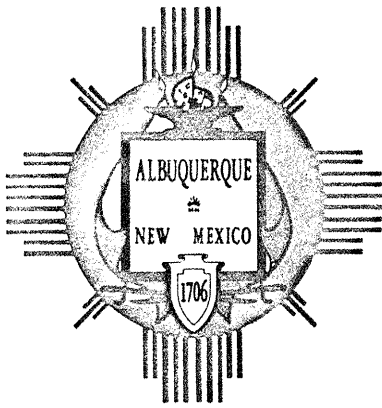




City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

February 20, 1984

Mr. Jeffrey Mortensen
Tom Mann & Associates
811 Dallas NE
Albuquerque, NM 87110

REF: REVISED PLAN FOR DON BARKHURST WAREHOUSE FACILITIES (K21-D7)
RECEIVED JANUARY 27, 1984

Dear Mr. Mortensen:

The above referenced plan, dated January 25, 1984, is approved.

Please attach a copy of this plan along with the approved "Construction Within Public R/W" document to the construction set prior to issuance of the building permit.

If I can be of further service, please contact me at 766-7644.

Sincerely yours,

Billy J. Goolsby, P.E.

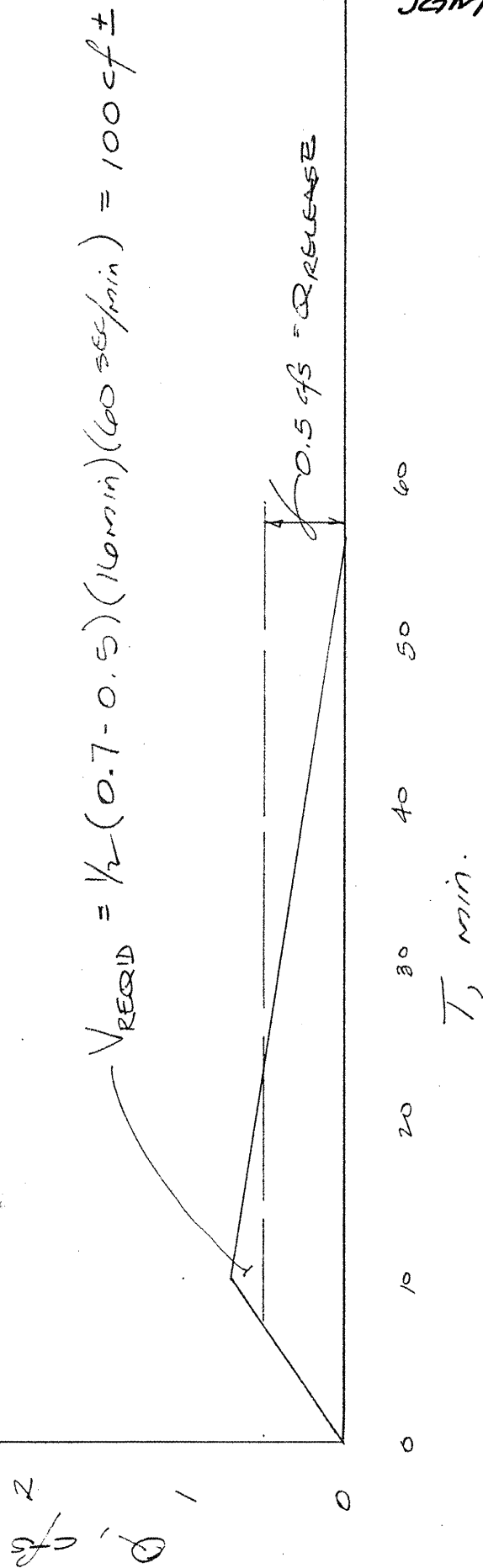
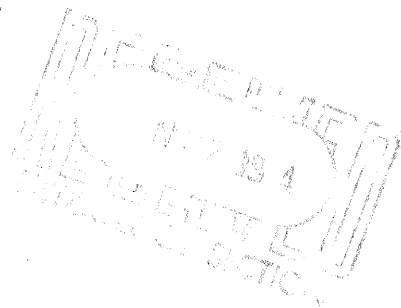
Billy J. Goolsby
City/County Flood Plain Admin.

BJG:mrk

308 MORIEL - BASIN 1

20792
01/24/84
JAM

$$\begin{aligned} Q_p &= Q_{100} = 0.7 \text{ cfs} \\ V_{100} &= 1173 \text{ cfs} \\ V_1 &= \frac{1}{2}(0.7)(10)(60) = 210 \text{ cf} \\ V_2 &= V_{100} - V_1 = 963 \text{ cf} \\ T_R &= \frac{2V_2}{Q_p} = 2751 \text{ SEC} \\ &= 46 \text{ min} \\ Q_{\text{RELEASE}} &= 0.5 \text{ cfs} \end{aligned}$$



PROJECT TITLE BARKHURST WAREHOUSE TYPE OF SUBMITTAL REVISED PLAN
ZONE ATLAS PAGE NO. K-21 CITY ADDRESS 308 MURIEL NE
LEGAL DESCRIPTION LOTS 1 & 2, BLOCK 19, E. CENTRAL BUS. ADDN.
ENGINEERING FIRM TOM MANN & ASSOC. CONTACT JEFFREY G. MORTENSEN
ADDRESS 811 DALLAS NE PHONE 285-5611
OWNER DON BARKHURST CONTACT ARCHITECT
ADDRESS _____ PHONE _____
ARCHITECT MILLER & ASSOC CONTACT JIM MILLER
ADDRESS _____ PHONE 884-1255
SURVEYOR TOM MANN CONTACT J.G. MORTENSEN
ADDRESS _____ PHONE _____
CONTRACTOR BARKHURST CONTACT _____
ADDRESS _____ PHONE _____

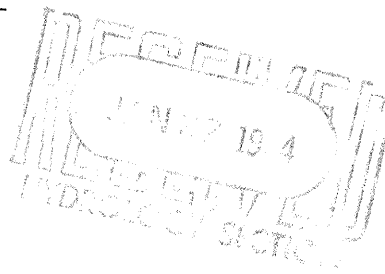
PRE-DESIGN MEETING:

☒ YES
☐ NO
☐ COPY OF CONFERENCE RECAP SHEET PROVIDED

PLEASE CHECK TYPE OF APPROVAL EXPECTED WITH THIS SUBMITTAL:

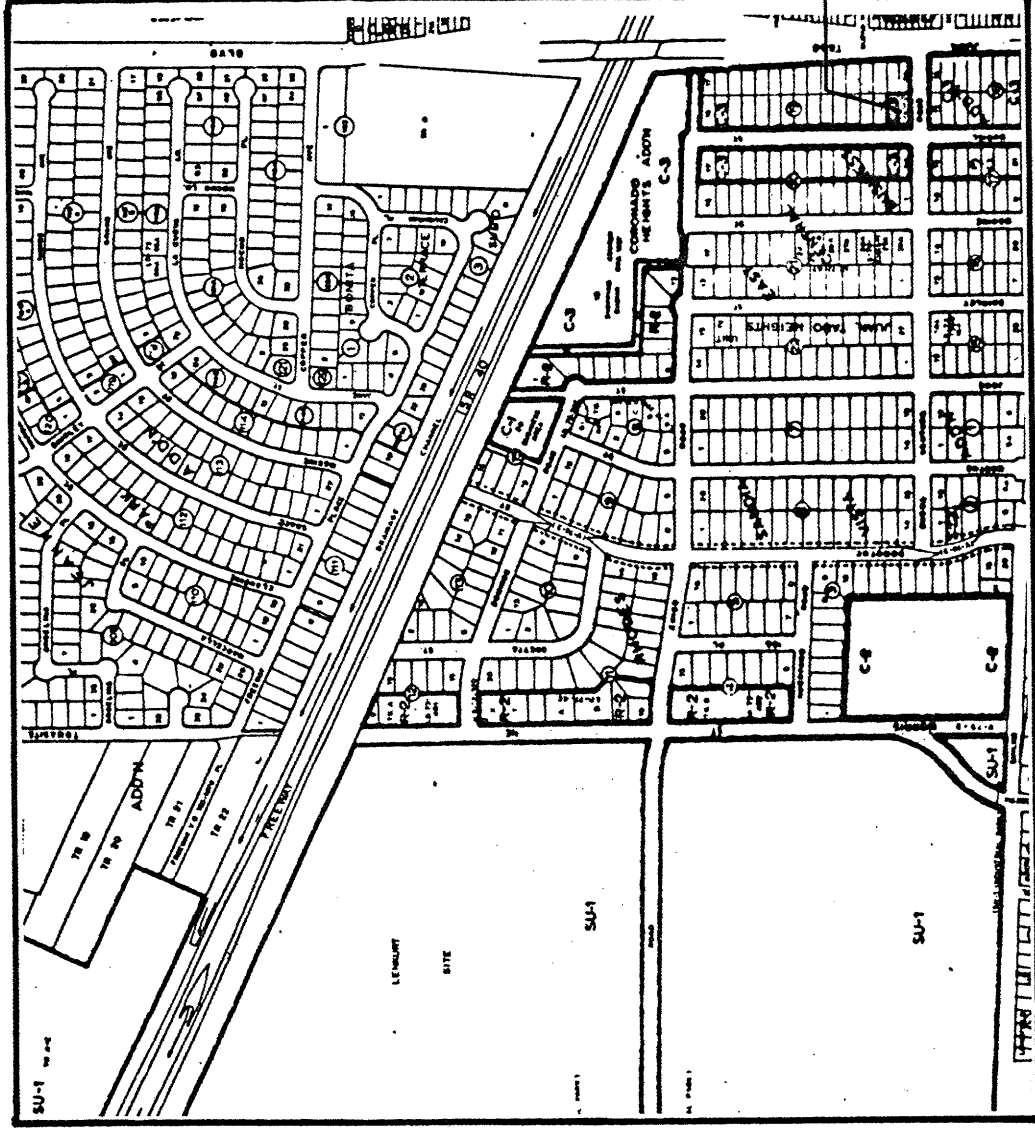
☐ SKETCH PLAT APPROVAL
☐ PRELIMINARY PLAT APPROVAL
☐ SITE DEVELOPMENT PLAN APPROVAL
☐ FINAL PLAT APPROVAL
☒ BUILDING PERMIT APPROVAL
☐ CERTIFICATE OF OCCUPANCY APPROVAL
☐ ROUGH GRADING PERMIT APPROVAL
☐ GRADING/PAVING PERMIT APPROVAL
☐ OTHER _____ (SPECIFY)

ATTN: JOHN ARMSTRONG

DATE SUBMITTED: 01/26/84BY: J.G. MORTENSEN

new report

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY

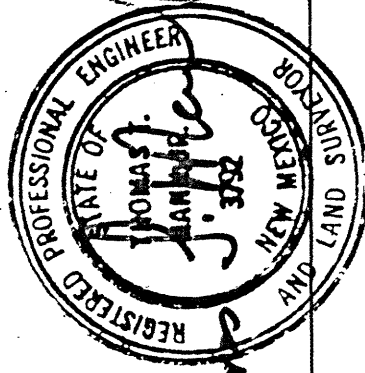


VICINITY MAP

SCALE: 1" = 800

K-21

PROJECT
SITE



K-21 - D-7

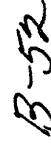
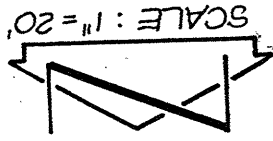
NOTICE TO CONTRACTOR

1. An excavation/construction permit will be required before beginning any work within City right-of-way. An approved copy of these plans must be submitted at the time of application for this permit.
2. All work detailed on these plans to be performed, except as otherwise stated or provided hereon, shall be constructed in accordance with "Contract Documents for City-Wide Utilities and Cash Paving No. 31" *Public Works Contract 87-1*
3. Two working days prior to any excavation, contractor must contact Line Locating Service, 765-1234, for location of existing utilities.
4. Prior to construction, the contractor shall excavate and verify the horizontal and vertical locations of all obstructions. Should a conflict exist, the contractor shall notify the engineer so that the conflict can be resolved with a minimum amount of delay.
5. Backfill compaction shall be according to RESIDENTIAL street use.

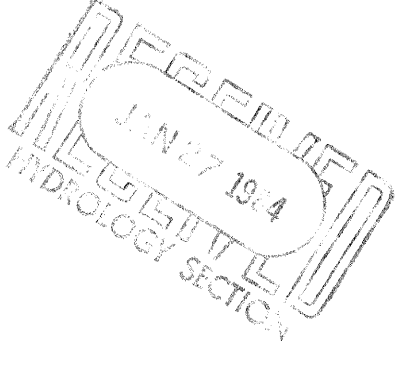
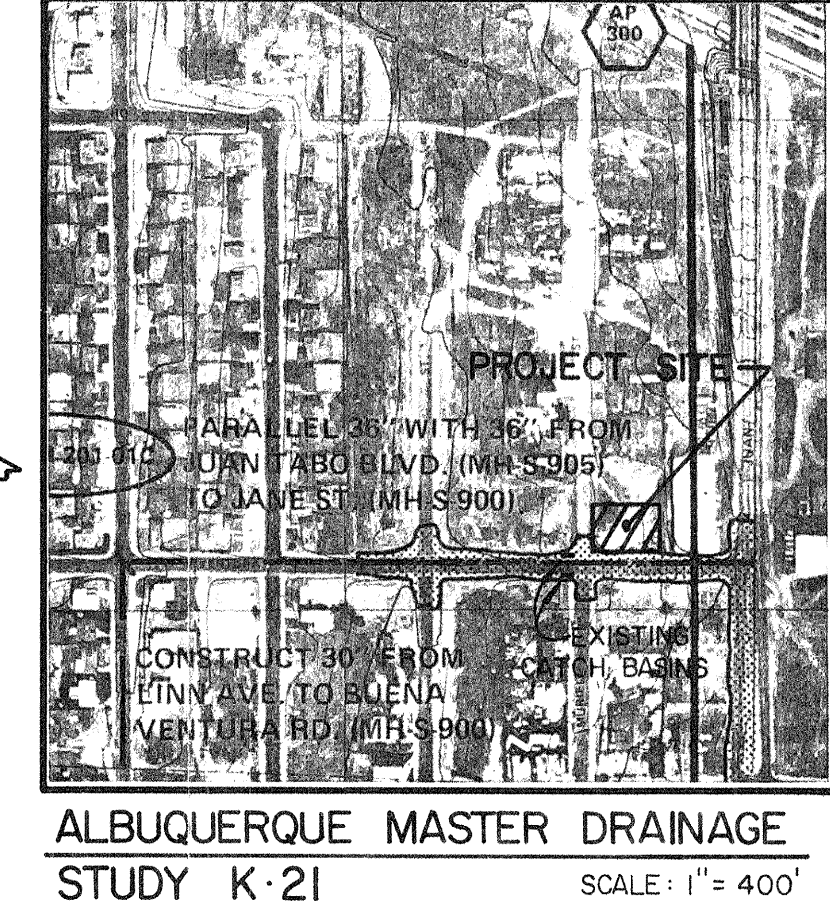
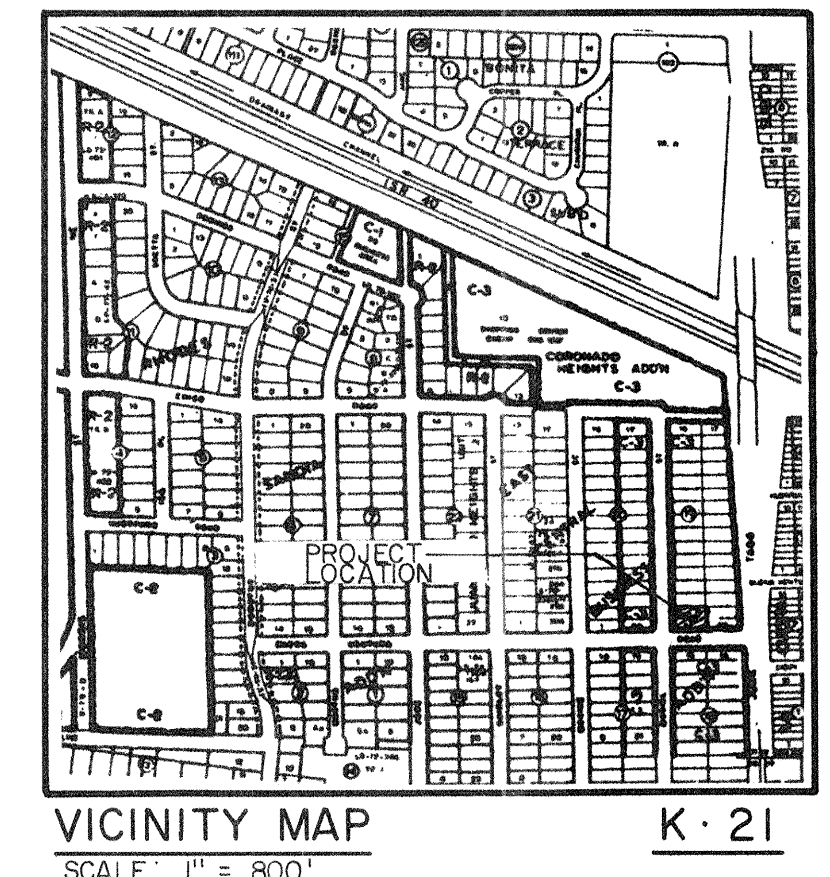
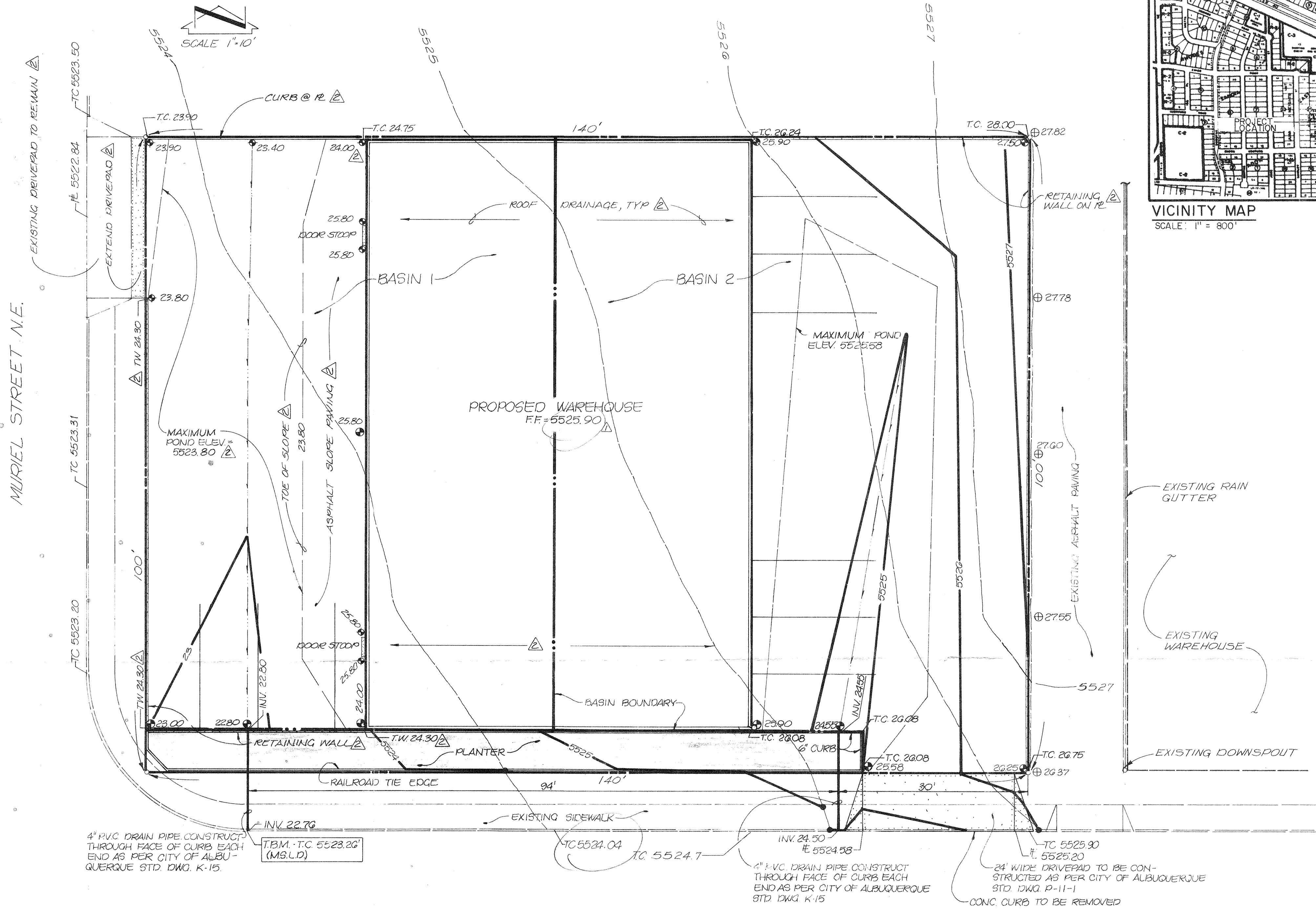
P-10277 Hep/Kins dwn Dr B-52

APPROVALS	NAME	DATE	TITLE:
A.C.E./DESIGN	<i>[Signature]</i>	<i>6 Oct 82</i>	DON BARKHURST WAREHOUSE
INSPECTOR	<i>[Signature]</i>	<i>3-6-83</i>	308 MURIEL ST. N.E.
		<i>4-6-84</i>	PERMIT NO.
			MAP K-21

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY



APPROVALS	NAME	DATE	TITLE: DON BARKHURST WAREHOUSE 308 MURIEL ST. N.E. PERMIT NO. SHEET 2 OF 2 MAP NO. K-21
A.C.E./DESIGN	<i>M. J. J. J.</i>	<i>5 Oct 82</i>	
INSPECTOR			
A.C.E./FIELD	<i>M. J. J. J.</i>	<i>3/8/82</i>	



The following items concerning the Don Barkhurst Warehouse Facilities Drainage Plan are contained hereon:

1. Vicinity Map
2. Flood Hazard Plate
3. Grading Plan
4. Calculations

The proposed improvements, as shown by the Vicinity Map, are located at the northeast corner of the intersection of Muriel Street N.E. and Buena Ventura Road N.E. The site is more particularly described as Lots 1 and 2, Block 19, East Central Business Addition. At present, the site is partially developed; a mobile home currently sits on the site. The land to the north of the site is undeveloped. The land to the east of the site is developed with an existing warehouse building.

The site lies adjacent to a designated flood hazard zone, as shown by Plate K-21 of the Albuquerque Master Drainage Study. Review of the Albuquerque Master Drainage Study shows that the 100-year flow depth in Buena Ventura Road reaches 0.83 feet. This depth just barely tops the existing curb in Buena Ventura Road. This flood water, however, does not enter private property. In addition, offsite flows do not enter this site. The existing warehouse to the east drains its roof water to a rain gutter which empties into two downspouts; one, adjacent to the southeast corner of the site and another approximately 100 feet north of the northeast corner of the site. Very little runoff is expected from the existing asphalt paving along the east property line. This paving is slightly inverted and drains to Buena Ventura Road.

The Grading Plan shows (1) existing and proposed grades indicated by spot elevations and contours at 1'-0" intervals, (2) continuity between existing and proposed elevations and (3) the limit and character of the proposed improvements. As shown by this plan, the proposed improvements include the construction of a new warehouse building and the construction of adjacent asphalt parking and necessary landscaping. The grading required by these improvements will not alter the existing drainage pattern. Runoff will continue to flow to the intersection of Muriel Street and Buena Ventura Road. This runoff will, however, be temporarily ponded in Basins 1 and 2 and will ultimately be discharged to Buena Ventura Road N.E. The two ponds almost have sufficient capacity to contain the runoff volume anticipated for the 100-year, 6-hour storm in the fully developed state. The flooding of the warehouse will not be a concern, however, because the proposed driveways will serve as spillways. Basin 1 will pond to elevation 5523.62 before it spills across the westerly driveway. Basin 2 will pond to elevation 5525.58 before it spills across the driveway at the southeast corner of the site. Both ponds will be paved with asphalt, therefore, ponding within 15 feet of the building foundation will not be a concern.

The calculations which appear below analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational method has been used for this analysis in accordance with the City of Albuquerque Development Process Manual, Volume 2. As shown by these calculations, only a minor increase in runoff is anticipated. Neither of the planter areas are considered in the calculations because they will contain all runoff which falls within them. Lastly, as shown by the AMDS Plate K-21, that runoff which is discharged from the site will be intercepted by the existing catch basins at the downstream intersection. This, of course, assumes that the catch basins are functioning properly, are not clogged, and are not full. The 4-inch PVC drain pipes which drain Basins 1 and 2 will control the discharge of storm water onto Buena Ventura Road. This hopefully will alleviate a part of the current flooding problem along Buena Ventura Road until such time as the recommendations of the Albuquerque Master Drainage Study are implemented.

Elev.	A, sf	Vol., cf	Σ Vol., cf
25.58	1,798		
25.00	567	686	686
24.55	-0-	128	814

OTHER INFORMATION

$V_{100} - V_{pond} = V_{direct} = 571 \text{ cf}$
 $C_{corrected} = 0.39$
 $Q_{direct} = 0.32 \text{ cfs}$

$Q_{pipe} = 19.636Kd^2/\sqrt{h} = 178 \text{ gpm} = 0.40 \text{ cfs}$
where $K = 0.61$
 $d = 4"$
 $h = 25.58 - 24.55 - \frac{2}{12} = 0.86'$

$Q_{100} = Q_{direct} + Q_{pipe} = 0.72 \text{ cfs}$

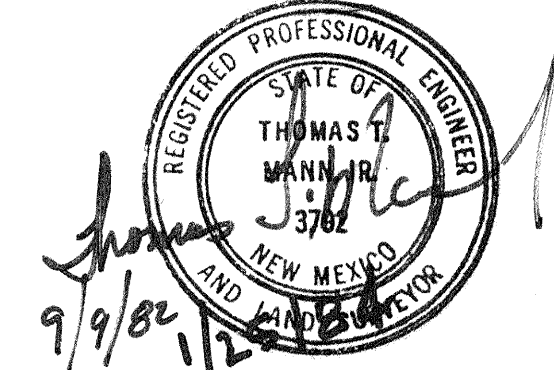
3. ENTIRE SITE
 $Q_{100} = Q_{100, 1} + Q_{100, 2} = 0.34 + 0.72 = 1.07 \text{ cfs}$
 $V_{100} = V_{100, 1} + V_{100, 2} = 2,558 \text{ cf}$

COMPARISON

$Q_{100} = 1.07 - 0.56 = 0.51 \text{ cfs}$
 $\Delta V_{100} = 2,558 - 972 = 1,586 \text{ cf}$
 $V_{direct} = 2,558 - 1,173 - 814 = 571 \text{ cf}$
 $V_{direct} < V_{100, existing}$

1. PROJECT BENCHMARK:
ACS Station 8-K21. A square chiseled in top of concrete curb at the WSW curb return at the intersection of Chico Road N.E. and Jane Street N.E.
Elevation: 5504.69 ft. (MSLD)
2. TEMPORARY BENCHMARK:
Top-of-curb elevation at ENE curb return at intersection of Muriel Street N.E. and Buena Ventura Road N.E. (SEE GRADING PLAN).
Elevation: 5523.26 ft. (MSLD)
3. LEGAL DESCRIPTION:
Lots 1 & 2, Block 19, East Central Business Addition, filed July 15, 1946, Book D, Page 12.
4. ADDRESS:
308 Muriel Street N.E.

APPROVED FOR DRAINAGE
2/20/82
D.E.E.
ADVISE DRAINAGE INSPECTION
WHEN GRADING EXECUTED



- LEGEND**
- EXISTING SPOT ELEVATION
 - PROPOSED SPOT ELEVATION
 - EXISTING CONTOUR
 - PROPOSED CONTOUR
 - SCALE
 - BASIN BOUNDARY

SOIL INFORMATION
From SCS Bernalillo County Soil Survey, Plate 32:
TgB Tijeras gravelly fine sandy loam B Soil

RATIONAL METHOD
1. Discharge: $Q = CiA$
 C varies
 $i = P_6(6.84)T_c^{-0.51} = 5.18 \text{ in/hr}$
where $P_6 = 2.45 \text{ in (DPM Plate 22.2D-1)}$
 $T_c = 10 \text{ min (Minimum)}$
 A varies, Ac.

2. Volume: $V = CP_6A(\frac{1}{12})$
 C varies
 $P_6 = 2.45 \text{ in (DPM Plate 22.2D-1)}$
 A varies, sf
EXISTING CONDITION
 $A = 100' \times 140' = 14,000 \text{ sf} = 0.32 \text{ Ac}$
 $\% \text{ impervious} = 0 \Rightarrow C = 0.34 \text{ (DPM Plate 22.2C-1)}$
 $Q_{100} = CiA = 0.34(5.18)(0.32) = 0.56 \text{ cfs}$
 $V_{100} = CP_6A(\frac{1}{12}) = 0.34(\frac{2.45}{12})(14,000) = 972 \text{ cf}$
DEVELOPED CONDITION
1. BASIN 1:
 $A = 6,050 \text{ sf} = 0.14 \text{ Ac}$
 $\% \text{ impervious} = 100 \Rightarrow C = 0.95$

By hydrograph analysis, $V_{required} = 100 \text{ cf}$

Elev.	A, sf	Vol., cf	Σ Vol., cf
23.80	1970	936	936
23.0	370	37	970
22.8	-0-		

$Q_{release} = CA/\sqrt{2gh}$ (Orifice Equation)
where $C = 0.8$
 $A = 0.0873 \text{ sf (4" dia pipe)}$
 $g = 32.2 \text{ ft/sec}^2$
 $h = 23.8 - 22.8 - 0.17 = 0.8'$
 $Q_{release} = 0.5 \text{ cfs}$

2. BASIN 2
 $A = 7,140 \text{ sf} = 0.16 \text{ Ac}$
 $\% \text{ impervious} = 100 \Rightarrow C = 0.95$
 $V_{100} = 1,385 \text{ cf}$
 $Q_{100} = 0.845 \text{ cfs}$
 $V_{pond} = 814 \text{ cf}$