

MASTER DRAINAGE PLAN  
ELDORADO PARK SUBDIVISION

AND

REVISED DRAINAGE REPORT  
FOR UNIT 1

Drainage File L9/D7  
DRB No. 97-13

Prepared for

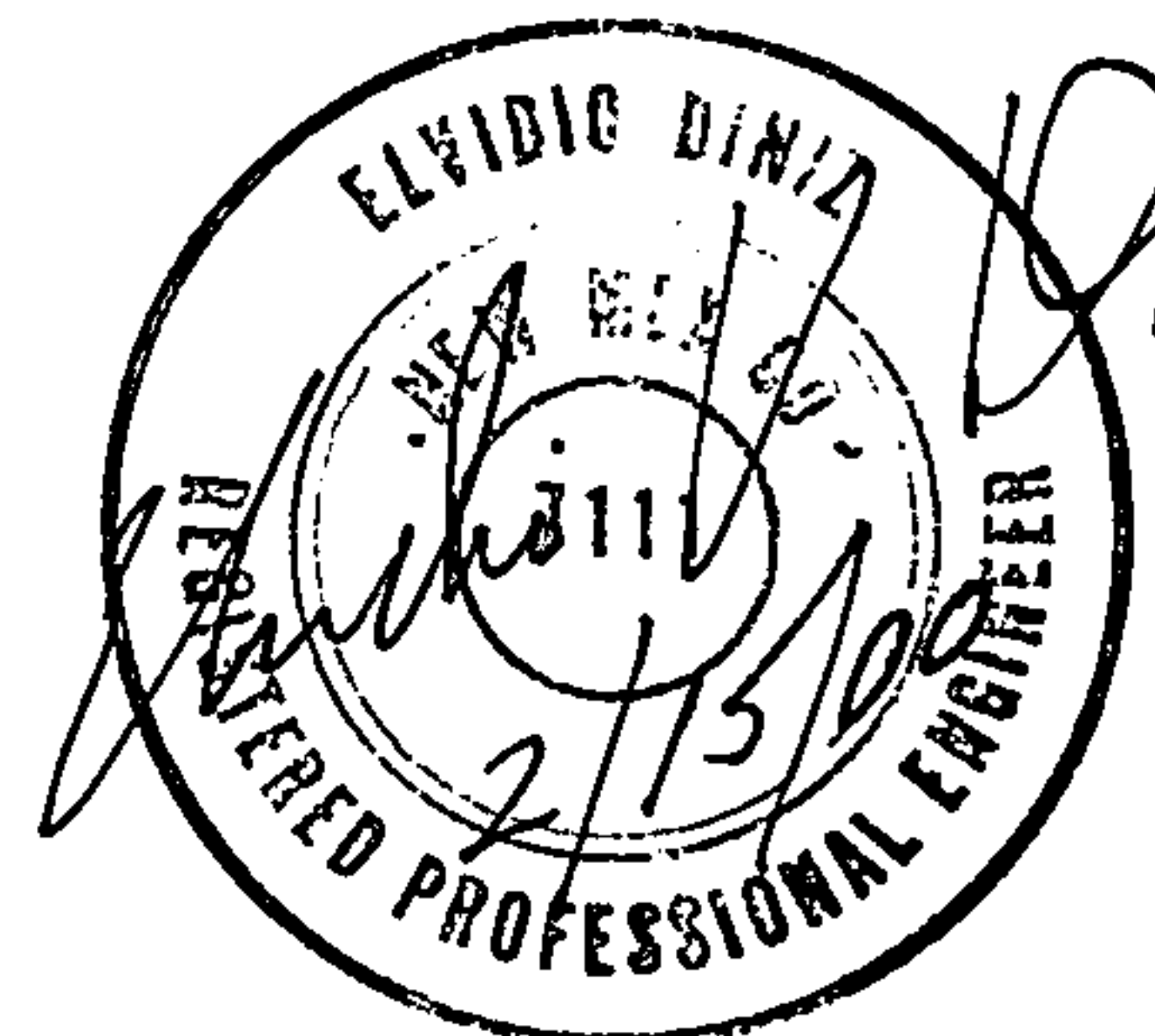
TOWER WEST JOINT VENTURE  
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Prepared by



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July 1999  
Revised February 2000





## A. DRAINAGE SUMMARY

The proposed development site, named Eldorado Park, is located on Tract A-1A (21.9 ac.) of the Tower West Joint Venture and Tract 27A (5.2 ac.), Unit 2 of the Town of Atrisco Grant. A copy of the proposed plat is attached. The site will be developed in four phases, Units 1 through 4, as shown on Sheet 1 of 3 of the Grading and Drainage Plan. The site is currently undeveloped with a cover of native shrubs and grass, with dumped refuse located over portions of the site.

The site is bounded by 98<sup>th</sup> Street on the west which collects local runoff and diverts this drainage to the existing arroyo located on the south side of the site. This runoff is conveyed to the arroyo by drainage swales on both sides of the roadway. No runoff from 98<sup>th</sup> Street or the area west of 98<sup>th</sup> Street currently flows onto the site, except into the arroyo.

Approximately 1000 feet west of 98<sup>th</sup> Street, Snow Vista Channel intercepts all runoff into the arroyo and diverts it to the southeast and out of the watershed. Because of the significantly reduced drainage area, the arroyo will be replaced by storm drains in 98<sup>th</sup> Street and Tower Road as planned in Special Assessment District 222 and described below.

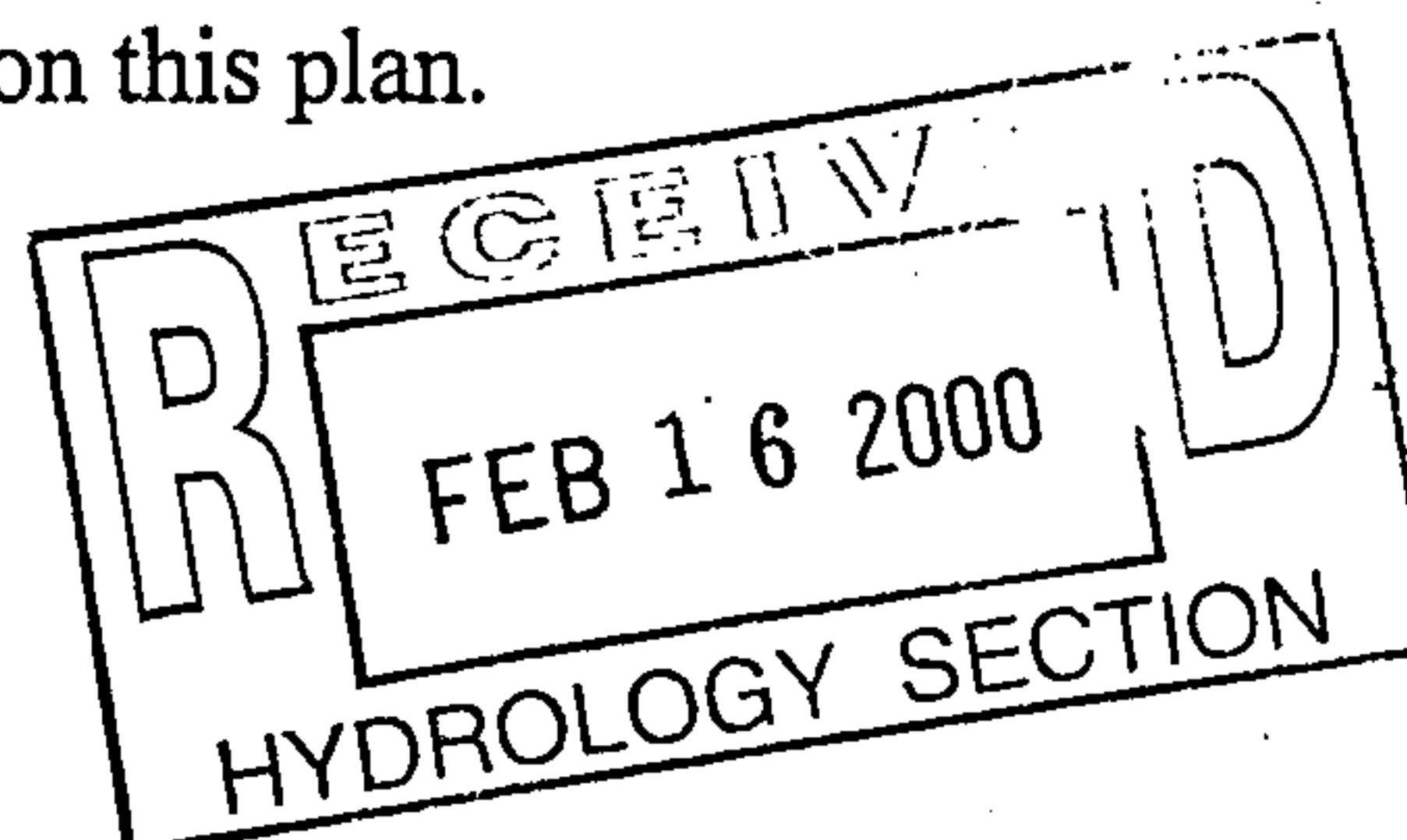
The existing FEMA map No. 35001C0328 D, September 20, 1996, shows a narrow floodplain along the arroyo banks and extending onto the project site, into Unit 3 (see Sheet 1 of 3). This part of Unit 3 will not be developed until a CLOMR is submitted to remove the floodplain for the reach north of Tower Road. The floodplain along Tower Road is expected to be addressed in SAD 222.

The entire project area is included within SAD 222 which provides for future storm drains as follows:

- A 54-inch storm drain draining south along 98<sup>th</sup> Street to Tower Road and by-passing the existing on-site arroyo (see Figure B);
- A 54-inch storm drain draining east along Tower Road;
- A 36-inch and 42-inch storm drain draining south along 94<sup>th</sup> Street to the Tower Road storm drain.

These three proposed storm drains are also shown on Sheet 1 of 3, and preliminary plan and profile sheets from the SAD 222 project are included as Figures A through F.

Consequently, there is no off-site flow under existing and future conditions. The 94<sup>th</sup> Street storm drain, which will be constructed with Unit 2 of this project will be the ultimate outfall for the entire project. However, until the Tower Road storm drain is constructed, the 94<sup>th</sup> Street storm drain will drain to temporary retention ponds, as shown on this plan.





## B. MASTER DRAINAGE PLAN

To facilitate the drainage analysis, the project area was divided into 4 basins numbered 101 through 104. Basin 101, with Tristani Road at its center, is the development area in Unit 1 (51 lots). Under undeveloped conditions Basin 101 drains overland to the east; when developed Basin 101 runoff will be collected in Tristani Road and drain east down Tristani and south on Dean Drive to its intersection with Rowen Road where all runoff is collected in a storm drain going east along Rowen Road and then south along future 94<sup>th</sup> Street. When SAD 222 is completed this storm drain will connect with the Tower Road storm drain.

Basin 102 consists of a small area (2.22 ac.) on the north side of Basin 101 that naturally flows overland across Basin 101 in a southeast direction. Unit 1 will be constructed so that Basin 102 runoff is collected in a swale along the rear of the lots on the north side of Tristani Road. The swale will discharge it's undeveloped condition flows into future 94<sup>th</sup> Street as shown on Sheet 2 of 3, until Unit 4 is constructed. With the future construction of Unit 4, the adjacent lots to the north of Unit 1 will be graded so that these lots will drain north to Eucariz Avenue.

Basin 103, which lies east of Basin 101, extends over future Unit 2 of this subdivision and is centered along 94<sup>th</sup> Street. Undeveloped condition runoff from Basin 103 consists of overland drainage to the southeast out of the proposed subdivision. With Unit 2 development, the storm drain in 94<sup>th</sup> Street from Rowen Road to Tower Road constructed with Unit 1 development) will be extended north to a battery of inlets in 94<sup>th</sup> Street north of its intersection with Eucariz Avenue. This system of inlets and the 94<sup>th</sup> Street storm drain was initially developed in SAD 222, and this plan fully complies with the overall SAD 222 drainage scheme.

Basin 104 is located south of Basin 101 and under undeveloped conditions is characterized by two existing flow paths - the northern part of Basin 104 drains eastwards overland into a broad swale which crosses future 94<sup>th</sup> Street and continues east; the southern part of Basin 104 is the existing arroyo previously discussed in this report. This arroyo drains east to the intersection of Tower Road and 94<sup>th</sup> Street and then continues east in Tower Road. As previously described, the flows in this arroyo have been significantly reduced with the construction of Snow Vista Channel west of 98<sup>th</sup> Street.

When developed, Basin 104 will be centered in Weems Avenue which will carry all drainage east and then north along Dean Drive to the storm drain inlets at the Dean Drive/Rowen Road intersection. After approval of a CLOMR, the arroyo will be filled in and the lots constructed over this area will drain north to Weems Avenue. SAD 222 originally proposed elimination of this arroyo by construction of storm drains in 98<sup>th</sup> Street and Tower Road as previously described.

For analytical convenience, the south half of Basin 103 (south of Rowen Road) was included in Basin 104 for developed conditions only. This was done to more accurately represent developed condition flows in the 94<sup>th</sup> Street storm drain north of Rowen Road.



Basins 103 and 104 represent drainage areas from which undeveloped condition runoff will not be impacted by development of Unit 1. Basins 103 and 104 will be developed in future phases of Eldorado Park and the runoff from these basins will ultimately be managed using the 94<sup>th</sup> Street storm drain, as described on the plan.

#### C. RUNOFF TO RETENTION PONDS

INTERIM

Unit I involves an 8.28-acre interior development of 51 lots as shown on Sheet 2 of 3. The remaining phases are also shown on the plan. Drainage from the proposed development will be retained in a temporary retention pond (Pond A) located east of 94<sup>th</sup> Street in a drainage easement (see Sheet 1 of 3). Consequently, with development of Unit 1 only, the 100-yr peak flow in the 94<sup>th</sup> Street storm drain will be 30.2 cfs (Basins 101F and 102); and with full development of the site, the 100-yr peak flow will be 88.5 cfs. However, the south end of the pipe will be temporarily plugged until the Tower Road storm drain is constructed under SAD 222. A temporary 48-inch CMP diversion pipe will be directed east from the last manhole (see Sheet 1 of 3) to daylight into Pond A.

The permanent 42-inch RCP storm drain in 94<sup>th</sup> Street, and temporary 48-inch CMP storm drain will convey runoff to Pond A as shown on Sheet 1 of 3. The pond is sized to retain the full 100-yr; 10-day runoff volume (1.43 ac-ft) generated by development of Basin 101, compared to the pond volume of 2.46 ac.ft. The CMP storm drain and pond are located on Tracts 24, 25, and 26 of Unit 2, Town of Atrisco Grant; consequently, a temporary drainage easement is provided. A copy of this easement is attached.

#### D. FUTURE DEVELOPMENT PHASES

To develop Unit 2 of Eldorado Park, located on both sides of 94<sup>th</sup> Street, the 94<sup>th</sup> Street storm drain will be extended to the north property line of Unit 2 where it will be temporarily plugged and extended further north under SAD 222 in the future. The 94<sup>th</sup> Street storm drain system will also serve Units 3 and 4 of Eldorado Park; the 100-yr; 10-day runoff amounts to 4.61 ac.ft. for full development of the entire site. Consequently, a second pond, Pond B, identical to Pond A, will be constructed east of Pond A (see Sheet 1 of 3). Total storage volume for both ponds is 4.92 ac.ft.

Development of Unit 3 will require submittal of a ~~CLOMR~~ or LOMR to remove the Zone AE designation along the arroyo. Because of the Snow Vista Diversion and the 98<sup>th</sup> Street storm drain, elimination of the Zone AE area is appropriate.

#### E. UNIT 1 DEVELOPMENT

Sheet 2 of 3 shows the grading plan for Unit 1. Demi Road will have a crest in its' centerline profile to exclude drainage from the valley gutter on 98<sup>th</sup> Street. Therefore, no runoff onto Unit 1 is expected from 98<sup>th</sup> Street or areas west of 98<sup>th</sup> Street.



Standard roll curb and gutter will be used for Demi Road, Hartman Drive, and the west part of Tristani Road. At Sta. 14+90 of Tristani Road there is a 10-foot transition to standard curb and gutter which extends east along Tristani Road, south along Dean Drive and east along Rowen Road to 94<sup>th</sup> Street. Similarly, the west end of Rowen Road (west of Dean Drive) will be roll curb and gutter.

Type A and single and double Type C inlets will be located on both sides of Dean Drive and Rowen Road at their intersection, as shown on Sheet 2 of 3. These inlets will drain to a 42-inch storm drain in Rowen Road which outfalls into the 42-inch storm drain in 94<sup>th</sup> Street. Sheet 3 of 3 provides details on the inlet locations, connector pipes and their respective elevations and capacities.

Table 1 provides hydrologic data for the major drainage subareas (basins) within the development for existing, Unit 1 development only, and full development conditions. In order to design the storm drain system at the intersection of Dean Drive and Rowen Road, and in 94<sup>th</sup> Street, individual street flows are computed in Tables 2A through 2D. These total computed flows for each street were divided between each side of the respective street based on approximate contributory area to that side of the street. These splits are listed on Table 3.

The computed flows and street capacities at a curb depth of 0.6 ft. are compared in Table 3 which also includes number of inlets determination using Type A and C inlets and a flow depth over the grate of 0.7 ft.

Table 4 lists the connector pipe lengths, flow requirements and computed pipe sizes. A 42-inch diameter storm drain in Rowen Road from Dean Drive to 94<sup>th</sup> Street is selected based on  $Q_{100} = 67.8$  cfs (Basins 101F and 104F only),  $S = 1\%$ , and  $n = 0.015$ . The storm drain in 94<sup>th</sup> Street was designed in SAD 222 (See Figures C and D), except that the 36-inch diameter pipe specified in the SAD was enlarged to 42-inch diameter from Rowen Road to Tower Road.

The lots south of Tristani Road, east of Hartman Drive and Dean Drive are all higher than the existing grade so that temporary slope grading is required but all runoff from these lots drains to the street in the front of these lots.

## **F. SUPPLEMENTARY MATERIALS**

Other materials submitted with this report include:

- A copy of the plat of the proposed subdivision

- A copy of the temporary drainage easement on Tracts 23, 24, 25, and 26, Town of Atrisco Grant



TABLE 1

## EXISTING CONDITION HYDROLOGY

| Basin | Area<br>(ac) | Land Treatment |    |    |    | Q-Peak (cfs) |        | Volume (ac-ft) |        |        |
|-------|--------------|----------------|----|----|----|--------------|--------|----------------|--------|--------|
|       |              | %A             | %B | %C | %D | 10-yr        | 100-yr | 10-yr          | 100-yr | 100-yr |
|       |              |                |    |    |    | 6-hr         |        | 6-hr           |        | 10-day |
| 101   | 8.28         | 100            | 0  | 0  | 0  | 2.0          | 10.7   | .055           | .304   | .304   |
| 102   | 2.22         | 100            | 0  | 0  | 0  | 0.5          | 2.9    | .015           | .081   | .081   |
| 103   | .78          | 100            | 0  | 0  | 0  | .2           | 1.0    | .005           | .029   | .029   |
| 104   | 6.12         | 100            | 0  | 0  | 0  | 1.5          | 7.9    | .041           | .224   | .224   |
| TOTAL |              |                |    |    |    | 4.2          | 22.5   | .116           | .638   | .638   |

INTERIM

## FUTURE CONDITION HYDROLOGY - ONLY UNIT 1 DEVELOPED

| Basin | Area<br>(ac) | Land Treatment |    |    |    | Q-Peak (cfs) |        | Volume (ac-ft) |        |        |
|-------|--------------|----------------|----|----|----|--------------|--------|----------------|--------|--------|
|       |              | %A             | %B | %C | %D | 10-yr        | 100-yr | 10-yr          | 100-yr | 100-yr |
|       |              |                |    |    |    | 6-hr         |        | 6-hr           |        | 10-day |
| 101F  | 8.28         | 0              | 35 | 16 | 49 | 15.9         | 27.4   | .521           | .937   | 1.430  |
| 102   | 2.20         | 100            | 0  | 0  | 0  | 0.5          | 2.8    | .015           | .081   | .081   |
| 103   | 0.78         | 100            | 0  | 0  | 0  | .2           | 1.0    | .005           | .029   | .029   |
| 104   | 6.12         | 90             | 0  | 10 | 0  | 1.5          | 7.9    | .041           | .224   | .224   |
| TOTAL |              |                |    |    |    | 18.1         | 39.1   | .582           | 1.271  | 1.764  |

## FUTURE CONDITION HYDROLOGY - FULL DEVELOPMENT

| Basin* | Area<br>(ac) | Land Treatment |    |    |    | Q-Peak (cfs) |        | Volume (ac-ft) |        |        |
|--------|--------------|----------------|----|----|----|--------------|--------|----------------|--------|--------|
|        |              | %A             | %B | %C | %D | 10-yr        | 100-yr | 10-yr          | 100-yr | 100-yr |
|        |              |                |    |    |    | 6-hr         |        | 6-hr           |        | 10-day |
| 101F   | 8.28         | 0              | 35 | 16 | 49 | 15.9         | 27.4   | .521           | 0.937  | 1.430  |
| 102F   | 0.47         | 100            | 0  | 0  | 0  | 7.2          | 12.8   | .235           | 0.435  | 0.656  |
|        | 3.69         | 0              | 35 | 16 | 49 |              |        |                |        |        |
| 103F   | 2.38         | 0              | 35 | 16 | 49 | 4.6          | 7.9    | .150           | .269   | .412   |
| 104F   | 12.21        | 0              | 35 | 16 | 49 | 23.5         | 40.4   | .768           | 1.382  | 2.115  |
| TOTAL  |              |                |    |    |    | 51.2         | 88.5   | 1.674          | 3.023  | 4.613  |

\*see Sheet 2 for basin boundaries



TABLE 2A

PROJECT: Tower West--Estimated Street Flows  
 BASIN : Flow from Hartman to Weems Ave to Dean Dr  
 CONDITION: Developed

## Part A -- Procedure for 40 Acre and Smaller Basins\*

## Input

| zone (x) | area (acres) | land treatment (%) |
|----------|--------------|--------------------|
| 1        | 67.000       | A                  |
| 2        |              | B                  |
| 3        |              | C                  |
| 4        |              | D                  |

## Output

|        |        |        | (ac-ft) |        |        |        | (cfs) |
|--------|--------|--------|---------|--------|--------|--------|-------|
| Volume | 2-YR   | 6-hr   | 0.273   | Q-Peak | 2-YR   | 8.026  |       |
|        |        | 24-hr  | 0.410   |        | 10-YR  | 16.064 |       |
|        |        | 4-day  | 0.506   |        | 100-YR | 26.276 |       |
|        |        | 10-day | 0.533   |        |        |        |       |
|        | 10-YR  | 6-hr   | 0.536   |        |        |        |       |
|        |        | 24-hr  | 0.673   |        |        |        |       |
|        |        | 4-day  | 0.769   |        |        |        |       |
|        |        | 10-day | 0.796   |        |        |        |       |
|        | 100-YR | 6-hr   | 0.941   |        |        |        |       |
|        |        | 24-hr  | 1.077   |        |        |        |       |
|        |        | 4-day  | 1.310   |        |        |        |       |
|        |        | 10-day | 1.570   |        |        |        |       |

\* City of Albuquerque Development Process Manual, Volume 1, 1997 Revision, pages 22-7 to 22-16.



TABLE 2B

PROJECT: Tower West—Estimated Street Flows  
 BASIN : Flow down 94th Street to Rowen Road  
 CONDITION: Developed

## Part A -- Procedure for 40 Acre and Smaller Basins\*

## Input

| zone (x) | area (acres) | land treatment (%) |
|----------|--------------|--------------------|
| 1        | 32000        | A                  |
| 2        |              | B                  |
| 3        |              | C                  |
| 4        |              | D                  |

## Output

|        |        |        | (ac-ft) |        |        |        | (cfs) |
|--------|--------|--------|---------|--------|--------|--------|-------|
| Volume | 2-YR   | 6-hr   | 0.130   | Q-Peak | 2-YR   | 3.833  |       |
|        |        | 24-hr  | 0.196   |        | 10-YR  | 7.672  |       |
|        |        | 4-day  | 0.241   |        | 100-YR | 12.550 |       |
|        |        | 10-day | 0.255   |        |        |        |       |
|        | 10-YR  | 6-hr   | 0.256   |        |        |        |       |
|        |        | 24-hr  | 0.321   |        |        |        |       |
|        |        | 4-day  | 0.367   |        |        |        |       |
|        |        | 10-day | 0.380   |        |        |        |       |
|        | 100-YR | 6-hr   | 0.449   |        |        |        |       |
|        |        | 24-hr  | 0.515   |        |        |        |       |
|        |        | 4-day  | 0.626   |        |        |        |       |
|        |        | 10-day | 0.750   |        |        |        |       |

\* City of Albuquerque Development Process Manual, Volume 1, 1997 Revision, pages 22-7 to 22-16.



TABLE 2C

PROJECT: Tower West—Estimated Street Flows  
 BASIN : Flow down Rowen Road  
 CONDITION: Developed

156750ft<sup>2</sup>

## Part A -- Procedure for 40 Acre and Smaller Basins\*

## Input

| zone (x) | area (acres) | land treatment (%) |
|----------|--------------|--------------------|
| 1        | 36000        | A                  |
| 2        |              | B                  |
| 3        |              | C                  |
| 4        |              | D                  |

## Output

| Output |        |        | (ac-ft) |        |        | (cfs)  |
|--------|--------|--------|---------|--------|--------|--------|
| Volume | 2-YR   | 6-hr   | 0.147   | Q-Peak | 2-YR   | 4.312  |
|        |        | 24-hr  | 0.220   |        | 10-YR  | 8.631  |
|        |        | 4-day  | 0.272   |        | 100-YR | 14.118 |
|        |        | 10-day | 0.286   |        |        |        |
|        | 10-YR  | 6-hr   | 0.288   |        |        |        |
| 24-hr  |        | 0.362  |         |        |        |        |
| 4-day  |        | 0.413  |         |        |        |        |
| 10-day |        | 0.428  |         |        |        |        |
|        | 100-YR | 6-hr   | 0.505   |        |        |        |
| 24-hr  |        | 0.579  |         |        |        |        |
| 4-day  |        | 0.704  |         |        |        |        |
| 10-day |        | 0.844  |         |        |        |        |

\* City of Albuquerque Development Process Manual, Volume 1, 1997 Revision, pages 22-7 to 22-16.



TABLE 2D

PROJECT: Tower West--Estimated Street Flows  
 BASIN : Flow down 94th Street from Rowen Road to Tower  
 CONDITION: Developed

156750ft<sup>2</sup>

## Part A -- Procedure for 40 Acre and Smaller Basins\*

## Input

| zone (x) | area (acres) | land treatment (%) |
|----------|--------------|--------------------|
| 1        | 36000        | A                  |
| 2        |              | B                  |
| 3        |              | C                  |
| 4        |              | D                  |

## Output

| (ac-ft) |      |        |       | (cfs)  |        |        |
|---------|------|--------|-------|--------|--------|--------|
| Volume  | 2-YR | 6-hr   |       | Q-Peak | 2-YR   |        |
|         |      | 24-hr  | 0.147 |        |        | 4.312  |
|         |      | 4-day  | 0.220 |        | 10-YR  | 8.631  |
|         |      | 10-day | 0.272 |        | 100-YR | 14.118 |
|         |      |        | 0.286 |        |        |        |
| 10-YR   |      | 6-hr   | 0.288 |        |        |        |
|         |      | 24-hr  | 0.362 |        |        |        |
|         |      | 4-day  | 0.413 |        |        |        |
|         |      | 10-day | 0.428 |        |        |        |
| 100-YR  |      | 6-hr   | 0.505 |        |        |        |
|         |      | 24-hr  | 0.579 |        |        |        |
|         |      | 4-day  | 0.704 |        |        |        |
|         |      | 10-day | 0.844 |        |        |        |

\* City of Albuquerque Development Process Manual, Volume 1, 1997 Revision, pages 22-7 to 22-16.



TABLE 3

## STREET AND INLET CAPACITIES

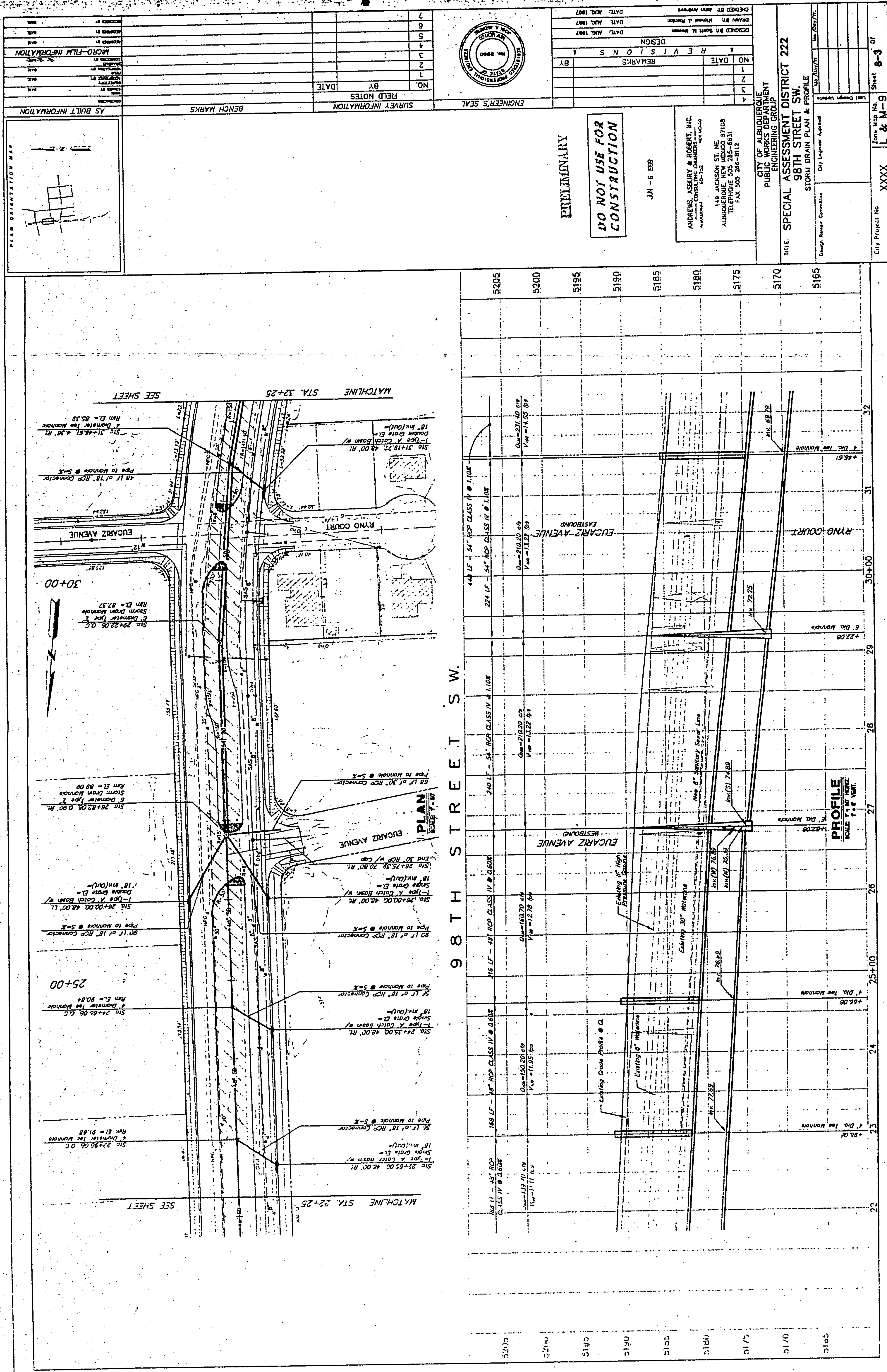
| Street             | Total Flow | Right Curb Flow | Left Curb Flow | Street Slope | Half Street Capacity (1) | 100-Yr Flow Depth (2) | Inlet Capacity (3) | No. of Inlets Required |
|--------------------|------------|-----------------|----------------|--------------|--------------------------|-----------------------|--------------------|------------------------|
|                    | cfs        | cfs             | cfs            | %            | cfs                      | ft                    | cfs                |                        |
| Dean Drive - North |            |                 |                |              |                          |                       |                    |                        |
| Total              | 25.3       |                 |                | 1.74         |                          |                       |                    |                        |
| Right Gutter       |            | 12.6            |                |              | 32                       | 0.43                  | 5.5                | 1 - Type "A"           |
|                    |            |                 |                |              |                          |                       | 6.8                | 1 - Double Type "C"    |
| Left Gutter        |            |                 | 12.6           |              | 32                       | 0.43                  | 5.5                | 1 - Type "A"           |
|                    |            |                 |                |              |                          |                       | 6.8                | 1 - Double Type "C"    |
| Dean Drive - South |            |                 |                |              |                          |                       |                    |                        |
| Total              | 26.3       |                 |                | 0.66         |                          |                       |                    |                        |
| Right Gutter       |            | 13.5            |                |              | 20                       | 0.52                  | 5.5                | 1 - Type "A"           |
|                    |            |                 |                |              |                          |                       | 7.4                | 1 - Double Type "C"    |
| Left Gutter        |            |                 | 13.5           |              | 20                       | 0.52                  | 5.5                | 1 - Type "A"           |
|                    |            |                 |                |              |                          |                       | 7.4                | 1 - Double Type "C"    |
| Rowen Road - West  |            |                 |                |              |                          |                       |                    |                        |
| Total              | 14.1       |                 |                | 2.45         |                          |                       |                    |                        |
| Right Gutter       |            | 7               |                |              | 38                       | 0.36                  | 4.5                | 1 - Single Type "A"    |
|                    |            |                 |                |              |                          |                       | 4.5                | 1 - Single Type "C"    |
| Left Gutter        |            |                 | 7              |              | 38                       | 0.36                  | 4.5                | 1 - Single Type "A"    |
|                    |            |                 |                |              |                          |                       | 4.5                | 1 - Single Type "C"    |

(1) Based on Plate 22.3 D-1 for 32 ft. street; d = 0.6 ft (0.67 ft. available)

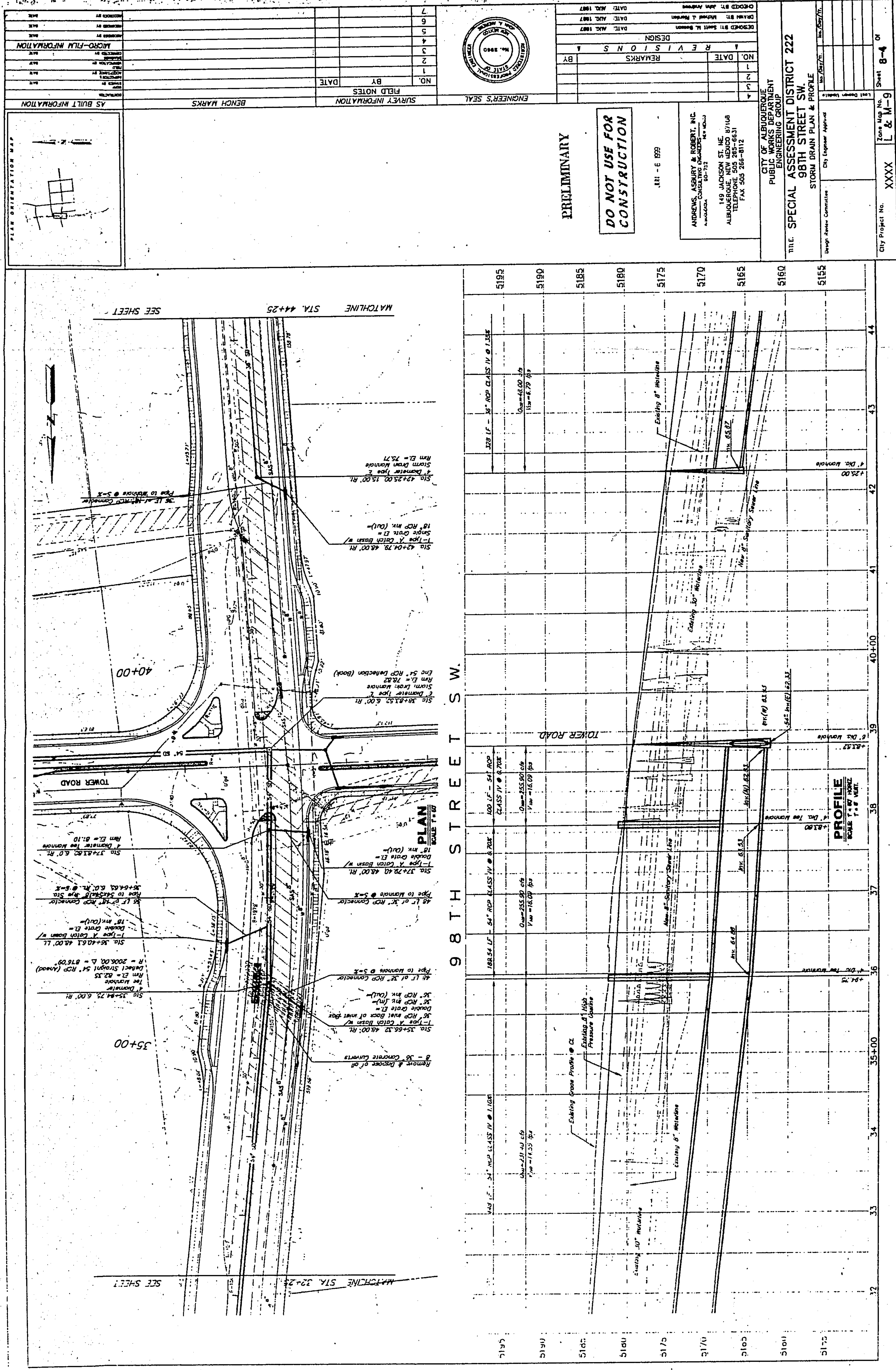
(2) From Plate 22.3 D-1 for 32' Street

(3) Based on Plate 22.3 D-5 for single "A" or "C" inlet or Plate 22.3 D-6 for double "C" inlets















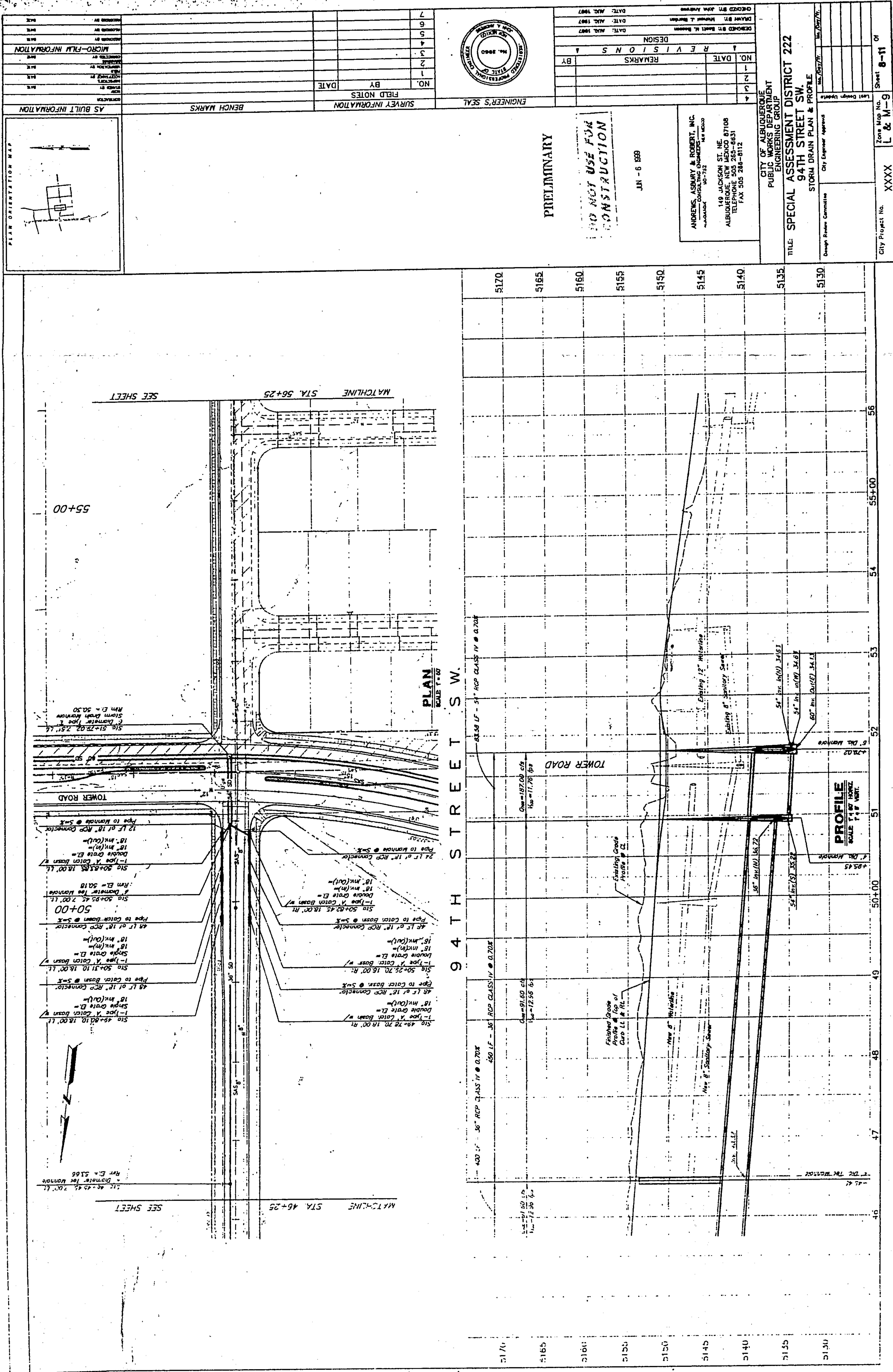
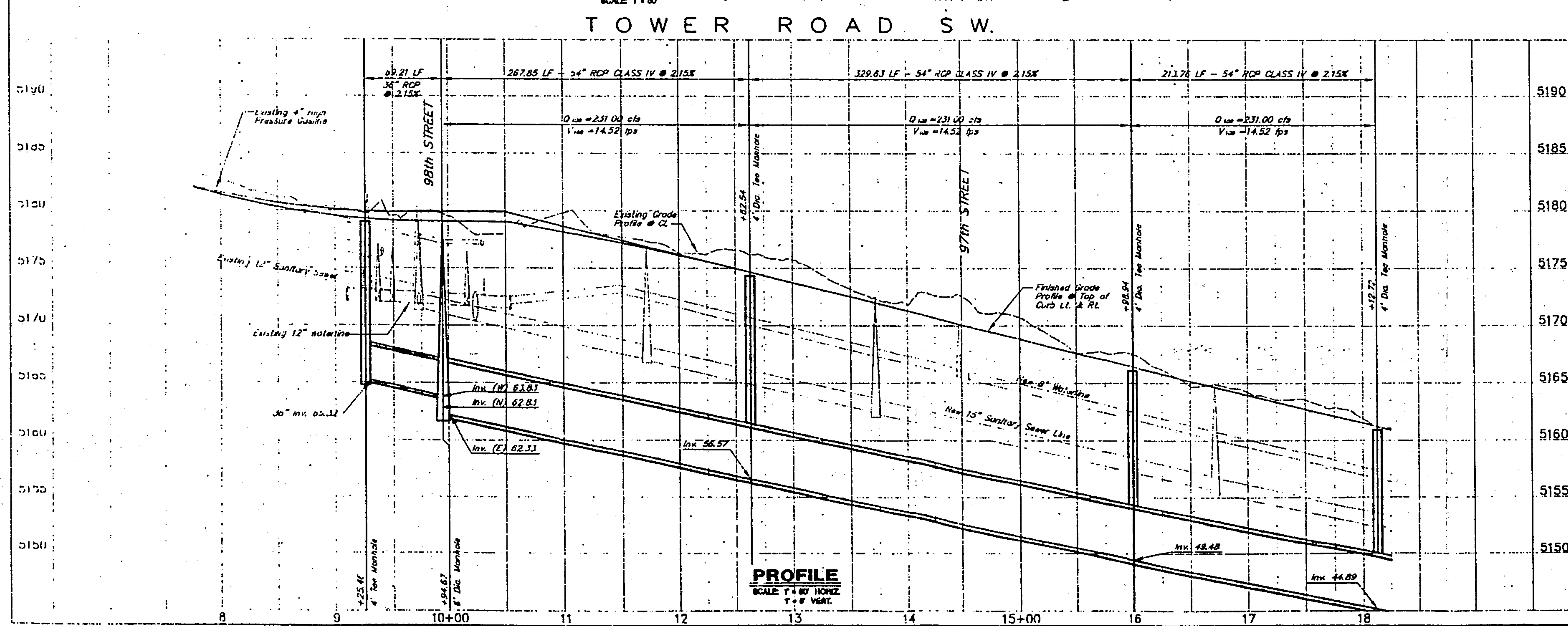
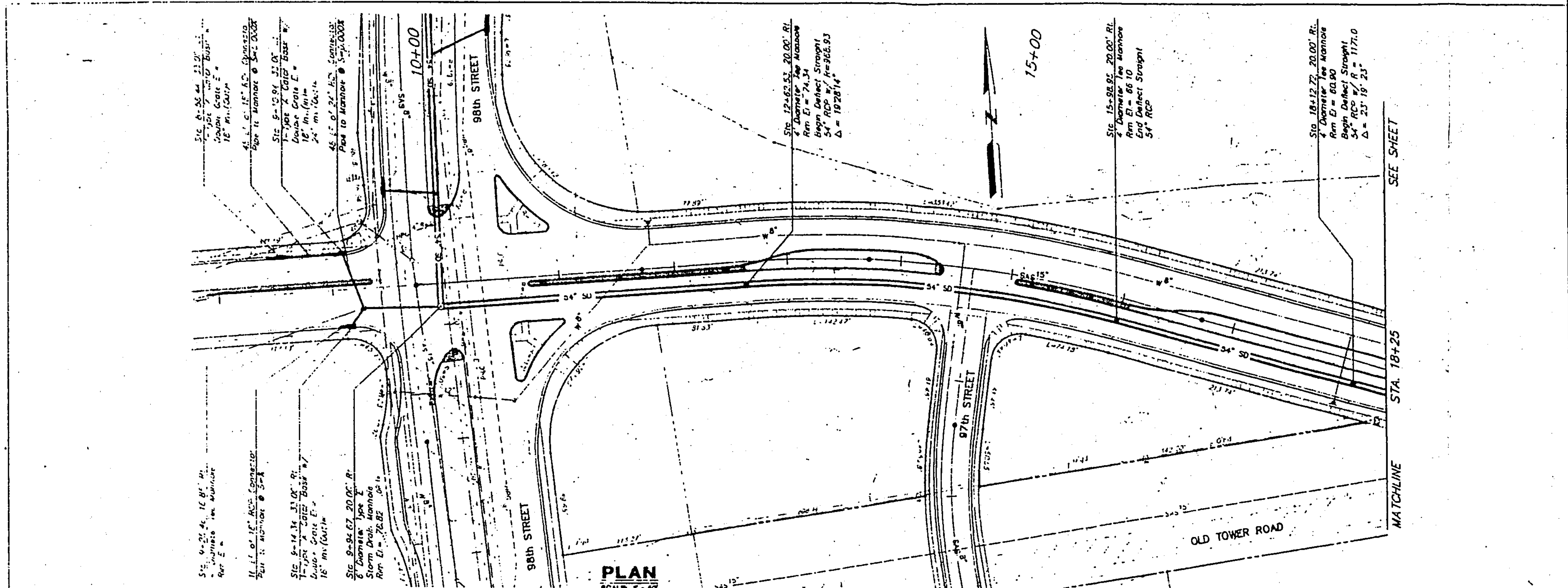


FIGURE D





PLAN ORIENTATION MAP

AS BUILT INFORMATION

| NO. | DATE | BY |
|-----|------|----|
| 1   |      |    |
| 2   |      |    |
| 3   |      |    |
| 4   |      |    |
| 5   |      |    |
| 6   |      |    |
| 7   |      |    |

BENCH MARKS

SURVEY INFORMATION

FIELD NOTES

ENGINEER'S SEAL

REVISIONS

| NO. | DATE | REMARKS | BY |
|-----|------|---------|----|
| 1   |      |         |    |
| 2   |      |         |    |
| 3   |      |         |    |
| 4   |      |         |    |
| 5   |      |         |    |
| 6   |      |         |    |
| 7   |      |         |    |

**PRELIMINARY**

**DO NOT USE FOR CONSTRUCTION**

JUN - 6 1999

**ANDREWS, ASBURY & ROBERT, INC.**  
CONSULTING ENGINEERS  
ALBUQUERQUE, NEW MEXICO 87108  
TELEPHONE 505 265-6631  
FAX 505 266-8112

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CITY OF ALBUQUERQUE  
PUBLIC WORKS DEPARTMENT  
ENGINEERING GROUP

**TITLE: SPECIAL ASSESSMENT DISTRICT 222  
TOWER ROAD SW.  
STORM DRAIN PLAN & PROFILE**

Design Review Committee

City Engineer Approval

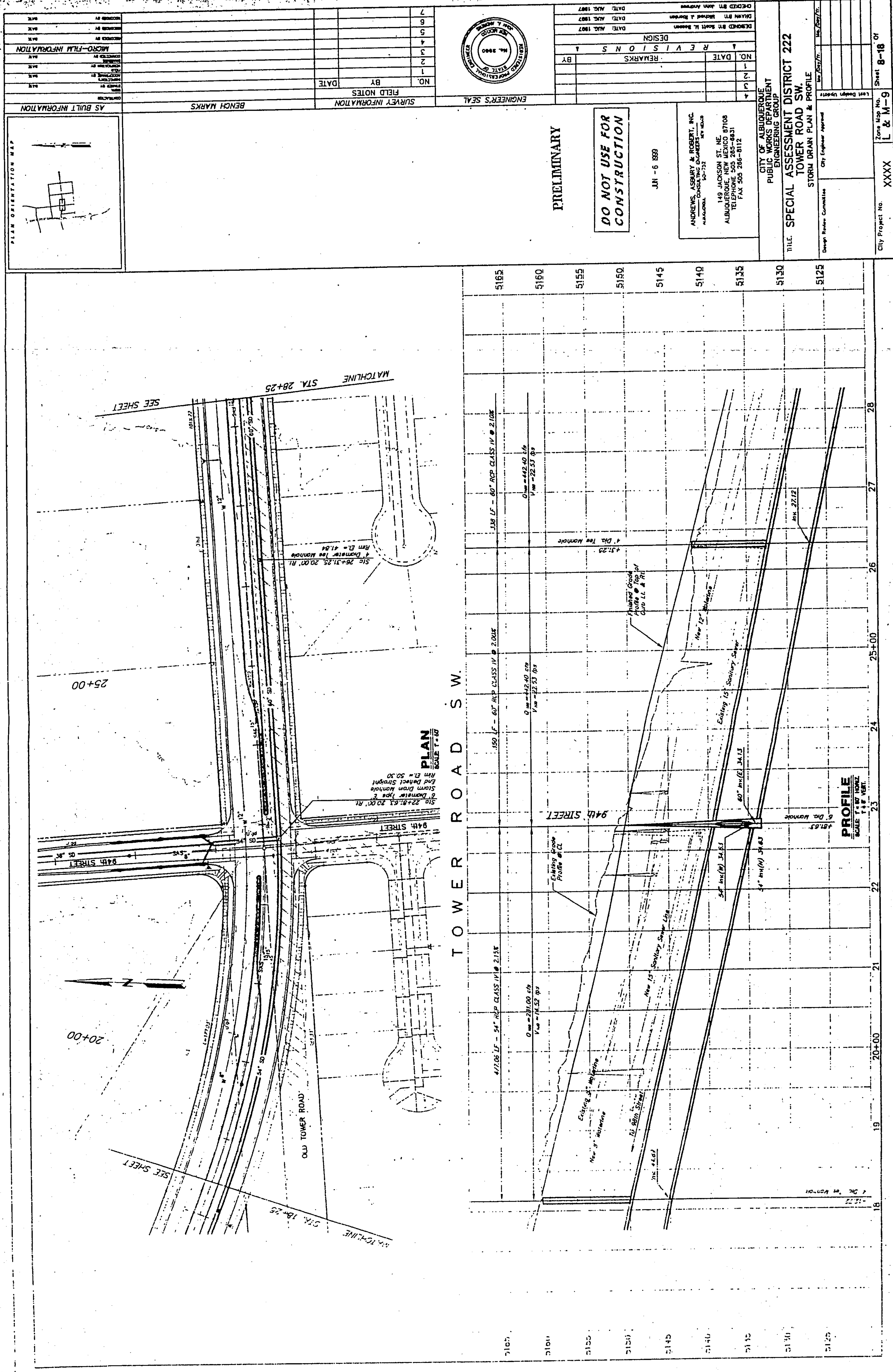
City Project No. XXXX

Zone Map No. L & M-9

Sheet 8-17 of

FIGURE E







LEGAL DESCRIPTION - TEMPORARY DRAINAGE EASEMENT

A certain tract of land, reserved as a Temporary Drainage Easement, being and comprising a portion of Tract A-1A, TOWER WEST, as the same is shown and designated on the plat thereof, filed in the office of the County Clerk of Bernalillo County, New Mexico on May 27, 1999 in Bk. 99C, Pg. 125 and being Ten feet (10') on either side of the following described centerline:

BEGINNING at the most easterly point of said tract herein described, a point on the westerly right-of-way line of 94th Street S.W., whence a capped rebar monumenting a point of curvature on 94th Street S.W. bears S 05°59'34" E, 514.02 feet distant; thence, from said point of beginning S 84°00'26" W, 116.94 feet to the terminus point of said tract herein described, containing 2338.80 square feet, more or less (0.0537 acres, more or less).

LEGAL DESCRIPTION - TEMPORARY DRAINAGE EASEMENT

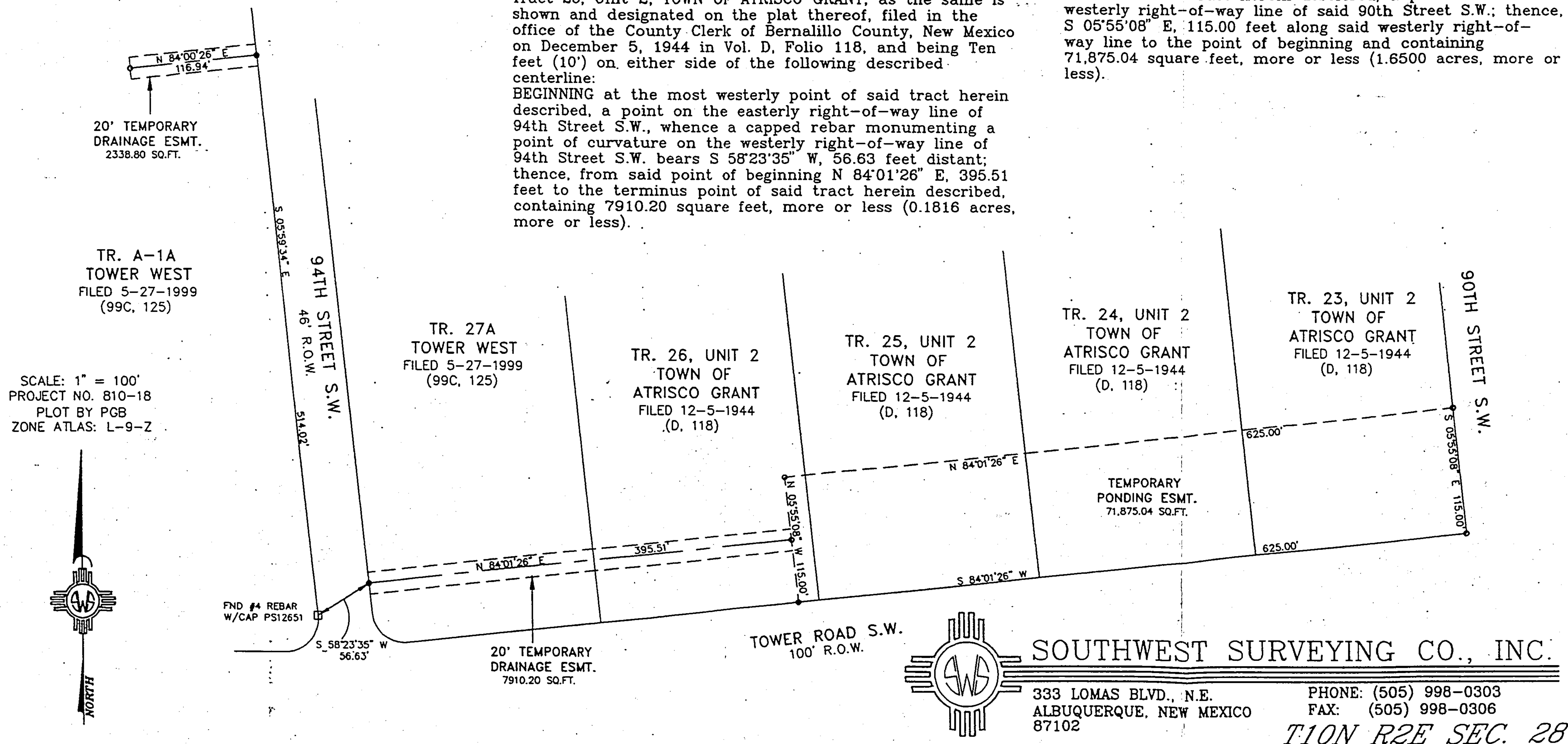
A certain tract of land, reserved as a Temporary Drainage Easement, being and comprising a portion of Tract 27A, TOWER WEST, as the same is shown and designated on the plat thereof, filed in the office of the County Clerk of Bernalillo County, New Mexico on May 27, 1999 in Bk. 99C, Pg. 125; TOGETHER WITH a portion of Tract 26, Unit 2, TOWN OF ATRISCO GRANT, as the same is shown and designated on the plat thereof, filed in the office of the County Clerk of Bernalillo County, New Mexico on December 5, 1944 in Vol. D, Folio 118, and being Ten feet (10') on either side of the following described centerline:

BEGINNING at the most westerly point of said tract herein described, a point on the easterly right-of-way line of 94th Street S.W., whence a capped rebar monumenting a point of curvature on the westerly right-of-way line of 94th Street S.W. bears S 58°23'35" W, 56.63 feet distant; thence, from said point of beginning N 84°01'26" E, 395.51 feet to the terminus point of said tract herein described, containing 7910.20 square feet, more or less (0.1816 acres, more or less).

LEGAL DESCRIPTION - TEMPORARY PONDING EASEMENT

A certain tract of land, reserved as a Temporary Ponding Easement, being and comprising a portion of Tracts 23, 24, 25 & 26, Unit 2, TOWN OF ATRISCO GRANT, as the same is shown and designated on the plat thereof, filed in the office of the County Clerk of Bernalillo County, New Mexico on December 5, 1944 in Volume D, Folio 118 and being more particularly described as follows:

BEGINNING at the southeast corner of said tract herein described, the northwesterly point of intersection between 90th Street S.W. and Tower Road S.W., being the southeast corner of said Tract 23; thence, from said point of beginning S 84°01'26" W, 625.00 feet along the northerly right-of-way line of said Tower Road S.W. to the southwest corner of said tract herein described; thence, leaving said northerly right-of-way line, N 05°55'08" W, 115.00 feet to the northwest corner of said tract herein described; thence, N 84°01'26" E, 625.00 feet to the northeast corner of said tract herein described, a point on the westerly right-of-way line of said 90th Street S.W.; thence, S 05°55'08" E, 115.00 feet along said westerly right-of-way line to the point of beginning and containing 71,875.04 square feet, more or less (1.6500 acres, more or less).



T10N R2E SEC. 28





# ***City of Albuquerque***

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

January 25, 2002

June Page, P.E.  
Resource Technology, Inc.  
1720-B Randolph Rd SE  
Albuquerque, New Mexico 87106

RE: **ELDORADO PARK SUBD UNIT 3** (L-9/D7)  
**Engineers Certification For Release of Financial Guaranty**  
**Engineers Stamp dated 6/21/2001**  
**Engineer's Certification dated 1/23/2002**

Dear Ms. Page:

Based upon the information provided in your submittal dated 1/24/2002, the above referenced plan is adequate to satisfy the Grading and Drainage Certification requirements for release of financial guaranty for the above mentioned project.

As noted in the Engineers Certification, due to the construction of SAD 222 along Tower Rd and 98th St., the drainage swale and rip-rap called out on the drawings for the above mentioned project will not require construction.

If you have any questions, please call me at 924-3981.

Sincerely,

Teresa A. Martin  
Hydrology Plan Checker  
Public Works Department  
*Bus*

C: Arlene Portillo, PWD – #665381

*File*



**DRAINAGE INFORMATION SHEET**  
(REV. 11/01/2001)

L-9/D7

PROJECT TITLE: Eldorado Unit 3

DRB #: \_\_\_\_\_ EPC#: \_\_\_\_\_ ZONE MAP/DRG. FILE #: \_\_\_\_\_  
WORK ORDER#: \_\_\_\_\_

LEGAL DESCRIPTION: \_\_\_\_\_  
CITY ADDRESS: \_\_\_\_\_

ENGINEERING FIRM: Resource Technology  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: One Page  
PHONE: 293-7360  
ZIP CODE: \_\_\_\_\_

OWNER: Tower West Ltd Ps  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
PHONE: \_\_\_\_\_  
ZIP CODE: \_\_\_\_\_

ARCHITECT: \_\_\_\_\_  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
PHONE: \_\_\_\_\_  
ZIP CODE: \_\_\_\_\_

SURVEYOR: \_\_\_\_\_  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
PHONE: \_\_\_\_\_  
ZIP CODE: \_\_\_\_\_

CONTRACTOR: \_\_\_\_\_  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
PHONE: \_\_\_\_\_  
ZIP CODE: \_\_\_\_\_

CHECK TYPE OF SUBMITTAL:

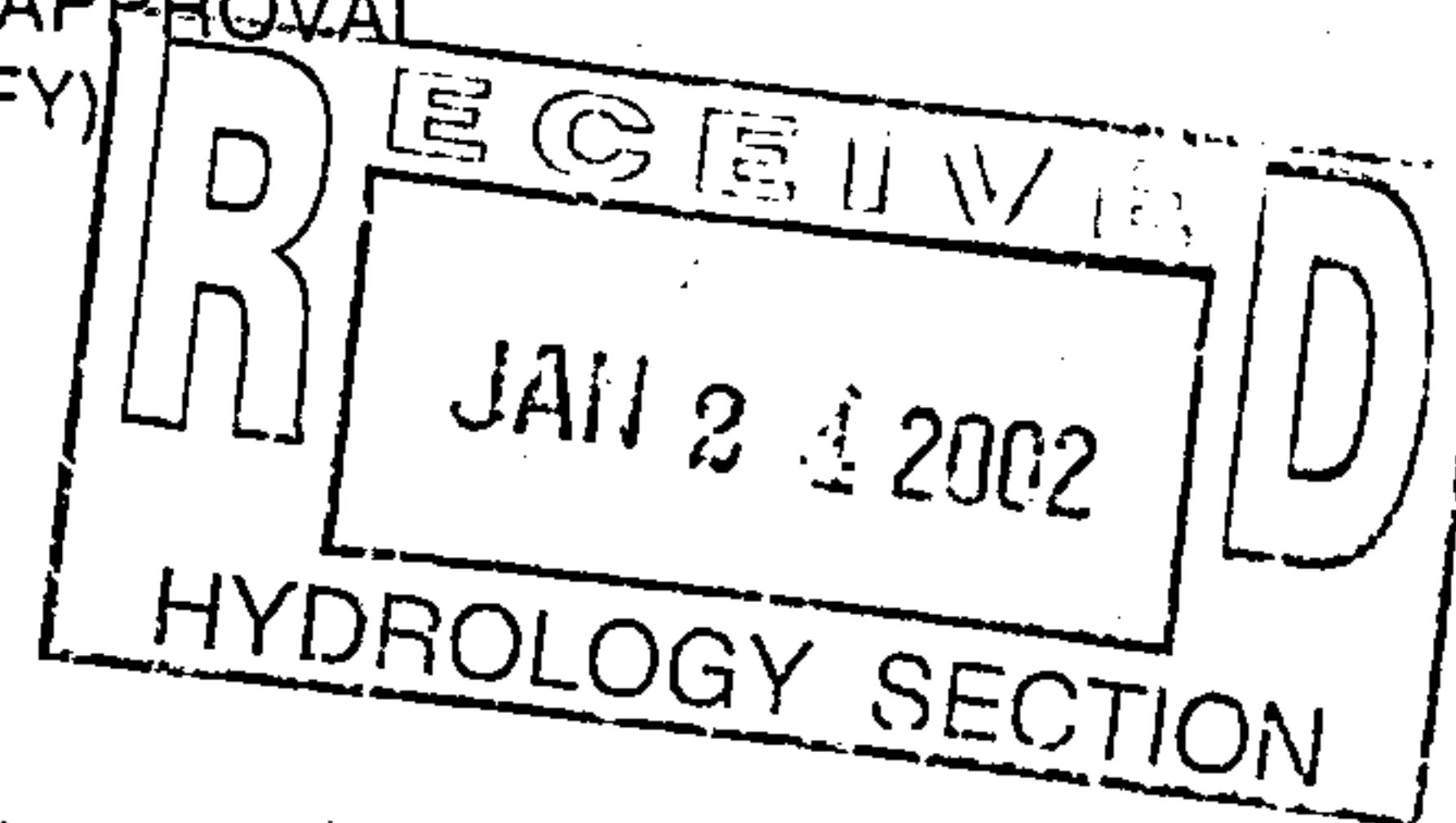
- ☐ DRAINAGE REPORT
- ☐ DRAINAGE PLAN
- ☐ CONCEPTUAL GRADING & DRAINAGE PLAN
- ☐ GRADING PLAN
- ☒ EROSION CONTROL PLAN
- ☐ ENGINEER'S CERTIFICATION (HYDROLOGY)
- ☐ CLOMR/LOMR
- ☐ TRAFFIC CIRCULATION LAYOUT (TCL)
- ☐ ENGINEERS CERTIFICATION (TCL)
- ☐ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
- ☐ OTHER

CHECK TYPE OF APPROVAL SOUGHT:

- ☐ SIA / FINANCIAL GUARANTEE RELEASE
- ☐ PRELIMINARY PLAT APPROVAL
- ☐ S. DEV. PLAN FOR SUB'D. APPROVAL
- ☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
- ☐ SECTOR PLAN APPROVAL
- ☐ FINAL PLAT APPROVAL
- ☐ FOUNDATION PERMIT APPROVAL
- ☐ BUILDING PERMIT APPROVAL
- ☐ CERTIFICATE OF OCCUPANCY (PERM.)
- ☐ CERTIFICATE OF OCCUPANCY (TEMP.)
- ☐ GRADING PERMIT APPROVAL
- ☐ PAVING PERMIT APPROVAL
- ☐ WORK ORDER APPROVAL
- ☐ OTHER (SPECIFY) \_\_\_\_\_

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
- ☐ NO
- ☐ COPY PROVIDED



DATE SUBMITTED: 1/24/02 BY: [Signature]

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plan:** Required for approval of Site Development Plans greater than five
2. **Drainage Plans:** Required for building permits, grading permits, paving permits and site plans less than five (5)
3. **Drainage Report:** Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or





# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

May 30, 2001

June Page, PE  
Resource Technology, Inc.  
1720-B Randolph SE  
Albuquerque, NM 87106

Re: Eldorado Park Subdivision, Unit 3 & 4 Grading and Drainage Plan  
Engineer's Stamp 5-25-01, (L9/D7)

Dear Ms. Page,

Based on your information contained in your resubmittal dated 5-29-01, the above referenced plan is approved for Preliminary Plat action by the DRB.

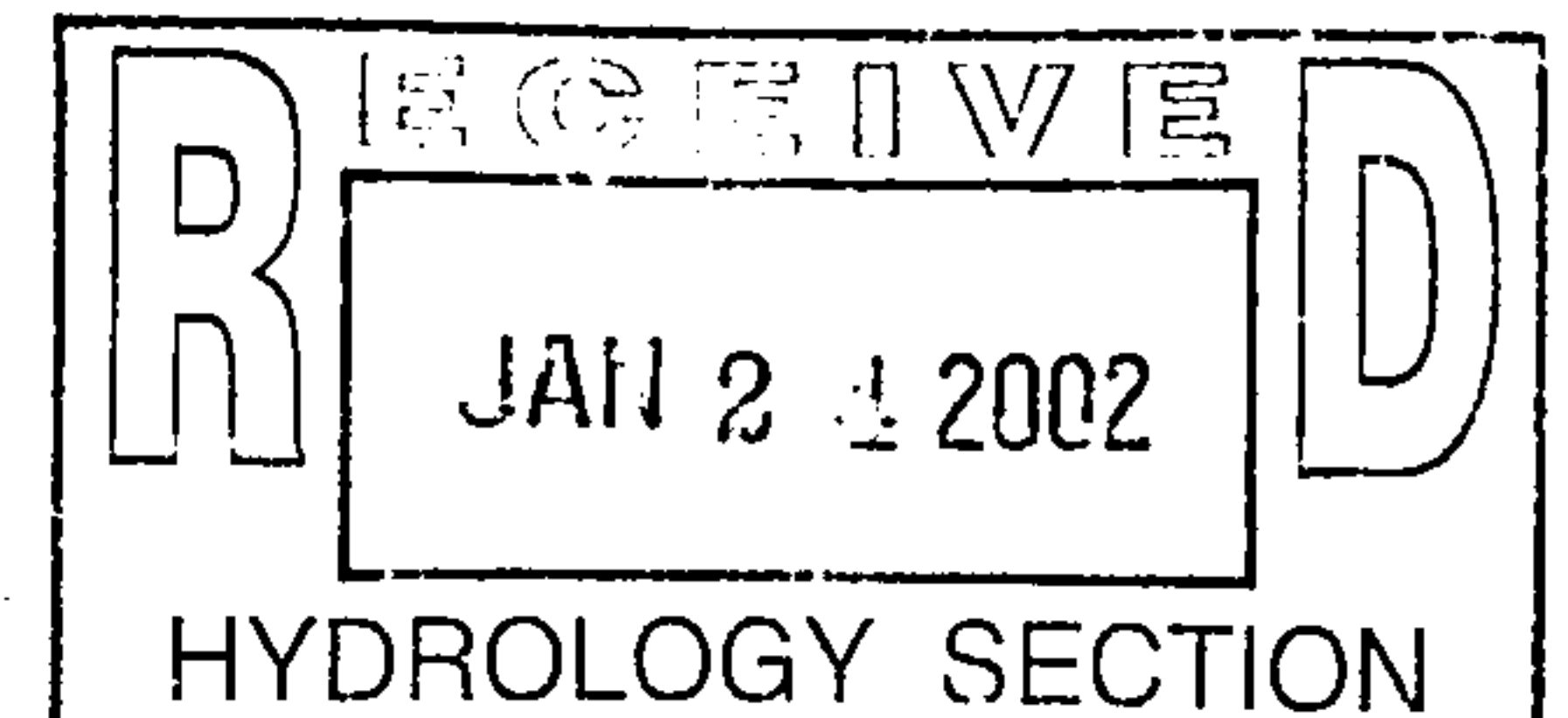
Prior to Final Plat sign-off, the Subdivision Improvements Agreement must be recorded. Please be advised that the Grading and Drainage Certification is required prior to release of the SIA for this project.

If you have any questions, you can contact me at 924-3986.

Sincerely,

*Bradley L. Bingham*  
Bradley L. Bingham, PE  
Sr. Engineer, Hydrology

C: file







# ***City of Albuquerque***

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

March 6, 2002

June Page, P.E.  
Resource Technology  
5501 Jefferson NE Suite 200  
Albuquerque, New Mexico 87109

RE: **ELDORADO PARK SUBD UNIT 4** (L-9/D7)  
**Engineers Certification For Release of Financial Guaranty**  
**Engineers Stamp dated 9/4/2001**  
**Engineers Certification dated 2/28/2002 & 3/5/2002**

Dear Ms. Page:

Based upon the information provided in your Engineers Certification dated 2/28/2002 and 3/5/2002, the above referenced plan is adequate to satisfy the Grading and Drainage Certification for Release of Financial Guaranty for the above referenced project.

If you have any questions, please call me at 924-3981.

Sincerely,

Teresa A. Martin

Hydrology Plan Checker

Public Works Department

**BLB**

C: Arlene Portillo, PWD - #665481  
File



# DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/11/2002)

L-9/07

PROJECT TITLE: El Dorado Park Subdivision <sup>Unit 4</sup> ZONE MAP/DRG. FILE #: ~~2002-0001~~  
 DRB #: \_\_\_\_\_ EPC#: \_\_\_\_\_ WORK ORDER#: \_\_\_\_\_

LEGAL DESCRIPTION: \_\_\_\_\_  
 CITY ADDRESS: \_\_\_\_\_

ENGINEERING FIRM: Resource Technology Inc  
 ADDRESS: 5301 Jefferson NE Ste 200  
 CITY, STATE: ABQ NM 87109

CONTACT: June Page  
 PHONE: 505-730-7300  
 ZIP CODE: \_\_\_\_\_

OWNER: Tower West Lta Partnership  
 ADDRESS: 333 Lomas NE  
 CITY, STATE: ABQ NM 87102

CONTACT: Frank Wilson  
 PHONE: 998-0305  
 ZIP CODE: \_\_\_\_\_

ARCHITECT: N/A  
 ADDRESS: \_\_\_\_\_  
 CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
 PHONE: \_\_\_\_\_  
 ZIP CODE: \_\_\_\_\_

SURVEYOR: Surveys Southwest  
 ADDRESS: 333 Lomas NE  
 CITY, STATE: ABQ NM 87102

CONTACT: Dan Graney  
 PHONE: 998-0303  
 ZIP CODE: \_\_\_\_\_

CONTRACTOR: New Concepts Inc  
 ADDRESS: 200 Box 9555  
 CITY, STATE: ABQ NM 87119

CONTACT: Burr Dickinson  
 PHONE: 432-8910  
 ZIP CODE: \_\_\_\_\_

## CHECK TYPE OF SUBMITTAL:

- ☐ DRAINAGE REPORT
- ☐ DRAINAGE PLAN
- ☐ CONCEPTUAL GRADING & DRAINAGE PLAN
- ☐ GRADING PLAN
- ☒ EROSION CONTROL PLAN
- ☐ ENGINEER'S CERTIFICATION (HYDROLOGY)
- ☐ CLOMR/LOMR
- ☐ TRAFFIC CIRCULATION LAYOUT (TCL)
- ☐ ENGINEERS CERTIFICATION (TCL)
- ☐ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
- ☐ OTHER

## CHECK TYPE OF APPROVAL SOUGHT:

- ☒ SIA / FINANCIAL GUARANTEE RELEASE
- ☐ PRELIMINARY PLAT APPROVAL
- ☐ S. DEV. PLAN FOR SUB'D. APPROVAL
- ☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
- ☐ SECTOR PLAN APPROVAL
- ☐ FINAL PLAT APPROVAL
- ☐ FOUNDATION PERMIT APPROVAL
- ☐ BUILDING PERMIT APPROVAL
- ☐ CERTIFICATE OF OCCUPANCY (PERM.)
- ☐ CERTIFICATE OF OCCUPANCY (TEMP.)
- ☐ GRADING PERMIT APPROVAL
- ☐ PAVING PERMIT APPROVAL
- ☐ WORK ORDER APPROVAL
- ☐ OTHER (SPECIFY)

## WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
- ☐ NO
- ☐ COPY PROVIDED

N/A

DATE SUBMITTED: 3/1/02

BY: Franklin E Wilson

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plan:** Required for approval of Site Development Plans greater than five
2. **Drainage Plans:** Required for building permits, grading permits, paving permits and site plans less than five (5)
3. **Drainage Report:** Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or



**TOWER WEST LTD. PARTNERSHIP**

**333 Lomas Blvd., N.E.**

**Albuquerque, NM 87102**

**(505) 998-0305 / Fax 998-7974**

March 1, 2002

Terri Martin  
City of Albuquerque  
Plaza Del Sol Bldg. 2<sup>nd</sup> Floor  
Albuquerque, NM 87102

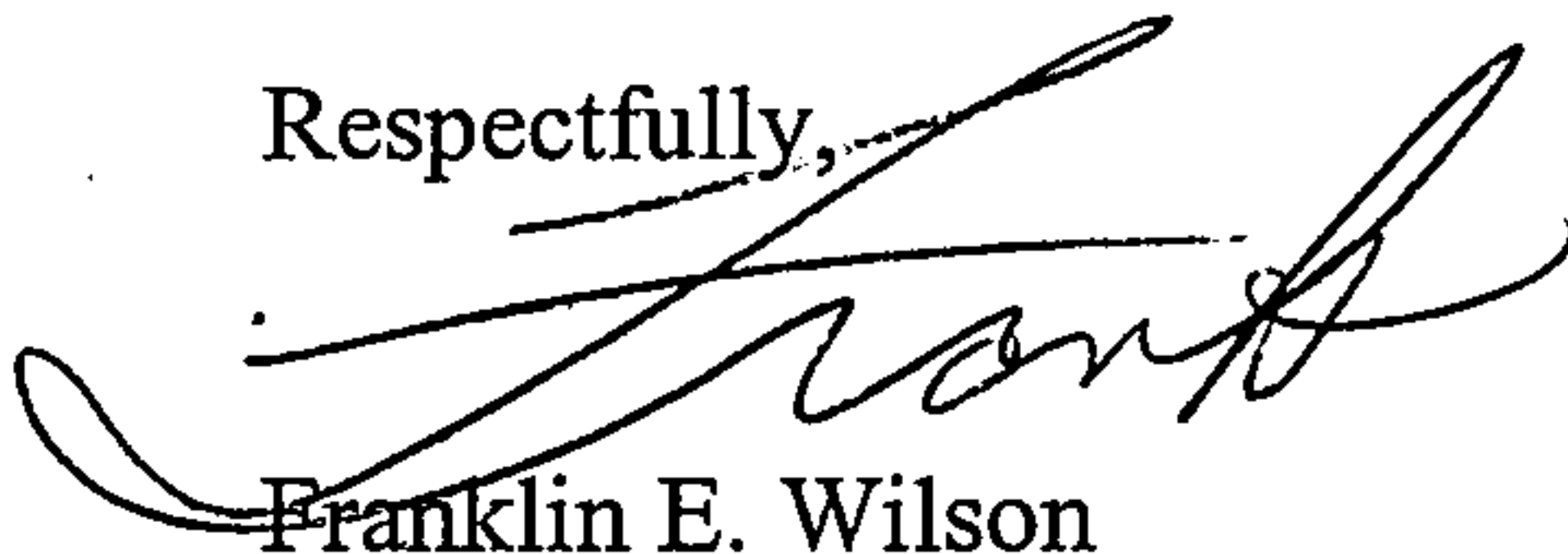
Re : Eldorado Park Subdivision, Unit 4 / #6654.81

Dear Terri:

Attached please find the Drainage and Transportation Information Sheet for Eldorado Park Subdivision, Unit 4. Please prepare the acceptance/approval letter for release of financial guarantee as soon as possible. Your letter is the final item required to enable us to submit the close out package to Public Works.

If you have any questions, please give me a call.

Respectfully,



Franklin E. Wilson  
General Partner



**TOWER WEST LTD. PARTNERSHIP**

**333 Lomas Blvd., N.E.  
Albuquerque, NM 87102  
(505) 998-0305 / Fax 998-7974**

March 5, 2002

Terri Martin  
City of Albuquerque  
Plaza Del Sol Bldg. 2<sup>nd</sup> Floor  
Albuquerque, NM 87102

Re : Eldorado Park Subdivision, Unit 4 / #6654.81

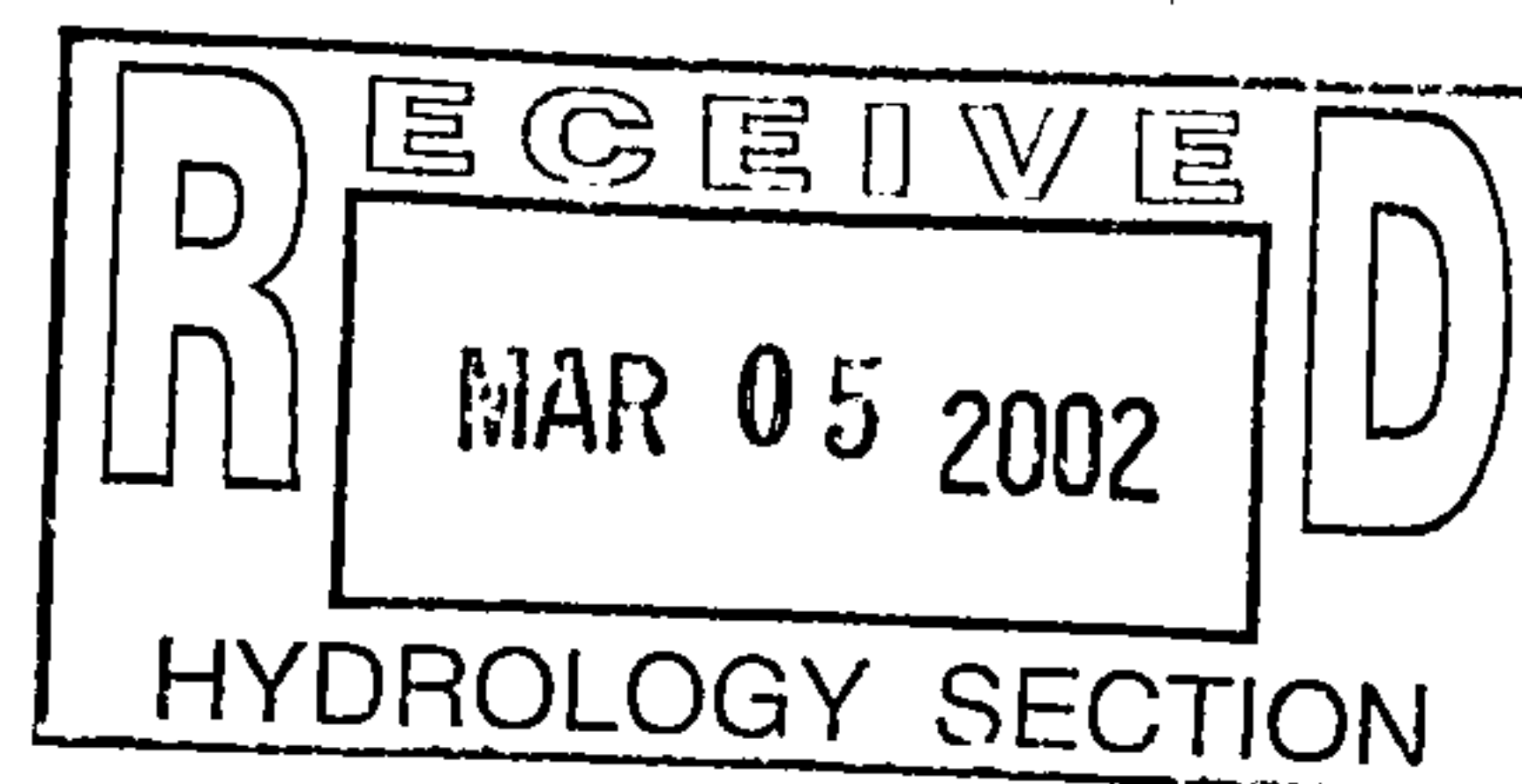
Dear Terri:

Attached please find page 10, the Details sheet for Eldorado Park Subdivision, Unit 4 reflecting the engineers certification. Please prepare the acceptance/approval letter for release of financial guarantee as soon as possible.

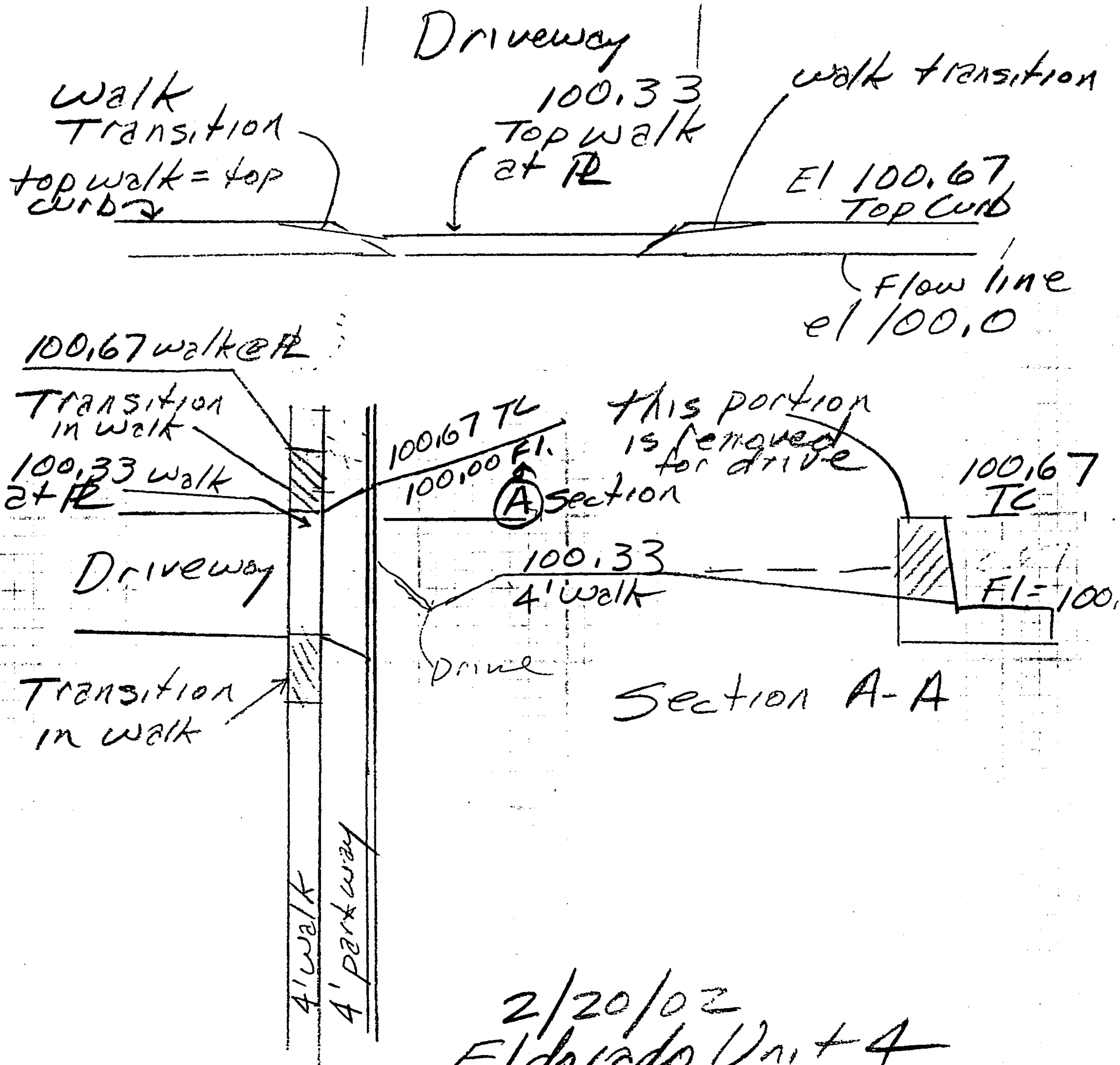
If you have any questions, please give me a call.

Respectfully,

*Franklin E. Wilson for.*  
Franklin E. Wilson  
General Partner







Customer

Frank Wilson

Date

Sketch for Review

Address

BRAD BINGHAM PE

Phone



th

80'

100'

EXIST. SWALE

EXIST. EDGE OF PAVEMENT

# 98 STREET

CL OF EXIST. PAVEMENT

10+00

30+57.37± 98TH  
10+05.75± EUCAF

EXISTING

TEMP. PAVEMENT

TW 5185.35  
EXISTING RETAINING  
WALL TOP

TO BE CONSTRUCTED BY OTHERS

5.6

3-P1

5180

4-P1

72085.3  
72085.1

72085.1  
72084.4

5-P1

72084.4  
72083.7

6-P1 72083.7 P1

8-P1

9-P1

10

RET. 5183.3 TW EXIST. EXIST.  
GARDEN 5185.3 TW

5182.6 EXIST.  
GARDEN WALL TOP

5180.6 TW

5179.99 TW 5180.6 TW

5178.7 TW

5175.3 TW

5173.0 TW

5185.2  
5185.4  
5185.1  
5184.44

151-P1

152-P1

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RECEIVED MAY 16 2001

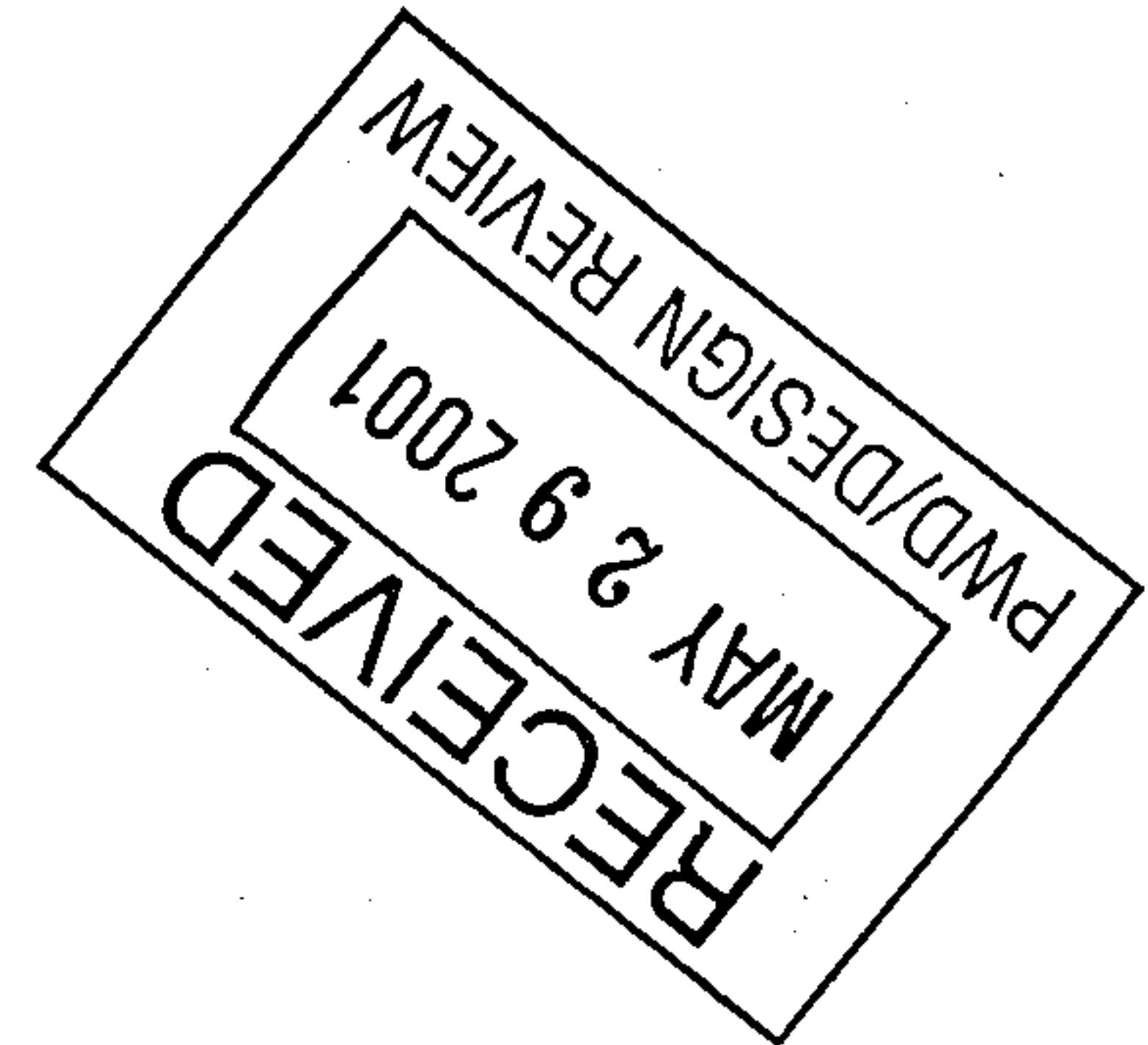


## City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

May 14, 2001

June Page, PE  
Resource Technology, Inc.  
1720-B Randolph SE  
Albuquerque, NM 87106



**Re: Eldorado Park Subdivision, Unit 3 & 4 Grading and Drainage Plan**  
**Engineer's Stamp not dated, (L9/D7)**

Dear Ms. Page,

Based on your information contained in your submittal dated 4-13-01, the above referenced plan cannot be approved for Preliminary Plat until the following comments are addressed.

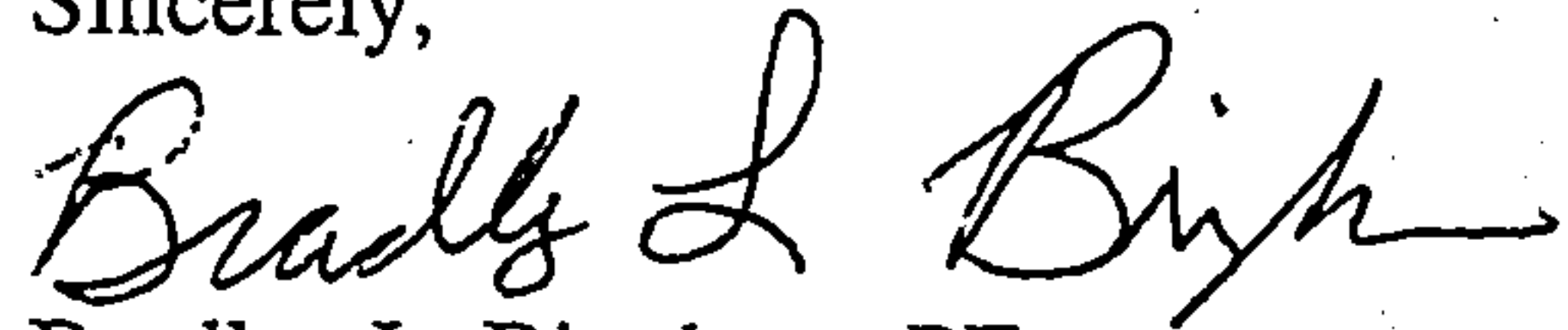
- The Work Order plans from Unit 2 show a 48" RCP in 94<sup>th</sup> but your report states that a 36" RCP is in 94<sup>th</sup> St. Please clarify this.
- You mention that you are adjusting the rainfall amount to Zone 1 and increasing the density. Does this make the runoff increase or decrease from the original report? If it increases, you will need to verify capacity downstream.
- Tower Rd is a minor arterial. Draining backyards across the sidewalk and into the roadway is not prudent for a facility of that designation. Please provide agreement that allows for this (as stated on the grading plan).
- On the Units 3&4 plans, please show the existing pad elevations from Units 1& 2 and any slope or wall needed to fit with these lots. All side yard retaining walls must be CMU or equal (not railroad ties). There appears to be a 4' bust at the return. Please add elevations from the SAD (at 100' intervals) for Tower and 98<sup>th</sup> on these grading plans.
- Please add spot elevations at all four corners (and extensions to the R/W) of each lot and applicable slope or retaining wall symbols to the unit-specific grading plans.
- Please add a date to the report and grading plans.



- By diverting the arroyo as proposed, the remaining three lots from Unit 2 can be developed. It would be prudent to include these in your plan as well. Please show actual grading of the arroyo and how it will fit in with the ultimate grades of the subdivision.
- Sheets 1 of 3 and 2 of 3 were difficult to read. Please resubmit these readable.

If you have any questions about my comments, you can contact me at 924-3986.

Sincerely,

A handwritten signature in cursive script, appearing to read "Bradley L. Bingham".

Bradley L. Bingham, PE  
Sr. Engineer, Hydrology

C: file





# ***City of Albuquerque***

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

May 30, 2001

June Page, PE  
Resource Technology, Inc.  
1720-B Randolph SE  
Albuquerque, NM 87106

**Re: Eldorado Park Subdivision, Unit 3 & 4 Grading and Drainage Plan  
Engineer's Stamp 5-25-01, (L9/D7)**

Dear Ms. Page,

Based on your information contained in your resubmittal dated 5-29-01, the above referenced plan is approved for Preliminary Plat action by the DRB.

Prior to Final Plat sign-off, the Subdivision Improvements Agreement must be recorded. Please be advised that the Grading and Drainage Certification is required prior to release of the SIA for this project.

If you have any questions, you can contact me at 924-3986.

Sincerely,

*Bradley L. Bingham*  
Bradley L. Bingham, PE  
Sr. Engineer, Hydrology

C: file



# DRAINAGE INFORMATION SHEET

PROJECT TITLE: EL DORADO <sup>L-9</sup> ZONE MAP/DRG. FILE #: (?) 665381  
DRB #: \_\_\_\_\_ EPC#: \_\_\_\_\_ WORK ORDER#: \_\_\_\_\_

LEGAL DESCRIPTION: \_\_\_\_\_  
CITY ADDRESS: \_\_\_\_\_

ENGINEERING FIRM: RESOURCE TECHNOLOGY  
ADDRESS: 1720-B RANDOLPH RD SE  
CITY, STATE: ALBQ

CONTACT: JUNE PALE  
PHONE: 402 243 7300  
ZIP CODE: \_\_\_\_\_

OWNER: Frank Wilson  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
PHONE: 610-3303  
ZIP CODE: \_\_\_\_\_

ARCHITECT: \_\_\_\_\_  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
PHONE: \_\_\_\_\_  
ZIP CODE: \_\_\_\_\_

SURVEYOR: \_\_\_\_\_  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
PHONE: \_\_\_\_\_  
ZIP CODE: \_\_\_\_\_

CONTRACTOR: \_\_\_\_\_  
ADDRESS: \_\_\_\_\_  
CITY, STATE: \_\_\_\_\_

CONTACT: \_\_\_\_\_  
PHONE: \_\_\_\_\_  
ZIP CODE: \_\_\_\_\_

TYPE OF SUBMITTAL:  
☐ DRAINAGE REPORT  
☐ DRAINAGE PLAN  
☐ CONCEPTUAL GRADING & DRAINAGE PLAN  
☐ GRADING PLAN  
☐ EROSION CONTROL PLAN  
☐ ENGINEER'S CERTIFICATION  
☐ CLOMR/LOMR  
☐ OTHER

CHECK TYPE OF APPROVAL SOUGHT:  
☐ SIA / FINANCIAL GUARANTEE RELEASE  
☐ PRELIMINARY PLAT APPROVAL  
☐ S. DEV. PLAN FOR SUB'D. APPROVAL  
☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL  
☐ SECTOR PLAN APPROVAL  
☐ FINAL PLAT APPROVAL  
☐ FOUNDATION PERMIT APPROVAL  
☐ BUILDING PERMIT APPROVAL  
☐ CERTIFICATE OF OCCUPANCY APPROVAL  
☐ GRADING PERMIT APPROVAL  
☐ PAVING PERMIT APPROVAL  
☐ WORK ORDER APPROVAL  
☐ OTHER (SPECIFY)

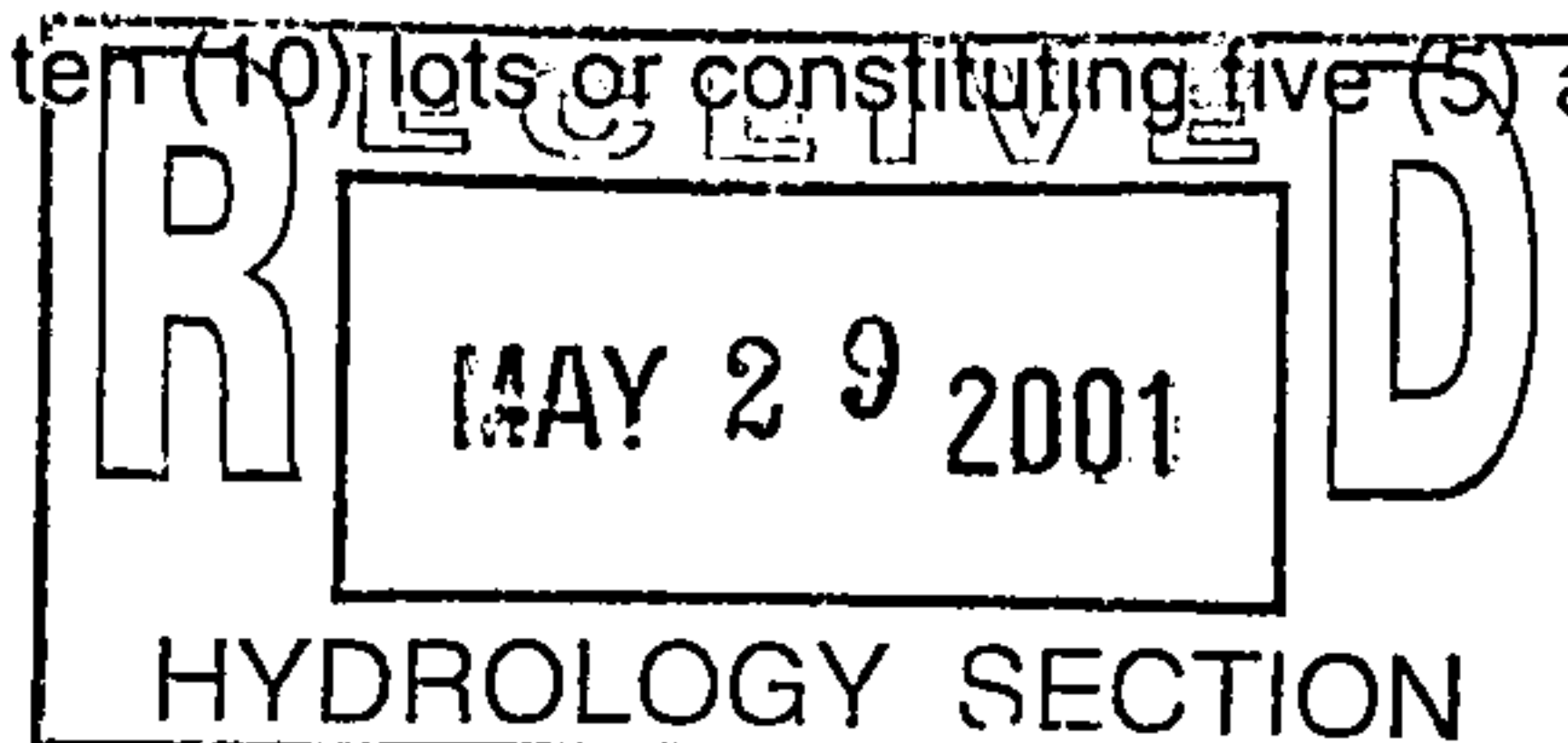
WAS A PRE-DESIGN CONFERENCE ATTENDED:  
☐ YES  
☐ NO  
☐ COPY PROVIDED

DATE SUBMITTED: \_\_\_\_\_ BY: \_\_\_\_\_

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plan:** Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
2. **Drainage Plans:** Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
3. **Drainage Report:** Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

5/24 Response to G & D issues *[Signature]*







May 25, 2001

1720 - B Randolph Rd. SE Albuquerque, NM 87106

Telephone: (505) 243-7300

FAX: (505) 243-7400

E-Mail: rti@nm.net

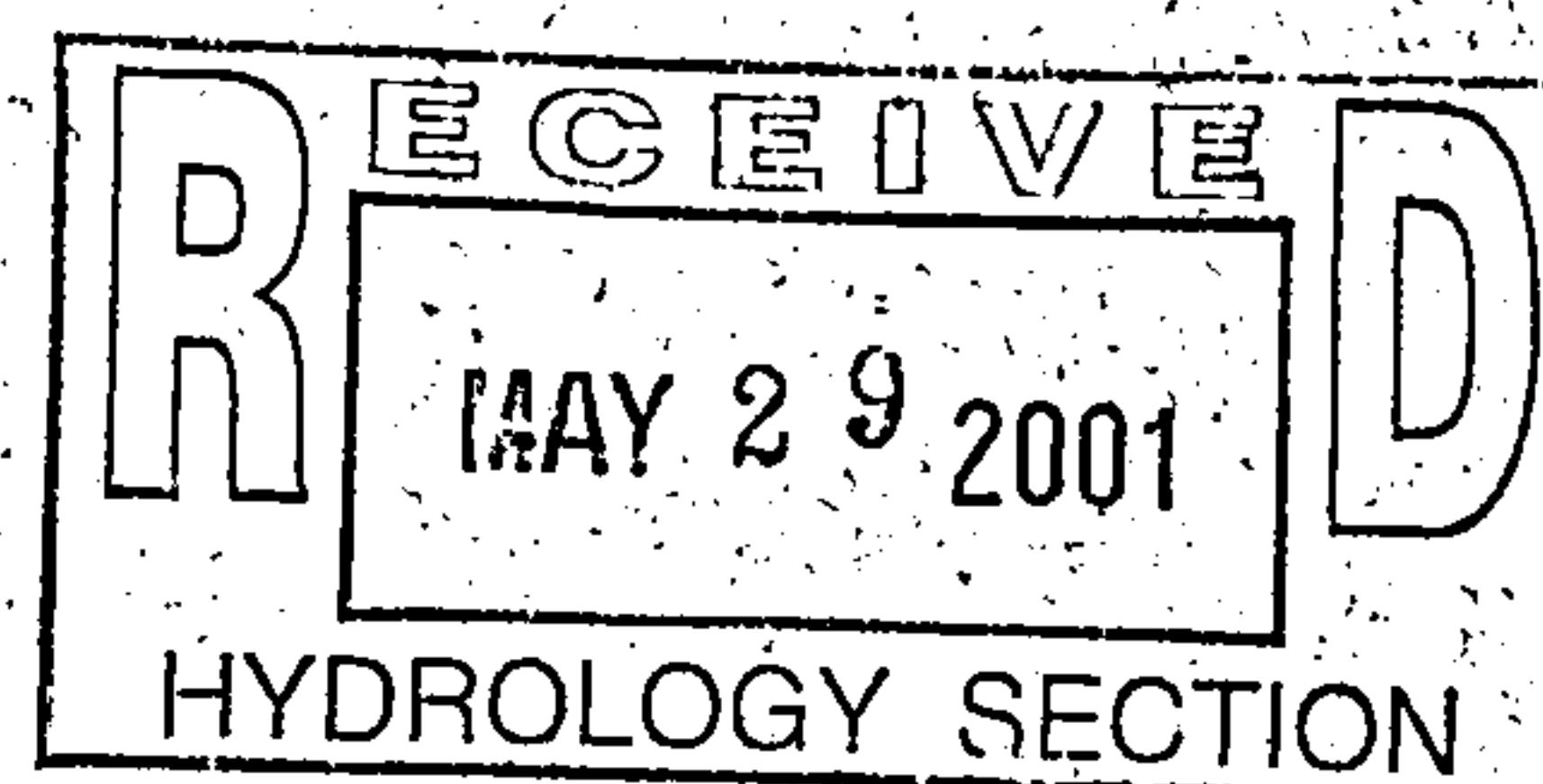
Bradley L. Bingham, P.E.  
Senior Engineer, Hydrology  
City of Albuquerque  
P.O. Box 1293  
Albuquerque, New Mexico 87103

**RE: Eldorado Park Subdivision, Unit 3 & 4 Grading and Drainage Plan, your letter dated: May 14, 2001**

Dear Mr. Bingham:

Thank you, for your review. In response to your comments, we are sending corrected plans herewith. Addressing each issue of your letter:

- The RCP in 94<sup>th</sup> has been corrected to be 48" in diameter.
- The net result of adjusting the rainfall to Zone 1 (reducing rainfall) and increasing the density to more closely represent the planned land use was a slight reduction in the 100-year flow values. The most significant of which was runoff from Dean Drive. The flows from Dean, south of Rowen, decreased from 26 cfs in the 100-year storm to 23 cfs.
- Weems has been lowered as much as practical and yet, there are a few places where the back yards have to drain to Tower. This had been agreed to in a meeting with Fred Aguirre and Elvidio Diniz, while Mr. Aguirre was still the head of hydrology at the City. Mr. Diniz was under the impression that you had been aware of this. Mr. Aguirre and Mr. Diniz had agreed that this is the most practical alternative. As you'll see on the plans, back-yard drainage to Tower Road has been kept to a minimum.
- We have provided spot elevations and the cross section detail of the pads, side to side, as you had requested. All slopes between lots are 3H to 1V or flatter. It is typical of the side lot slopes to start at 3:1 at the back of the lot and end up close to the slope of the street at the front. We say these slopes vary, maximum of 3:1.
- I have re-stamped these plans and put the date on them. Thanks!
- The grading for the last 2 lots in Unit 2 are now shown. As we discussed, no new retaining walls are needed there.





May 25, 2001

Bradley L. Bingham, P.E.

Page 2

- I am sorry the reference plans from Units 1 & 2 were difficult to read. Blue lines were all that I had inherited. We are supplying black-line prints of those approved drawings (from the Units 1 & 2 drainage report) for sheets 1 and 2 of 3. You mentioned that sheet 3 was ok (because the lettering is so large).

You will notice that the grades have changed in the central part of the plans. And we have incorporated ideas from the DRC meeting, by tightening the knuckles. A hammerhead configuration at Dean and Weems would not work.

Thank you for your assistance.

Sincerely,

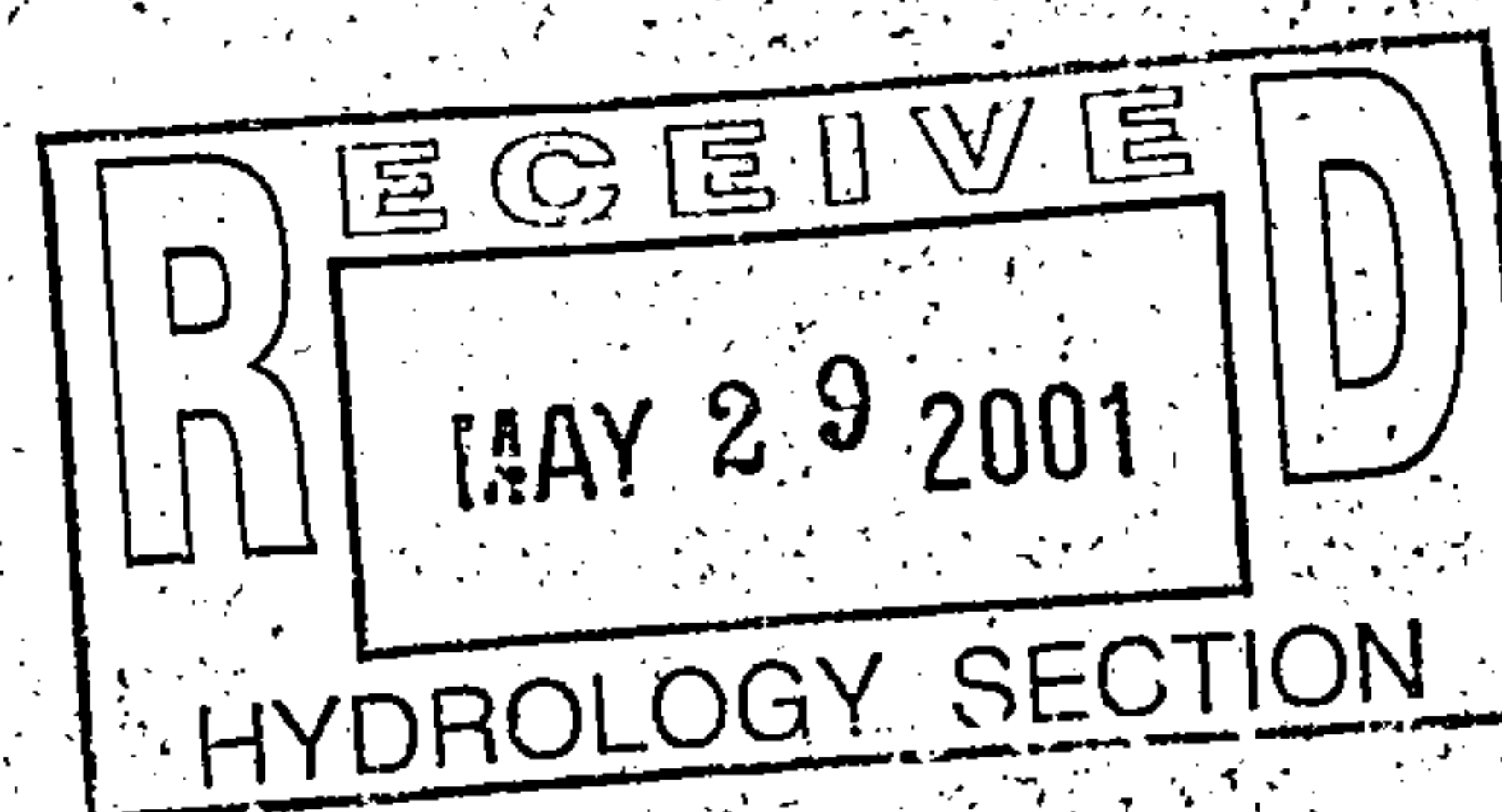
**RESOURCE TECHNOLOGY, INC.**

*June Page*

June Page, P.E., CFM

JP:tew

Encl: Sheets 1 & 2 of 3 of Units 1 & 2 Drainage Report  
Sheets 1 thru 5 of 5 of Units 3 & 4 Drainage Report, corrected, stamped and dated





**Tower West Ltd. Partnership / Ten West L.L.C.**  
**333 Lomas Blvd., N.E.**  
**Albuquerque, New Mexico 87102**  
**(505) 998-0303 / Fax 998-0306**

April 3, 2000

Mr. Fred Aguirre, PE  
City Engineer / City of Albuquerque  
Plaza Del Sol Blvd. 2<sup>nd</sup> Floor  
Albuquerque, NM 87102  
HAND DELIVER

L9/D7

Re : Eldorado Park / Temporary Drainage

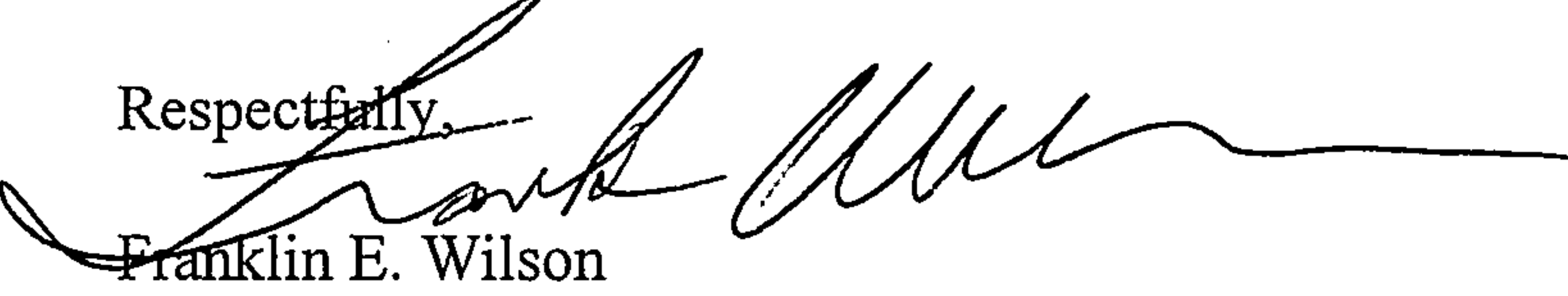
Dear Fred:

Thank you and Susan Calongne for meeting with me last week. As I understand, your concern is that the ultimate solution for conveyance of the storm water be guaranteed by either Tower West or the City. In the case of the Eldorado Park Arroyo, the ultimate solution is placing the runoff into the Special Assessment District 222 Storm Drainage System. Given the high cost of a financial guarantee for the storm sewer in Tower Road, the most practical approach for Eldorado Park Subdivision is the following:

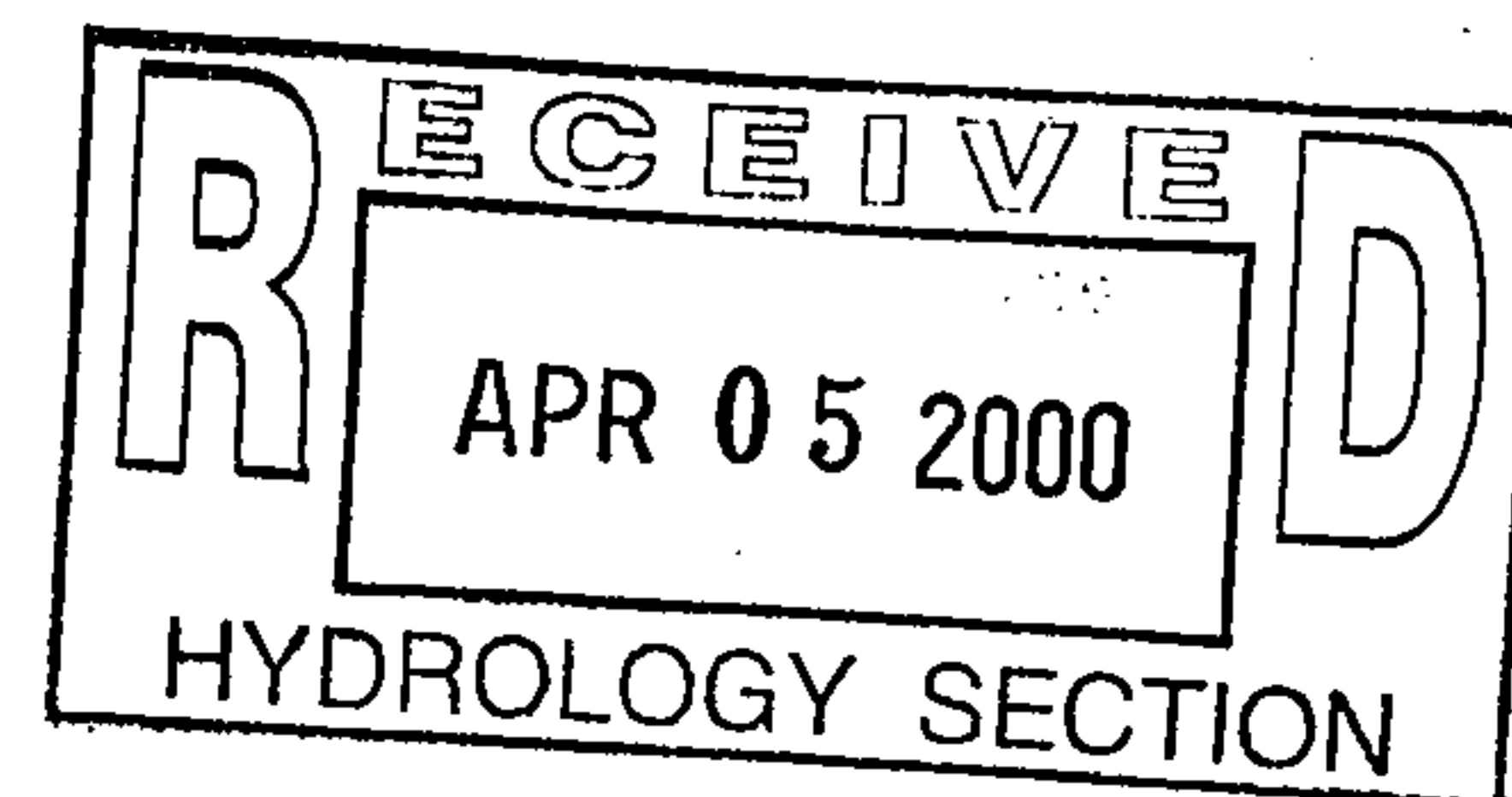
1. Resource Technology Inc. does the design of Unit 3, Eldorado Park Subdivision and shows the Eldorado Park Arroyo shifted into a temporary drainage channel within the Tower right-of-way. After the Special Assessment District Engineer has submitted the entire project to DRC then Resource Technology Inc. will submit the plans for review and request approval subject to the developer guaranteeing the temporary channel and the Special Assessment District 222 guaranteeing final storm drainage system.
2. The City Council passes Resolution 5 for Special Assessment District 222.
3. Tower West builds Unit 3 and a temporary drainage channel in the Tower right-of-way. A guarantee would not be needed for construction because Resolution 5 is passed. Eldorado Park Subdivision would maintain the temporary channel.
4. Construction begins on Special Assessment District 222 which is scheduled to be complete within 18 months from start. The temporary channel is eliminated and all drainage is placed in Special Assessment District 222 storm sewer.

This is set out to present an overview which should be adequate for now. I will present more detail as Special Assessment District 222 progresses through the approval process.

Respectfully,

  
Franklin E. Wilson  
General Partner

crh/enc.  
c:\tower\aguirre





MASTER DRAINAGE PLAN  
ELDORADO PARK SUBDIVISION

AND

REVISED DRAINAGE REPORT  
FOR UNITS 3 & 4

Prepared for

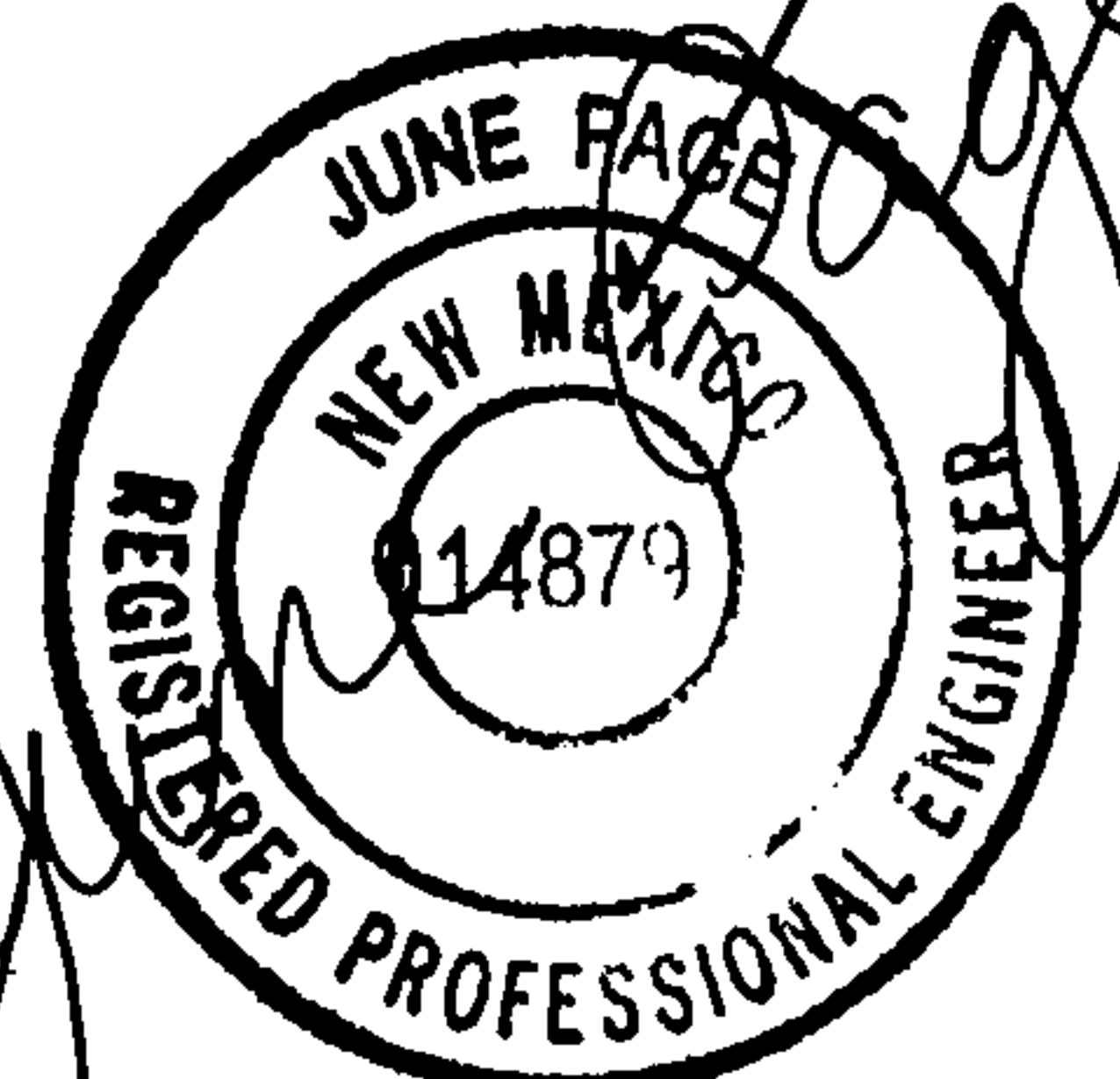
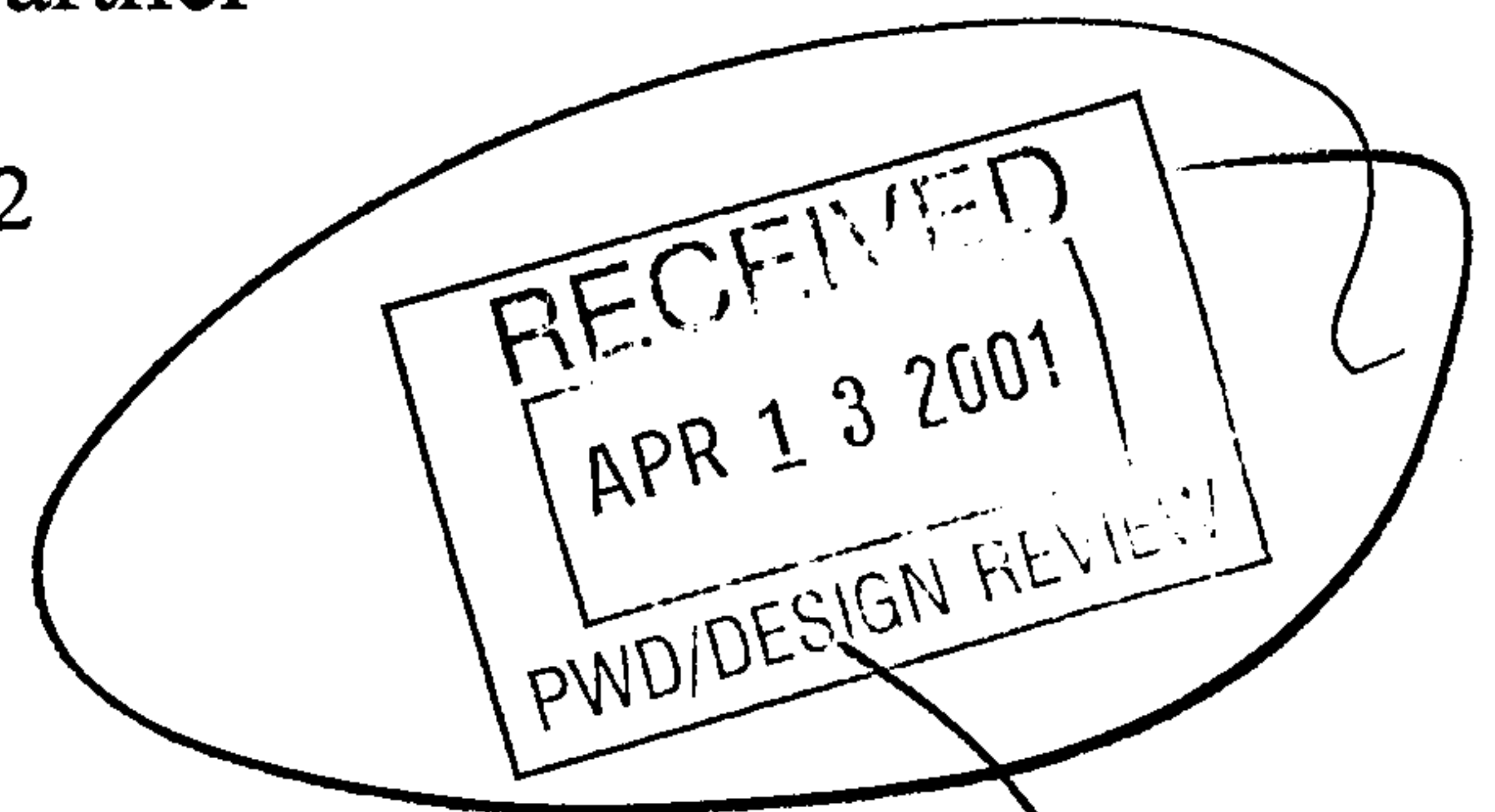
TOWER WEST JOINT VENTURE  
Frank Wilson, Managing Partner  
333 Lomas NE  
Albuquerque, NM 87102

Prepared by



1720-B Randolph Rd. SE Albuquerque, NM 87106  
Phone (505)243-7300 Fax (505)243-7400  
e-mail: rti@nm.net

April 2001







1720 - B Randolph Rd. SE Albuquerque, NM 87106  
Telephone: (505) 243-7300  
FAX: (505) 243-7400

## TRANSMITTAL

TO: BRAD BINGHAM  
CITY OF ALBUQUERQUE  
Playa del Sol  
ATTN: \_\_\_\_\_  
RE: EL DORADO

FROM: June Page, PE, CFM  
DATE: 29 May 01  
OUR PROJECT NO.: 98 260 3E4

### WE TRANSMIT THE FOLLOWING:

- |                                            |                                         |                                                         |
|--------------------------------------------|-----------------------------------------|---------------------------------------------------------|
| <input type="checkbox"/> SHOP DRAWINGS     | <input type="checkbox"/> SPECIFICATIONS | <input type="checkbox"/> CHANGE ORDER                   |
| <input checked="" type="checkbox"/> PRINTS | <input type="checkbox"/> COPY OF LETTER | <input type="checkbox"/> UNDER SEPARATE COVER VIA _____ |
|                                            | <input type="checkbox"/> DOCUMENTS      | <input type="checkbox"/> _____                          |

### FOR YOUR:

- |                                           |                                                  |
|-------------------------------------------|--------------------------------------------------|
| <input type="checkbox"/> APPROVAL         | <input type="checkbox"/> DISTRIBUTION TO PARTIES |
| <input type="checkbox"/> REVIEW & COMMENT | <input type="checkbox"/> RECORD                  |
| <input type="checkbox"/> USE              | <input type="checkbox"/> INFORMATION             |

IF MATERIAL RECEIVED IS NOT AS LISTED, PLEASE  
NOTIFY US AT ONCE.

| COPIES | DATE      | ITEM DESCRIPTION             |
|--------|-----------|------------------------------|
| 1      | 29 MAY 01 | Sheet 3 of 5. Unit 4 GRADING |
| 1      | 14 May    | your letter -                |
|        |           |                              |
|        |           |                              |
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|        |           |                              |
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### REMARKS:

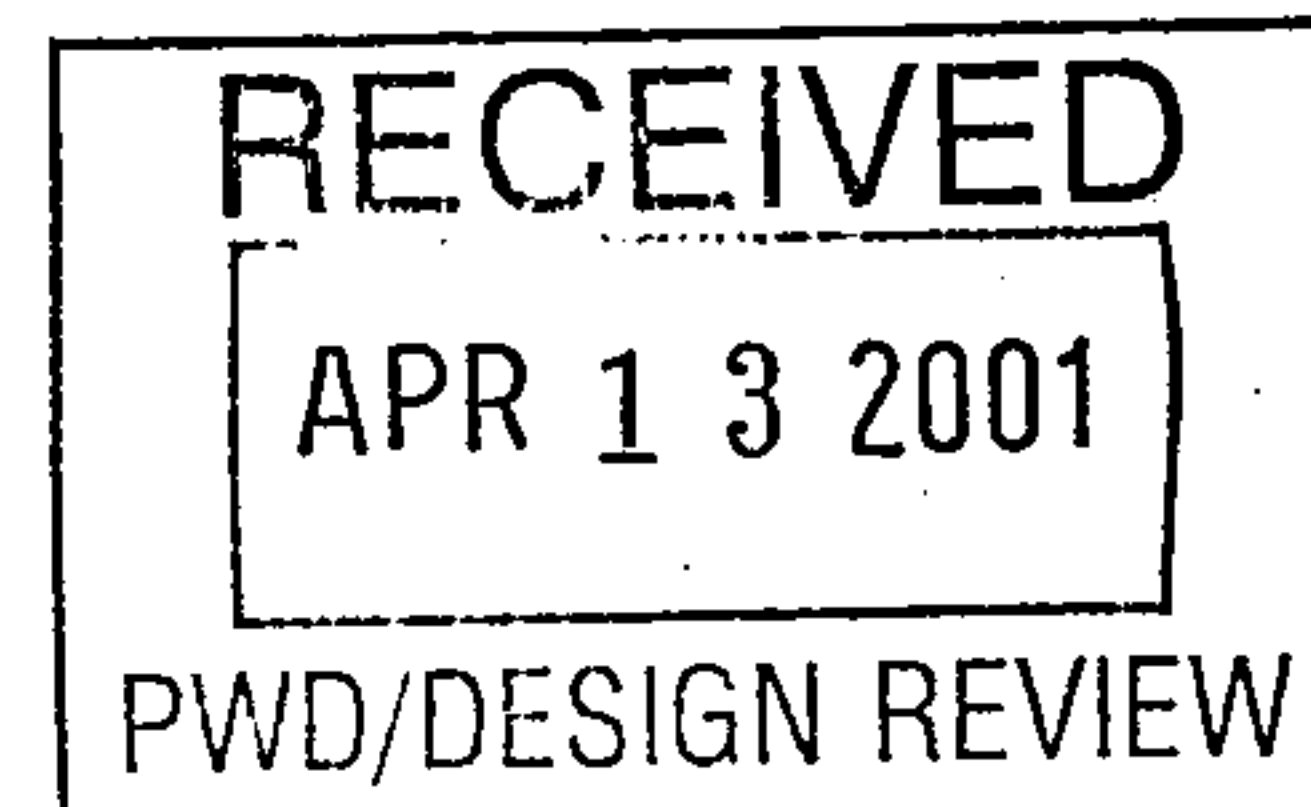
Brad, please substitute this better print - smaller  
Boundaries - more read-able. Thanks.

COPY TO: \_\_\_\_\_



# TABLE OF CONTENTS

|                                                                                          |             |
|------------------------------------------------------------------------------------------|-------------|
| <b>Location</b>                                                                          | <b>Page</b> |
|                                                                                          | 1           |
| <b>Hydrology</b>                                                                         | 1           |
| Unit 3                                                                                   | 1           |
| Unit 4                                                                                   | 2           |
| <b>Hydraulics</b>                                                                        | 3           |
| Arroyo                                                                                   | 3           |
| <b>Unit 3 and 4 Development</b>                                                          | 4           |
| <b>TABLES</b>                                                                            |             |
| TABLE A:                                                                                 | 4           |
| FUTURE CONDITION HYDROLOGY, Units 3 & 4<br>Basin A, B, & C at time of DEVELOPMENT Zone 1 |             |
| TABLE B:                                                                                 | 5           |
| STREET AND INLET CAPACITIES                                                              |             |
| TABLE C:                                                                                 | 6           |
| PRESENTED AS TABLE 4 IN THE UNIT 1 REPORT<br>CONNECTOR PIPE CAPACITIES                   |             |
| TABLE D1:                                                                                | 7           |
| TOWER ROAD DIVERSION SWALE AT 1%                                                         |             |
| TABLE D2:                                                                                | 8           |
| TOWER ROAD DIVERSION SWALE AT 2%                                                         |             |
| <b>PLATES</b>                                                                            | 9           |
| Plate 1. Street Capacity Graph                                                           | 10          |
| Plates 2 & 3. Grating Capacities Graph                                                   | 11          |
| Plate 4 Pipe Size Graph                                                                  | 12          |
| <b>APPENDICES (In folders)</b>                                                           | 14          |
| ELDORADO PARK SUBDIVISION UNIT 1,<br>GRADING AND DRAINAGE PLAN Sheets 1, 2, and 3        |             |





## Location

The proposed development sites are Unit 3 and Unit 4 of the Eldorado Park Subdivision. The subdivision is located between the proposed intersection of Tower Road S. W. and Eucariz Avenue on the east side of 98<sup>th</sup> Street S. W. on the south half of Tract A-1A (21.9 acres) of the Tower West Joint Venture.

Units 1 and 2 of this subdivision have been developed. Unit 3 is currently undeveloped and partially disturbed. Unit 4 is undeveloped with a cover of native shrubs and grass.

## Hydrology

### Unit 3

Storm flow values for Unit 3 were developed in the Drainage Report for Eldorado Park Subdivision, Unit 1, prepared for Tower West Joint Venture, by Resource Technology, Inc. dated July 1999. The proposed Unit 3 grading and drainage remain in essence the same as presented in the July 1999 report. "ELDORADO PARK SUBDIVISION UNIT 1, GRADING AND DRAINAGE PLAN" Sheets 1, 2, and 3 from that report are included in the appendix of this report for reference and are referred to as Sheets A.1, A.2 and A.3 in this report. Two changes to the 1999 report are necessary; the lot density in Unit 3 has increased to 6.2 units/acre, and Zone 3 rainfall was used although the area is located in Zone 1. The runoff from Unit 3 was revised in this report to reflect Zone 1 rainfall and the higher lot density.

As discussed in the Unit 1 report, this site will receive no off-site drainage. Approximately 1000 feet west of 98<sup>th</sup> Street, Snow Vista Channel intercepts all runoff into the unnamed arroyo that cuts through the south side of this project. The Snow Vista Channel diverts drainage to the southeast and out of the watershed. 98<sup>th</sup> Street intercepts and delivers a small remaining amount of runoff to the existing arroyo. It is proposed to install a temporary diversion swale around Unit 3 as part of the grading and drainage plans.

SAD 222 includes the construction of a 54 in. storm drain along 98<sup>th</sup> Street, (southerly) to Tower Road. The 54 in. storm drain continues easterly along Tower Road. Under SAD 222 the need for drainage into this arroyo will be eliminated. The temporary swale will not be needed after the construction of the 54 in. storm drain.

The Amole-Hubbell Drainage Master Plan Prepared for AMAFCA and dated July 22, 1999 identifies Subbasin A3-D as the tributary area to this unnamed arroyo. That report's AHYMO analysis for 100-year, 24 hour storm with sediment indicates a peak flow of 94.8 cubic feet per second (cfs) for the arroyo. The subbasins north of A3-D will be intercepted by the proposed 54 in. storm drain to be placed in 98<sup>th</sup> street. However in the watershed's current condition that drainage spills to the east on the north side of Eldorado Park Subdivision and only the drainage from Subbasin A3-D will reach the existing unnamed arroyo or the temporary interceptor swale.



48" in 94<sup>th</sup>

The 24 in. storm drain in Dean Drive was constructed with Unit 1 up to the division between Units 1 and 3. Unit 1 construction included the inlets and connections to this storm drain. Storm water goes from the Dean Drive 24 in. storm drain to a 42 in. storm drain in Rowen Road and 94th Street. These storm drains were constructed with Unit 1 of this project. Table A presents the runoff calculations for storm water flowing north on Dean Drive from Unit 3. This is identified as Basin A.

This storm drain system also receives runoff from the western part of Rowen Road. This is identified as Basin B on Table A and on the Revised Grading and Drainage Plan.

Until the Tower Road storm drain is constructed, the 94<sup>th</sup> Street storm drain discharges into two temporary retention ponds as shown on the Master Grading and Drainage plan (Sheet 1) included in a pocket in the back of this report. These ponds are existing and had been built with Units 1 and 2 of this subdivision. The 1999 drainage report for Unit 1 presents the design calculations that demonstrate the ponds' capacity to accept the 100-year, 10-day runoff from the fully developed units 1, 2, 3, and 4.

#### Unit 4

At the northern side of the subdivision is Eucariz Avenue, S. E. The south half of Eucariz Avenue will be built with Unit 4. Basin C is comprised of Unit 4 and the south half of Eucariz Avenue. Until the north half of Eucariz Avenue is built, a small (0.6 acre) triangle of existing ground, called C-Temp, will drain onto the south half just east of 98<sup>th</sup> Street. See Table A.

The combination of C-Temp and C were used to check the street capacity and size of the inlets which will drain into the 36 in. storm drain which was placed with Unit 2 in 94<sup>th</sup> Street. See Table B.

48"



## Hydraulics

Inlets and storm drains sizes were calculated and presented in the above referenced July 1999 report. The Unit 3 Dean Drive and Rowen Road West street and inlet capacities are included in this report on Table B. The inlet connector pipe capacities are included in Table C. These calculations plan for all of the flow from Unit 3 (except offsite flows into the arroyo) to be picked up by inlets at the intersection of Rowen Road and Dean Drive. Details of this intersection are presented on Sheet 5 of 5 in the pockets in the back of this report. Dean Drive at Rowen Road is the point of concentration is at the northern boundary of Unit 3.

## Arroyo

The existing FEMA Map No. 35001C0328D, dated September 20, 1996, shows a narrow floodplain along the banks of an unnamed arroyo that extends into Unit 3. See Sheet 1 of 5, the Master Grading and Drainage plan in the pocket in the back of this report. As mentioned above, the 54 in. storm drain along 98<sup>th</sup> and Tower Road will convey the storm water now reaching that arroyo. The CLOMR and LOMR to remove the FEMA flood plain along this arroyo will be submitted by the SAD 222 engineer. Until the Tower Road storm drain is completed, all flows reaching the arroyo from areas west of 98<sup>th</sup> street will be diverted into a shallow swale along the proposed Tower Road alignment as shown on Sheets 1 and 2 of 5. The proposed Tower Road alignment is new and currently undeveloped. Existing Tower Road is located further south from this alignment.

Until the time that the 54 in. storm drain is constructed, an earth swale will divert the arroyo flow around this project. The swale will have the dimensions of 12 feet bottom width, a minimum depth of one and a quarter feet at a minimum slope of one percent or a minimum depth of one foot at a minimum slope of two percent. This is based upon the 95 cubic feet per second as described in the hydrology section of this report. Design calculations are presented on Table D-1 and D-2 of this report.



## Unit 3 and 4 Development

The storm drain system in Units 1 and 2 was designed to accept storm water from Unit 3 and 4. The design calculations for the now-existing storm drain system were presented in the July 1999 Drainage Report, Eldorado Park Subdivision Unit 1, prepared for Tower West Joint Venture by Resource Technology, Inc. A copy of that table is presented in this report as Table C.

Standard curb and gutter will be used for Dean Drive in Unit 3. The computed flows and street capacities at a curb depth of 0.6 ft. were presented in the July 1999 report. Table A and B of this report are copied out of that report. Plate 1 is a reproduction of the street capacity curves from Albuquerque DMP. Standard roll curb and gutter will be used for the rest of the project.

Sheet 2 of 5 shows the grading plan for Unit 3. The inlets at the northern boundary of Unit 3 on both sides of Dean Drive and Rowen Road were designed to intercept all of the 100-year flow from this unit. This is the point of concentration for the unit, with the exception of some back yard drainage to Tower Road from several lots along the southern boundary of the project. No other surface runoff is expected from the Unit 3.

Retention Ponds A and B, built with Units 1 and 2 were designed to hold the 100-year, 10-day storm amount of 4.75 acre feet from all 4 units in developed condition. The total volume of storage for Ponds A and B is 4.92 acre-feet.

TABLE A

FUTURE CONDITION HYDROLOGY, Units 3 & 4  
Basin A, B, & C at time of DEVELOPMENT zone 1

| Basin    | Area (ac) | Land Treatment | Land Treatment | Land Treatment | Land Treatment | Qpeak (cfs) | Qpeak (cfs) | Volume (ac-ft) | Volume (ac-ft) | Volume (ac-ft) |
|----------|-----------|----------------|----------------|----------------|----------------|-------------|-------------|----------------|----------------|----------------|
|          |           | %A             | %B             | %C             | %D             | 10-yr       | 100-yr      | 10-yr          | 100-yr         | 100-yr         |
|          |           |                |                |                |                | 6-hr        | 6-hr        | 6-hr           | 6-hr           | 10-day         |
| A        | 6.70      | 0              | 29             | 13             | 58             | 14.0        | 23.4        | 0.469          | 0.818          | 1.294          |
| B        | 1.40      | 0              | 29             | 13             | 58             | 2.9         | 4.9         | 0.098          | 0.171          | 0.270          |
| C-future | 2.90      | 0              | 32             | 15             | 53             | 5.4         | 9.9         | 0.179          | 0.317          | 0.492          |
| C-temp   | 0.62      | 100            | 0              | 0              | 0              | 0.1         | 0.8         | 0.004          | 0.023          | 0.023          |
| C-Unit 4 | 3.52      |                |                |                |                |             | 10.7        |                |                |                |

\* City of Albuquerque Development Process Manual,  
Volume 1, 1997 Revision, Pages 22-7 to 22-16

*How does this compare to Masterplan*

*where does this come from*



## DRAINAGE INFORMATION SHEET

L-9/D7

PROJECT TITLE: Eldorado Park Units 3 & 4 ZONE ATLAS/DRNG. FILE #: \_\_\_\_\_

DRB #: \_\_\_\_\_ EPC#: \_\_\_\_\_ WORK ORDER #: \_\_\_\_\_

LEGAL DESCRIPTION: Tract A-1A, Tower West Joint VentureCITY ADDRESS: 94th St. and Tower Rd. NWENGINEERING FIRM: Resource Technology, Inc. CONTACT: June Page, P.E.ADDRESS: 1720-B Randolph Rd. SE Albuquerque NM 87106 PHONE: (505) 243-7300OWNER: Tower West Joint Venture CONTACT: Frank WilsonADDRESS: 333 Lomas NE Albuquerque NM 87102 PHONE: (505) 998-0305

ARCHITECT: \_\_\_\_\_ CONTACT: \_\_\_\_\_

ADDRESS: \_\_\_\_\_ PHONE: \_\_\_\_\_

SURVEYOR: Survey Southwest CONTACT: Dan GraneyADDRESS: 333 Lomas NE Albuquerque NM 87102 PHONE: (505) 998-0305

CONTRACTOR: \_\_\_\_\_ CONTACT: \_\_\_\_\_

ADDRESS: \_\_\_\_\_ PHONE: \_\_\_\_\_

## TYPE OF SUBMITTAL:

- ☒ DRAINAGE REPORT  
☒ DRAINAGE PLAN  
☐ CONCEPTUAL GRADING & DRAINAGE PLAN  
☒ GRADING PLAN  
☐ EROSION CONTROL PLAN  
☐ ENGINEER'S CERTIFICATION  
☐ OTHER \_\_\_\_\_

## PRE-DESIGN MEETING:

- ☐ YES  
☐ NO  
☐ COPY PROVIDED



## CHECK TYPE OF APPROVAL SOUGHT:

- ☐ SKETCH PLAT APPROVAL  
☒ PRELIMINARY PLAT APPROVAL  
☐ S. DEV. PLAN FOR SUB'D. APPROVAL  
☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL  
☐ SECTOR PLAN APPROVAL  
☐ FINAL PLAN APPROVAL  
☐ FOUNDATION PERMIT APPROVAL  
☐ BUILDING PERMIT APPROVAL  
☐ CERTIFICATE OF OCCUPANCY APPROVAL  
☒ GRADING PERMIT APPROVAL  
☐ PAVING PERMIT APPROVAL  
☐ S.A.D. DRAINAGE REPORT  
☐ DRAINAGE REQUIREMENTS  
☐ SUBDIVISION CERTIFICATION  
☐ OTHER \_\_\_\_\_ (SPECIFY)

DATE SUBMITTED: April 12, 2001BY: June Page

Received  
4-31-01  
AP





1720 - B Randolph Rd. SE Albuquerque, NM 87106  
Telephone: (505) 243-7300  
FAX: (505) 243-7400

## TRANSMITTAL

TO: Brad Bingham

FROM: June Page

DATE: April 13, 2001

ATTN: \_\_\_\_\_

RE: ELDORADO UNITS 3 & 4

OUR PROJECT NO.: 98-260.30

& 98-260.40

### WE TRANSMIT THE FOLLOWING:

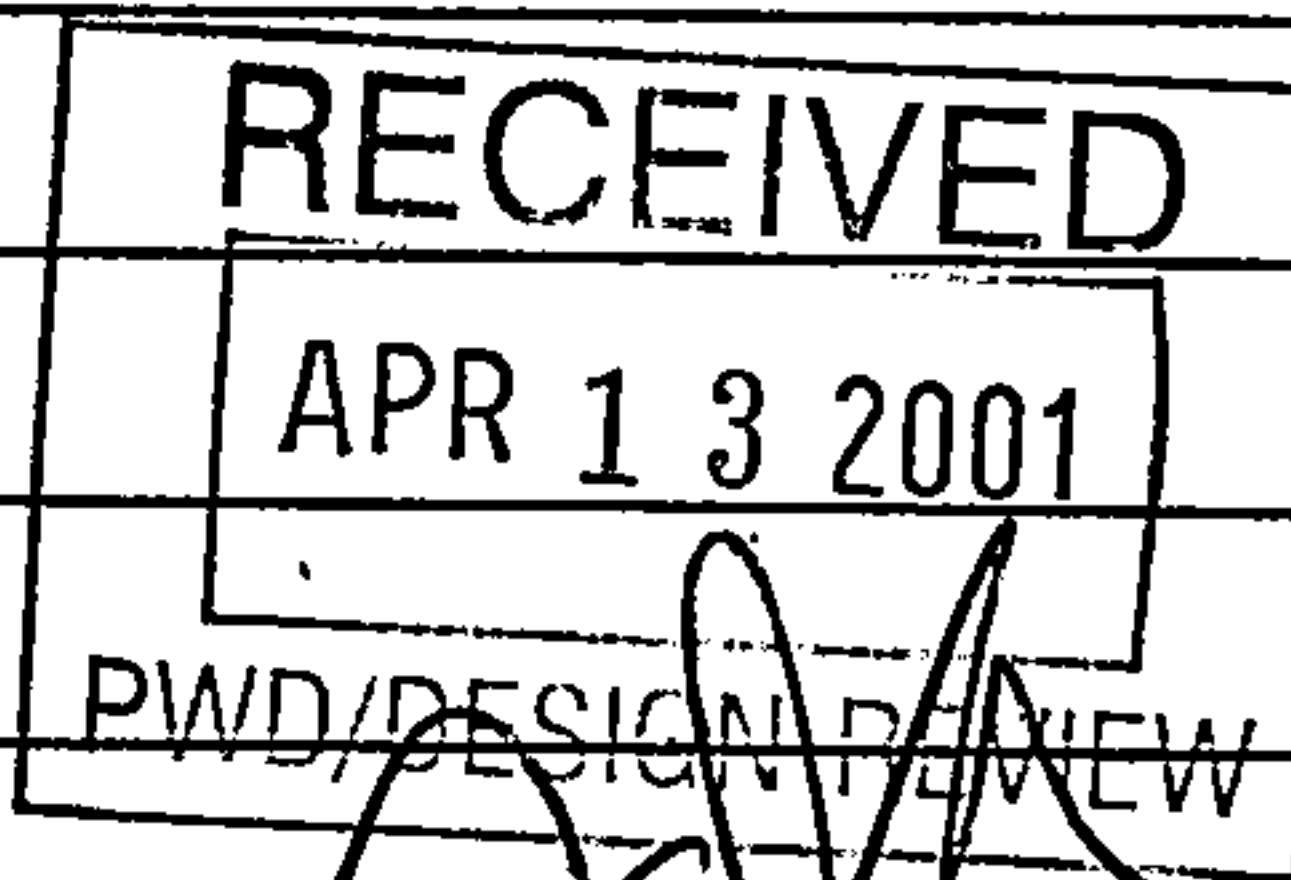
- |                                            |                                               |                                                         |
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| <input type="checkbox"/> SHOP DRAWINGS     | <input type="checkbox"/> SPECIFICATIONS       | <input type="checkbox"/> CHANGE ORDER                   |
| <input checked="" type="checkbox"/> PRINTS | <input type="checkbox"/> COPY OF LETTER       | <input type="checkbox"/> UNDER SEPARATE COVER VIA _____ |
|                                            | <input checked="" type="checkbox"/> DOCUMENTS | <input type="checkbox"/> _____                          |

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- |                                           |                                                  |
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| <input type="checkbox"/> APPROVAL         | <input type="checkbox"/> DISTRIBUTION TO PARTIES |
| <input type="checkbox"/> REVIEW & COMMENT | <input type="checkbox"/> RECORD                  |
| <input type="checkbox"/> USE              | <input type="checkbox"/> INFORMATION             |

IF MATERIAL RECEIVED IS NOT AS LISTED, PLEASE NOTIFY US AT ONCE.

| COPIES | DATE | ITEM DESCRIPTION                                     |
|--------|------|------------------------------------------------------|
| 1      | 4-12 | Revised Drainage Report<br>& Grading & Drainage Plan |
|        |      |                                                      |
|        |      |                                                      |
|        |      |                                                      |
|        |      |                                                      |
|        |      |                                                      |
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REMARKS: \_\_\_\_\_

COPY TO: \_\_\_\_\_



show:  $\frac{v^2}{2g} + \text{depth}$

TABLE B

STREET AND INLET CAPACITIES

| Street                                                                                                        | South Side 100-yr. Flow | Right Curb Flow | Left Curb Flow | Street Slope | Half Street Capacity (1) | 100-Yr Flow Depth (2) | Inlet Capacity (3) | No. of Inlets Required |
|---------------------------------------------------------------------------------------------------------------|-------------------------|-----------------|----------------|--------------|--------------------------|-----------------------|--------------------|------------------------|
|                                                                                                               | cfs                     | cfs             | cfs            | %            | cfs                      | ft                    | cfs                |                        |
| Eucariz Road, South half, Unit 4                                                                              |                         |                 |                |              |                          |                       |                    |                        |
| Total                                                                                                         |                         |                 |                | 2.9          |                          |                       |                    |                        |
|                                                                                                               | 10.7                    | 10.7            |                |              | 41                       | 0.46                  | 7.5                | 1 - Type A             |
|                                                                                                               |                         |                 |                |              |                          | 0.39                  | 4.2                | 1 - Single Type C      |
|                                                                                                               |                         |                 |                |              |                          |                       |                    |                        |
| Dean Drive – Unit 3 Inlet design from Unit 1. Flows were revised for more lots and corrected for Zone 1 rain. |                         |                 |                |              |                          |                       |                    |                        |
| Total                                                                                                         | 23.4                    |                 |                | 0.66         |                          |                       |                    |                        |
| Right Gutter                                                                                                  |                         | 11.7            |                |              | 20                       | 0.56                  | 5.5                | 1 - Type A             |
|                                                                                                               |                         |                 |                |              |                          |                       | 7.4                | 1 - Double Type C      |
| Left Gutter                                                                                                   |                         |                 | 11.7           |              | 20                       | 0.56                  | 5.5                | 1 - Type A             |
|                                                                                                               |                         |                 |                |              |                          |                       | 7.4                | 1 - Double Type C      |
| Rowen Road – West Unit 1 and 3 as presented in the Unit 1 Drainage Report, of which, Basin B is a part        |                         |                 |                |              |                          |                       |                    |                        |
| Total                                                                                                         | 14.1                    |                 |                | 2.45         |                          |                       |                    |                        |
| Right Gutter                                                                                                  |                         | 7               |                |              | 38                       | 0.36                  | 4.5                | 1 - Single Type A      |
|                                                                                                               |                         |                 |                |              |                          |                       | 4.5                | 1 - Single Type C      |
| Left Gutter                                                                                                   |                         |                 | 7              |              | 38                       | 0.36                  | 4.5                | 1 - Single Type A      |
|                                                                                                               |                         |                 |                |              |                          |                       | 4.5                | 1 - Single Type C      |

(1) Based on Plate 22.3 D-1 for 32 ft. street; d = 0.6 ft (0.67 ft. available). See Plate 1 of this report.

(2) From Plate 22.3 D-1 for 32' Street See Plate 1.

(3) Based on Plate 22.3 D-5 for single AA@ or AC@ inlet (See Plate 2.) or Plate 22.3 D-6 for double AC@ inlets (See Plate 3.).



**TABLE C**  
PRESENTED AS TABLE 4 IN THE UNIT 1 REPORT  
CONNECTOR PIPE CAPACITIES

| Street             | Line | Flow | Length | Dia. Required <sup>(1)</sup> |
|--------------------|------|------|--------|------------------------------|
|                    |      | cfs  | ft.    | in.                          |
| Dean Drive - North | A-1  | 14   | 28     | 18                           |
|                    | A-2  | 28   | 65     | 27                           |
| Dean Drive - South | B-1  | 27   | 50     | 24                           |
|                    | B-2  | 20   | 40     | 21                           |
|                    | B-3  | 7    | 40     | 18                           |
| Rowen Road         | C-1  | 7    | 25     | 18                           |
|                    | C-2  | 14   | 65     | 21                           |

(1) Based on Plate 22.3 D-8; H = 2.0 ft.

UNIT 4  
CONNECTOR PIPE CAPACITIES

| Street        | Line | Flow | Length | Dia. Required <sup>(2)</sup> |
|---------------|------|------|--------|------------------------------|
|               |      | cfs  | ft.    | in.                          |
| Eurcariz Road | D-1  | 8    | 40     | 18                           |
|               | D-2  | 4    | 24     | 18                           |

(2) Based on Plate 22.3 D-8; H = 2.0 ft.



TOWER ROAD DIVERSION SWALE AT 1%  
Worksheet for Trapezoidal Channel

| Project Description |                                        |
|---------------------|----------------------------------------|
| Project File        | c:\projects\eldoradounit3\eldorado.fm2 |
| Worksheet           | tower road arroyo                      |
| Flow Element        | Trapezoidal Channel                    |
| Method              | Manning's Formula                      |
| Solve For           | Channel Depth                          |

| Input Data           |                |
|----------------------|----------------|
| Mannings Coefficient | 0.030          |
| Channel Slope        | 0.010000 ft/ft |
| Left Side Slope      | 3.000000 H : V |
| Right Side Slope     | 3.000000 H : V |
| Bottom Width         | 12.00 ft       |
| Discharge            | 95.00 cfs      |

| Results              |                       |
|----------------------|-----------------------|
| Depth                | 1.24 ft               |
| Flow Area            | 19.43 ft <sup>2</sup> |
| Wetted Perimeter     | 19.82 ft              |
| Top Width            | 19.42 ft              |
| Critical Depth       | 1.13 ft               |
| Critical Slope       | 0.013804 ft/ft        |
| Velocity             | 4.89 ft/s             |
| Velocity Head        | 0.37 ft               |
| Specific Energy      | 1.61 ft               |
| Froude Number        | 0.86                  |
| Flow is subcritical. |                       |



# TOWER ROAD DIVERSION SWALE AT 2%

## Worksheet for Trapezoidal Channel

| Project Description |                                        |
|---------------------|----------------------------------------|
| Project File        | c:\projects\eldoradounit3\eldorado.fm2 |
| Worksheet           | tower road arroyo                      |
| Flow Element        | Trapezoidal Channel                    |
| Method              | Manning's Formula                      |
| Solve For           | Channel Depth                          |

| Input Data           |                |
|----------------------|----------------|
| Mannings Coefficient | 0.030          |
| Channel Slope        | 0.020000 ft/ft |
| Left Side Slope      | 5.000000 H : V |
| Right Side Slope     | 5.000000 H : V |
| Bottom Width         | 12.00 ft       |
| Discharge            | 95.00 cfs      |

| Results                |                       |
|------------------------|-----------------------|
| Depth                  | 0.97 ft               |
| Flow Area              | 16.44 ft <sup>2</sup> |
| Wetted Perimeter       | 21.94 ft              |
| Top Width              | 21.74 ft              |
| Critical Depth         | 1.07 ft               |
| Critical Slope         | 0.014204 ft/ft        |
| Velocity               | 5.78 ft/s             |
| Velocity Head          | 0.52 ft               |
| Specific Energy        | 1.49 ft               |
| Froude Number          | 1.17                  |
| Flow is supercritical. |                       |



## PLATES

Plate 1. Street Capacity Graph from the DPM, City of Albuquerque Plate 22.3 D-1  
Plates 2 & 3. Grating Capacities for Type A, C, and D From DPM Plates 22.3 D-5 & 6



STREET CAPACITY

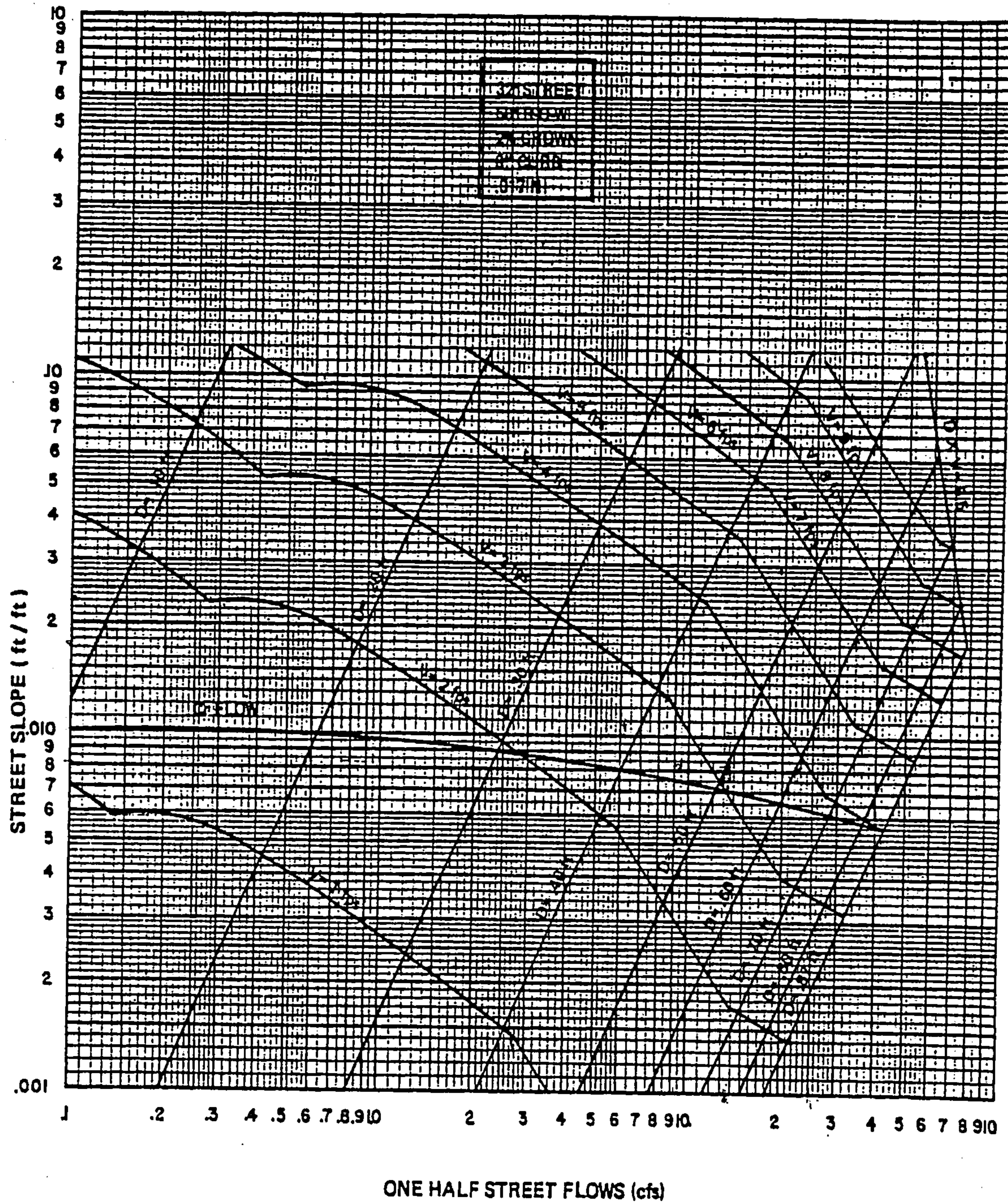
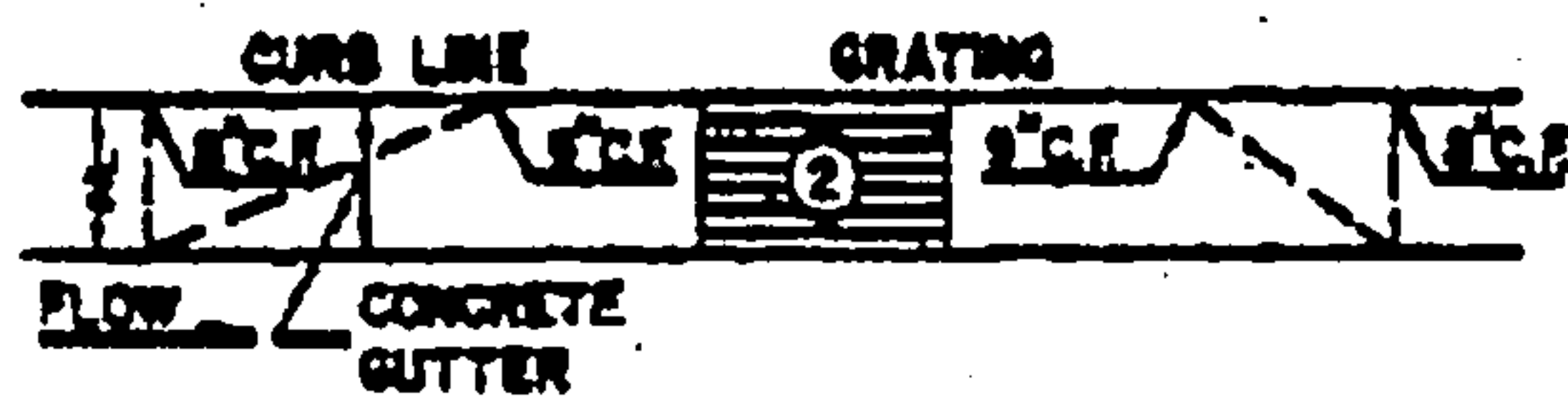


PLATE 22.3 D-1

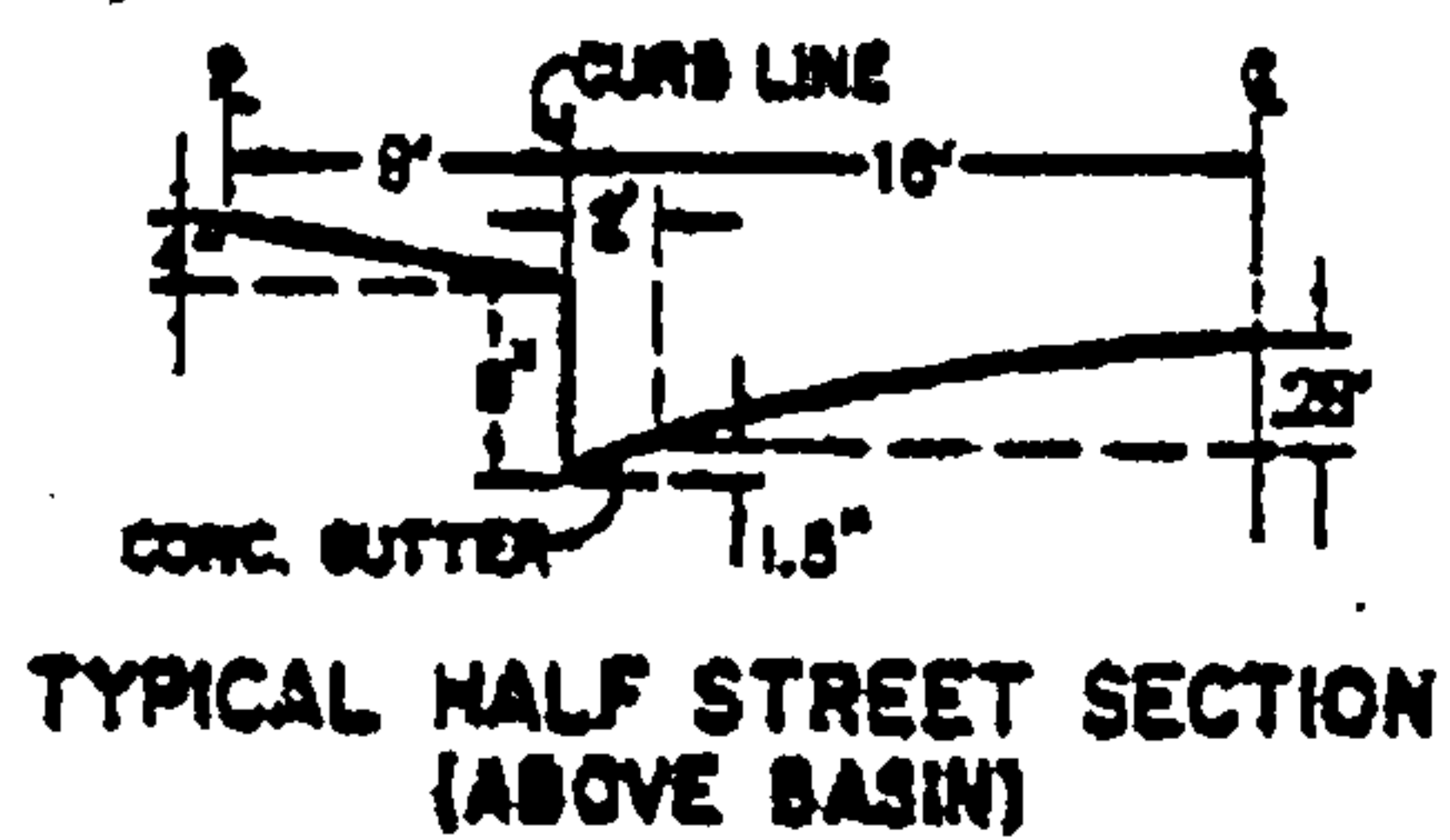
PLATE 1



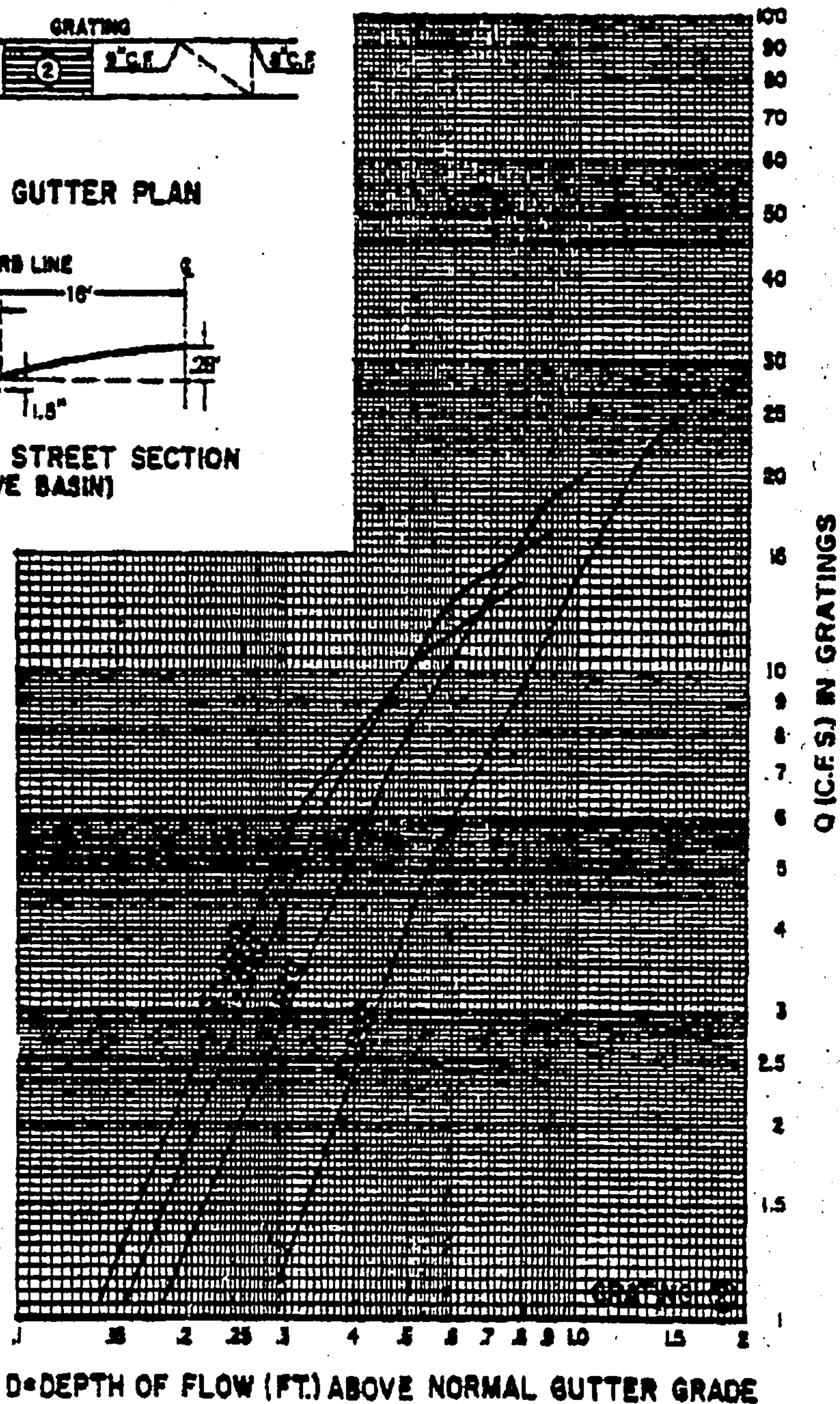
# GRATING CAPACITIES FOR TYPE 'A' , 'C' and 'D'



GRATING & GUTTER PLAN

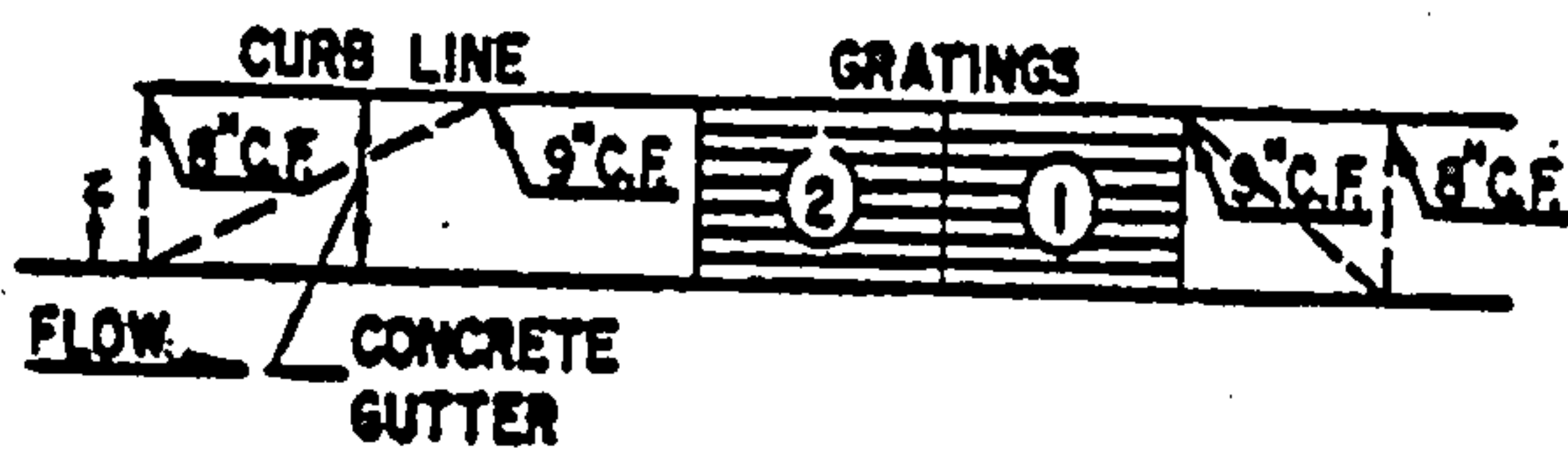


TYPICAL HALF STREET SECTION  
(ABOVE BASIN)

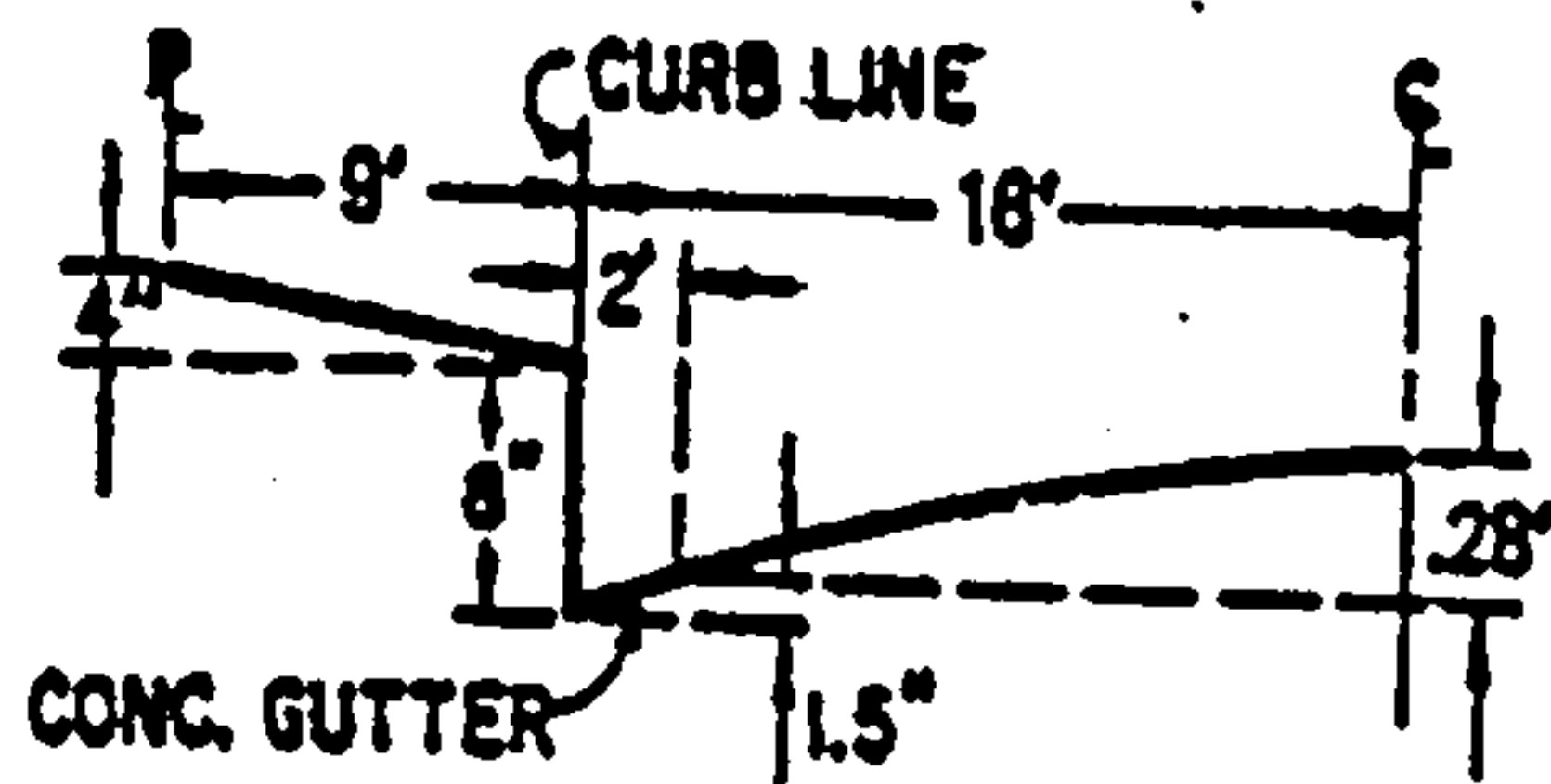




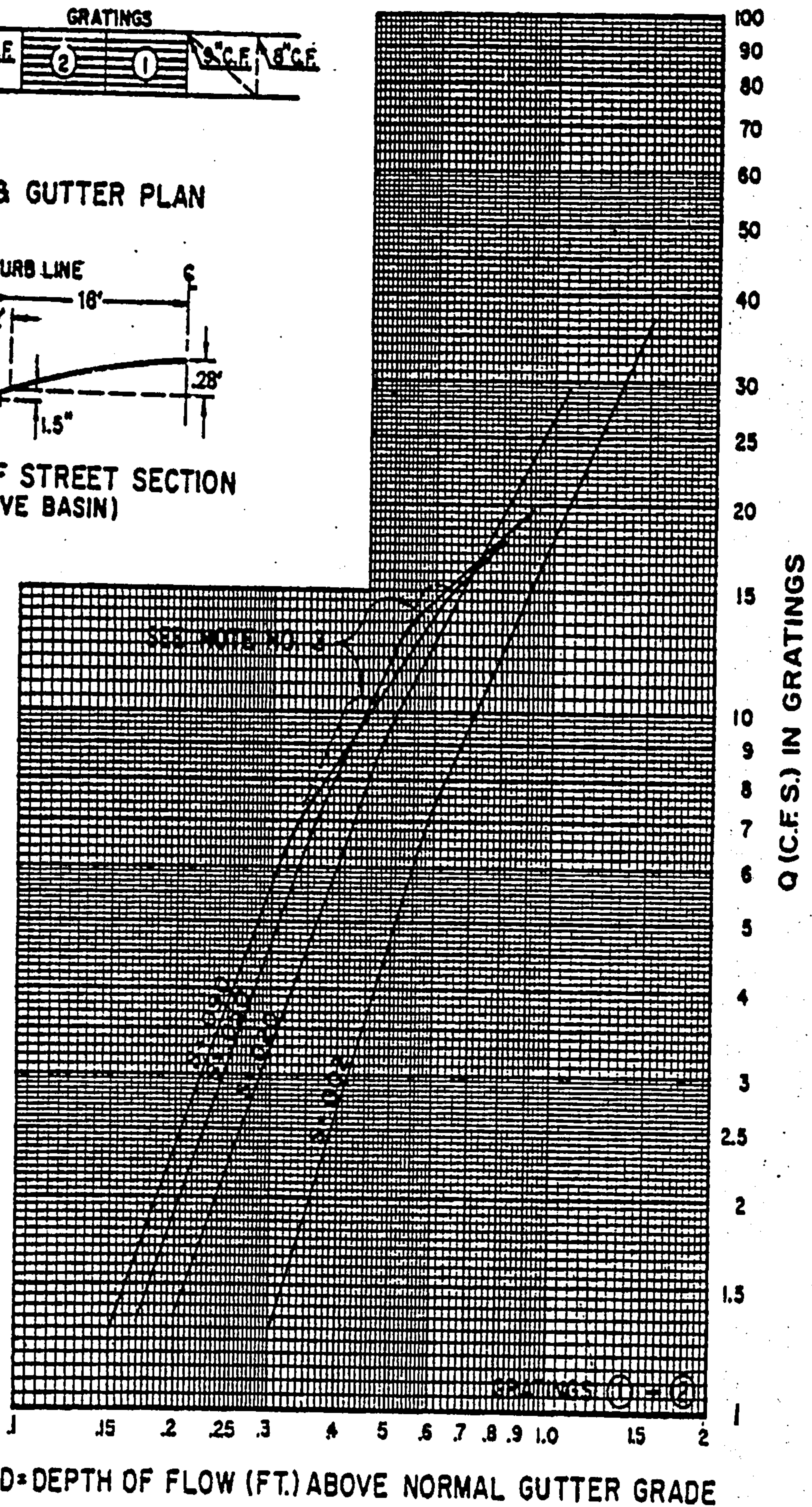
# GRATING CAPACITIES FOR TYPE DOUBLE "C," AND "D"



GRATING & GUTTER PLAN



TYPICAL HALF STREET SECTION  
(ABOVE BASIN)





DESIGN OF SPUN CONCRETE  
CONNECTOR PIPES FLOWING FULL

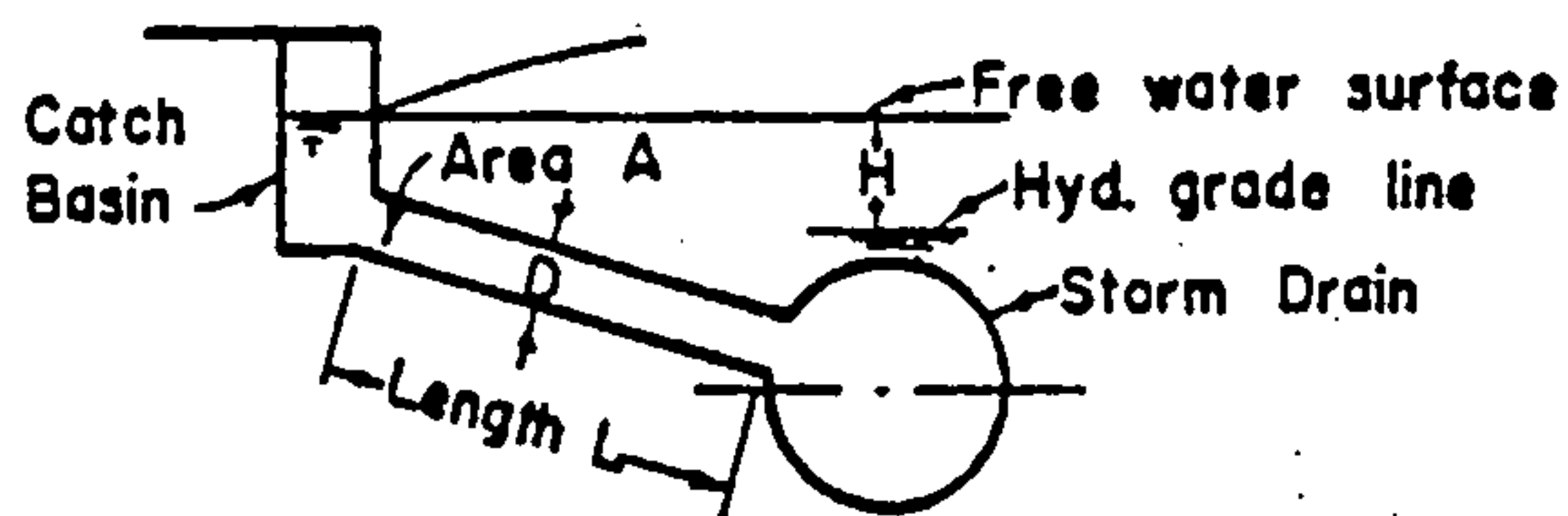
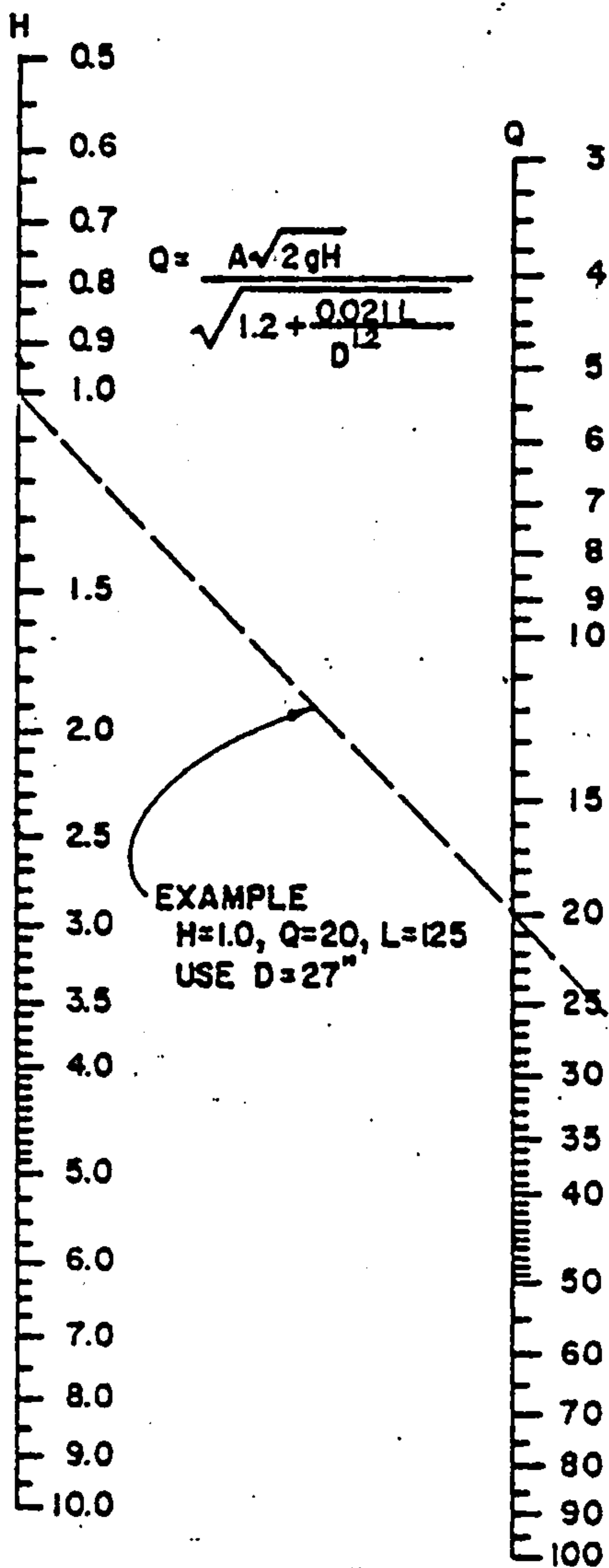


PLATE 22.3 D-8

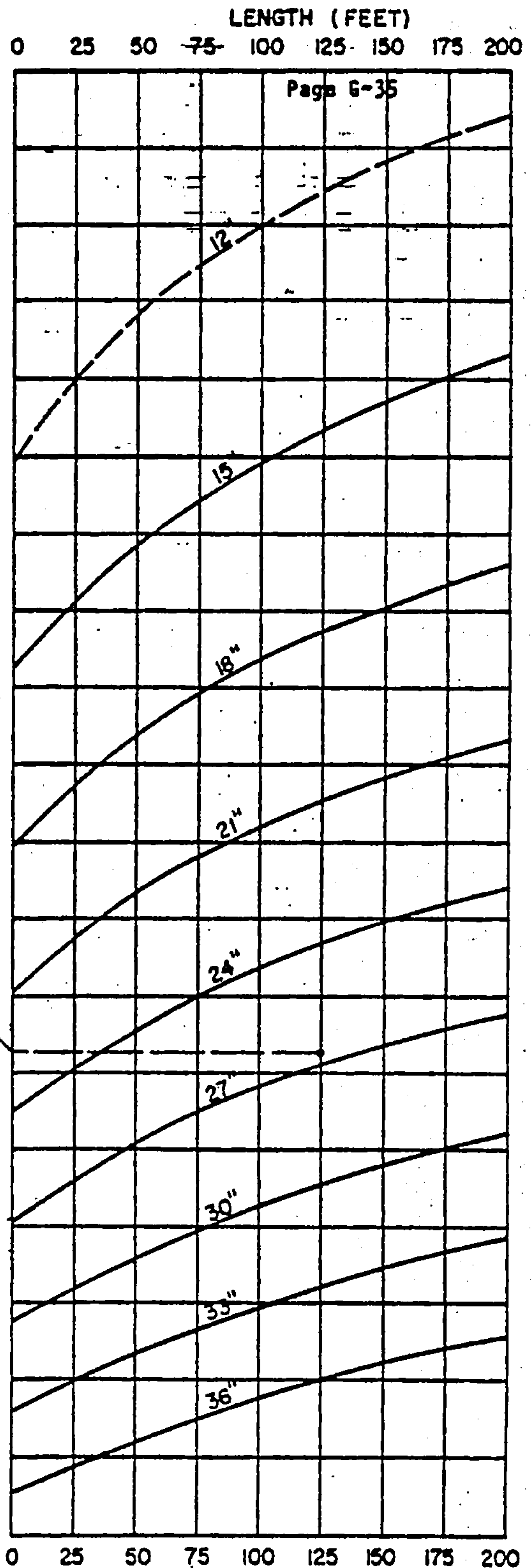


PLATE 4



**APPENDIX**  
(In folders)

**ELDORADO PARK SUBDIVISION UNIT 1, GRADING AND DRAINAGE PLAN**  
Sheets 1, 2, and 3