO: 693 DATE: March 23, 2006 REVISIONS

SHEET NO.

EXISTING CONDITIONS:

The site is located on the north side of Skyline Drive about 200' east of Elizabeth Avenue, NE. The site is almost completly unoccupied except for a mobile home and some small storage sheds. The area north of the site is occupied by an apartment complex and RV storage sheds similar to the ones proposed for this project. The Area to the East is also RV storage with no runoff leaving the site. The area west of the site is unplatted property which is presumed to be owned, or possibly leased, by the City of Albuquerque for a water well site.

PROPOSED CONDITIONS:

It is proposed to construct an RV storage building on the site as shown. The building roof will slope away from the property line to the front of the building and be conveyed to a swale via which it will be directed to a small stilling basin and channeled to Morris Avenue. No attempt has been made to use the stilling basin to reduce the peak discharge. The basin serves only to collect site runoff for discharge via a concrete channel and sidewalk culvert into Morris Avenue.

DRAINAGE CRITERIA:

The calculations shown on this plan were prepared in accordance with Section 22.2, Hydrology, of the Development Process Manual, Volume 2, Design Criteria, for the City of Albuquerque in cooperation with Bernalillo County, New Mexico and the Metropolitan Arroyo Flood Control Authority, January, 1993.

PRECIPITATION ZONE:

The site is east of Eubank Blvd. However, it is also south of I-40 and is, therefore, in Precipitation Zone 3.

LAND TREATMENT AREAS, ETC.:

The peak discharge per acre and excess precipitation are shown for the four land treatments in Zone 3 in the table below, and the values shown are from the City of Albuquerque D. P. M. Also shown are the existing and proposed land treatment areas. LAND q(cfs/ac) E (in) Existing Site Areas Developed Site Areas TREAT. 100-yr. 10-yr. 100-yr. 10-yr. % Sq. Ft. Acres % Sq. Ft. Acres A 1.87 0.58 0.66 0.19 0.0 0 0.0000 0.0 0 0.0000 B 2.60 1.19 0.92 0.36 0.0 0 0.0000 0.0 0 0.0000 C 3. 45 2. 00 1. 29 0. 62 100. 0 117, 799 2. 7043 86. 1 101, 419 2. 3283 D 5. 02 3. 39 2. 36 1. 50 0. 0 0 0. 0000 13. 9 16, 380 0. 3760 100. 0 117, 799 2. 7043 100. 0 117, 799 2. 7043

PEAK DISCHARGE:

EXISTING CONDITIONS: Q100 = 2.7043 * 3.45 = 9.33 cfs

Q10 = 2.7043 * 2.00 = 5.41 cfs

DEVELOPED CONDITIONS:

Q100 = 2.3283 * 3.45 + 0.3760 * 5.02 = 9.92 cfsQ10 = 2.3283 * 2.00 + 0.3760 * 3.39 = 5.93 cfs

VOLUME, 100-YEAR AND 10-YEAR, 6-HOUR:

EXISTING CONDITIONS:

V100 = (117, 799 * 1. 29) / 12 = 12,663 cf V10 = (117,799 * 0.62) / 12 = 6,086 cf

DEVELOPED CONDITIONS:

V100 = (101, 419 * 1.29 + 16, 380 * 2.36)/12 = 14, 124 cf $V10 = (101, 419 \times 0.62 + 16, 380 \times 1.50)/12 = 7,287 \text{ cf}$

SUMMARY OF ON-SITE VOLUMES AND DISCHARGE RATES:

	V100(CF)	V10(CF)	Q100(CF5)	Q10(CF3)
DEVELOPED	14, 124	7,287	9. 92	5, 93
EXISTING	12, 663	6, 086	9. 33	5. 41
INCREASE	1, 461	1,201	0. 59	0. 52

SIDEWALK CULVERT:

USE MANNING'S EQUATION EQUATION, $Q = A(1.486 / N)(R)^{2/3}(S)^{1/2}$ $A = 0.667 \times 2.0 = 1.33 \text{ SF} \quad P = 0.667 + 2.0 + 0.667 = 3.33 \text{ FT}, \quad R = A / P = 1.33 / 3.33 = 0.3994$ Manning's n = 0.013 (concrete) Slope (S = 0.0200 ft./ft.) $Q = 1.33 (1.486 / 0.013) (0.3994)^{2/3} (0.0200)^{1/2} = 11.66 CFS > 9.92 CFS (ADEQUATE)$

CHANNEL NORTH 40 FEET TO DOG LEG:

USE MANNING'S EQUATION $Q = A(1.486 / N)(R)^{2/3}(S)^{1/2}$ USE 3' WIDE CHANNEL WITH 8' SIDES. $A = 0.667 \times 3.0 = 2.00 \text{ SF} \quad P = 0.667 + 3.0 + 0.667 = 4.33 \text{ FT}, \quad R = A / P = 2.00 / 4.33 = 0.4619$ At owner'w request, try using concrete curbs to contain the flow and line bottom of channel with gravel. Slope = 10 inch drop in first 40 feet = 0.0208 ft./ft. Manning's n = 0.013 (concrete) Manning's n 0.035 (rock bottom) Weighted n = $(0.013 \times 16 + 0.035 \times 36) / 52 = 0.0282$ $Q = 2.00 (1.486 / 0.0282) (0.4619)^{2/3} (0.0208)^{1/2} = 9.08 CFS (9.92 CFS (NOT ADEQUATE)$ USE CONCRETE BOTTOM INSTEAD OF ROCK. Manning's n = 0.013 $Q = 2.00 (1.486 / 0.013) (0.4619)^{2/3} (0.0208)^{1/2} = 19.70 CFS > 10.66 CFS (TDD LARGE)$ TRY 6" DEPTH. $A = 3.0 \times 0.5 = 1.5 \text{ SF} P=0.5 + 3.0 + 0.5 = 4.0 \text{ FT} R=A / P = 1.5 / 4.0 = 0.375$ Q = 1.50 (1.486 / 0.013) (0.375)2/3 (0.0208)1/2 = 12.85 CFS > 9.92 CFS (ADEQUATE)

CHANNEL - FROM DOG LEG TO SIDEWALK CULVERT:

Manning's n = 0.013 Width = 3.0' Depth = 4' Area = 1.00 sf P = 0.33 + 3.0 + 0.33 = 3.66' Slope = 1.29 / 23.2 = 0.0556 ft/ft R = A / P = 1.00 / 3.66= 0.2732 ft $Q = 1.00 (1.486 / 0.013) (0.2732)^{2/3} (0.0556)^{1/2} = 11.34 CFS > 9.92 CFS (ADEQUATE)$

FLOOD HAZARD MAP:

THERE ARE NO FLOOD ZONES ON OR ADJACENT TO THIS SITE.

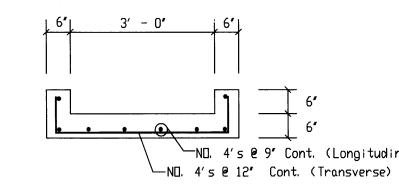
DOWNSTREAM CAPACITY:

THE RUNDFF FROM THIS SITE ENTERS MORRIS AND IS CONVEYED NORTH VIA THE STREET AND STORM DRAINAGE SYSTEM TO THE DRAINAGE CHANNEL BETWEEN THE EAST-BOUND AND WEST-BOUND LANES OF I-40. DOWNSTREAM CAPACITY IS ADEQUATE. **OFF-SITE FLOW:**

THERE IS NO OFF-SITE FLOW EFFECTING THIS SITE.

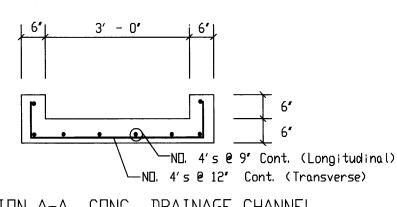
GENERAL NOTES:

- 1: OWNER OF RECORD MICHAEL J. and CAROL WARREN TRUST
- 2: LEGAL DESCRIPTION AND EASEMENTS SHOWN ARE BASED ON THE PLAT OF RECORD AS STATED BELOW.
- 3: PLATS USED TO ESTABLISH BOUNDARY. A: SUMMARY PLAT ONDES TRACT "A" FILED: FEBRUARY 14, 1979 IN VOLUME C14, FOLIO 109
- 4: FIELD WORK PERFORMED ON: <u>December, 2005</u>
- 5: CONTOUR INTERVAL IS ONE (1) FOOT.
- 6: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE STATION No. "11-L21", HAVING AN ELEVATION OF 5498.56 .
- 7: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND/ OR DEPTH PRIOR TO EXCAVATION OR DESIGN CON-SIDERATIONS.



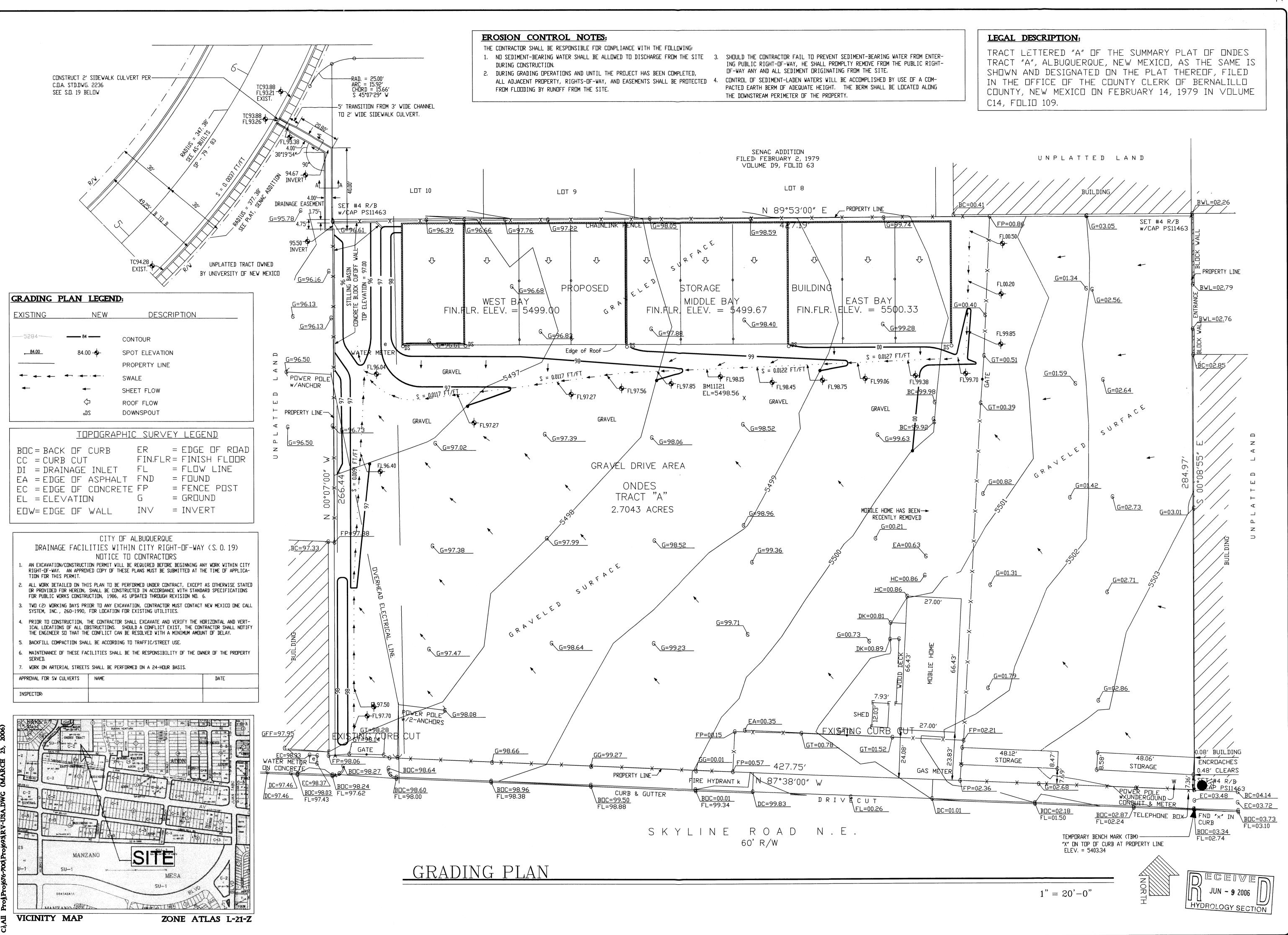
SECTION A-A CONC. DRAINAGE CHANNEL

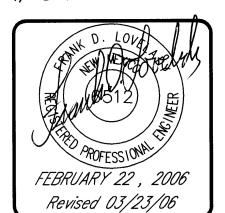
1/2'' = 1' - 0''



Revised 03/23/06

JOB NO	693
DATE:	March 23, 2006
	REVISIONS





GRADING AND DRAINAGE PLAN
RV-USA
TITT SKYLINE ROAD NE

FRANK D. LOUELADY, P. E.(505) 345-2267 * Fax (505) 345-2115 * 300 ALAMOSA RD. NW * Albuquerque, NM * 87107

JOB NO: **693**DATE: March 23, 2006

REVISIONS

TOF 2

EXISTING CONDITIONS:

The site is located on the north side of Skyline Drive about 200' east of Elizabeth Avenue, NE. The site is almost completly unoccupied except for a mobile home and some small storage sheds. The area north of the site is occupied by an apartment complex and RV storage sheds similar to the ones proposed for this project. The Area to the East is also RV storage with no runoff leaving the site. The area west of the site is unplatted property which is presumed to be owned, or possibly leased, by the City of Albuquerque for a water well site.

PROPOSED CONDITIONS:

It is proposed to construct an RV storage building on the site as shown. The building roof will slope away from the property line to the front of the building and be conveyed to a swale via which it will be directed to a small stilling basin and channeled to Morris Avenue. No attempt has been made to use the stilling basin to reduce the peak discharge. The basin serves only to collect site runoff for discharge via a concrete channel and sidewalk culvert into

Morris Avenue. DRAINAGE CRITERIA:

The calculations shown on this plan were prepared in accordance with Section 22.2, Hydrology, of the Development Process Manual, Volume 2, Design Criteria, for the City of Albuquerque in cooperation with Bernalillo County, New Mexico and the Metropolitan Arroyo Flood Control Authority, January, 1993.

PRECIPITATION ZONE:

The site is east of Eubank Blvd. However, it is also south of I-40 and is, therefore, in Precipitation Zone 3.

LAND TREATMENT AREAS, ETC.:

The peak discharge per acre and excess precipitation are shown for the four land treatments in Zone 3 in the table below, and the values shown are from the City of Albuquerque D. P. M. Also shown are the existing and proposed land treatment areas. LAND q(cfs/ac) E (in) Existing Site Areas Developed Site Areas TREAT. 100-yr. 10-yr. 100-yr. 10-yr. % Sq.Ft. Acres % Sq.Ft. Acres A 1.87 0.58 0.66 0.19 0.0 0 0.0000 0.0 0 0.0000 B 2, 60 1, 19 0, 92 0, 36 0, 0 0 0, 0000 0, 0 0 0, 0000 C 3, 45 2, 00 1, 29 0, 62 100, 0 117, 799 2, 7043 86, 1 101, 419 2, 3283 D 5. 02 3. 39 2. 36 1. 50 0. 0 0 0. 0000 13. 9 16, 380 0. 3760 100. 0 117, 799 2. 7043 100. 0 117, 799 2. 7043

PEAK DISCHARGE:

EXISTING CONDITIONS:

Q100 = 2.7043 * 3.45 = 9.33 cfsQ10 = 2.7043 * 2.00 = 5.41 cfs

DEVELOPED CONDITIONS:

Q100 = 2.3283 * 3.45 + 0.3760 * 5.02 = 9.92 cfsQ10 = 2.3283 * 2.00 + 0.3760 * 3.39 = 5.93 cfs

VOLUME, 100-YEAR AND 10-YEAR, 6-HOUR:

EXISTING CONDITIONS:

V100 = (117,799 * 1.29) / 12 = 12,663 cfV10 = (117,799 * 0.62) / 12 = 6,086 cf

DEVELOPED CONDITIONS:

V100 = (101, 419 * 1.29 + 16, 380 * 2.36)/12 = 14, 124 cfV10 = $(101, 419 \times 0.62 + 16, 380 * 1.50)/12 = 7,287 \text{ cf}$

SUMMARY OF ON-SITE VOLUMES AND DISCHARGE RATES:

	V100(CF)	V10(CF)	Q100(CF3)	Q10(CF5)
DEVELOPED	14, 124	7,287	9. 92	5. 93
EXISTING	12, 663	6, 086	9, 33	5, 41
INCREASE	1,461	1,201	0. 59	0. 52

SIDEWALK CULVERT:

USE MANNING'S EQUATION EQUATION, $Q = A(1.486 / N)(R)^{2/3}(S)^{1/2}$ $A = 0.667 \times 2.0 = 1.33 \text{ SF} \quad P = 0.667 + 2.0 + 0.667 = 3.33 \text{ FT}, \quad R = A / P = 1.33 / 3.33 = 0.3994$ Manning's n = 0.013 (concrete) Slope (S = 0.0200 ft./ft.) $Q = 1.\overline{33}$ (1. 486 / 0. 013) (0. 3994)2/3 (0. 0200)1/2 = 11. 66 CFS > 9. 92 CFS (ADEQUATE)

CHANNEL NORTH 40 FEET TO DOG LEC:

USE MANNING'S EQUATION $Q = A (1.486 / N) (R)^{2/3} (S)^{1/2}$ USE 3' WIDE CHANNEL WITH 8' SIDES. $A = 0.667 \times 3.0 = 2.00 \text{ SF}$ P = 0.667 + 3.0 + 0.667 = 4.33 FT, R = A / P = 2.00 / 4.33 = 0.4619At owner'w request, try using concrete curbs to contain the flow and line bottom of channel with gravel. Slope = 10 inch drop in first 40 feet = 0.0208 ft./ft. Manning's n = 0.013 (concrete) Manning's n 0.035 (rock bottom) Weighted n = $(0.013 \times 16 + 0.035 \times 36) / 52 = 0.0282$ $Q = 2.00 (1.486 / 0.0282) (0.4619)^{2/3} (0.0208)^{1/2} = 9.08 CFS (9.92 CFS (NDT ADEQUATE)$ USE CONCRETE BOTTOM INSTEAD OF ROCK. Manning's n = 0.013 $Q = 2.00 (1.486 / 0.013) (0.4619)^{2/3} (0.0208)^{1/2} = 19.70 CFS > 10.66 CFS (TDD LARGE)$ TRY 6" DEPTH. A = 3.0 X 0.5 = 1.5 SF P=0.5 + 3.0 + 0.5 = 4.0 FT R=A / P = 1.5 / 4.0 = 0.375 $Q = 1.50 (1.486 / 0.013) (0.375)^{2/3} (0.0208)^{1/2} = 12.85 CFS > 9.92 CFS (ADEQUATE)$

CHANNEL - FROM DOG LEG TO SIDEWALK CULVERT:

Manning's n = 0.013 Width = 3.0' Depth = 4' Area = 1.00 sf P = 0.33 + 3.0 + 0.33 = 3.66' Slope = 1.29 / 23.2 = 0.0556 ft/ft R = A / P = 1.00 / 3.66= 0.2732 ft $Q = 1.00 (1.486 / 0.013) (0.2732)^{2/3} (0.0556)^{1/2} = 11.34 CFS > 9.92 CFS (ADEQUATE)$

FLOOD HAZARD MAP:

THERE ARE NO FLOOD ZONES ON OR ADJACENT TO THIS SITE.

DOWNSTREAM CAPACITY:

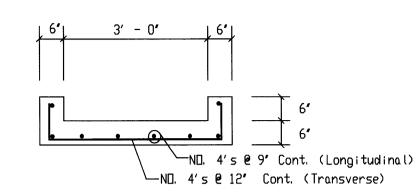
THE RUNDFF FROM THIS SITE ENTERS MORRIS AND IS CONVEYED NORTH VIA THE STREET AND STORM DRAINAGE SYSTEM TO THE DRAINAGE CHANNEL BETWEEN THE EAST-BOUND AND WEST-BOUND LANES OF I-40. DOWNSTREAM CAPACITY IS ADEQUATE.

OFF-SITE FLOW: THERE IS NO OFF-SITE FLOW EFFECTING THIS SITE.

GENERAL NOTES:

- 1: OWNER OF RECORD MICHAEL J. and CAROL WARREN TRUST
- 2: LEGAL DESCRIPTION AND EASEMENTS SHOWN ARE BASED
- 3: PLATS USED TO ESTABLISH BOUNDARY. A: SUMMARY PLAT ONDES TRACT "A"

- 7: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND / OR DEPTH PRIOR TO EXCAVATION OR DESIGN CON-SIDERATIONS.

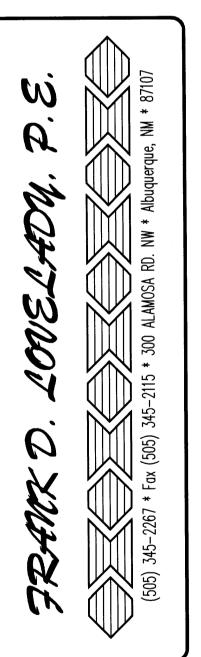


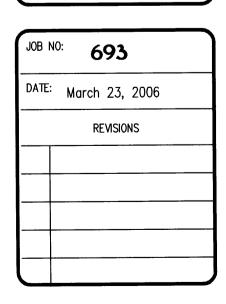
SECTION A-A CONC. DRAINAGE CHANNEL

1/2'' = 1' - 0''

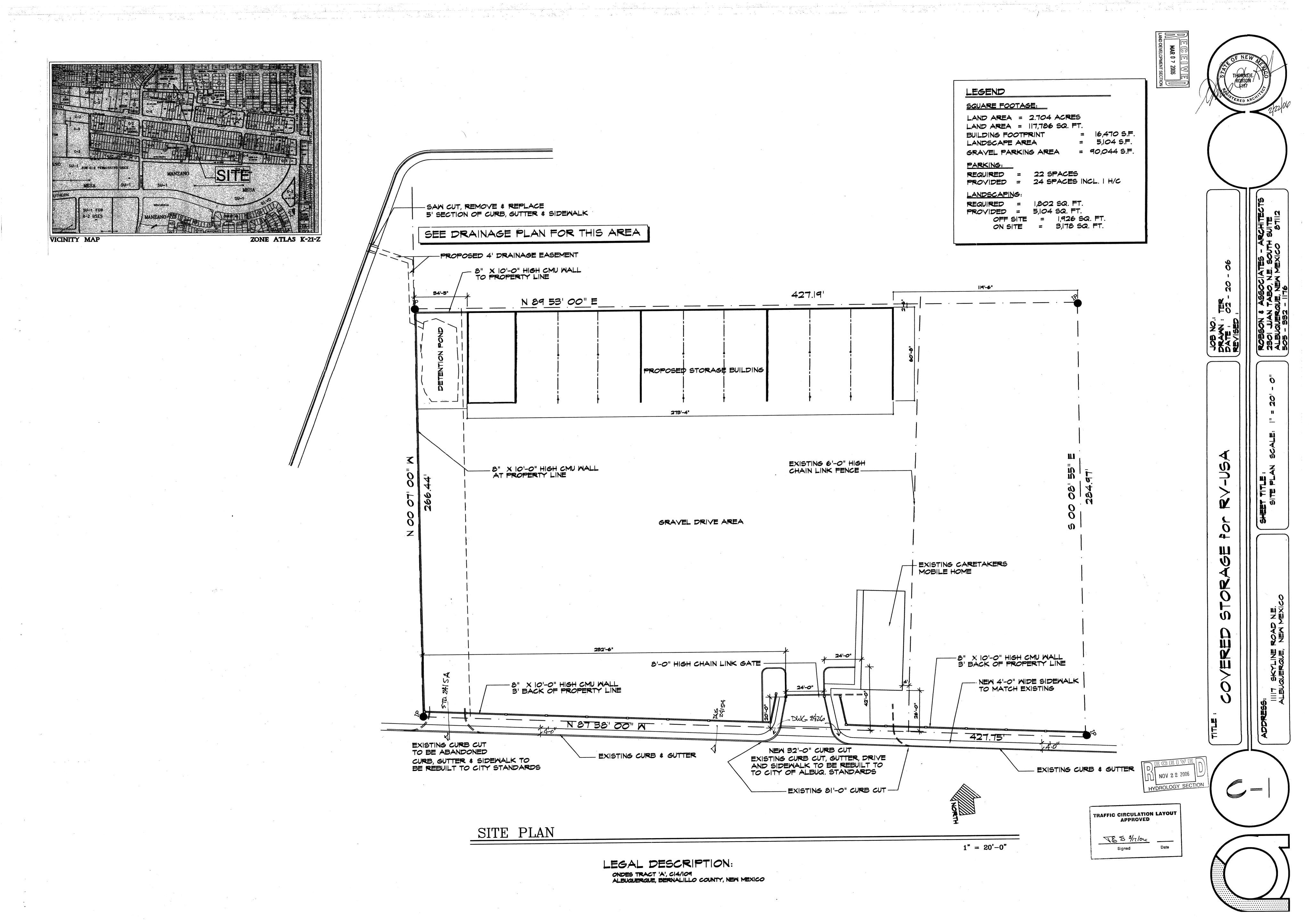


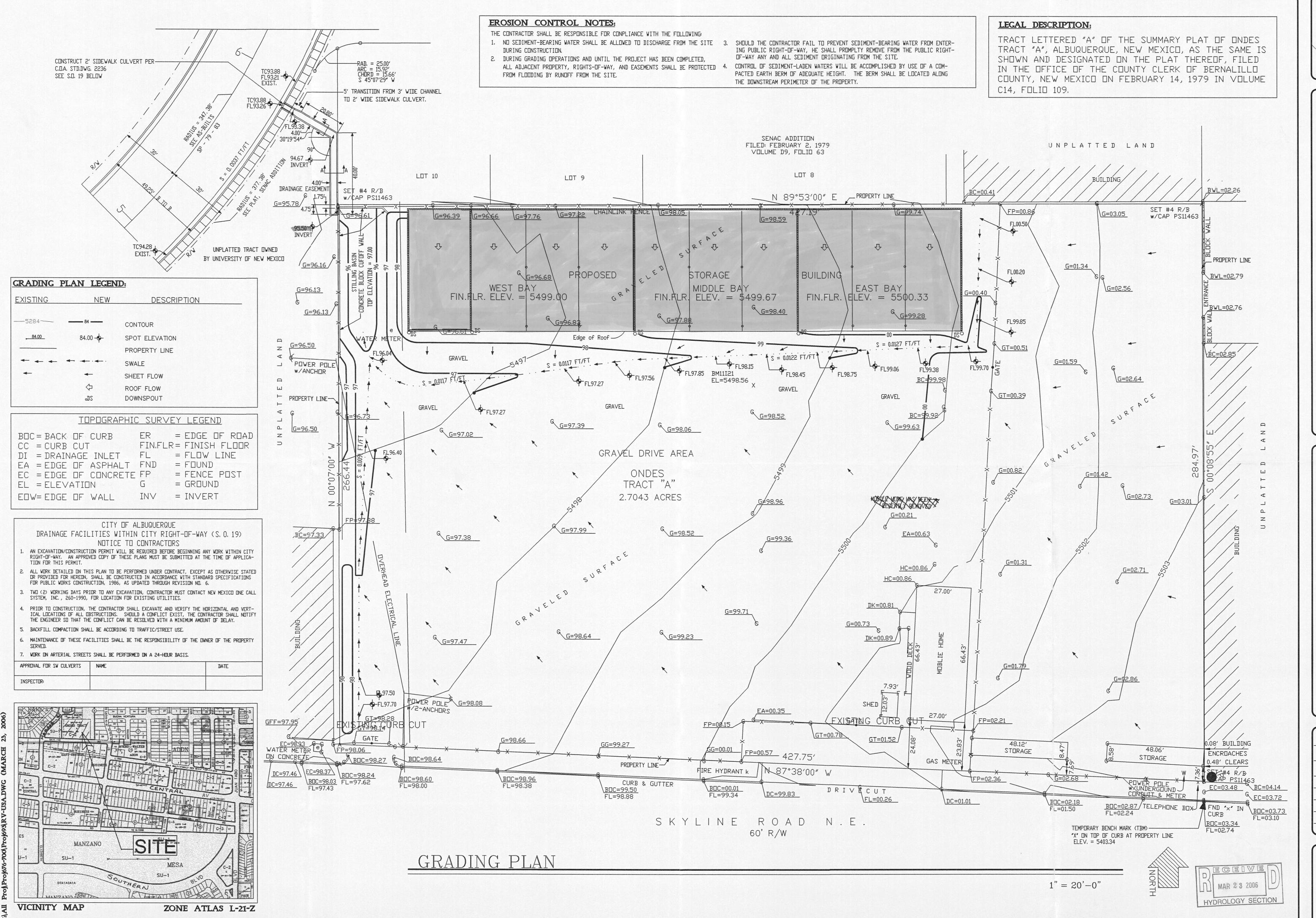
FEBRUARY 22 , 2006 Revised 03/23/06

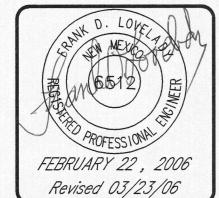




- ON THE PLAT OF RECORD AS STATED BELOW.
- FILED: FEBRUARY 14, 1979 IN VOLUME C14, FOLIO 109
- 4: FIELD WORK PERFORMED ON: <u>December, 2005</u>
- 5: CONTOUR INTERVAL IS ONE (1) FOOT.
- 6: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE STATION No. "11-L21", HAVING AN ELEVATION OF <u>5498.56</u>







ERADING AND DRAINAGE PLAN
RV-USA

TITT SKYLINE ROAD NE

72.47.404. P. E.

345-2267 * Fax (505) 345-2115 * 300 ALAMOSA RD. NW * Albuquerque, NM * 87107

JOB NO: 693

DATE: March 23, 2006

REVISIONS

REVISIONS

TOF 2

EXISTING CONDITIONS:

The site is located on the north side of Skyline Drive about 200' east of Elizabeth Avenue, NE.

The site is almost completly unoccupied except for a mobile home and some small storage sheds.

The area north of the site is occupied by an apartment complex and RV storage sheds similar to the ones proposed for this project. The Area to the East is also RV storage with no runoff leaving the site. The area west of the site is unplatted property which is prosumed to be considered or possibly leased, by the City of Albuquerque for a water well site.

By the University of Albuquerque for a water well site.

PROPOSED CONDITIONS:

It is proposed to construct an RV storage building on the site as shown. The building roof will slope away from the property line to the front of the building and be conveyed to a swale via which it will be directed to a small stilling basin and channeled to Morris Avenue. No attempt has been made to use the stilling basin to reduce the peak discharge. The basin serves only to collect site runoff for discharge via a concrete channel and sidewalk culvert into Morris Avenue.

DRAINAGE CRITERIA:

The calculations shown on this plan were prepared in accordance with Section 22.2, Hydrology, of the Development Process Manual, Volume 2, Design Criteria, for the City of Albuquerque in cooperation with Bernalillo County, New Mexico and the Metropolitan Arroyo Flood Control Authority, January, 1993.

PRECIPITATION ZONE:

The site is east of Eubank Blvd. However, it is also south of I-40 and is, therefore, in Precipitation Zone 3.

LAND TREATMENT AREAS, ETC.:

The peak discharge per acre and excess precipitation are shown for the four land treatments in Zone 3 in the table below, and the values shown are from the City of Albuquerque D. P. M. Also shown are the existing and proposed land treatment areas.

LAND q(cfs/ac) E (in) Existing Site Areas Developed Site Areas TREAT. 100-yr. 10-yr. 10-yr. ½ Sq.Ft. Acres ½ Sq.Ft. Acres

A 1.87 0.58 0.66 0.19 0.0 0 0.0000 0.0 0 0.0000

B 2.60 1.19 0.92 0.36 0.0 0 0.0000 0.0 0 0.0000

C 3.45 2.00 1.29 0.62 100.0 117,799 2.7043 86.1 101,419 2.328340

D 5.02 3.39 2.36 1.50 0.0 0 0.0000 13.9 16,380 0.3760

PEAK DISCHARGE:

EXISTING CONDITIONS:

Q100 = 2.7043 * 3.45 = 9.33 cfs Q10 = 2.7043 * 2.00 = 5.41 cfs

DEVELOPED CONDITIONS:

Q100 = 2. 3283 * 3. 45 + 0. 3760 * 5. 02 = 9. 92 cfs Q10 = 2. 3283 * 2. 00 + 0. 3760 * 3. 39 = 5. 93 cfs

VOLUME, 100-YEAR AND 10-YEAR, 6-HOUR:

EXISTING CONDITIONS:

V100 = (117, 799 * 1.29) / 12 = 12,663 cf V10 = (117, 799 * 0.62) / 12 = 6,086 cf

DEVELOPED CONDITIONS:

V100 = (101, 419 * 1.29 + 16, 380 * 2.36)/12 = 14, 124 cf

V10 = $(101, 419 \times 0.62 + 16, 380 * 1.50)/12 = 7,287 \text{ cf}$

SUMMARY OF ON-SITE VOLUMES AND DISCHARGE RATES:

	V100(CF)	V10(CF)	Q100(CF5)	Q10(CF3)
DEVELOPED	14, 124	7,287	9. 92 🗸	5. 93
EXISTING	12, 663	6, 086	9. 33 🗸	5. 41
INCREASE	1,461	1,201	0. 59 🗸	0. 52

SIDEWALK CULVERT:

USE MANNING'S EQUATION EQUATION, Q = A (1.486 / N) $(R)^{2/3}$ (S)^{1/2} A = 0.667 X 2.0 = 1.33 SF P = 0.667 + 2.0 + 0.667 = 3.33 FT, R = A / P = 1.33 / 3.33 = 0.3994 Manning's n = 0.013 (concrete) Slope (S = 0.0200 ft./ft.)

Q = 1. 33 (1. 486 / 0. 013) (0. 3994)2/3 (0. 0200)1/2 = 11. 66 CFS > 9. 92 CFS (ADEQUATE) CHANNEL NORTH 40 FEET TO DOG LEG:

USE MANNING'S EQUATION Q = A (1. 486 / N) $(R)^{2/3}$ ($S)^{1/2}$ USE 3' WIDE CHANNEL WITH 8' SIDES. A = 0. 667 X 3. 0 = 2. 00 SF P = 0. 667 + 3. 0 + 0. 667 = 4. 33 FT, R = A / P = 2. 00 / 4. 33 = 0. 4619 At owner' w request, try using concrete curbs to contain the flow and line bottom of channel with gravel. Slope = 10 inch drop in first 40 feet = 0. 0208 ft. /ft. Manning's n = 0. 013 (concrete) Manning's n 0. 035 (rock bottom) Weighted n = (0.013 x 16 + 0.035 x 36) / 52 = 0.0282 Q = 2.00 (1. 486 / 0.0282) (0.4619) Q = 2.00 (1.486 / 0.013) (0.4619) Q = 2.00

Q = 2.00 (1.486 / 0.013) (0.4619) $^{2/3}$ (0.0208) $^{1/2}$ = 19.70 CFS > 10.66 CFS (TIII LARGE) TRY 6' DEPTH. A = 3.0 X 0.5 = 1.5 SF P=0.5 + 3.0 + 0.5 = 4.0 FT R=A / P = 1.5 / 4.0 = 0.375 Q = 1.50 (1.486 / 0.013) (0.375) $^{2/3}$ (0.0208) $^{1/2}$ = 12.85 CFS > 9.92 CFS (ADEQUATE)

CHANNEL - FROM DOG LEG TO SIDEWALK CULVERT:

Manning's n = 0.013 Width = 3.0' Depth = 4' Area = 1.00 sf P = 0.33 + 3.0 + 0.33 = 3.66' Slope = 1.29 / 23.2 = 0.0556 ft/ft R = A / P = 1.00 / 3.66= 0.2732 ft Q = 1.00 (1.486 / 0.013) (0.2732) $^{2/3}$ (0.0556) $^{1/2}$ = 11.34 CFS > 9.92 CFS (ADEQUATE)

FLOOD HAZARD MAP:

THERE ARE NO FLOOD ZONES ON OR ADJACENT TO THIS SITE.

DOWNSTREAM CAPACITY:

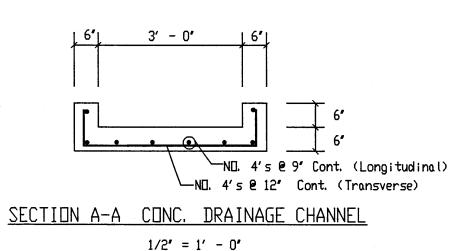
THE RUNDFF FROM THIS SITE ENTERS MORRIS AND IS CONVEYED NORTH VIA THE STREET AND STORM DRAINAGE SYSTEM TO THE DRAINAGE CHANNEL BETWEEN THE EAST-BOUND AND WEST-BOUND LANES OF I-40. DOWNSTREAM CAPACITY IS ADEQUATE.

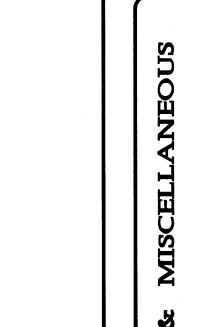
OFF-SITE FLOW:

THERE IS NO OFF-SITE FLOW EFFECTING THIS SITE.

GENERAL NOTES:

- 1: OWNER OF RECORD <u>MICHAEL J. and CAROL WARREN TRUST</u>
- 2: LEGAL DESCRIPTION AND EASEMENTS SHOWN ARE BASED ON THE PLAT OF RECORD AS STATED BELOW.
- 3: PLATS USED TO ESTABLISH BOUNDARY.
 - A: SUMMARY PLAT ONDES TRACT "A"
 - FILED: FEBRUARY 14, 1979 IN VOLUME C14, FOLIO 109
- 4: FIELD WORK PERFORMED ON: _____December, 2005___
- 5: CONTOUR INTERVAL IS ONE (1) FOOT.
- 6: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE STATION No. "11-L21", HAVING AN ELEVATION OF <u>5498.56</u>.
- 7: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS—BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND/OR DEPTH PRIOR TO EXCAVATION OR DESIGN CON—SIDERATIONS.





ALBUQUERQUE, N

FEBRUARY 22 , 2006

Revised 03/23/06

