AGREEMENT AND COVENANT

K-21/D009F

12-14-00

This Agreement and Covenant, between the City of Albuquerque, New Mexico ("City") and Sam's East, Inc., an Arkansas Corporation, ("User") is made in Albuquerque, New Mexico and is entered into as of the date of recording this Agreement with the Bernalillo County Clerk.

1. Recital. The User is the owner of certain real property ("User's Property") located at Chico and Eubank, in Albuquerque, New Mexico, and more particularly described as: (give legal description and filing information)

Lot 4, The Lenkurt Properties filed for record on 4/13/99 in Vol. 99C, Fol. 84C records of Bernalillo County Clerk, Bernalillo County, New Mexico.

The City is the owner of a certain, easement ("City's Property") in the vicinity of, contiguous to, abutting or within User's Property, and more particularly described as:

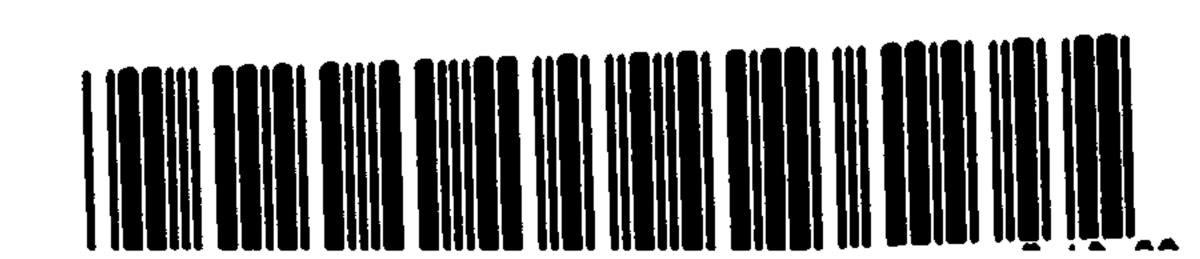
SEE ATTACHED EXHIBIT "A"

The User wishes to construct upon, improve or repair and to maintain the following "Improvement" on the City's Property (or already has done so):

A sketch of the proposed or existing Improvement is attached as Exhibit A and made a part of this Agreement.

The City agrees to permit the Improvement to exist on the City's Property provided the User complies with the terms of this Agreement.

- 2. <u>City Use of City's Property and City Liability</u>. The City has the right to enter upon the City's Property at any time and perform whatever inspection, installation, maintenance, repair, modification or removal ("Work") it deems appropriate without liability to the User, so long as the work does not unreasonably interfere with the operation of the Improvement.
- 3. <u>User's Responsibility for Improvement</u>. The User will be solely responsible for constructing, maintaining, repairing and, if required, removing the Improvement, all in accordance with standards required by the City as per the approved Grading and Drainage Plan <u>K21/D09F</u> on file at the City Engineer's office. The User will be solely responsible for paying all related costs. The User will not permit the Improvement to constitute a hazard to the health or safety of the general public. The User will not interfere with the purpose of the City's Easement or to interfere with the City's use of the City's Property. The User will conform with all applicable laws, ordinances and regulations.
 - 4. Demand for Repair, Modification or Removal. The City may send written notice ("Notice") to



2001004335 5454963 Page: 1 of 7 81/16/2001 10:14A the User requiring the User to repair, modify or remove the Improvement within 30 days ("Deadline") or such longer period of time if reasonably required and the User will comply promptly with the requirements of the Notice. The User will perform all required work by the Deadline, at User's sole expense.

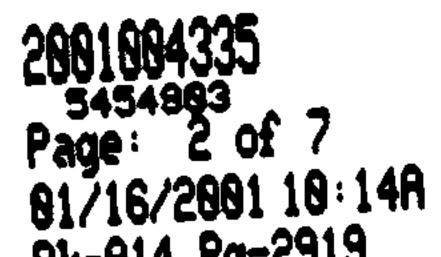
- 5. Failure to Perform by User and Emergency Work by City. If the User fails to comply with the terms of the Notice by the Deadline stated, or, if the City determines that an emergency condition exists, the City may perform the work itself. The City then may assess the User for the cost of the work and for any other expenses or damages which result from User's failure to perform. The User agrees promptly to pay the City the amount assessed. If the User fails to pay the City within thirty (30) days after the City gives the User written notice of the amount due, the City may impose a lien against User's Property for the total resulting amount.
- 6. <u>Cancellation of Agreement and Release of Covenant</u>. Upon prior written approval of the City Engineer, this Agreement may be canceled and User's covenants released by the City by the City's mailing to the User notice of the City's intention to record a Cancellation and Release with the Bernalillo County Clerk. The Cancellation and Release will be effective thirty (30) days after the date of mailing the notice to the User unless a later date is stated in the notice or the Cancellation and Release. After the effective date, the City will record the Cancellation and Release with the Bernalillo County Clerk.
- 7. <u>Condemnation</u>. If any part of the User's Property is ever condemned by the City, the User will forego all claims to compensation for any portion of User's structure which encroaches on City Property and for severance damage to the remaining portion of User's structure on User's Property.
- 8. <u>Assessment</u>. Nothing in this Agreement shall be construed to relieve the User, his heirs, assigns and successors from an assessment against User's Property for improvements to the City Property under a duly authorized and approved Special Assessment District. The parties specifically agree that the value of the Improvement will not reduce the amount assessed by the City.
 - 9. Notice. For purposes of giving formal written notice to the User, User's address is:

Sam's East, Inc. 2001 S.E. 10th Street Bentonville, AR 72712-6489 Attn: Randy Crossno

with copy to: Tierra West LLC Attn. Ron Bohannan 8509 Jefferson NE Albuquerque, NM 87113

Notice may be given to the User either in person or by mailing the notice by regular U.S. mail, postage





paid. Notice will be considered to have been received by the User within 3 days after the notice is mailed if there is no actual evidence of receipt. The User may change User's address by giving written notice of the change by certified mail, return receipt requested, to the City Engineer at P.O. Box 1293, Albuquerque, New Mexico 87103.

- Indemnification. The User agrees to defend, indemnify and hold harmless the City, its 10. officials, agents and employees from and against any and all claims, actions, suits or proceedings of any kind brought against said parties as a result of User's use of the City's Property. To the extent, if at all, Section 56-7-1 NMSA 1978 is applicable to this Agreement, this Agreement to indemnify will not extend to liability, claims, damages, losses or expenses, including attorney's fees, arising out of (1) the preparation or approval of maps, drawings, opinions, reports, surveys, change orders, designs or specifications by the indemnitee, or the agents or employees of the indemnitee; or (2) the giving of or the failure to give direction or instructions by the indemnitee, where such giving or failure to give directions or instructions is the primary cause of bodily injury to persons or damage to property.
 - Term. This Agreement shall continue until revoked by the City pursuant to Section 7 above. 11.
- Binding on User's Property. The covenants and obligations of the User set forth herein shall be binding on User, his heirs assigns and successors and on User's Property and constitute covenants running with User's Property until released by the City.
- 13. Entire Agreement. This Agreement contains the entire agreement of the parties and supersedes any and all other agreements or understandings, oral or written, whether previous to the execution hereof or contemporaneous herewith.
- Changes of Agreement. Changes to this Agreement are not binding unless made in writing, 14. signed by both parties.
- Construction and Severability. If any part of this Agreement is held to be invalid or 15. unenforceable, the remainder of the Agreement will remain valid and enforceable if the remainder is reasonably capable of completion.
- 16. Captions. The captions to the sections or paragraphs of this Agreement are not part of this Agreement and will not affect the meaning or construction of any of its provisions.

CITY OF ALBUQUERQUE:

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USER: Sam's East, Inc., an Arkansas corp.

Assistant Vice President

Dated:

10-13-00 Dated:

x:\pubprop\share\agrekic\samsagr2.cov

Approyed as to legal terms only Wal-Mart Legal Team 10-13-00 Date:

APPROVED: Director, Public Works Depty 2/14	He island
	CITY'S ACKNOWLEDGMENT
STATE OF NEW MEXICO) COUNTY OF BERNALILLO)	SS
This instrument was acknown an acknown and a Chief Chief Chief Corporation, on behalf of the corporation of the corporation and the corporation are the corporation ar	wledged before me on <u>Necember 14</u> ,2000, by ef Administrative Officer for the City of Albuquerque, a New Mexicone corporation.
	Aloria S. Saavedra Notary Public
My Commission Expires:	
11-15-2003	
	USER'S ACKNOWLEDGMENT
STATE OF ARKANSAS COUNTY OF BENTONVILLE	SS.
This instrument was acknowled M. BEDARD ASST. V. PRES, or	edged before me on $Oct-13$, 2000 , by $Robert$ hobels of $SAMSEAST$, INC
My Commission Expires:	Notary Public "OFFICIAL SEAL" BIANCA MARIA THORNTON Notary Public, State of Arkenses Councy of Benton My Commission Exp. 07/01/2010



ExHIBIT "A" PS. 100-3-

LEGAL DESCRIPTION

An Easement situate within Section 21, Township 10 North, Range 4 East, New Mexico Principal Meridian, City of Albuquerque, Bernalillo County, New Mexico, lying within Lot 4, The Lenkurt Properties as the same is shown and designated on said plat filed for record in the office of the County Clerk of Bernalillo County, New Mexico on April 13, 1999 in Volume 99C, Folio 84, and being more particularly described by survey performed by Russ P. Hugg, New Mexico Professional Surveyor number 9750 using plat bearings and ground distances as follows:

BEGINNING at the Southwest corner of the easement herein described, whence the Southwest corner of said Lot 4, The Lenkurt Properties bears S 84°11'34" W, 94.90 feet; Thence,

N 00°14'29" E, 44.64 feet to a point; Thence,

N 90°00'00" E, 45.84 feet to a point of curvature; Thence,

Northeasterly, 39.27 feet along the arc of a curve to the left (said curve having a radius of 25.00 feet, a central angle of 90°00'00" and a chord which bears N 45°00'00" E, 35.36 feet) to a point of tangency; Thence,

N 00°00'00" E, 277.14 feet to a point of curvature; Thence,

Northeasterly, 25.00 feet along the arc of a curve to the right (said curve having a radius of 15.00 feet, a central angle of 95°30'30" and a chord which bears N 47°45'15" E, 22.21 feet) to a point of tangency; Thence,

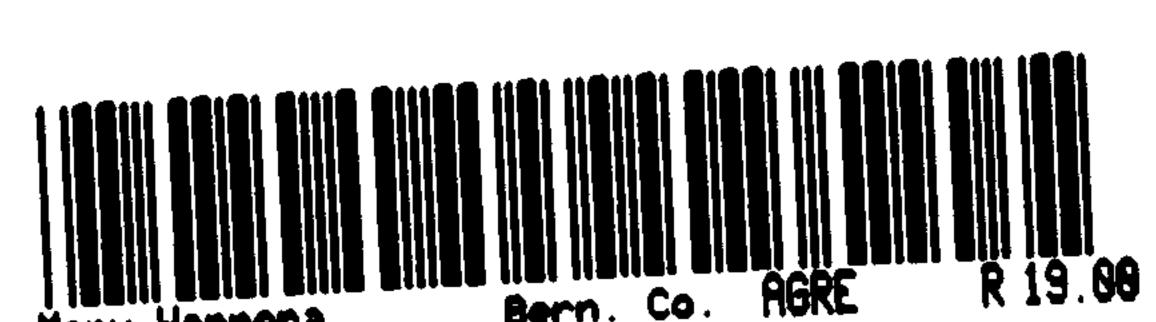
S 84°29'30" E, 24.84 feet to a point of curvature; Thence,

Southeasterly, 19.54 feet along the arc of a curve to the right (said curve having a radius of 15.00 feet, a central angle of 74°38'38" and a chord which bears S 47°10'11" E, 18.19 feet) to a point of tangency; Thence,

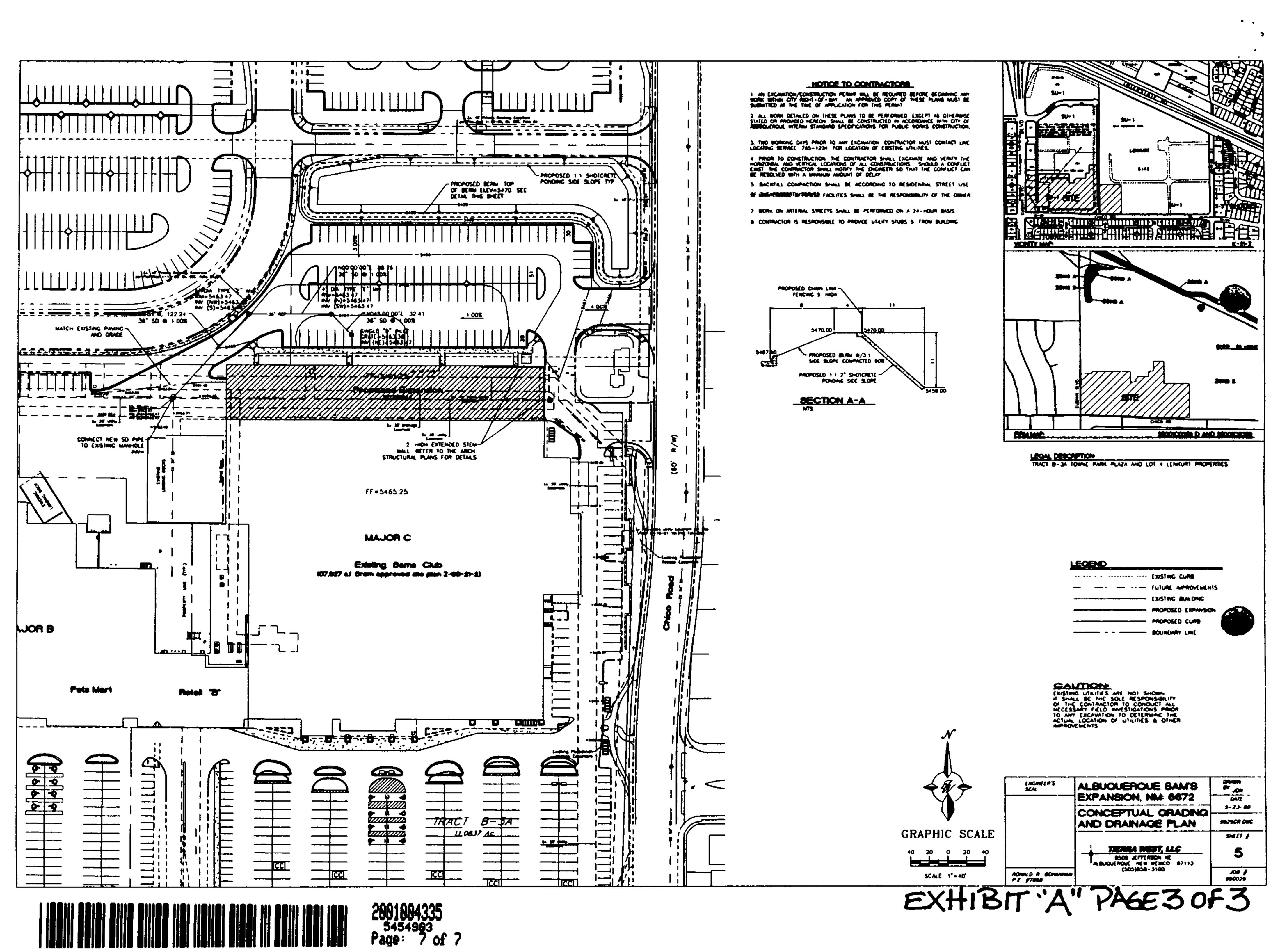
S 00°14'29" W, 283.42 feet to the Southeast corner of the easement herein described; Thence,

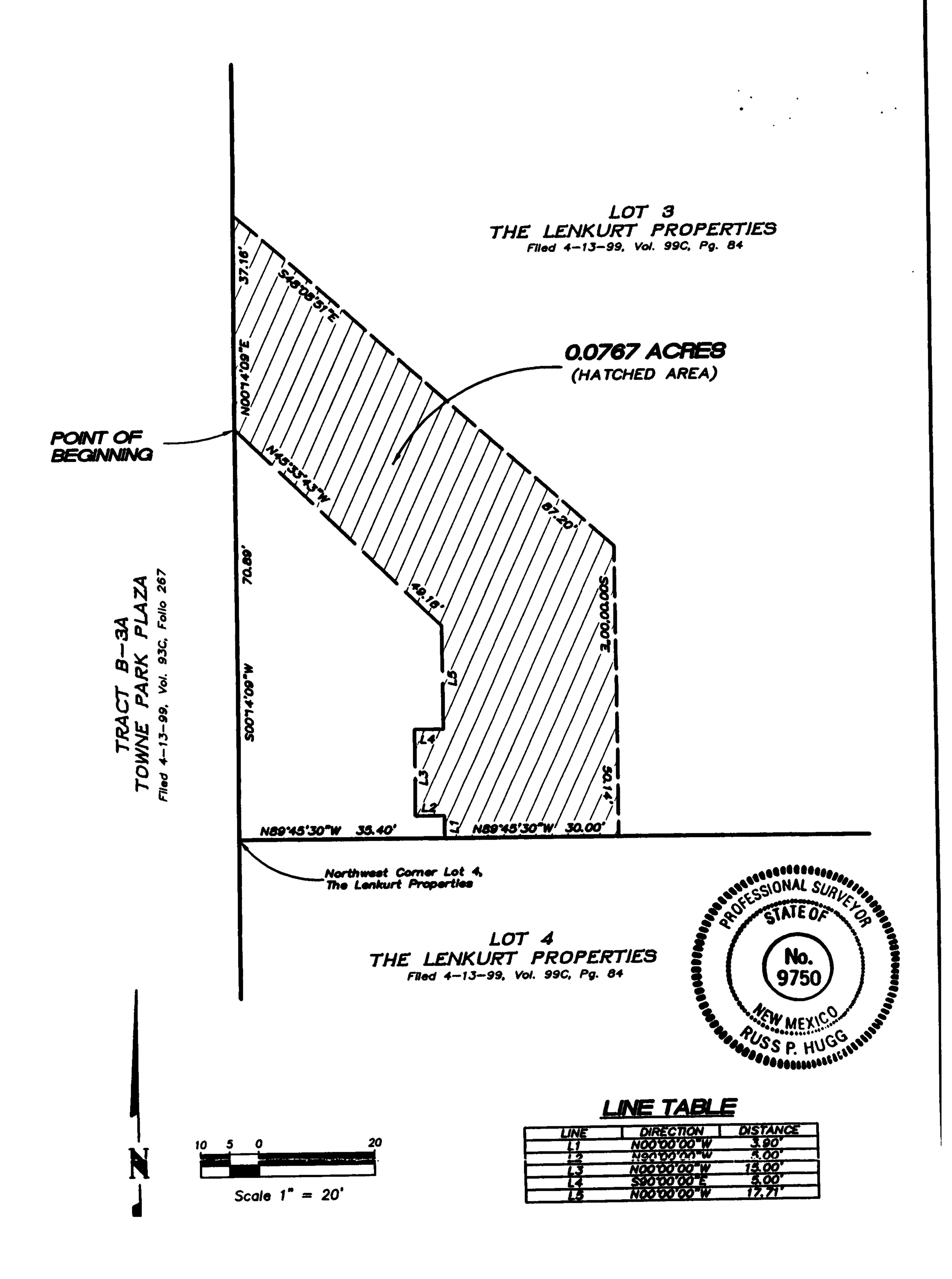
N 89°45'31" W, 135.47 feet to the point of beginning of the easement herein described.

Said easement contains 0.6024 acres, more or less.



2001004335 5454903 Page: 5 of 7 01/16/2001 10:14A Bk-A14 Pg-2919





EXOMIBIT "A"
PJ. 2 0 × 3-1 LOT 3 THE LENKURT PROPERTIES Filed 4-13-99, Vol. 89C, Folio 84C LOT 4
THE LENKURT PROPERTIES Filed 4-13-99, Vol. 99C, Folio 84C POINT OF BEGINNING 58471'34"W 94.90' N6945'31"W 135.47 Mary Herrera R 19.00 Bk-A14 Pg-2919 *60* 15 **30** Scale 1" = 60' NE TABLE DISTANCE 44.64 45.84 24.84 65.08 N90'00'00'E 58429'30 E S09'50'51'E CURVE TABLE DELTA BEARING CHORD TANGENT RADIUS LENGTH CURVE 9000000 N45'00'00'E 25.00° 16.52° 35.36 39.27 25.00 95'30'30" N474515E 22.21 25.00 15.00 \$4770'11'E 74'38'38" 18.19 11.44 19.54 15.00 CJ SURV DIEK, NC.

Consuling Burveyors

From New Marine Risk For A

Phone: 505-897-3366



PRELIMINARY REVIEW

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Contact Person <u>\\\ \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</u>			CCN#		113
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Utilitiy Div					
Hydrology Div	10-3/-00		BUB	11/3/00	
Transportation Div					
DRC Chairman	11-08-00 Aug		148	11-17-00	
Legal Dept	11-20-00 Me		141		د / ۱۱ ک
City Engineer	12-11-00 Aux		J-A	12/8/02	
PWD Director					_
Finance				·	•
City Clerk					-
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City Clerk	1/101	(DM)	HARACO VIIO BUDE	• •	
Treasury			91.8. Hd 61	MAL. FO	
Other:			91 % Md 01	***	

ADDENDUM TO COVER PAGE

10-21-00

		(Date)		
TO:	Kevin Curran, Assistant City Attorn	ey, Legal Departmen	t	
FROM:	Project Administrator, Project Review			
SUBJECT:	PROJECT TITLE: SAM S	LUB COPANS'I	<u>0</u> /PF	ROJECT # 64698
	ed documents have been review, app ed for your action as noted.	roved, initialed and/o	or signed by the	DRC Chairman and
IT	<u>EM</u>	Review & Approval	ACTION Reference	Comments
Procedure Proced	"A"" "B" Modified Non Work Order "C"			

if you have any questions regarding Please Call _ the above or when the documents are ready to be picked up.

No. of Attachments



Other:

City of Albuquerque

July 18, 2000

Ron Bohannan, PE Tierra West LLC 8509 Jefferson NE Albuquerque, NM 87113

Sam's Club East - Albuquerque Store #6672-01 Drainage Report Re: Engineer's Stamp dated 7-14-00 (K21/D09F)

Dear Mr. Bohannan,

Based upon the information provided in your resubmittal dated 7-17-00, the above referenced plan is approved for Site Development Plan for Subdivision, Site Development Plan for Building Permit and Building Permit.

Please be advised that the Grading Permit cannot be approved until the Site Plan is signed off by DRB.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Also, prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

If you have any questions, you can contact me at 924-3986.

Sincerely,
Bulley J. Biryham Bradley L. Bingham, PE Senior Engineer, Hydrology

file

8509 Jefferson NE Albuquerque, NM 87113

(505) 858-3100 fax (505) 858-1118 e-mail: twdms@aol.com 1-800-245-3102

B(C) E I V E

HYDROLOGY SECTION

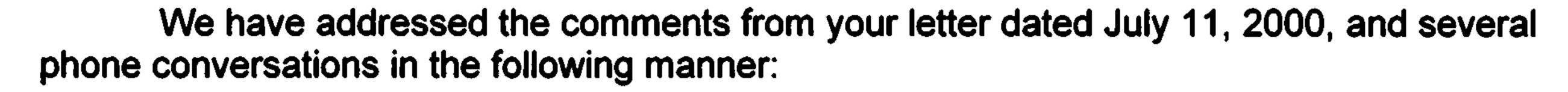
July 14, 2000

Mr. Brad Bingham Senior Engineer/Hydrology City of Albuquerque PO Box 1293 Albuquerque, NM 87103

RE:

Sam's Club East (K21/D09F)

Dear Mr. Bingham:



- Please stamp the Grading Plan and make sure that it has the same date as the report. Also, the north arrow should point to plan left, not as shown. The Grading Plan and drainage report have the same stamp date. Also, the north arrow has been corrected on the Grading Plan and in the report exhibits.
- 2. Please add and label the pipe work needed for interim and ultimate conditions. Also, add any reference to orifice plates as needed. We labeled all pipes needed and included the orifice plate at the pond. We have revised the AHYMO to show the pond draining to the Towne Park Plaza storm drain system. The pond discharges 7.48 cfs to the system. However, the peak flow from the pond reaches the system after the peak has passed for Towne Park Plaza. Consequently, the pond only increases the flow to Eubank by 0.1 cfs. We are no longer showing an ultimate and interim solution. Since the pond has no adverse affect on Towne Park Plaza, it will continue to drain there in the future. A storm drain connection to the Cinemark system is no longer needed.
- 3. You will need to adjust the invert at your proposed Double D inlet and also provide inverts at all pertinent manholes as well. We have shown inverts at all inlets and manholes on the site.
- Show the Water Surface Elevation in the pond 4. This is now shown on the Grading Plan.
- 5.∵ Demonstrate downstream capacity within the Towne Park Plaza storm drain system for the discharge from the pond. This has been shown in the new AHYMO included in the drainage report. The system has capacity for the additional flows.
- 6. Clean up the notes on the Grading Plan and various typos in the drainage report. The notes on the Grading Plan were revised and the drainage report checked.

We addressed all your comments and, if possible, we would like approval for Building Permit as well as Site Plan for Subdivision and Site Plan for Building Permit. If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

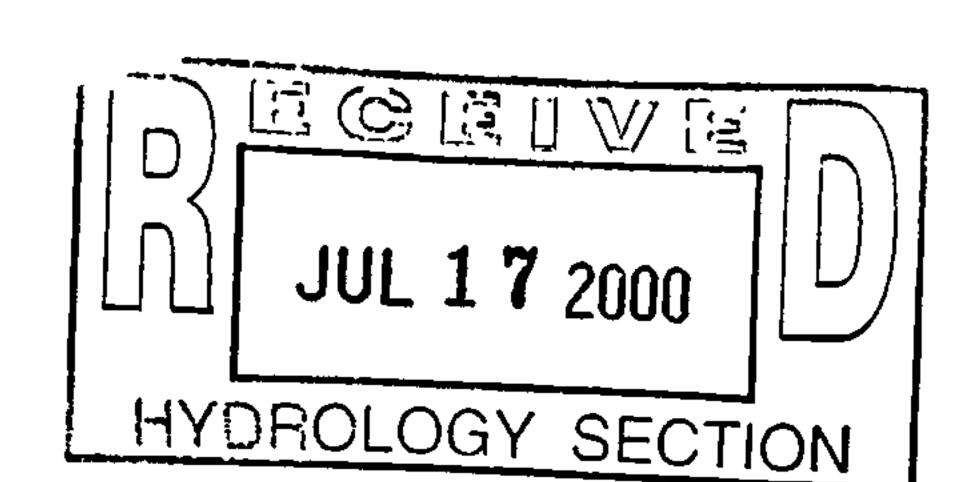
Sara Lavv

Enclosures

cc: Mohsen Ghadimkhani

JN: 990029

SCI 9929 9929 hydrology resubmittal.ttr



DRAINAGE INFORMATION SHEET

PROJEC	TITLE:	Sam's Club East	ZONE ATLAS/DRN	G. FILE #:	K-21/D9F	
DRB #:		EPC #:	WORK ORDER #:		<u>, </u>	
LEGAL 1	DESCRIPTIO	ON: Tract B3A Towne Park Plaza				
CITY AD	DRESS:	Northeast Corner of Chico and Eubank	·			
ENGINE	ERING FIR	TIERRA WEST, LLC	CONTACT:	RONALD	R. BOHANNAN OR SARA	LAVY
ADDRES	3 S :	8509 Jefferson NE, ABQ, NM 87113	PHONE:	(505) 858-	3100	
OWNER	•		CONTACT:		•	
ADDRES	3S :	·	PHONE:		•	
ARCHITI	ECT:		CONTACT:	•		
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BY:

SARA LAVY

HYDDCLOGY SECTION

DRAINAGE REPORT

for

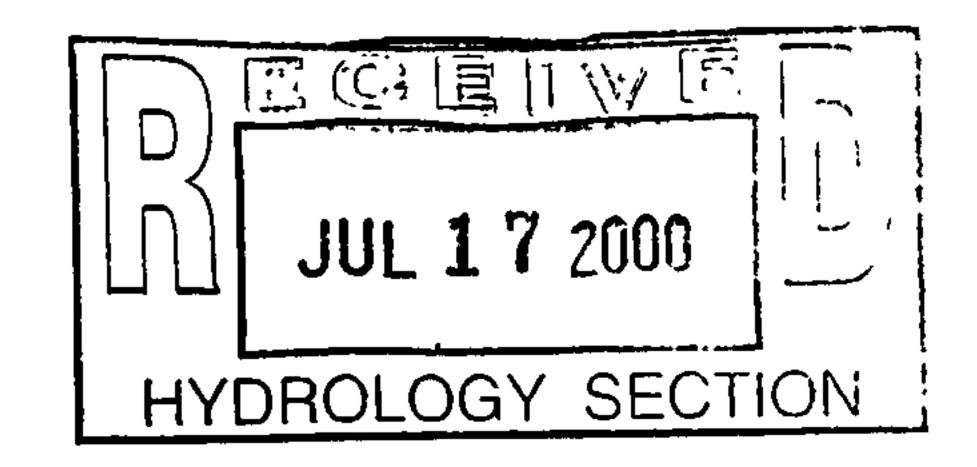
Sam's Club East Store # 6672-01

Prepared by

Tierra West, LLC 8509 Jefferson NE Albuquerque, New Mexico 87113

Prepared for

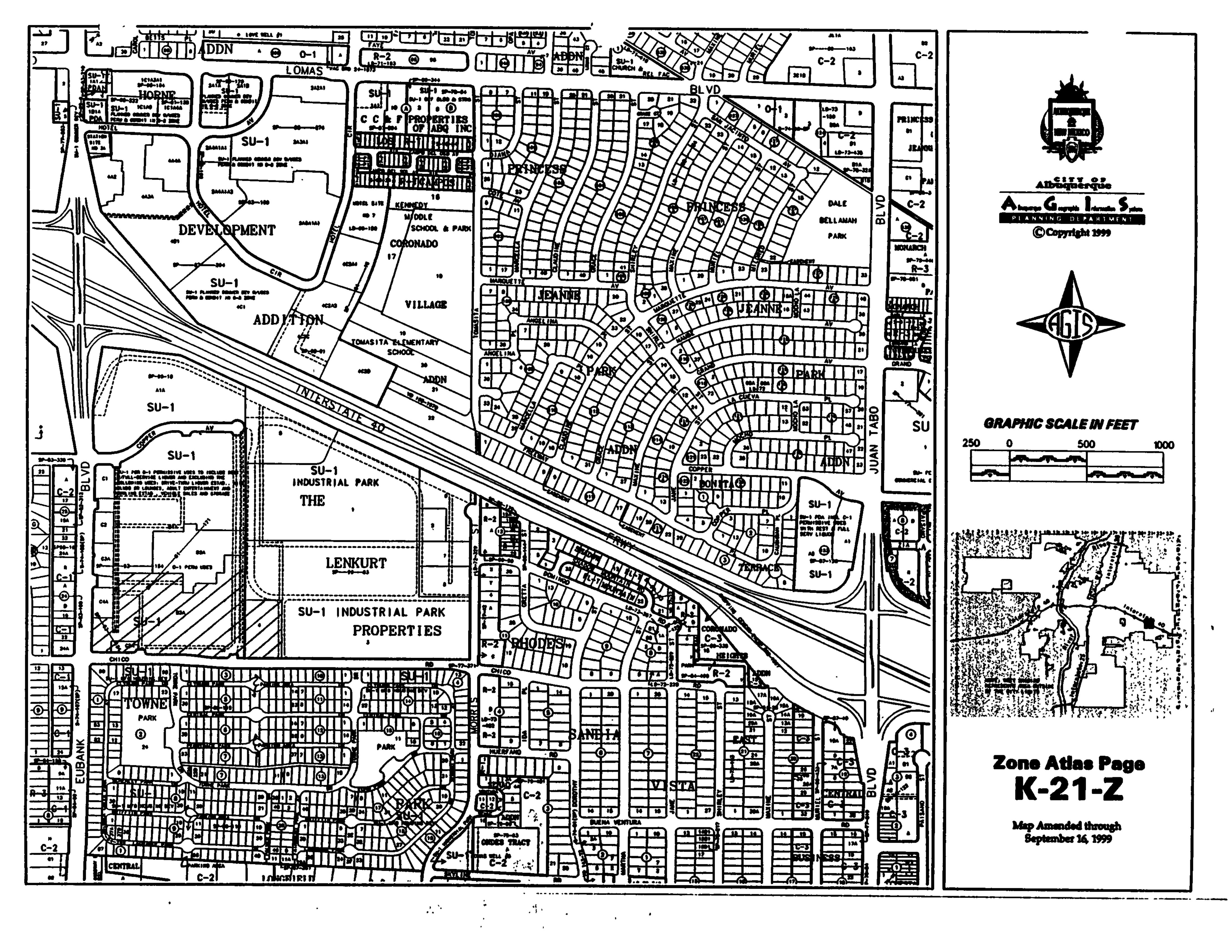
Sam's Club, Inc. 2001 Southeast 10th Street Bentonville, AR 72712-6489



Ronald R. Bohannan P.E. No. 7868

PROFESSIONAL

PROFESSION



Location

The site is located at the northeast corner of Eubank Boulevard and Chico Road and is shown on the attached Zone Atlas Map K-21. The site is the location of an existing Sam's Club and Lot 4 of the adjacent Lenkurt site. The proposed improvements include an expansion of the Sam's Club on the north side of the building and a service station in the southwest corner of the site. The purpose of this report is to provide the drainage analysis and management plan for the new improvements.

Existing Drainage Conditions

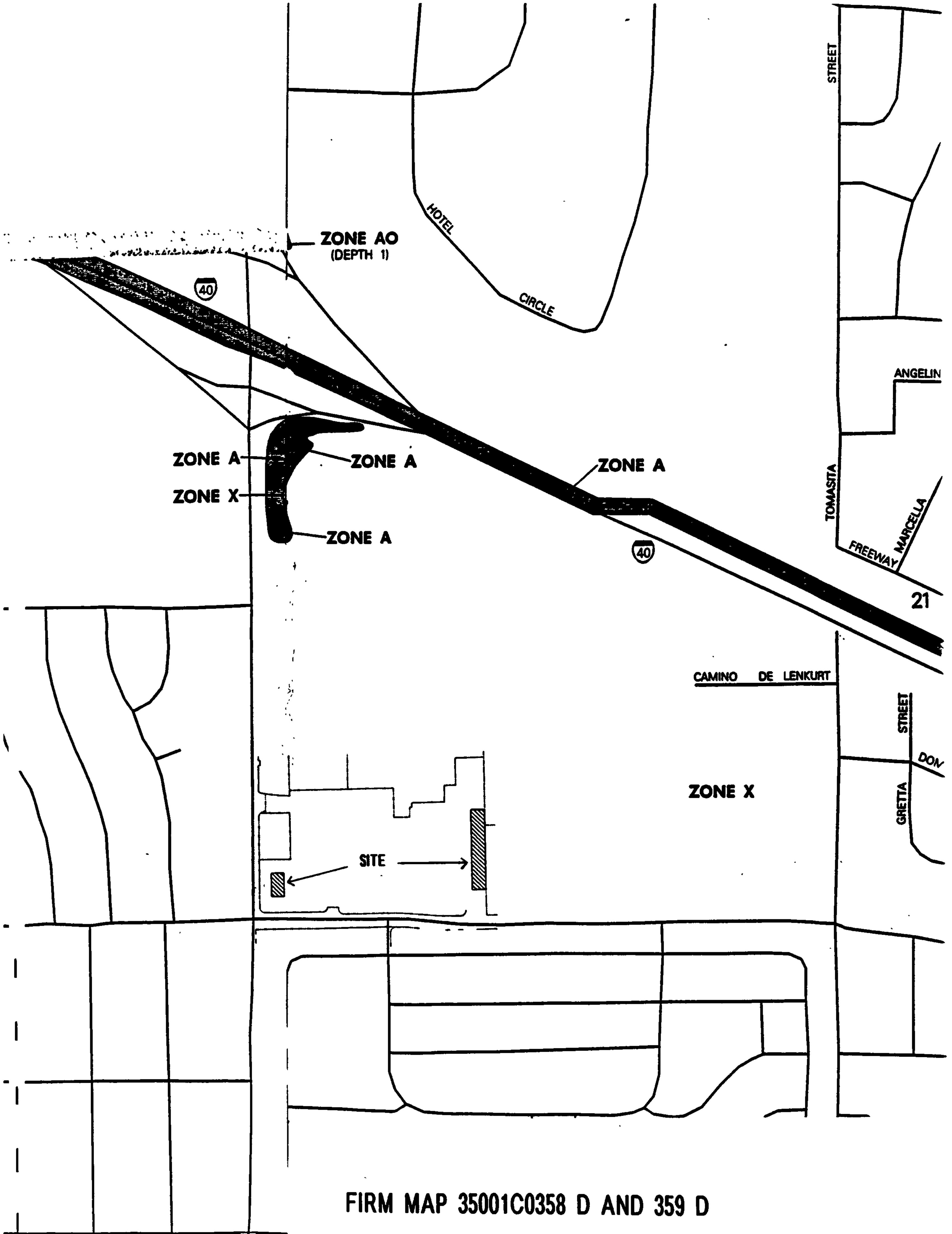
The majority of the site is currently developed. There is an existing Sam' Club and parking lot on the site. The drainage was analyzed by Bohannan-Huston Inc. in the "Master Drainage Report for Towne Park Plaza" in 1991 (K21-D9A). We are not changing the existing drainage patterns of this site.

Lot 4 of the Lenkurt site is undeveloped at this time. A drainage plan for Cinemark was approved and includes this site. That report was done by Bohannan-Huston Inc. in 1999 and is titled "Cinemark Drainage Report" (K21-D9). Lot 4 has been bought by Sam's Club and will be used for additional parking for the site.

No flows enter the site from the south or west sides of the site. A large offsite basin of approximately 180 cfs enters Lot 4 from Chico Road (east of the site). A new detention pond will be constructed east of the expansion to contain these flows. The undeveloped Lenkurt site discharges 9.7 cfs to the existing Wal-Mart and Home Base site. This flow comes from east of the Wal-Mart site. Most of the offsite flow is located on Lot 4 and will be accounted for in Basin 1100-A. In the future, when Cinemark is developed, the remainder of the off-site flow will be contained on-site. In the interim, the portion of the 9.7 cfs that is not within Lot 4 will continue its existing drainage pattern.

FIRM Map and Soil Conditions

The site is located on FIRM Maps 35001C0358 D and 359 D. The maps shows that the site does not lie within any 100 year flood plains.



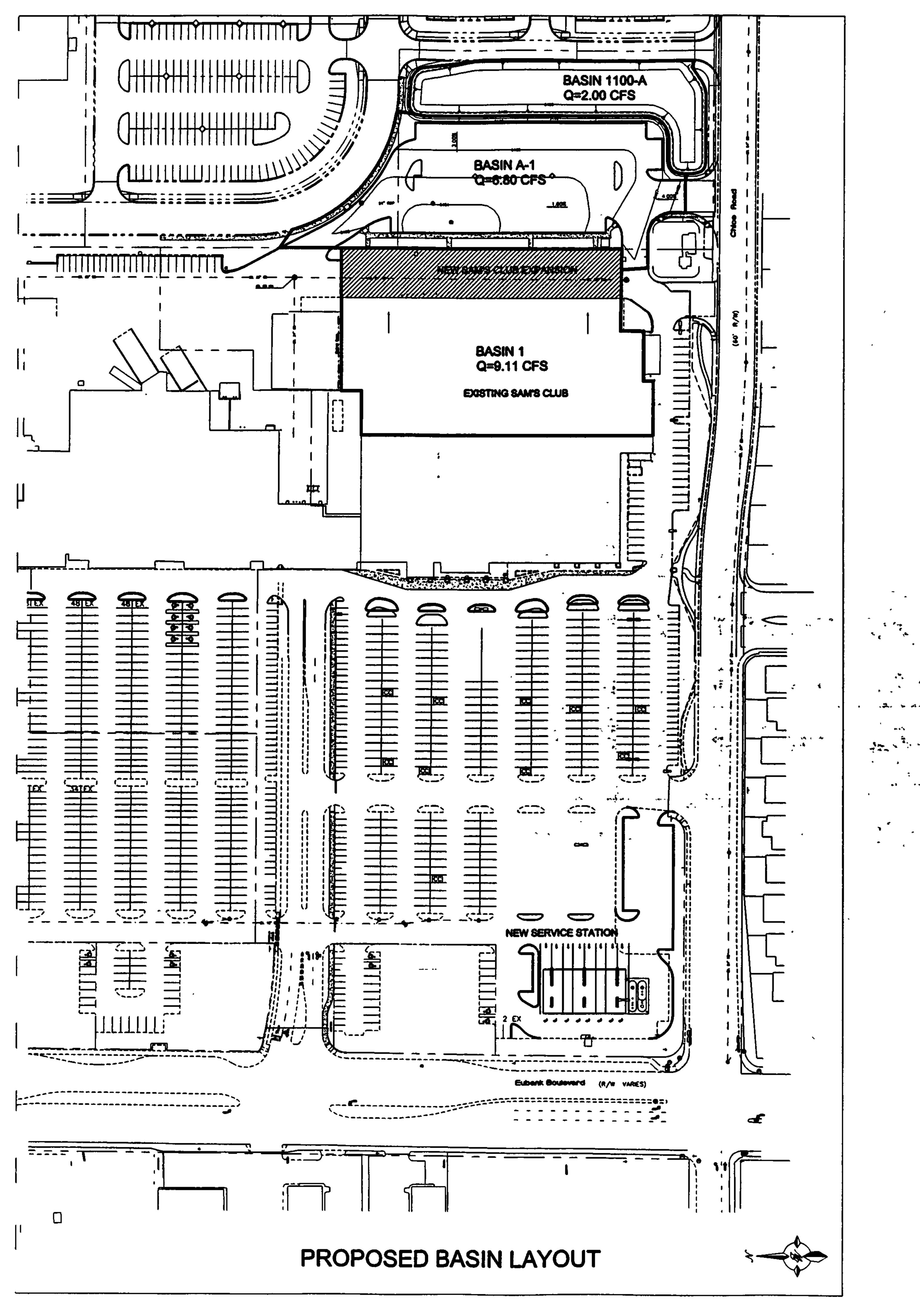
The site contains one soil from the Soil Conservation Service Soil Survey of Bernalillo County. The soil is a Tijeras gravelly fine sandy loam. This soil has moderate runoff and a moderate hazard of water erosion.

On-Site Drainage Management Plan

The onsite drainage management is to continue the existing drainage patterns on the Sam's Club site and construct a pond to contain the offsite flows that enter Lot 4. The new service station and the expansion are located on the existing Sam's Club site and do not change the existing drainage patterns or rates. The new service station does not change the land treatments for the site and will continue to drain in the same manner as previously designed. Currently, the Sam's Club drains through roof drains to the rear of the building. A storm drain collects the flows and they are routed through the Towne Park Shopping Center to the storm drain system in Eubank Boulevard. The expansion interferes with the existing storm drain line behind Sam's Club and will cause the storm drain to be rerouted to Lot 4. The new expansion will continue allowing roof drains to drain into the parking lot. There will be no increase in flows because of the expansion, as the land treatment has not changed. The new relocated storm drain will collect the flows from the expansion and the new parking lot and connect to the existing Towne Park Plaza system.

Lot 4 has been divided into two basins. Basin 1 has a developed flow of 9.11 cfs and consists of the roof drainage from the Sam's Club. Basin A-1 consists of the new parking lot behind Sam's Club and has a developed flow of 6.80 cfs. Both basins will drain to a new drop inlet located in the parking lot that will collect 15.91 cfs. This inlet will connect to the Towne Park Plaza storm drain system. The pipe calculations and the AHYMO show the system has capacity for the flows.

Basin 1100-A has a developed flow of 2.00 cfs and consists of the pond area for the offsite overflow from Chico Road. The drainage report for Cinemark showed all of Lot 4 as a pond for the offsite flows. This pond has been reduced in area to allow for the expansion but increased in depth to continue to provide the required volume of approximately four acre-feet. The new pond was incorporated into the Cinemark AHYMO for the offsite basins. This shows the pond has capacity for the overflow from Chico Road.

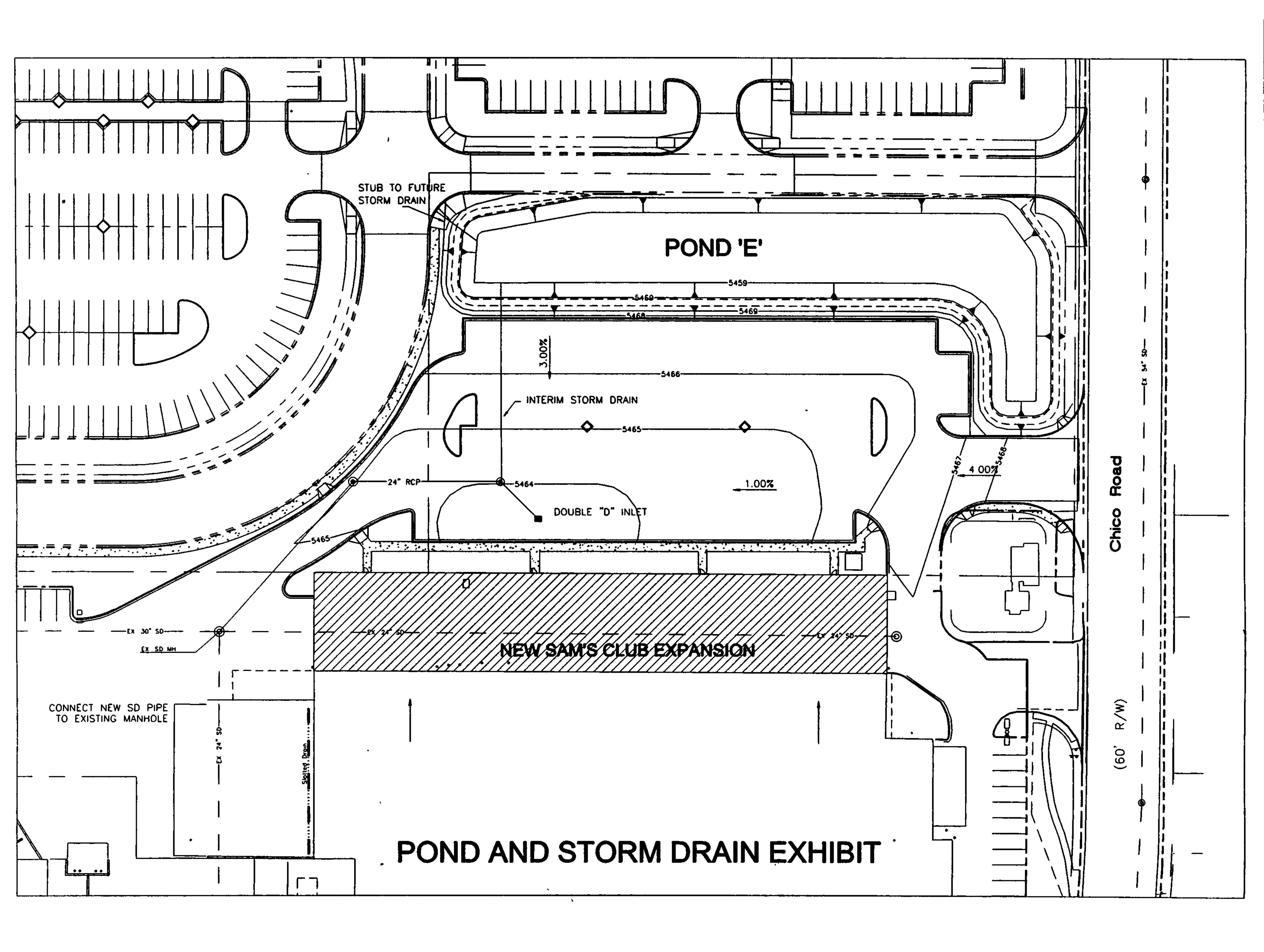


The Cinemark report shows the outfall from the pond connecting to a new storm drain system built with the Cinemark project. However, the Cinemark project has not been constructed to date and the storm drain connection not possible. Consequently, the pond will discharge to the existing Towne Park Plaza storm drain system. The pond does not affect the Towne Park Plaza system because the peak flow time is later than the peak flow from Towne Park Plaza. The Towne Park Plaza storm drain system was originally designed for 17 cfs in the 24" pipe behind Sam's Club. The on-site basins have a total flow of 15.91 cfs and the peak discharge with the pond is increased to 16.01 cfs. This is an increase of 0.1 cfs and less than the system was designed for. We feel that the pond will not adversely affect the Towne Park Plaza system.

In the event of a storm greater than a 100-year storm, the pond will discharge through a 62' wide emergency overflow. The overflow will flow north behind the Sam's Club and will not flood any buildings.

Summary

A new service station and Sam's Club expansion are being added to the existing Sam's Club site. These improvements will not change the existing drainage patterns or flows. Lot 4 of the Lenkurt property has been bought by Sam's Club and is part of this submittal. Lot 4 will consist of a new parking lot and a new pond. The pond and upland flows were analyzed in a previous report by Bohannan-Huston, Inc. for the Cinemark project. The pond will have the same volume and has been analyzed with the Cinemark AHYMO. The new parking lot will discharge 7.48 cfs to the Towne Park Plaza storm drain system that is being relocated with this project. The Towne Park 24" storm drain has capacity for the additional flow.



Runoff Calculations

Drainage Basins

BASIN	AREA (SF)	AREA (AC)	AREA (MI ²)
A-1	63961.00	1.4683	0.002294
1100-A	26878.00	0.6170	0.000964
1	81632.00	1.8740	0.002928

Runoff Calculation Results

BASIN	Q-100	V-100
	CFS	AC-FT
A-1	6.80	0.314
1100-A	2.00	0.074
1	9.11	0.431

RUNOFF CALCULATIONS

The site is @ Zone 3

LAND TREATMENT

Basin	A (%)	B (%)	C (%)	D (%)
A-1	0	10	0	90
1100-A	0	30	0	70
1	0	0	0	100

DEPTH (INCHES) @ 100-YEAR STORM

 $P_{60} = 2.14$ inches

 $P_{360} = 2.56 \text{ inches}$

 $P_{1440} = 3.10 \text{ inches}$

DEPTH (INCHES) @ 10-YEAR STORM

 $P_{60} = 2.14 \times 0.667$

 $\dot{}$ = 1.43 inches

 $P_{360} = 1.73$

 $P_{1440} = 2.07$

Storm Sewer

Pipe Capacity

Pipe	D	Slope	Area	R	Q Provided	Q Required	Velocity
	(in)	(%)	(ft^2)		(cfs)	(cfs)	(ft/s)
1	24	1.3	3.14	0.5	25.86	16.01	5.10
2	15	3.5	1.23	0.3125	12.12	7.48	6.10

Max Outflow from Basin A-1, Basin 1 and Pond Outflow = 16.01 cfs

Manning's Equation:

 $Q = 1.49/n * A * R^{2/3} * S^{1/2}$

A = Area

R = D/4

S = Slope

n = 0.013

DROP INLET CALCULATIONS

TYPE OF	AREA	Q	Н	H ALLOW
INLET	(SF)	(CFS)	(FT)	(FT)
Double 'D'	4.21	15.91	0.6160	0.67

Basin A-1 and Basin 1 = 6.80 + 9.11 = 15.91 cfs

ORIFICE EQUATION

Q = CA sqrt(2gH)

0.

y = 32.2

STORM DRAIN INLET EFFECTIVE AREA ASSUMING A 50% CLOGGING FACTOR

DOUBLE D:

Area at the grate:

Ponding Calculations

VOLUME CALCULATIONS

Ab - Bottom Of The Pond Surface Area

At - Top Of The Pond Surface Area

D - Water Depth

Dt - Total Pond Depth

C - Change In Surface Area / Water Depth

Volume = $Ab * D + 0.5 * C * D^2$

C = (At - Ab) / Dt

Ab = 15,208.32 @ Elevation = 5460

At = 23,167.31 @ Elevation = 5469

Dt = 9.00

C = 884.33

ACTUAL	DEPTH	VOLUME	Q
ELEV.	(FT)	(AC-FT)	(CFS)
5460.00	0.0	0.0000	0.000
5461.00	1.0	0.3593	2.006
5462.00	2.0	0.7389	3.305
5463.00	3.0	1.1388	4.221
5464.00	4.0	1.5590	4.971
5465.00	5.0	1.9994	5.622
5466.00	6.0	2.4602	6.205
5467.00	7.0	2.9413	6.738
5468.00	8.0	3.4427	7.232
5469.00	9.0	3.9644	7.694

Orifice Equation

Q = CA SQRT(2gH)

C = 0.6 Diameter (i 10 Area (ft^2) 0.545

g = 32.2

H (Ft) = Depth of water above center of orifice

Q (CFS)= Flow

Emergency Overflow for Pond 'E'

Weir Equation:

$$Q=CLH^{3/2}$$

Q = 182.05 cfs

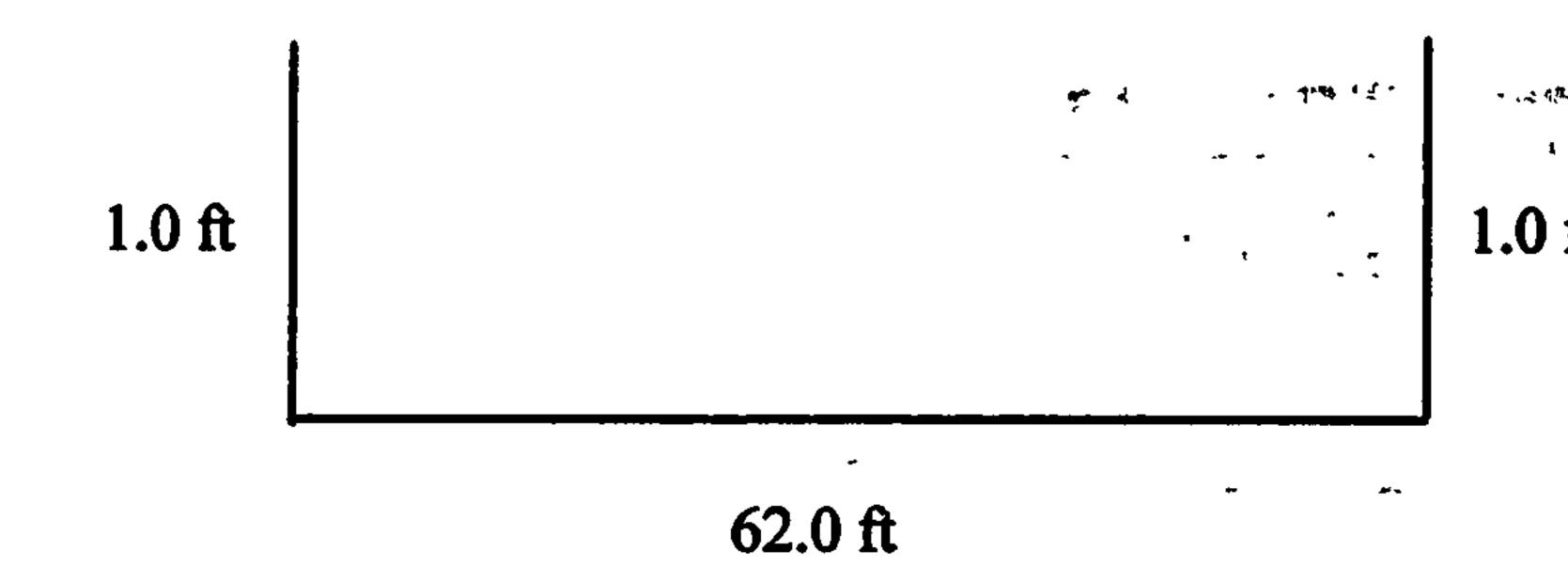
C = 2.95

H = 1.00 ft

L = Length of weir

$$L = \frac{182.05}{2.95(1.0)^{3/2}}$$

L = 61.71 ft Use 62.0 feet for length of weir



AHYMO Summary and Output

RUN DATE (MON/DAY/YR) =07/13/2000 USER NO.= AHYMO-I-9702a0100011K-SH

	HYDROGRAPH	FROM	TO ID	AREA	PEAK DISCHARGE	RUNOFF	DIMORE	TIME TO	CFS	PAGE =	= 1
COMMAND	IDENTIFICATION		NO.	(SQ MI)	(CFS)	VOLUME (AC-FT)	RUNOFF (INCHES)	PEAK (HOURS)	PER ACRE	NOTATI	ION
START										m mare	~ ~
	E= 2	•								TIME= RAIN24=	.00
COMPUTE NM HY	104.00	_	12	.04955	96.18	4.533	1.71541	1.533	3.033		3.100 50.00
COMPUTE NM HY	105.00	_	16	.01810	51.06	2.661	2.75634	1.533	4.408		
COMPUTE NM HY	106.00	_	17	.03425	68.27	3.405	1.86416	1.567	3.115		48.00
COMPUTE NM HY	107.00	-	19	.01986	51.82	2.293	2.16527	1.500		PER IMP=	
ADD HYD	107.10	17&19	13	.05411	115.01	5.699	1.97468	1.533	3.321		, , , ,
ROUTE	107.12	13	18	.05411	104.96	5.688	1.97098	1.600	3.031		
COMPUTE NM HY	108.00	_	13	.02297	54.58	2.367	1.93234	1.500	3.712	PER IMP=	60.00
ROUTE	104.02	12	17	.04955	83.59	4.521	1.71080	1.600	2.636		
ADD HYD	105.10		19	.06765	129.86	7.182	1.99054	1.567	2.999		
ADD HYD	107.20		17	.12176	233.61	12.870	1.98185	1.567	2.998		
ADD HYD	108.10	13&17	16	.14473	279.59	15.237	1.97399	1.567	3.018		
DIVIDE HYD	CB3	16	17	.09993	88.00	10.520	1.97399	1.400	1.376		
	108.13	and	18	.04480	191.59	4.717	1.97399	1.567	6.681		
COMPUTE NM HY			31	.05794	99.24	6.312	2.04268	1.633	2.676	PER IMP=	62.00
ROUTE	CB3.02	17	32	.09993	88.16	10.511	1.97233	1.467	1.379		
ROUTE	108.12		13	.04480	156.87	4.717	1.97410	1.633	5.471		
ADD HYD	301.10			.10274	256.12	11.029	2.01277	1.633	3.895		
DIVIDE HYD	CB8	33	34	.04229	34.00	4.540	2.01277	1.433	1.256		
אטט מעט	301.03		35	.06045	222.12	6.489	2.01277	1.633	5.741		
ADD HYD		34&32	_	.14222	122.16	15.051	1.98436	1.467	1.342		
ROUTE ROUTE	CB8.12	-	31	.14222	122.56	15.040	1.98287	1.467	1.347		
COMPUTE NM HY	301.02	35	32	.06045	204.21	6.490	2.01285	1.700	5.278		
DIVIDE HYD			33	.02999	43.91	2.770	1.73180	1.633		PER IMP=	49.00
DIAIDE UID	CB9	32	35 36	.01624	30.00	1.743	2.01285	1.533	2.887		
ADD HYD	301.06		36 34	.04421	174.21	4.746	2.01285	1.700	6.157		
ROUTE	CB9.12	31&35 34	_	.15846	152.02	16.783	1.98594	1.533	1.499		
ADD HYD	302.10		31	.15846	152.00	16.769	1.98423	1.833	1.499		
ROUTE	301.12		32	.07420	216.59	7.516	1.89926	1.700	4.561		
COMPUTE NM HYI			33	.07420 .08070	206.67	7.510	1.89764	1.766	4.352		
ADD HYD	303.10			.15490	128.73 330.97	8.788	2.04190	1.667		PER IMP=	62.00
DIVIDE HYD	CB10	34	32	.03779	10.00	16.298 3.976	1.97280 1.97280	1.733	3.339		
,	303.13	and	33	.11712	320.97	12.323	1.97280	1.333 1.733	.414		
ADD HYD	CB10.1		35	.19624	162.00	20.744	1.98203	1.833	4.282 1.290		
ROUTE	CB10.12	35	36	.19624	162.00	20.705	1.97829	. 2.033	1.290		
COMPUTE NM HYD		-	31	.01342	32.85	1.591	2.22262	1.533		PER IMP=	75 00
ROUTE	304.03	31	32	.01342	22.87	1.583	2.21110	1.600	2.662		75.00
COMPUTE NM HYI		-	31	.03919	59.45	3.425	1.63841	1.633		PER IMP=	42 00
ADD HYD	305.10	31&32	35	.05261	82.29	5.007	1.78450	1.633	2.370	LEK TML-	43.00
ADD HYD	305.20		32	.16973	390.03	17.330	1.91443	1.733	3.591		
DIVIDE HYD	305.30	32	33	.13208	209.83	13.485	1.91443	1.567	2.482		
	305.40	_	34	.03765	180.20	3.844	1.91443	1.733	7.478		
COMPUTE NM HYD			1	.00910	27.16	1.255	2.58599	1.500	_	PER IMP=	90.00
ROUTE RESERVOI	R 1300.01	1	2	.00910	3.86	1.255	2.58594	2.133		AC-FT=	.677
COMPUTE NM HYE	1100.A	-	3	.00097	2.00	.074	1.44107	1.500		PER IMP=	30.00
ADD HYD	1100.01	3&34	4	.03862	181.00	3.918	1.90260	1.733	7.324		• •

ROUTE RESERVOIR COMPUTE NM HYD COMPUTE NM HYD ADD HYD	1100.02	4	5	.03862	7.48	3.913	1.90014	1.966	.303 AC-FT= 3.724
	100.10	-	1	.00293	9.11	.431	2.75761	1.500	4.859 PER IMP= 100.00
	100.A	-	2	.00229	6.80	.314	2.56953	1.500	4.632 PER IMP= 90.00
	101.A	1& 2	3	.00522	15.91	.745	2.67499	1.500	4.759
ADD HYD ROUTE FINISH	101.A1 201.10	5& 3 4	4 20	.04384	16.01 15.91	4.658	1.99245	1.500	.571

AHYMO PROGRAM (AHYMO_97) - Version: 1997.02a

RUN DATE (MON/DAY/YR) = 07/13/2000

START TIME (HR:MIN:SEC) = 15:28:23 USER NO.= AHYMO-I-9702a0100011K-SH

INPUT FILE = A:POND-INT.DAT

RAIN DAY=3.10 IN DT=0.03333 HR

COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.

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DT =
        .033330 HOURS
                            END TIME =
                                          19.964670 HOURS
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* BASIN 104
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COMPUTE NM HYD ID=12 HYD NO=104 AREA=0.04955 SQ MI PER A=40.00 PER B=8.00 PER C=2.00 PER D=50.00 TP=-0.1649 HR MASS RAINFALL=-1

TP =

.164900HR

.545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = 79.069 CFS UNIT VOLUME = 1.000 526.28 B = P60 = 2.0700.024775 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR AREA = RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K/TP RATIO =

TP = .164900HR.188637HR K/TP RATIO = 1.143948 SHAPE CONSTANT, N = 3.095848 UNIT PEAK = 43.502CFS UNIT VOLUME = .9997 B = 289.55 P60 = 2.0700.024775 SQ MI IA = .61400 INCHES INF = 1.56920 INCHES PER HOURAREA = RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=12 CODE=1

.089871HR

RUNOFF VOLUME = 1.71541 INCHES 4.5332 ACRE-FEET PEAK DISCHARGE RATE = 96.18 CFS AT 1.533 HOURS BASIN AREA = .0496 SQ. MI.

*

* BASIN 105

* (

COMPUTE NM HYD

ID=16 HYD NO=105 AREA=0.01810 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00TP=-0.1665 HR MASS RAINFALL=-1

.090743HR .166500HR K/TP RATIO = .545000SHAPE CONSTANT, N =

7.106420

UNIT PEAK = 57.211 CFS UNIT VOLUME = 1.000 B = 526.28 P60 = 2.0700.018100 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR AREA = RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=16 CODE=1

PARTIAL HYDROGRAPH 105.00

RUNOFF VOLUME = 2.75634 INCHES 2.6608 ACRE-FEET PEAK DISCHARGE RATE = 51.06 CFS 1.533 HOURS BASIN AREA - .0181 SQ. MI. _ AT

* BASIN 106

ID=17 HYD NO=106 AREA=0.03425 SQ(MI COMPUTE NM HYD

PER A=0.00 PER B=26.00 PER C=26.00 PER D=48.00 *** ** TP=-0.1968 HR MASS RAINFALL=-1

7.106420

TP = .196800HRK = .107256HRK/TP RATIO =545000 SHAPE CONSTANT, N =

UNIT VOLUME = $\frac{1}{2}.9998$ 1 B = 526.28 UNIT PEAK = 43.963CFS $\sim P60 = 2.0700$.016440 SQ MI AREA = IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT -

TP = .196800HRK = .177854HRK/TP RATIO = .903729SHAPE CONSTANT, N = 3.920589

UNIT PEAK = 31.660 UNIT VOLUME = .9996 CFS B =349.84 P60 = 2.0700.017810 SQ MI AREA = IA = .42500 INCHES INF = 1.04000 INCHES PER HOURRUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = . .033330

PRINT HYD ID=17 CODE=1

PARTIAL HYDROGRAPH 106.00

RUNOFF VOLUME = 1.86416 INCHES = 3.4052 ACRE-FEET PEAK DISCHARGE RATE = 68.27 CFS AT 1.567 HOURS BASIN AREA = .0343 SQ. MI.

^{*} BASIN 107

*

COMPUTE NM HYD ID=19 HYD NO=107 AREA=0.01986 SQ MI

PER A=10.00 PER B=20.00 PER C=0.00 PER D=70.00

TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =

7.106420

UNIT PEAK = 54.886 CFS UNIT VOLUME = .9992 B = 526.28 P60 = 2.0700 AREA = .013902 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .141422HR TP = .133300HR K/TP RATIO = 1.060927 SHAPE CONSTANT, N =

3.328408

UNIT PEAK = 13.748 CFS UNIT VOLUME = .9991 B = 307.59 P60 = 2.0700 AREA = .005958 SQ MI IA = .55000 INCHES INF = 1.39000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=19 CODE=1

PARTIAL HYDROGRAPH 107.00

RUNOFF VOLUME = 2.16527 INCHES = 2.2934 ACRE-FEET
PEAK DISCHARGE RATE = 51.82 CFS AT 1.500 HOURS BASIN AREA = .0199 SQ. MI.

. *******************

* * ADD BASIN 106 TO BASIN 107

+

ADD HYD ID=13 HYD NO=107.1 ID I=17 ID II=19

PRINT HYD ID=13 CODE=1

PARTIAL HYDROGRAPH 107.10

RUNOFF VOLUME = 1.97468 INCHES = 5.6986 ACRE-FEET PEAK DISCHARGE RATE = 115.01 CFS AT 1.533 HOURS BASIN AREA = .0541 SQ. MI.

* ROUTE HYD 107.1 THRU 105/108

COMPUTE RATING CURVE CID=1 VS NO=1 NO SEGS=1

60

MIN ELEV=0 MAX ELEV=0.67
CH SLOPE=0.0193 FP SLOPE=0.0193
N=0.017 DIST=60
DIST ELEV DIST ELEV
0 0.67 0.1 0
30 0.60 59.9 0

0.67

RATING CURVE	VALLEY SECTION	1.0	
WATER	FLOW	FLOW	TOP
SURFACE	AREA	RATE	WIDTH
ELEV	SQ FT	CFS	FT
.00	.00	.00	.00
.04	.06	.05	3.53
.07	.25	.32	7.05
.11	.56	.95	10.58
.14	.99	2.04	14.10
.18	1.55	3.69	17.63

.21	2.24	6.01	21.15
.25	3.05	9.06	24.68
.28	3.98	12.94	28.20
.32	5.03	17.71	31.73
.35	6.22	23.46	35.25
.39	7.52	30.25	38.78
.42	8.95	38.15	42.30
.46	10.50	47.23	45.83
.49	12.18	57.54	49.35
.53	13.98	69.17	52.88
.56	15.91	82.16	56.40
. 60	17.96	96.57	59.93
. 63	20.08	116.11	59.99
. 67	22.19	137.10	60.00

COMPUTE TRAVEL TIME ID=18 REACH NO=1 NO VS=1 L=1350 SLOPE=0.0193

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS
.035	.062	.05	.4611
.071	.249	.32	.2905
.106	.559	. 95	.2217
.141	.994	2.04	.1830
.176	1.554	3.69	.1577
.212	2.238	6.01	.1397
.247	3.045	9.06	.1260
.282	3.978	12.94	.1153
.317	5.034	17.71	.1066
.353	6.215	23.46	.0993
.388	7.520	30.25	.0932
.423	8.950	38.15	.0880
.458	10.504	47.23	.0834
.494	12.182	57.54	.0794
.529	13.984	69.17	.0758
.564	15.911	82.16	.0726
.599	17.962	96.57	.0697
.635	20.077	116.11	.0648
.670	22.193	137.10	.0607

ROUTE PRINT HYD

*

ID=18 HYD NO=107.12 INFLOW ID=13 ID=18 CODE=1

PARTIAL HYDROGRAPH 107.12

RUNOFF VOLUME = 1.97098 INCHES = 5.6880 ACRE-FEET PEAK DISCHARGE RATE = 104.96 CFS AT 1.600 HOURS BASIN AREA = .0541 SQ. MI.

COMPUTE NM HYD ID=13 HYD NO=108 AREA=0.02297 SQ MI
PER A=30.00 PER B=7.00 PER C=3.00 PER D=60.00
TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420

UNIT PEAK = 54.412 CFS UNIT VOLUME = .9992 B = 526.28 P60 = 2.0700 AREA = .013782 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .150333HR TP = .133300HR K/TP RATIO = 1.127777 SHAPE CONSTANT, N =

3.137813

UNIT PEAK = 20.187 CFS UNIT VOLUME = .9992 B = 292.87 P60 = 2.0700 AREA = .009188 SQ MI IA = .60125 INCHES INF = 1.53350 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD

ID=13 CODE=1

PARTIAL HYDROGRAPH 108.00

RUNOFF VOLUME = 1.93234 INCHES = 2.3672 ACRE-FEET
PEAK DISCHARGE RATE = 54.58 CFS AT 1.500 HOURS BASIN AREA = .0230 SQ. MI.

~ *********************

* ROUTE HYD 104 THRU 105

COMPUTE RATING CURVE CID=1 VS NO=1 NO SEGS=1

MIN ELEV=0 MAX ELEV=1.00 CH SLOPE=0.008 FP SLOPE=0.008 N=0.017DIST=100 DIST **ELEV** DIST ELEV 0.67 1.0 20.0 20.1 0.0 50.0 0.60 79.9 0.0 80.0 0.67 100.0 1.0

> RATING CURVE WATER FLOW FLOW FLOW FLOW SURFACE **AREA** -- RATE WIDTH **ELEV** SQ FT .00 . · · · 00 5.26 .05 .14 .11 **~.** 60 10.52 . 55 .16 1.25 15.78 2.22 .21 21.05 .26 3.46 26.31 6.92 .32 4.98 11.25 31.57 .37 6.78 16.98 36.83 .42 8.86 24.24 42.09 .47 11.21 33.18 47.35 .53 13.85 43.94 52.61 .58 16.75 56.66 57.87 . 63 19.89 73.58 59.99 . 68 23.06 92.35 61.72 .74 26.47 109.01 68.10 .79 30.23 128.20 74.48 .84 34.31 150.06 80.86 .89 38.74 174.72 87.24

COMPUTE TRAVEL TIME ID=17 REACH NO=1 NO VS=1 L=1200 SLOPE=0.008

TRAVEL TIME TABLE

.95

1.00

REACH= 1.0

43.50

48.59

202.32

233.00

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS

93.62

100.00

.053	.138	.09	.4875
.105	.554	.60	.3071
.158	1.246	1.77	.2344
.211	2.215	3.82	.1935
.263	3.461	6.92	.1667
.316	4.984	11.25	.1476
.368	6.784	16.98	.1332
.421	8.861	24.24	.1219
.474	11.215	33.18	.1127
.526	13.846	43.94	.1050
.579	16.753	56.66	.0986
.632	19.888	73.58	.0901
.684	23.058	92.35	.0832
.737	26.474	109.01	.0810
.789	30.227	128.20	.0786
.842	34.314	150.06	.0762
.895	38.738	174.72	.0739
.947	43.498	202.32	.0717
1.000	48.593	233.00	.0695

*

ROUTE ID=17 HYD NO=104.02 INFLOW ID=12

PRINT HYD ID=17 CODE=1

> PARTIAL HYDROGRAPH 104.02

1.71080 INCHES RUNOFF VOLUME = 4.5210 ACRE-FEET 83.59 CFS PEAK DISCHARGE RATE = 1.600 HOURS .0496 SQ. MI. AT BASIN AREA =

*

* ADD HYD 104.02 TO BASIN 105

ID=19 HYD NO=105.1 ID I=16 ID II=17 ADD HYD

PRINT HYD ID=19 CODE=1

PARTIAL HYDROGRAPH 105.10

RUNOFF VOLUME = 1.99054 INCHES = 7.1818 ACRE-FEET PEAK DISCHARGE RATE = 129.86 CFS AT 1.567 HOURS BASIN AREA = .0677 SQ. MI.

* ADD HYD 105.1 TO HYD 107.12

ID=17 HYD NO=107.2 ID I=19 ID II=18 ADD HYD

PRINT HYD ID=17 CODE=1

PARTIAL HYDROGRAPH 107.20

1.98185 INCHES = 12.8698 ACRE-FEET RUNOFF VOLUME = PEAK DISCHARGE RATE = 233.61 CFS AT 1.567 HOURS BASIN AREA = .1218 SQ. MI.

* ADD HYD 107.2 TO HYD 108

ADD HYD

* TOTAL FLOW ENTERING JUAN TABO'S CATTLE GUARD

ID=16 HYD NO=108.1 ID I=13 ID II=17 PRINT HYD ID=16 CODE=1

PARTIAL HYDROGRAPH 108.10

RUNOFF VOLUME = 1.97399 INCHES = 15.2370 ACRE-FEET PEAK DISCHARGE RATE = 279.59 CFS AT 1.567 HOURS BASIN AREA = .1447 SQ. MI.

* DIVIDE HYD 108.1 BY INFLOW OF CATCH BASINS AND CATTLE GUARD IN JUAN TABO

* CATTLE GUARD AND CATCH BASINS BETWEEN MH# S-993 TO MH# S-903

DIVIDE HYD INFLOW ID=16 Q=88 ID I=17 HYD NO=CB3

• ID II=18 HYD NO=108.13

PRINT HYD ID=17 CODE=1

HYDROGRAPH FROM AREA CB3

RUNOFF VOLUME = 1.97399 INCHES = 10.5200 ACRE-FEET
PEAK DISCHARGE RATE = 88.00 CFS AT 1.400 HOURS BASIN AREA = .0999 SQ. MI.

PRINT HYD ID=18 CODE=1

PARTIAL HYDROGRAPH 108.13

RUNOFF VOLUME = 1.97399 INCHES = 4.7170 ACRE-FEET PEAK DISCHARGE RATE = 191.59 CFS AT 1.567 HOURS BASIN AREA = .0448 SQ. MI.

COMPUTE NM HYD ID=31 HYD NO=301 AREA=0.05794 SQ MI
PER A=11.00 PER B=15.00 PER C=12.00 PER D=62.00
TP=-0.2713 HR MASS RAINFALL=-1

K = .147859HR TP = .271300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =

7.106420

UNIT PEAK = 69.684 CFS UNIT VOLUME = .9999 B = 526.28 P60 = 2.0700

AREA = .035923 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = 1.269656HR TP = 1.271300HR K/TP RATIO = 1.993940 SHAPE CONSTANT, N = 1.552084

UNIT PEAK = 26.303 CFS UNIT VOLUME = .9995 B = 324.11 P60 = 2.0700 AREA = .022017 SQ MI IA = .49605 INCHES INF = 1.23895 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=31 CODE=1

HYDROGRAPH FROM AREA 301.00

RUNOFF VOLUME = 2.04268 INCHES = 6.3121 ACRE-FEET

PEAK DISCHARGE RATE = 99.24 CFS AT 1.633 HOURS BASIN AREA = .0579 SQ. MI.

* ROUTING HYD CB3 FOR 36" IN BUENA VENTURA STREET

* FROM MH# S-973 TO MH# S-993 IN BUENA VENTURA STREET

COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.024
DIA=3 FT N=0.014

RATING	CURVE	PIPE	SECTION	1.0		
WAT	rer -	F1	LOW	FLOW	M	ΑX
SURI	FACE	A	REA	RATE	W]	HTG
ELE	EV	S	FT	CFS	F	ľ
•	.00		.00	.00		.00
•	.16		.14	.50	. 1	1.33
•	.31		.39	2.18	1	1.83
•	. 47		.71	5.08	2	2.18
•	. 63	1	1.07	9.14	2	2.44
•	.78	1	46	14.26	2	2.63
,	. 94	1	L.89	20.35	2	2.78
1.	.09	2	2.33	27.27	2	2.89
1.	. 25	2	2.79	.34.86	2	2.96
1.	. 41	3	3.26	42.97	2	2.99
1.	. 56	3	3.72	51.43	3	3.00
1.	.72	4	1.19	60.04	3	3.00
1.	. 88	4	. 65	68.60	3	3.00
2.	.03	. 5	5.10	76.89	3	3.00
2.	.19	Ę	5.53	84.65	. 3	3.00
2.	. 35	5	5.93	91.60	3	3.00
2.	. 50	6	5.30	97.38	3	3.00
2.	. 66	6	5.62	101.51	3	3.00
2.	81	6	5.89	103.21		3.00
3.	.00	7	1.07	103.21		.00
		_	ه م ح سیسی ، ،	A Superior		

COMPUTE TRAVEL TIME ID=32 REACH NO=1 NO VS=1 L=1246 SLOPE=0.024

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA .	RATE ~	- TIME
FEET	SQ.FT.	CFS ,	· HRS
.156	.140	.50	.0966
.313	.391	2.18	.0619
.469	.706	5.08	.0481
. 625	1.068	9.14	.0405
.782	1.465	14.26	.0355
.938	1.889	20.35	.0321
1.094	2.332	27.27	.0296
1.251	2.790	34.86	.0277
1.407	3.255	42.97	.0262
1.563	3.724	51.43	.0251
1.720	4.191	60.04	.0242
1.876	4.650	68.60	.0235
2.032	5.097	76.89	.0229
2.189	5.525	84.65	.0226
2.345	5.928	91.60	.0224
2.501	6.297	97.38	.0224
2.658	6.622	101.51	.0226
2.814	6.887	103.21	.0231
3.000	7.069	103.21	.0237

ROUTE PRINT HYD ID=32 HYD NO=CB3.02 INFLOW ID=17 ID=32 CODE=1

HYDROGRAPH FROM AREA CB3.02

0.40

0.67

RUNOFF VOLUME = 1.97233 INCHES = 10.5112 ACRE-FEET
PEAK DISCHARGE RATE = 88.16 CFS AT 1.467 HOURS BASIN AREA = .0999 SQ. MI.

100.0

120.0

*

* ROUTE 108.13 THRU 301: 40' WIDTH

* HYD 108.13 THRU 301 ON BUENA VENTURA STREET

COMPUTE RATING CURVE CID=1 VS NO=1 NO SEGS=1
MIN ELEV=0 MAX ELEV=1.00
CH SLOPE=0.016 FP SLOPE=0.016
N=0.017 DIST=200
DIST ELEV DIST ELEV
0 1.0 80.0 0.67

80.10.0119.90.0200.01.0

RATING CURVE	VALLEY SECT	ION 1.0	
WATER	FLOW	FLOW	TOP
SURFACE	AREA	RATE	WIDTH
ELEV	SQ FT	CFS	FT
.00	.00	.00	.00
.05	.14	.13	5.25
.11	.55	.85	10.51
.16	1.24	2.50	15.76
.21	2.21	5.39	21.01
.26	3.46	9.77	26.26
.32	4.98	15.89	31.52
.37	6.77	23.97	36.77
.42	8.82	35.24	39.93
. 47	10.93	50.22	39.94
.53	13.03	67.22	39.96
.58	15.13	86.12	39.97
. 63	17.24	106.80	39.99
. 68	19.39	117.07	46.89
.74	22.53	113.18	72.41
.79	27.01	125.57	97.93
.84	32.84	149.24	123.44
.89	40.01	183.18	148.96
. 95	48.52	227.53	174.48
1.00	58.37	282.89	200.00

COMPUTE TRAVEL TIME ID=13 REACH NO=1 NO VS=1 L=1250 SLOPE=0.016

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS
.053	.138	.13	.3591
.105	.553	.85	.2262
.158	1.244	2.50	.1726
.211	2.212	5.39	.1425
.263	3.456	9.77	.1228
.316	4.976	15.89	.1087
.368	6.773	23.97	.0981
.421	8.824	35.24	.0870
.474	10.926	50.22	.0755
.526	13.029	67.22	.0673

.579 15.132 .0610 86.12 .632 17.236 106.80 .0560 . 684 19.390 117.07 .0575 . 664 18.573 113.18 .0570 .789 27.012 125.57 .0747 .842 32.838 149.24 .0764 .895 40.006 183.18 .0758 .947 .0740 48.518 227.53 1.000 282.89 58.373 .0716

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*

*

ROUTE ID=13 HYD NO=108.12 INFLOW ID=18

PRINT HYD ID=13 CODE=1

PARTIAL HYDROGRAPH 108.12

RUNOFF VOLUME = 1.97410 INCHES = 4.7173 ACRE-FEET
PEAK DISCHARGE RATE = 156.87 CFS AT 1.633 HOURS BASIN AREA = .0448 SQ. MI.

* ADD BASIN 301 TO HYD 108.12 * TOTAL OF JANE AND BUENA VENTURA

ADD HYD

ID=33 HYD NO=301.1 ID I=13 ID II=31

PRINT HYD

ID=33 CODE=1

HYDROGRAPH FROM AREA 301.10

RUNOFF VOLUME = 2.01277 INCHES = 11.0294 ACRE-FEET
PEAK DISCHARGE RATE = 256.12 CFS AT 1.633 HOURS BASIN AREA = .1027 SQ. MI.

* DIVIDE BASIN 301 BY MAXIMUM INFLOW OF CATCH BASIN

* BETWEEN MH# S-973 AND MH# S-993 ON BUENA VENTURA STREET

*

*

DIVIDE HYD INFLOW ID=33 Q=34 ID I=34 HYD NO=CB8

ID II=35 HYD NO=301.03

PRINT HYD ID=34 CODE=1

HYDROGRAPH FROM AREA CB8

RUNOFF VOLUME = 2.01277 INCHES = 4.5400 ACRE-FEET

PEAK DISCHARGE RATE = 34.00 CFS AT 1.433 HOURS BASIN AREA = .0423 SQ. MI.

PRINT HYD ID=35 CODE=1

HYDROGRAPH FROM AREA 301.03

RUNOFF VOLUME = 2.01277 INCHES = 6.4894 ACRE-FEET
PEAK DISCHARGE RATE = 222.12 CFS AT 1.633 HOURS BASIN AREA = .0605 SQ. MI.

* ADD HYD CB8 TO HYD CB3.02

HYDROGRAPH FROM AREA CB8.1

RUNOFF VOLUME = 1.98436 INCHES = 15.0512 ACRE-FEET

PEAK DISCHARGE RATE = 122.16 CFS AT 1.467 HOURS BASIN AREA = .1422 SQ. MI.

COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.012 DIA=4 FT N=0.014

RATING CURVE	PIPE SECTION	1.0	
WATER	FLOW	FLOW	MAX
SURFACE	AREA	RATE	WIDTH
ELEV	SQ FT	CFS	FT
.00	.00	.00	.00
.21	.25	.77	1.78
.42	69	3.33	2.44
. 63	1.25	7.73	2.91
.83	1.90	13.91	3.25
1.04	2.60	21.72	3.51
1.25	3.36	30.99	3.71
1.46	4.15	- 41.52	3.85
1.67	4.96	53.09	3.94
1.88	. 5.79	. 65.44	
	6. 62.	**···78.32	4.00 ····
2.29	7.45	91.43	4.00
2.50	8.27	104.47	4.00
	9.06	117.09	4.00
2.92	9.82	128.91	` 4.00
3.13	10.54	139.49	4.00
3.34	. 11.20	148.29	
3.54 [^]	11.77	-154.59	4.00
3.75	12.24	157.18	4.00
4.00	12.57	157.18	4.00

COMPUTE TRAVEL TIME ID=31 REACH NO=1 NO VS=1 L=944 SLOPE=0.012

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS
.208	.250	.77	.0855.
.417	. 695	3.33	.0548
. 625	1.255	7.73	.0425
.834	1.898	13.91	.0358
1.042	2.604	21.72	.0314
1.251	3.358	30.99	.0284
1.459	4.146	41.52	.0262
1.668	4.960	53.09	.0245
1.876	5.788	65.44	.0232
2.084	6.621	78.32	.0222
2.293	7.451	91.43	.0214
2.501	8.267	104.47	.0208
2.710	9.062	117.09	.0203

general and the

^{*} ROUTE HYD CB8.1 FOR 48" PIPE IN JANE STREET

^{*} FROM MH# S-772 TO MH# S-973 IN JANE STREET

^{*} Q(CAP)=146 CFS

2.918	9.823	128.91	.0200
3.127	10.539	139.49	.0198
3.335	11.195	148.29	.0198
3.544	11.773	154.59	.0200
3.752	12.243	157.18	.0204
4.000	12.566	157.18	.0210

ROUTE

*

ID=31 HYD NO=CB8.12 INFLOW ID=36

PRINT HYD ID=31 CODE=1

HYDROGRAPH FROM AREA CB8.12

RUNOFF VOLUME = 1.98287 INCHES = 15.0398 ACRE-FEET
PEAK DISCHARGE RATE = 122.56 CFS AT 1.467 HOURS BASIN AREA = .1422 SQ. MI.

* ROUTE 301.03 THRU 302 ON JANE STREET: 40' WIDTH

COMPUTE RATING CURVE CID=1 VS NO=1 .NO SEGS=1 MIN ELEV=0 MAX ELEV=1.50 CH SLOPE=0.0056 FP SLOPE=0.0056 N=0.017DIST=200 DIST DIST **ELEV** ELEV 1.50 0.67 80.0 80.1 0.0 119.9 0.00 120.9 0.67 200.0 1.50

RATING CURVE	VALLEY SECT	ION 1.0	-
WATER	FLOW	FLOW	TOP
SURFACE	AREA	RATE	WIDTH
ELEV	SQ FT	CFS	FT
.00	.00	.00	39.80
.08	3.15	3.78	39.93
.16	6.30	11.98	40.06
.24	9.47	23.53	40.19
.32	12.65	37.98	40.32
.39	15.84	55.04	40.45
.47	19.04	74.51	40.58
.55	22.25	96.25	40.71
. 63	25.46	120.14	40.84
.71	28.85	131.76	48.67
.79	33.29	139.99	63.80
.87	38.92	157.87	78.93
. 95	45.75	184.07	94.07
1.03	53.78	218.32	109.20
1.11	62.99	260.79	124.33
1.18	73.41	311.86	139.47
1.26	85.02	372.01	154.60
1.34	97.82	441.75	169.73
1.42	111.82	521.63	184.87
1.50	127.01	612.19	200.00

COMPUTE TRAVEL TIME ID=32 REACH NO=1 NO VS=1 L=900 SLOPE=0.0056

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS
.079	3.147	3.78	.2082
.158	6.305	11.98	.1315
.237	9.472	23.53	.1006
.316	12.650	37.98	.0833

.395	15.838	55.04	.0719
.474	19.037	74.51	.0639
.553	22.245	96.25	.0578
. 632	25.464	120.14	.0530
.711	28.849	131.76	.0547
.789	33.289	139.99	.0595
.868	38.923	157.87	.0616
.947	45.752	184.07	.0621
1.026	53.776	218.32	.0616
1.105	62.995	260.79	.0604
1.184	73.408	311.86	.0588
1.263	85.016	372.01	.0571
1.342	97.818	441.75	.0554
1.421	111.816	521.63	.0536
1.500	127.008	612.19	.0519

ROUTE

ID=32 HYD NO=301.02 INFLOW ID=35

PRINT HYD

*

*

ID=32 CODE=1

HYDROGRAPH FROM AREA 301.02

RUNOFF VOLUME = 2.01285 INCHES = 6.4897 ACRE-FEET

PEAK DISCHARGE RATE = 204.21 CFS AT 1.700 HOURS BASIN AREA = .0605 SQ. MI.

COMPUTE NM HYD ID=33 HYD NO=302 AREA=0.02999 SQ MI
PER A=30.00 PER B=14.00 PER C=7.00 PER D=49.00
TP=-0.2725 HR MASS RAINFALL=-1

K = .148513HR TP = .272500HR K/TP RATIO = .545000 SHAPE CONSTANT, N =

7.106420

UNIT PEAK = 28.380 CFS UNIT VOLUME = .9996 B = 526.28 P60 = 2.0700

AREA = .014695 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .0333330

K = .295635HR TP = .272500HR K/TP RATIO = 1.084900 SHAPE CONSTANT, N = 3.256680

UNIT PEAK = 16.957 CFS UNIT VOLUME = .9992 B = 302.12 P60 = 2.0700 AREA = .015295 SQ MI IA = .56765 INCHES INF = 1.43941 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=33 CODE=1

HYDROGRAPH FROM AREA 302.00

RUNOFF VOLUME = 1.73180 INCHES = 2.7699 ACRE-FEET
PEAK DISCHARGE RATE = 43.91 CFS AT 1.633 HOURS BASIN AREA = .0300 SQ. MI.

* DIVIDE 301.03 BY MAXIMUM INFLOW OF JANE STREET'S CATCH BASINS

DIVIDE HYD INFLOW ID=32 Q=30 ID I=35 HYD NO=CB9 ID II=36 HYD NO=301.06

PRINT HYD ID=35 CODE=1

HYDROGRAPH FROM AREA CB9

RUNOFF VOLUME = 2.01285 INCHES = 1.7433 ACRE-FEET

PEAK DISCHARGE RATE = 30.00 CFS AT 1.533 HOURS BASIN AREA = .0162 SQ. MI.

PRINT HYD

ID=36 CODE=1

HYDROGRAPH FROM AREA 301.06

RUNOFF VOLUME = 2.01285 INCHES = 4.7463 ACRE-FEET
PEAK DISCHARGE RATE = 174.21 CFS AT 1.700 HOURS BASIN AREA = .0442 SQ. MI.

* ADD PIPE HYD CB8.1 TO PIPE HYD CB9

* TOTAL FLOW IN CHICO AND JANE

ADD HYD

ID=34 HYD NO=CB9.1 ID I=31 IDII=35

PRINT HYD ID=34 CODE=1

HYDROGRAPH FROM AREA CB9.1

RUNOFF VOLUME = 1.98594 INCHES = 16.7831 ACRE-FEET
PEAK DISCHARGE RATE = 152.02 CFS AT 1.533 HOURS BASIN AREA = .1585 SQ. MI.

* ROUTE HYD CB9.1 FOR 48" PIPE IN CHICO ROAD

* FROM MH# S-743 TO MH# S-772

* Q(CAP) = 169 CFS

COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.016 DIA = 4 FT N=0.014

> RATING CURVE PIPE SECTION 1.0 WATER FLOW FLOW MAX SURFACE AREA RATE WIDTH **ELEV** SQ FT CFS FT .00 .00 39.80 .00 .21 .25 1.78 .88 .42 .69 3.84 2.4463 1.25 8.93 2.91 .83 1.90 16.06 3.25 1.04 2.60 25.08 3.51 1.25 3.36 35.79 3.71 1.46 4.15 47.94 3.85 1.67 4.96 61.30 3.94 1.88 5.79 3.99 75.56 2.08 6.62 90.44 4.00 2.29 105.58 7.45 4.00 2.50 8.27 120.64 4.00 2.71 9.06 135.21 4.00 2.92 9.82 148.86 4.00 3.13 10.54 161.07 4.00 3.34 11.20 171.23 4.00 3.54 11.77 178.50 4.00 3.75 12.24 181.49 4.00

4.00 12.57 181.49 4.00

COMPUTE TRAVEL TIME ID=31 REACH NO=1 NO VS=1 L=1331 SLOPE=0.016

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS
.208	.250	.88	.1044
.417	. 695	3.84	.0669
. 625	1.255	8.93	.0520
.834	1.898	16.06	.0437
1.042	2.604	25.08	.0384
1.251	3.358	35.79	.0347
1.459	4.146	47.94	.0320
1.668	4.960	61.30	.0299
1.876	5.788	75.56	.0283
2.084	6.621	90.44	.0271
2.293	7.451	105.58	.0261
2.501	8.267	120.64	.0253
2.710	9.062	135.21	.0248
2.918	9.823	148.86	.0244
3.127	10.539	161.07	.0242
3.335	11.195	171.23	.0242
3.544	11.773	178.50	.0244
3.752	12.243	181.49	.0249
4.000	12.566	181.49	.0256

ROUTE ID=31 HYD NO=CB9.12 INFLOW ID=34 PRINT HYD ID=31 CODE=1

HYDROGRAPH FROM AREA CB9.12

RUNOFF VOLUME = 1.98423 INCHES = 16.7687 ACRE-FEET
PEAK DISCHARGE RATE = 152.00 CFS AT 1.833 HOURS BASIN AREA = .1585 SQ. MI.

* ADD HYD 302 TO HYD 301.06

*

ADD HYD ID=34 HYD NO=302.1 ID I=33 ID II=36

PRINT HYD ID=34 CODE=1

HYDROGRAPH FROM AREA 302.10

RUNOFF VOLUME = 1.89926 INCHES = 7.5163 ACRE-FEET

PEAK DISCHARGE RATE = 216.59 CFS AT 1.700 HOURS BASIN AREA = .0742 SQ. MI.

* ROUTE 302.1 THRU 303 ON CHICO STREET

COMPUTE RATING CURVE CID=1 VS NO=1 NO SEGS=1 MIN ELEV=0 MAX ELEV=1.00 CH SLOPE=0.0172 FP SLOPE=0.0172 N=0.017DIST=88 DIST ELEV DIST ELEV 1.00 0 20.0 0.67 20.1 0.0 44.0 0.48 67.9 0.0 68.0 0.67 88.0 1.00

RATING CURVE	VALLEY SECTION	1.0	
WATER	FLOW	FLOW	TOP
SURFACE	AREA	RATE	WIDTH
ELEV	SQ FT	CFS	FT
.00	.00	.00	.00
.05	.14	.14	5.26
.11	. 55	.88	10.51
.16	1.25	2.60	15.77
.21	2.21	5.59	21.03
.26	3.46	10.14	26.28
.32	4.98	16.49	31.54
.37	6.78	24.87	36.80
.42	8.85	35.51	42.06
.47	11.21	48.61	47.31
.53	13.73	67.49	47.96
.58	16.25	89.29	47.97
. 63	18.78	113.43	. 47.99
. 68	21.32	136.80	49.72
.74	24.10	155.15	56.10
.79	27.22	177.14	62.48
.84	30.68	202.85	68.86
.89	34.47	232.43	75.24
. 95	38.60	266.04	81.62
1.00	43.06	303.85	88.00

COMPUTE TRAVEL TIME ID=32 REACH NO=1 NO VS=1 L=1280 SLOPE=0.0172

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS
.053	.138	.14	.3546
.105	.553	.88	.2234
.158	1.245	2.60	.1705
.211	2.213	5.59	.1407
.263	3.459	10.14	.1213
.316	4.980	16.49	.1074
.368	6.779	24.87	.0969
.421	8.854	35.51	.0887
.474	11.206	48.61	.0820
.526	13.727	67.49	.0723
.579	16.252	89.29	.0647
.632	18.777	113.43	.0589
.684	21.315	136.80	.0554
.737	24.100	155.15	.0552
.789	27.221	177.14	.0546
.842	30.677	202.85	.0538
.895	34.469	232.43	.0527
.947	38.597	266.04	.0516
1.000	43.061	303.85	.0504

ROUTE PRINT HYD ID=32 HYD NO=301.12 INFLOW ID=34 ID=32 CODE=1

HYDROGRAPH FROM AREA 301.12

RUNOFF VOLUME = 1.89764 INCHES = 7.5099 ACRE-FEET

PEAK DISCHARGE RATE = 206.67 CFS AT 1.766 HOURS BASIN AREA = .0742 SQ. MI.

*

^{*} BASIN 303

COMPUTE NM HYD

ID=33 HYD NO=303 AREA=0.08070 SQ MI PER A=11.00 PER B=15.00 PER C=12.00 PER D=62.00 TP=-0.3042 HR MASS RAINFALL=-1

K = .166204HRTP =K/TP RATIO = .546364.304200HR SHAPE CONSTANT, N =

7.083708

*

UNIT PEAK = 86.385CFS UNIT VOLUME = .9999 B = 525.21P60 = 2.0700AREA = .050034 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

.299341HR K =TP =.304200HR K/TP RATIO = .984026 SHAPE CONSTANT, N =

3.588462

UNIT PEAK = 32.937 CFS UNIT VOLUME = .9996 **B** = 326.73 P60 = 2.0700.030666 SQ MI .49605 INCHES IA = INF = 1.23895 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=33 CODE=1

> HYDROGRAPH FROM AREA 303.00

2.04190 INCHES RUNOFF VOLUME = 8.7883 ACRE-FEET 128.73 CFS PEAK DISCHARGE RATE = AΤ 1.667 HOURS .0807 SQ. MI. BASIN AREA =

* ADD BASIN 303 TO HYD 302.12

ADD HYD

ID=34 HYD NO=303.1 ID I=33 ID II=32

PRINT HYD

ID=34 CODE=1

HYDROGRAPH FROM AREA 303.10

1.97280 INCHES RUNOFF VOLUME = 16.2982 ACRE-FEET PEAK DISCHARGE RATE = 330.97 CFS AT 1.733 HOURS .1549 SQ. MI. BASIN AREA =

* DIVIDE HYD 303.1 BY MAXIMUM INFLOW OF CHICO'S CATCH BASINS

* BETWEEN MH# S-743 AND MH# S-772

DIVIDE HYD

INFLOW ID=34 Q=10 ID I=32 HYD NO=CB10

ID II=33 HYD NO=303.13

PRINT HYD

ID=32 CODE=1

HYDROGRAPH FROM AREA CB10

1.97280 INCHES RUNOFF VOLUME = 3.9756 ACRE-FEET PEAK DISCHARGE RATE = 10.00 CFS AT 1.333 HOURS BASIN AREA = .0378 SQ. MI.

PRINT HYD ID=33 CODE=1

> HYDROGRAPH FROM AREA 303.13

RUNOFF VOLUME = 1.97280 INCHES 12.3225 ACRE-FEET

PEAK DISCHARGE RATE = 320.97 CFS AT 1.733 HOURS BASIN AREA = .1171 SQ. MI.

* ADD PIPE HY CB9.12 WITH CB10

* TOTAL PIPE FLOW AT CHICO AND MORRIS

ADD HYD ID=35 HYD NO=CB10.1 ID I=31 ID II=32

PRINT HYD ID=35 CODE=1

HYDROGRAPH FROM AREA CB10.1

RUNOFF VOLUME = 1.98203 INCHES = 20.7443 ACRE-FEET
PEAK DISCHARGE RATE = 162.00 CFS AT 1.833 HOURS BASIN AREA = .1962 SQ. MI.

* ROUTING PIPE HYD CB10.1 FOR 54" PIPE IN CHICO ROAD

*

COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.014
DIA=4.5 FT N=0.014

RATING CURVE	PIPE SECTION	1.0	
WATER	FLOW	FLOW	MAX
SURFACE	AREA	RATE	WIDTH
ELEV	SQ FT	CFS	FT
.00	.00	.00	.00
.23	.32	1.13	2.00
.47	.88	4.92	2.75
.70	1.59	11.44	3.27
. 94	2.40	20.57	3.66
1.17	3.30	32.12	3.95
1.41	4.25	45.83	4.17
1.64	5.25	61.40	4.33
1.88	6.28	78.50	4.44
2.11	7.32	96.77	4.49
2.35	8.38	115.81	4.50
2.58	9.43	135.20	4.50
2.81	10.46	154.48	4.50
3.05	11.47	173.15	4.50
3.28	12.43	190.62	4.50
3.52	13.34	206.27	4.50
3.75	14.17	219.28	4.50
3.99	14.90	228.59	4.50
4.22	15.50	232.42	4.50
4.50	15.90	232.42	4.50

COMPUTE TRAVEL TIME ID=36 REACH NO=1 NO VS=1 L=2523 SLOPE=0.014

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS
.235	.316	1.13	.1955
.469	.880	4.92	.1253
.704	1.588	11.44	.0973
.938	2.402	20.57	.0818
1.173	3.296	32.12	.0719
1.407	4.249	45.83	.0650
1.641	5.248	61.40	.0599

•			
1.876	6.277	78.50	.0560
2.111	7.325	96.77	.0530
2.345	8.380	115.81	.0507
2.579	9.430	135.20	.0489
2.814	10.463	154.48	.0475
3.049	11.468	173.15	.0464
3.283	12.432	190.62	.0457
3.518	13.338	206.27	.0453
3.752	14.169	219.28	.0453
3.987	14.900	228.59	.0457
4.221	15.495	232.42	.0467
4.500	15.904	232.42	.0480

ROUTE PRINT HYD ID=36 HYD NO=CB10.12 INFLOW ID=35

PRINT HYD ID=36 CODE=1

HYDROGRAPH FROM AREA CB10.12

RUNOFF VOLUME = 1.97829 INCHES = 20.7051 ACRE-FEET
PEAK DISCHARGE RATE = 162.00 CFS AT 2.033 HOURS BASIN AREA = .1962 SQ. MI.

COMPUTE NM HYD ID=31 HYD NO=304 AREA=0.01342 SQ MI
PER A=25.00 PER B=0.00 PER C=0.00 PER D=75.00
TP=-0.1520 HR MASS RAINFALL=-1

K = .082840 HR TP = .152000HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420

UNIT PEAK = 34.848 CFS UNIT VOLUME = .9998 B = 526.28 P60 = 2.0700 AREA = .010065 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .180897HR TP = .152000HR K/TP RATIO = 1.190112 SHAPE CONSTANT, N = 2.983819

UNIT PEAK = 6.1913 CFS UNIT VOLUME = .9976 B = 280.50 P60 = 2.0700

AREA = .003355 SQ MI IA = .65000 INCHES INF = 1.67000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=31 CODE=1

HYDROGRAPH FROM AREA 304.00

RUNOFF VOLUME = 2.22262 INCHES = 1.5908 ACRE-FEET
PEAK DISCHARGE RATE = 32.85 CFS AT 1.533 HOURS BASIN AREA = .0134 SQ. MI.

* ROUTING BASIN 304 THRU BASIN 305 ON MORRIS STREET

COMPUTE RATING CURVE CID=1 VS NO=1 NO SEGS=1

MIN ELEV=0 MAX ELEV=0.67

CH SLOPE=0.0084 FP SLOPE=0.0084

N=0.017 DIST=48

DIST ELEV DIST ELEV

0 0.67 0.1 0

24 0.48 47.9 0 48 0.67

RATING CURVE	VALLEY SECTION	1.0	
WATER	FLOW	FLOW	TOP
SURFACE	AREA	RATE	WIDTH
ELEV	SQ FT	CFS	FT
.00	.00	.00	.00
.04	.06	.03	3.52
.07	.25	.21	7.04
.11	.56	. 62	10.57
.14	.99	1.34	14.09
.18	1.55	2.44	17.61
.21	2.24	3.96	21.13
.25	3.04	5.97	24.66
.28	3.97	8.53	28.18
.32	5.03	11.68	. 31.70
.35	6.21	15.46	35.22
.39	7.51	19.94	38.74
.42	8.94	25.15	42.27
.46	10.50	31.13	45.79
.49	12.16	38.58	47.95
.53	13.85	47.89	47.96
.56	15.54	57.97	47.97
. 60	17.24	68.79	47.98
. 63	18.93	80.33	47.99
. 67	20.62	92.57	48.00

COMPUTE TRAVEL TIME ID=32 REACH NO=1 NO VS=1 L=1660 SLOPE=0.0084

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	· RATE	TIME ~
FEET .	SQ.FT.	- CFS	HRS *
.035	.062		.8595
.071	.248	.21	.5415
.106	.559	. 62	.4132
.141	.994	1.34	.3411
.176	1.553	2.44	.2939
.212	2.236	3.96	.2603
.247	3.043	5.97	2349
.282	3.974	8.53	.2149
.317	5.030	.11.68	.1986
.353	6.210	15.46	.1852
.388	7.514	19.94	.1738
.423	8.943	25.15	.1640
.458	10.495	31.13	.1555
.494	12.162	38.58	.1453
.529	13.853	47.89	.1334
.564	15.545	57.97	.1237
.599	17.236	68.79	.1155
.635	18.929	80.33	.1087
.670	20.621	92.57	.1027

ROUTE PRINT HYD

ID=32 HYD NO=304.03 INFLOW ID=31

PRINT HYD ID=32 CODE=1

HYDROGRAPH FROM AREA 304.03

RUNOFF VOLUME = 2.21110 INCHES = 1.5826 ACRE-FEET

PEAK DISCHARGE RATE = 22.87 CFS AT 1.600 HOURS BASIN AREA = .0134 SQ. MI.

*
COMPUTE NM HYD

ID=31 HYD NO=305 AREA=0.03919 SQ MI PER A=25.00 PER B=23.00 PER C=9.00 PER D=43.00

TP=-0.2442 HR MASS RAINFALL=-1

K = .133089HR TP = .244200HR K/TP RATIO = .545000 SHAPE CONSTANT, N =

7.106420

UNIT PEAK = 36.317 CFS UNIT VOLUME = .9997 B = 526.28 P60 = 2.0700 AREA = .016852 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .256918HR TP = .244200HR K/TP RATIO = 1.052081 SHAPE CONSTANT, N =

3.355912

UNIT PEAK = 28.326 CFS UNIT VOLUME = .9996 B = 309.66 P60 = 2.0700 AREA = .022338 SQ MI IA = .54211 INCHES INF = 1.36789 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=31 CODE=1

HYDROGRAPH FROM AREA 305.00

RUNOFF VOLUME = 1.63841 INCHES = 3.4245 ACRE-FEET
PEAK DISCHARGE RATE = 59.45 CFS AT 1.633 HOURS BASIN AREA = .0392 SQ. MI.

*

* ADD HYD 304.03 TO BASIN 305

ADD HYD

ID=35 HYD NO=305.1 ID I=31 ID II=32

PRINT HYD ID=35 CODE=1

HYDROGRAPH FROM AREA 305.10

RUNOFF VOLUME = 1.78450 INCHES = 5.0070 ACRE-FEET
PEAK DISCHARGE RATE = 82.29 CFS AT 1.633 HOURS BASIN AREA = .0526 SQ. MI.

* ADD HYD 305.1 TO HYD 303.13

* TOTAL AT CHICO AND MORRIS

ADD HYD ID=32 HYD NO=305.2 ID I=35 ID II=33

PRINT HYD ID=32 CODE=1

HYDROGRAPH FROM AREA 305.20

RUNOFF VOLUME = 1.91443 INCHES = 17.3296 ACRE-FEET PEAK DISCHARGE RATE = 390.03 CFS AT 1.733 HOURS BASIN AREA = .1697 SQ. MI.

DIVIDE HYD ID=32 Q=209.83 ID=33 HYD=305.3 ID=34 HYD=305.4

PRINT HYD ID=33 CODE=1

HYDROGRAPH FROM AREA 305.30

RUNOFF VOLUME = 1.91443 INCHES = 13.4854 ACRE-FEET PEAK DISCHARGE RATE = \cdot 209.83 CFS AT 1.567 HOURS BASIN AREA = .1321 SQ. MI.

PRINT HYD ID=34 CODE=1

HYDROGRAPH FROM AREA 305.40

RUNOFF VOLUME = 1.91443 INCHES = 3.8442 ACRE-FEET
PEAK DISCHARGE RATE = 180.20 CFS AT 1.733 HOURS BASIN AREA = .0377 SQ. MI.

COMPUTE NM HYD ID=1 HYD NO=1300 AREA=0.0091 SQ MI
PER A=0.00 PER B=5.00 PER C=5.00 PER D=90.00
TP=-0.1333 HR MASS RAINFALL=-1 *

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420

32.335 UNIT PEAK = CFS UNIT VOLUME = .9990 B = 526.28P60 = 2.0700.008190 SQ MI AREA = IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT -.033330

K = .120467HR TP = .133300HR K/TP RATIO = .903729 SHAPE CONSTANT, N = 3.920589

UNIT PEAK = 2.3883 CFS UNIT VOLUME = .9949 B = 349.84 P60 = 2.0700

AREA = .000910 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=1 CODE=1

HYDROGRAPH FROM AREA 1300.00

RUNOFF VOLUME = 2.58599 INCHES = 1.2551 ACRE-FEET

PEAK DISCHARGE RATE = 27.16 CFS AT 1.500 HOURS BASIN AREA = .0091 SQ. MI.

* ROUTE BASIN 1300 THROUGH POND 'A' ON CINEMARK SITE

ID=2 HYD NO=1300.01 INFLOW ID=1 CODE=24 ROUTE RESERVOIR OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT) 0.00 0.0000 5459.80 0.50 0.0001 5460.00 1.27 0.0108 5461.00 1.84 0.0585 5462.00 2.27 0.1505 5463.00 2.63 0.2653 5464.00 2.95 0.3875 5465.00 3.23 0.5057 5466.00 3.49 0.6066 5467.00

INFLOW TIME **ELEV** VOLUME OUTFLOW (HRS) (CFS) (FEET) (AC-FT) (CFS) .00 .00 5459.80 .000 .00 .80 .00 5459.80 .000 .00 1.60 18.73 5465.33 .427 3.04 2.40 1.25 5467.54 .640 3.63 3.20 .34 5465.61 .459 3.12 4.00 .25 5464.17 .286 2.68 5462.88 4.80 .23 .139 2.22 5.60 .029 .24 5461.38 1.49 .24 6.40 5459.89 .000 .23 7.20 5459.89 .23 .000 .23 8.00 .22 5459.89 .000 .22 8.80 5459.88 .20 .000 .20 9.60 5459.88 .20 .000 .20 10.40 5459.88 .19 .000 .19 11.20 .18 5459.87 .000 .18 12.00 .17 5459.87 .000 .17 12.80 5459.87 .17 .17 .000 13.60 .16 5459.86 .000 .16 14.40 5459.86 .16 .16 .000 15.20 .15 5459.86 .000 .15 16.00 .15 5459.86 .000 .15 16.80 5459.86 .14 .000 .14 17.60 5459.85 .14 .000 .14 18.40 .13 5459.85 • .13 .000 5459.85 ,19.20 .13 .000 .13 3.863 CFS -PEAK DISCHARGE = PEAK OCCURS AT HOUR 2.13 MAXIMUM WATER SURFACE ELEVATION = 5468.547 MAXIMUM STORAGE = .6766 AC-FT INCREMENTAL TIME= .033330HRS PRINT HYD ID=2 CODE=1 HYDROGRAPH. FROM AREA 1300.01 2.58594 INCHES RUNOFF VOLUME = 1.2550 ACRE-FEET 3.86 CFS PEAK DISCHARGE RATE AT 2.133 HOURS .0091 SQ. MI. BASIN AREA = * BASIN 1100-A ID=3 HYD NO=1100.A AREA=0.000965 SQ MI COMPUTE NM HYD PER A=0.00 PER B=70.00 PER C=0 PER D=30.00 TP=-0.1333 HR MASS RAINFALL=-1 .072649HR TP = .133300HR K/TP RATIO =.545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = 1.1430 CFS UNIT VOLUME = .9897B = 526.28P60 = 2.0700.000290 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR AREA = RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330 K = .132811HR TP = .133300HR K/TP RATIO = .996335 SHAPE CONSTANT, N = 3.543441 UNIT PEAK = 1.6393 CFS UNIT VOLUME = .9921 B = 323.49 P60 = 2.0700.000676 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR AREA =

Page 24 of 28

0.6675

0.6874

5468.00

5469.20

3.74

4.01

*

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD

ID=3 CODE=1

HYDROGRAPH FROM AREA 1100.A

RUNOFF VOLUME = 1.44107 INCHES = .0742 ACRE-FEET
PEAK DISCHARGE RATE = 2.00 CFS AT 1.500 HOURS BASIN AREA = .0010 SQ. MI.

* ADD THE CHICO ROAD OVERFLOW TO BASIN 1100-A

ADD HYD

*

ID=4 HYD NO=1100.01 ID I=3 ID II=34

PRINT HYD

ID=4 CODE=1

HYDROGRAPH FROM AREA 1100.01

5467.00

5468.00

5469.00

RUNOFF VOLUME = 1.90260 INCHES = 3.9184 ACRE-FEET

PEAK DISCHARGE RATE = 181.00 CFS AT 1.733 HOURS BASIN AREA = .0386 SQ. MI.

2.9413

3.4427

3.9644

* ROUTE CHICO ROAD OVERFLOW PLUS BASIN 1100-A THROUGH POND 'E'

*			
ROUTE. RESERVOIR	ID=5 HYD NO=1100.02 INFLOW ID=4 CODE=24		
•	OUTFLOW (CFS)	STORAGE (AC-FT)	ELEVATION (FT)
•	0.000	0.000	5460.00
-	2.006	0.3593	5461.00
•	3.305	0.7389	5462.00
	4.221	1.1388	5463.00
	4.971	1.5590	5464.00
	5.622	1.9994	5465.00
	6.205	2.4602	5466.00

* * * * * * * * * * * * *

6.738

7.232

7.694

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
^	00	E 4 C O O O		
.00	.00	5460.00	.000	.00
.80	.00	5460.00	.000	.00
1.60	91.22	5460.60	.214	1.20
2.40	.07	5468.05	3.467	7.25
3.20	.01	5467.13	3.005	6.80
4.00	.01	5466.23	2.572	6.33
4.80	.01	5465.37	2.170	5.84
5.60	.01	5464.55	1.801	5.33
6.40	.01	5463.78	1.466	4.81
7.20	.01	5463.07	1.167	4.27
8.00	.01	5462.41	.905	3.68
8.80	.01	5461.84	.680	3.10
9.60	.01	5461.36	.496	2.48
10.40	.01	5460.98	.350	1.96
11.20	.01	5460.68	.243	1.35
12.00	.01	5460.47	.168	.94
12.80	.01	5460.32	.117	. 65
13.60	.01	5460.23	.081	.45

```
14.40
                      .01
                           5460.16
                                         .056
                                                    .31
                                         .039
         15.20
                      .01
                                                    .22
                           5460.11
                                         .027
         16.00
                      .01
                                                    .15
                           5460.08
         16.80
                      .00
                           5460.05
                                         .019
                                                    .11
         17.60
                      .00
                           5460.04
                                                    .08
                                         .014
         18.40
                      .00
                           5460.03
                                         .010
                                                    .05
         19.20
                      .00
                           5460.02
                                         .007
                                                    .04
      PEAK DISCHARGE =
                             7.481 CFS - PEAK OCCURS AT HOUR
      MAXIMUM WATER SURFACE ELEVATION =
                                           5468.540
                              3.7244 AC-FT
      MAXIMUM STORAGE =
                                                                      .033330HRS
                                                INCREMENTAL TIME=
     PRINT HYD
                         ID=5 CODE=1
                                                                   1100.02
                                             HYDROGRAPH FROM AREA
                            1.90014 INCHES
         RUNOFF VOLUME =
                                                       3.9133 ACRE-FEET
                                    7.48 CFS
         PEAK DISCHARGE RATE =
                                                                                .0386 SQ. MI.
                                                   1.966 HOURS
                                              AT
                                                                 BASIN AREA =
     * BASIN 1 (TOWNE PARK PLAZA)
     COMPUTE NM HYD
                        ID=1 HYD NO=100.1 AREA=0.002928 SQ MI
                        PER A=0.00 PER B=0.00 PER C=0 PER D=100.00
                        TP=-0.1333 HR MASS RAINFALL=-1
                .072649HR
                                                K/TP RATIO =
                                   .133300HR
                                                               .545000
                                                                           SHAPE CONSTANT,
7.106420
                       11.560
                                 CFS
                                                        .9984
                                                                          526.28
                                                                                      P60 = 2.0700
                                       UNIT
                                            VOLUME =
                     .002928 SQ MI
          AREA =
                                              .10000 INCHES
                                      IA =
                                                               INF =
                                                                        .04000 INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD
                        ID=1 CODE=1
     PRINT HYD
                                        PARTIAL HYDROGRAPH
                                                             100.10
                            2.75761 INCHES
                                                        .4306 ACRE-FEET
        RUNOFF VOLUME =
        PEAK DISCHARGE RATE =
                                    9.11 CFS
                                              ΑT
                                                   1.500 HOURS
                                                                 BASIN AREA =
     * BASIN A-1 (TOWNE PARK PLAZA)
                        ID=2 HYD NO=100.A AREA=0.002294 SQ MI
    COMPUTE NM HYD
                        PER A=0.00 PER B=10.00 PER C=0 PER D=90.00
                        TP=-0.1333 HR MASS RAINFALL=-1
               .072649HR
                                   .133300HR
                                                K/TP RATIO =
                                                               .545000
                                                                           SHAPE CONSTANT, N =
7.106420
         UNIT PEAK = 8.1511 CFS UNIT VOLUME = .9981
                                                                   B = 526.28
                                                                                     P60 = 2.0700
                .002065 SQ MI IA \Rightarrow .10000 INCHES INF \Rightarrow .04000 INCHES PER HOUR
         AREA =
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330
         K = .132811HR TP =
                                  .133300HR
                                               K/TP RATIO = .996335
                                                                          SHAPE CONSTANT, N =
3.543441
         UNIT PEAK = .55670
                              CFS UNIT VOLUME = .9759 B = 323.49 P60 = 2.0700
                     .000229 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
         AREA =
```

Page 26 of 28

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=2 CODE=1

HYDROGRAPH FROM AREA 100.A

RUNOFF VOLUME = 2.56953 INCHES = .3144 ACRE-FEET
PEAK DISCHARGE RATE = 6.80 CFS AT 1.500 HOURS BASIN AREA = .0023 SQ. MI.

* ADD HYD ID=3

ID=3 HYD NO=101.A ID I=1 ID II=2

PRINT HYD ID=3 CODE=1

HYDROGRAPH FROM AREA 101.A

RUNOFF VOLUME = 2.67499 INCHES = .7450 ACRE-FEET

PEAK DISCHARGE RATE = 15.91 CFS AT 1.500 HOURS BASIN AREA = .0052 SQ. MI.

ADD HYD
PRINT HYD

ID=4 HYD NO=101.A1 ID I=5 ID II=3

YD ID=4 CODE=1

HYDROGRAPH FROM AREA 101.A1

RUNOFF VOLUME = 1.99245 INCHES = 4.6583 ACRE-FEET
PEAK DISCHARGE RATE = 16.01 CFS AT 1.500 HOURS BASIN AREA = .0438 SQ. MI.

COMPUTE RATING CURVE CID=1 VS NO=1 CODE=-1 SLP=0.013 DIA=2.0 FT N=0.013

WATER FLOW FLOW MAX SURFACE AREA RATE WIDTH ELEV SQ FT CFS FT .00 .00 .00 .00 .00 .10 .06 .14 .89 .21 .17 .59 1.22 .31 .31 1.37 1.45 .42 .47 2.46 1.62 .52 .65 3.83 1.76 .63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00 1.04 1.66 13.83 2.00	RATING CURVE	PIPE SECTION	1.0	
ELEV SQ FT CFS FT .00 .00 .00 .00 .10 .06 .14 .89 .21 .17 .59 1.22 .31 .31 1.37 1.45 .42 .47 2.46 1.62 .52 .65 3.83 1.76 .63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00	WATER	FLOW	FLOW	MAX
.00 .00 .00 .10 .06 .14 .89 .21 .17 .59 1.22 .31 .31 1.37 1.45 .42 .47 2.46 1.62 .52 .65 3.83 1.76 .63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00	SURFACE	AREA	RATE	WIDTH
.10 .06 .14 .89 .21 .17 .59 1.22 .31 .31 1.37 1.45 .42 .47 2.46 1.62 .52 .65 3.83 1.76 .63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00	ELEV	SQ FT	CFS	FT
.21 .17 .59 1.22 .31 .31 1.37 1.45 .42 .47 2.46 1.62 .52 .65 3.83 1.76 .63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00	.00	.00	.00	.00
.31 .31 1.37 1.45 .42 .47 2.46 1.62 .52 .65 3.83 1.76 .63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00	.10	.06	.14	.89
.42 .47 2.46 1.62 .52 .65 3.83 1.76 .63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00	.21	.17	.59	1.22
.52 .65 3.83 1.76 .63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00	.31	.31	1.37	1.45
.63 .84 5.47 1.85 .73 1.04 7.33 1.93 .83 1.24 9.37 1.97 .94 1.45 11.55 2.00	.42	.47	2.46	1.62
.731.047.331.93.831.249.371.97.941.4511.552.00	.52	. 65	3.83	1.76
.83 1.24 9.37 1.97 .94 1.45 11.55 2.00	. 63	.84	5.47	1.85
.94 1.45 11.55 2.00	.73	1.04	7.33	1.93
	.83	1.24	9.37	1.97
1.04 1.66 13.83 2.00	.94	1.45	11.55	2.00
	1.04	1.66	13.83	2.00

1.15	1.86	16.14	2.00
1.25	2.07	18.44	2.00
1.35	2.27	20.67	2.00
1.46	2.46	22.76	2.00
1.56	2.63	24.62	2.00
1.67	2.80	26.18	2.00
1.77	2.94	27.29	2.00
1.88	3.06	27.75	2.00
2.00	3.14	27.75	2.00

COMPUTE TRAVEL TIME ID=20 REACH NO=1 NO VS=1 L=244 SLOPE=0.013

TRAVEL TIME TABLE

REACH= 1.0

WATER	AVERAGE	FLOW	TRAVEL
DEPTH	AREA	RATE	TIME
FEET	SQ.FT.	CFS	HRS
.104	.062	.14	.0313
.208	.174	.59	.0200
.313	.314	1.37	.0156
.417	.475	2.46	.0131
.521	.651	3.83	.0115
.625	.839	5.47	.0104
.730	1.037	7.33	.0096
.834	1.240	9.37	.0090
.938	1.447	11.55	.0085
1.042	1.655	13.83	0081
1.146	1.863	. 16.14	0078
1.251	2.067	18.44	.0076
1.355	2.265	20.67	.0074
1.459	2.456	22.76	
1.563	2.635	24.62	.0073
1.668	2.799	26.18	.0072
1.772	2.943	27.29	.0073
1.876	3.061	27.75	***** · 0075
2.000	3.142	27.75	.0077

ROUTE PRINT HYD

ID=20 HYD NO=201.1 INFLOW ID=4

ID=20 CODE=1

PARTIAL HYDROGRAPH 201.10

RUNOFF VOLUME = 1.99233 INCHES = 4.6580 ACRE-FEET
PEAK DISCHARGE RATE = 15.91 CFS AT 1.500 HOURS BASIN AREA = .0438 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 15:28:25

DEVELOPMENT & BUILDING SERVICE CENTER ONE STOP SHOP 600 SECOND ST. N.W.

ATTENTION:	
•	505-924-3900

Records Withdrawai Form
Project No. K21/D9F' Date: 04-21-05 ABQ. WAL-MART EXPN: 835-02
Project Title: SAM'S CLUB EXPN
a. File b. Mylars c. Redlines/Comments d. Other 24×34 SIAT (2) PEAACEON
Requested by: FRED C. AREMAN, AREMAN, P.A. Phone No.: 268-8828 Name and Company
Comments: FOR BOND REPRODUCTION
Anticipated Return Date:
I hereby accept full responsibility for the security of the above noted records/plans until return receipt acknowledgement is completed. Records/plans will be returned to the Development and Building Services Center on or before the indicted anticipated return date.
Delivery Picked Up By:
Name: Jimmy ICK Organization: ABO, REPORTORANCE Print
Signed:
Office Use Only
Return Acknowledged:
Received By: Date: 4-25-05



K-21 D9F

City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

September 18, 2001

Ronald R. Bohannan, P.E. Tierra West, LLC 8509 Jefferson NE Albuquerque, New Mexico 87113

RE: SAM'S CLUB EXPANSION

(K-21/D9F)

(Chico & Eubank NE)

ENGINEERS CERTIFICATION FOR CERTIFICATE OF OCCUPANCY

ENGINEERS STAMP DATED 7/14/2000

ENGINEERS CERTIFICATION DATED 9/4/2001

Dear Mr. Bohannan:

Based upon the information provided in your Engineers Certification submittal dated 9/14/2001, the above referenced site is approved for Permanent Certificate of Occupancy.

If I can be of further assistance, please contact me at 924-3981.

Sincerely,

Teresa A. Martin

Hydrology Plan Checker Public Works Department

Teresa A. Martin

RIN

C: Vickie Chavez, COA approval file Idrainage file

DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Sam's Club Expansion	ZONE ATLAS/DRNG.	. FILE #: K-21/D9F		
DRB #: 00450-00	000-00774,00775 EPC #:	WORK ORDER #:	6469.81		
LEGAL DESCRIPTIO	ON: Tract B3A Towne Park Plaza				
CITY ADDRESS:	Northeast Corner of Chico and Cubank, 300 E	ubank Boulevard NE			
ENGINEERING FIRM	A: TIERRA WEST, LLC	CONTACT:	RONALD R. BOHANNAN OR SARA LAVY		
ADDRESS:	8509 Jefferson NE, ABQ, NM 87113	PHONE:	(505) 858-3100		
OWNER:	Sam's Club East	CONTACT:	Mohsen Ghadimkhani		
ADDRESS:	300 Eubank NE	PHONE:	(501) 273-4940		
ARCHITECT:	Harrison French Architects	CONTACT:	Trish		
ADDRESS:	502 SW "A" Street, Bentonville, AR 72712	PHONE:	(501) 273-7780		
SURVEYOR:	Jaynes Corporation	CONTACT:	Steve Young		
ADDRESS:	P. O. Box 26841, ABQ 87126	PHONE:	345-8591		
CONTRACTOR:	Jaynes Corporation	CONTACT:	Scott Anderson .		
ADDRESS:	P. O. Box 26841, ABQ 87125	PHONE:	345-8691		
TYPE OF SUBMITTA	AL: AGE REPORT	CHECK TYPE OF APP	PROVAL SOUGHT: PLAN APPROVAL		
	AGE PLAN		NARY PLAT APPROVAL		
	PTUAL GRADING & DRAINAGE PLAN		PLAN FOR SUB'D. APPROVAL		
GRADII	NG PLAN	S. DEV. F	PLAN FOR BLDG. PERMIT APPROVAL		
EROSK	ON CONTROL PLAN	SECTOR	PLAN APPROVAL		
X ENGIN	EER'S CERTIFICATION	FINAL PL	AT APPROVAL		
X OTHER	: Grading & Drainage As-Builts	FOUNDA	FOUNDATION PERMIT APPROVAL		
	· —	BUILDING	S PERMIT APPROVAL		
PRE-DESIGN MEET	ing:		CATE OF OCCUPANCY APPROVAL		
YES			S PERMIT APPROVAL		
<u>X</u> NO	<u>X</u> NO		PAVING PERMIT APPROVAL S. A. D. DRAINAGE REPORT		
COPY PROVIDED			DRAINAGE REQUIREMENTS		
		OTHER			
DAT	TE SUBMITTED: 9/4/01 BY: Ronald R. Bohannan, PE		D) 屋 © 国 I V 国 D) SEP 1 4 2001 HYDROLOGY SECT		
			SECTION		

SIERRA WEST, LLC

K-21 D9F

8509 Jefferson NE Albuquerque, NM 87113

(505) 858-3100 fax (505) 858-1118 e-mail: twdms@aol.com 1-800-245-3102

September 4, 2001

Mr. Brad Bingham Senior Engineer / Hydrology City of Albuquerque P. O. Box 1293 Albuquerque, NM 87103

RE:

Final Certification of Drainage for Certificate of Occupancy Sam's Club Expansion (K21/D09F), 300 Eubank Boulevard NE

Albuquerque, New Mexico

Dear Mr. Bingham:

We are requesting a Final Certification of Drainage for Certificate of Occupancy. Enclosed please find one copy of the as-built Grading and Drainage Plan and Information Sheet for the Sam's Expansion at Eubank and Copper. Jaynes Corporation completed the on-site paving and curb and gutter. Landscaping for the site is complete, and construction of the onsite pond is complete. The drainage outfall is existing and functional. Jaynes Corporation supplied as-built information, and we field verified the improvements.

If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

Ronald R. Bohannan, PE

HYDROLOGY SECTION

Enclosures

CC:

Mohsen Ghadimkhani Scott Anderson

JN 990029 RRB:js

1999misc#6 9929bb070501



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 29, 2001

Ronald R. Bohannan, P.E.
Tierra West, LLC
8509 Jefferson NE
Albuquerque, New Mexico 87113

RE: SAM'S CLUB EAST (GAS STATION)

(K-21/D9F)

(Eubank Blvd NE)

ENGINEERS CERTIFICATION FOR CERTIFICATE OF OCCUPANCY

ENGINEERS STAMP DATED 4/17/2001

ENGINEERS CERTIFICATION DATED 6/26/2001

Dear Mr. Bohannan:

Based upon the information provided in your Engineers Certification submittal dated 6/26/2001, the above referenced site is approved for a Permanent Certificate of Occupancy.

If I can be of further assistance, please contact me at 924-3981.

Sincerely,

Teresa A. Martin

Hydrology Plan Checker Public Works Department

13113

C: Vickie Chavez, CAO drainage file approval file

DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Sam's Club Expansion	ZONE ATLAS/DRN(G. FILE #: <u>K-21/D9F</u>	
DRB #:	EPC #:	WORK ORDER #:		
LEGAL DESCRIPTION	ON: Tract B3A Towne Park Plaza			
CITY ADDRESS:	Northeast Corner of Chico and Eubank, 300 l	Eubank Boulevard, NE		
ENGINEERING FIR	TIERRA WEST, LLC	CONTACT:	RONALD R. BOHANNAN OR SARA LAVY	
ADDRESS:	8509 Jefferson NE, ABQ, NM 87113	PHONE:	(505) 858-3100	
OWNER:	Sam's Club East	CONTACT:	Mohsen Ghadimkhani	
ADDRESS:	300 Eubank NE	PHONE:	(501) 273-4940	
ARCHITECT:	Harrison French Architects	CONTACT:	Trish	
ADDRESS:	502 SW "A" Street, Bentonville, AR 72712	PHONE:	(501) 273-7780	
SURVEYOR:	4	CONTACT:		
ADDRESS:		PHONE:		
CONTRACTOR:	Jaynes Corporation	CONTACT:	Scott Anderson	
ADDRESS:	P.O. Box 26841, Alb. 87125	PHONE:	345-8591	
DRAIN	AGE REPORT	SKETCH	1 PLAN APPROVAL	
DRAIN	AGE REPORT	SKETCH	I PLAN APPROVAL	
DRAIN	AGE PLAN	PRELIM	INARY PLAT APPROVAL	
	EPTUAL GRADING & DRAINAGE PLAN		PLAN FOR SUB'D. APPROVAL	
GRADI	NG PLAN	S. DEV.	PLAN FOR BLDG. PERMIT APPROVAL	
EROSI	ON CONTROL PLAN	SECTOR	R PLAN APPROVAL	
X ENGIN	EER'S CERTIFICATION	FINAL P	PLAT APPROVAL	
OTHER		FOUNDATION PERMIT APPROVAL		
		BUILDIN	NG PERMIT APPROVAL	
PRE-DESIGN MEET	TING:	X CERTIF	ICATE OF OCCUPANCY APPROVAL	
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DA	TE SUBMITTED: 6/26/01		HYDROLOGY SECTION	
	BY: Ron Wright			

8509 Jefferson NE Albuquerque, NM 87113 (505) 858-3100 fax (505) 858-1118

e-mail: twdms@aol.com 1-800-245-3102

June 26, 2001

Mr. Brad Bingham Senior Engineer/Hydrology City of Albuquerque PO Box 1293 Albuquerque, NM 87103

RE:

Final Certification of Drainage for Certificate of Occupancy

Sam's Club Expansion Gas Station (K21/D09F), 300A Eubank Boulevard, NE

Dear Mr. Bingham:

We are requesting a Final Certification of Drainage for Certificate of Occupancy. Enclosed please find one copy of the as-built Grading and Drainage Plan for the Sam's Expansion Gas Station at Eubank and Copper. Kachina Petroleum, Inc. and Jaynes Corporation have completed the on-site paving and curb and gutter. Landscaping for the site is complete. The drainage outfall for the gas station is existing and functional. As-built information was supplied by Jaynes Corporation and we have field verified the improvements.

An Administrative Amendment for the gas station was processed in April, because the station was constructed 12 feet south of the approved site plan. A new grading and drainage plan for the gas station dated 4-17-01 was included with the AA package. This plan has not been approved by Hydrology but the drainage pattern is the same and the grades have not changed more than 18 inches. We have shown the as-built grades on the new grading plan (dated 4-17-01) but have also included the approved grading and drainage plan as requested.

If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

Ronald R. Bohannan, PE

Enclosures

CC:

Mohsen Ghadimkhani Scott Anderson

Lee Woodmansee

JN: 990029

scl

JUN 2 6 2001

HYDROLOGY SECTION

9929 9929 final CO hydrology gas station ltr



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

Public Works Department Transportation Development Services Section

October 18, 2001

Ron Bohannan, Professional Engineer Tierra West, LLC 8509 Jefferson NE Albuquerque, NM 87113

Re:

Certification Submittal for Final Building Certificate of Occupancy for

Sam's Club Expansion, [K21/D09F]_____5

300 Eubank Blvd., NE

Certification Letter Dated 09/04/01

Dear Mr. Bohannan:

The TCL / Letter of Certification submitted is sufficient for acceptance by this office for final Certificate of Occupancy (C.O.). Notification has been made to Building and Safety and final C.O. has been logged in by Vicki Chavez in the Building Safety Section downstairs.

Sincerely,

Mike Zamora

Commercial Plan Checker

Development and Building Services

Public Works Department

C:

Terri Martin
Office File

MZ:gds

O*.

8509 Jefferson NE Albuquerque, NM 87113 (505) 858-3100 fax (505) 858-1118 e-mail: twdms@aol.com 1-800-245-3102

September 4, 2001

Mr. Mike Zamora
Public Works Department
City of Albuquerque
PO Box 1293
Albuquerque, NM 87103

RE:

Final Certification of Transportation for Certificate of Occupancy

Sam's Club Expansion (K21/D09F), 300 Eubank Boulevard NE

Dear Mr. Zamora:

We are requesting a Final Certification of Transportation for Certificate of Occupancy. Enclosed please find one copy of the as-built Site Plan (Traffic Circulation Plan) for the Sam's Expansion at Eubank and Copper. Jaynes Corporation has completed the on-site paving, curb and gutter, and parking lot striping. Landscaping for the site is complete. The drainage outfall for the site is in place and functional, and the pond for off-site flows has been constructed. All work is in substantial compliance with the approved plans. As-built information was supplied by Jaynes Corporation and field verified by our office.

If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

Ronald R. Bohannan, PE

Enclosures

CC:

Mohsen Ghadimkhani

Scott Anderson

JN: 990029 RRB:js

1999misc#6 9929mz070501

10/101-Calledin GT to Vicki - ASDPout
10/101-Sout leter for GT



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 1, 2001

Ronald R. Bohannan, P.E. Tierra West, LLC 8509 Jefferson NE Albuquerque, New Mexico 87113

RE:

SAM'S CLUB EAST EXPANSION GAS STATION

(K-21/D9F)

(EUBANK & CHICO NE)

CERTIFICATE OF OCCUPANCY APPROVAL-Temporary

ENGINEERS STAMP DATED 4/17/2001

ENGINEERS CERTIFICATION DATED 5/30/2001

Dear Mr. Bohannan:

Based on the information provided in your May 31, 2001 submittal, the above referenced project is approved for a TEMPORARY Certificate of Occupancy.

A Temporary Certificate of Occupancy has been issued for 60 days, allowing the remaining drainage issues in your May 31, 2001 Engineers Certification to be completed within this time scope.

When these remaining issues have been fully completed, are in substantial compliance, and an Engineers Certification has been resubmitted to the City's Hydrology office for approval, a Permanent Certificate of Occupancy can be issued.

If I can be of further assistance, please feel free to contact me at 924-3981.

Sincerely,

Jenesa A. Martin

Hydrology Plan Checker

C.O.A./Public Works Department

c: Vickie Chavez, COA
Drainage File
Approval File

8509 Jefferson NE Albuquerque, NM 87113

(505) 858-3100 fax (505) 858-1118

e-mail: twdms@aol.com 1-800-245-3102

May 29, 2001

Mr. Brad Bingham Senior Engineer/Hydrology City of Albuquerque PO Box 1293 Albuquerque, NM 87103

RE:

60-Day Temporary Certification of Drainage for Certificate of Occupancy

Sam's Club Expansion Gas Station (K21/D09F), 300A Eubank Boulevard, NE

Dear Mr. Bingham:

We are requesting a 60-day temporary Certification of Drainage for Certificate of Occupancy. Enclosed please find one copy of the as-built Grading and Drainage Plan for the Sam's Expansion Gas Station at Eubank and Copper. Kachina Petroleum, Inc. and Jaynes Corporation has completed the on-site paving and curb and gutter. Landscaping for the site is underway. The drainage outfall for the gas station is existing and functional. As-built information was supplied by Jaynes Corporation and we have field verified the improvements.

An Administrative Amendment for the gas station was processed in April, because the station was constructed 12 feet south of the approved site plan. Anew:grading:and:drainage plantforthe gas station dated 4-17-01-was included with the AA package. -This plan has not been approved by Hydrology but the drainage pattern is the same and the grades have not changed more than 18 inches. We have shown the as-built grades on the new grading plan (dated 4-17-01) but have also included the approved grading and drainage plan as requested.

If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

Ronald R. Bohannan, PE

Enclosures

CC:

Mohsen Ghadimkháni

Scott Anderson Lee Woodmansee

JN: 990029

scl

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DRAINAGE INFORMATION SHEET

PROJECT T	ITLE:	Sam's Club Gas Stat	ion	ZONE ATLAS/DRNG.	FILE #:	K-21/D19
DRB #:	1000317	EPC #:		WORK ORDER #:	N/A	
LEGAL DES	<u> </u>	4: Tract B3A T	owne Park Plaza			· · · · · · · · · · · · · · · · · · ·
CITY ADDR		300-A Eubank Blvd.	NE			<u> </u>
ENGINEER	ING FIRM:	TIERRA WE	ST, LLC	CONTACT:	RONALD	R. BOHANNAN
ADDR		8509 Jefferson NE, ABQ	, NM 87113	PHONE:	(505) 858	-3100
OWNER:		Sam's Club Inc.		CONTACT:		
ADDR	ESS:	8525 Jefferson, NE 8711.	3	PHONE:	(505) 298-	-5308
ARCHITEC		Harrison French Archite		CONTACT:		
		502 SW A Street, Benton		PHONE:	(501) 273	-7780
ADDR			THE TENTE	CONTACT:	Scott And	
SURVEYOR		Jaynes Corporation		PHONE:	(505) 345	
ADDR	RESS:	P.O. Box 26841, Alb. 871		•		
CONTRAC	TOR:	Kachina Petroleum I	nc.	CONTACT:	Tracy Nei	
ADDR	RESS:	9600 Bell, SE 87123	<u> </u>	PHONE:	(505) 292	2-3090
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	DAT	E SUBMITTED BY:	May 29, 2001 RONALD R. BOHANNA			DECEVE MAY 3 1 2001 HYDROLOGY SECTION



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

July 21, 1995'

Jerry Domke Dunaway Associates West Inc. 4500 Lakeshore Drive, STE 250 Tempe, Arizona 85282

RE: DRAINAGE PLAN FOR SAM'S CLUB ADDITION (K21-D9F) ENGINEER'S STAMP DATED 5/30/95.

Dear Mr. Domke:

Based on the information provided on your July 12, 1995 submittal, the above referenced site is approved for Building Permit.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

If I can be of further assistance, please feel free to contact me at 768-2667.

Sincerely,

Sernie J. Montoya, CE Engineering Associate

BJM/dl

c: Andrew Garcia

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