DRAINAGE INFORMATION SHEET

10025

PROJECT TITLE:	Sage Mobile Home Park	ZONE ATLAS/DRN	G. FILE #:	L-9-Z	
DRB #:	EPC #:	WORK ORDER #:			
LEGAL DESCRIPTION	ON: TRACT 2A, WESTGATE MOBILE H	OME PARK & TRAC	TS 1&3, SNC	OW VISTA	
CITY ADDRESS:	ON 98TH STREET BETWEEN TOWER RO	AD AND DEVARGAS	ROAD		
ENGINEERING FIRE	M: TIERRA WEST, LLC	CONTACT:	ca		
ADDRESS:	8509 Jefferson NE Albuqueque, NM 87113	PHONE:	(505) 858-3	3100	
OWNER:	SAGE WEST, LLC	CONTACT:			
ADDRESS:	3511 CARLISLE NE ALBUQUERQUE, NM 87110	PHONE:			
ARCHITECT:		CONTACT:	**************************************		· ·
ADDRESS:		PHONE:		——————————————————————————————————————	
SURVEYOR:	SOUTHWEST SURVEY CO.	CONTACT:	THOMAS F	PATRICK	
ADDRESS:	333 LOMAS BLVD NE ALBUQUERQUE,NM 87102	PHONE:	(505)998-0	303	
CONTRACTOR:		CONTACT:			
ADDRESS:		PHONE:			
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DAT	E SUBMITTED: 10/13/00		TYDHOL(OGY SECTION	

BY: Vincent Carrica

DRAINAGE INFORMATION SHEET

PROJECT TIT	TLE: <u>Sa</u>	ge Mobile Home Park	ZONE ATLAS/DRN	G. FILE #:	L-9-Z & M-9-Z
DRB #:		EPC #:	WORK ORDER #:		
LEGAL DESC	RIPTION:	TRACT 2A, WESTGATE MOBILE H	OME PARK & TRAC	TS 1&3, SN	IOW VISTA
CITY ADDRE	SS: <u>01</u>	N 98TH STREET BETWEEN TOWER ROA	AD AND DEVARGAS	ROAD	
ENGINEERIN	G FIRM:	TIERRA WEST, LLC	CONTACT:	Vincent Cari	rica
ADDRES	SS: <u>850</u>	99 Jefferson NE Albuqueque, NM 87113	PHONE:	(505) 858	-3100
OWNER:	<u>s/</u>	AGE WEST, LLC	CONTACT:		
ADDRES	351 351	I CARLISLE NE ALBUQUERQUE, NM 87110	PHONE:		
ARCHITECT:			CONTACT:		
ADDRES	SS:		PHONE:		
SURVEYOR:	<u>sc</u>	DUTHWEST SURVEY CO.	CONTACT:	THOMAS	PATRICK
ADDRES	SS: <u>333</u>	B LOMAS BLVD NE ALBUQUERQUE,NM 87102	PHONE:	(505)998-0303	
CONTRACTO)R:		CONTACT:		
ADDRES	SS:		PHONE:		-
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DATE SUBMITTED: 10/13/00 (Revised 12-28-00)

BY: Vincent Carrica



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

February 15, 2001

Ron Bohannan, PE
Tierra West LLC
8509 Jefferson NE
Albuquerque, NM 87113

Re: Sage Mobile Home Park Drainage Report (L9/D25)

Engineer Stamp date 12-26-00

Dear Mr. Bohannan,

Based on information provided in your submittal dated 10-19-00, the above referenced site is approved for Grading Permit. Since this is an integral part of the SAD and also is critical for the safety of the park residents, an Engineer's Certification of this plan will be required in order to satisfy Hydrology requirements for a Certificate of Occupancy.

Per your calculations, this site shall release 1.8 cfs from the new pond at Sage (with a water surface elevation of 5145.0) and 2.7 cfs from the connection at the new inlet midblock.

If you have any questions, you can contact me at 924-3986.

Sincerely, Bradley L. Bryham

Bradley L. Bingham, PE

Sr. Engineer, Hydrology

C:

Ed Adams, CoA

file

8509 Jefferson NE Albuquerque, NM 87113 (505) 858-3100 fax (505) 858-1118

e-mail: twdms@aol.com 1-800-245-3102

December 20, 2000

Mr. Bradley L. Bingham, PE Hydrology Review Engineer City of Albuquerque PO Box 1293 Albuquerque, NM 87103

RE: Drainage Report and Grading and Drainage Plan

Sage Mobile Home Park (L9/D25)

Dear Brad:

We are in receipt of your comments regarding grading and drainage for the referenced project. The comments are addressed in the order they appear in your letter of November 9th.

1. Please define a swale along the eastern edge of Basins 12, 13, and 9. Please add calculations to size this swale.

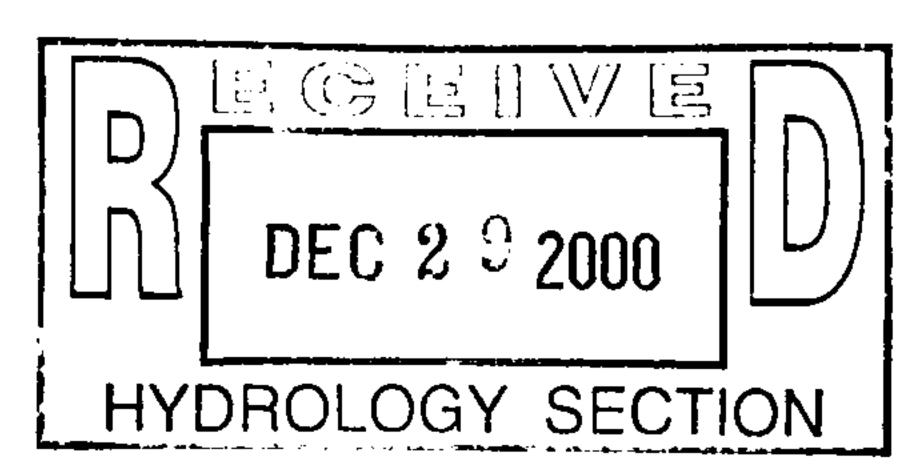
This swale is shown on plan sheet A. A detail of the swale was added and also shown on sheet A. The calculations for sizing the swale are attached.

2. Please add elevations along 98th Street that are coordinated with SAD 222 design. The entrance to Tract I needs more detail. Are you using the area north of this entrance as part of your ROW pond also? Either way, show how you propose to grade this area.

Additional elevations were added to the proposed 98th Street in the vicinity of the two entrances to our site and adjacent to the proposed ROW pond as shown on Sheet B of the grading and drainage plans. The Larkin Group provided these elevations to our office. We also added more detail to the commercial center entrance. The area north of the second entrance will be graded to drain at a 6.6 percent slope through a 12" reinforced concrete culvert pipe to the ROW pond.

3. Please provide enough detail to ensure that all runoff will get to the pond at AP-2B and AP-1. I will need to see rundowns and a low flow channel in the design of you pond. The SAD will build this pond.

We designed the grades for the park entrance and the commercial center with a one-foot minimum water block to direct the flows from the park into the drive for the commercial center. Once in the commercial center parking lot, the flows will continue south and enter the ROW pond through a rundown to be constructed just south of the second entrance. The park's south section will continue to flow to the southeast corner and drain through a break in the fence to a concrete rundown and into the ROW pond. We revised the plans to include the required rundowns to the ROW pond at AP-1 and AP-2B and provided a detail of the rundowns. We added a two-foot concrete swale from the two rundowns to the pond outlet pipe for low flow conditions.



4. Some contours in Basin 4 are incomplete so I cannot determine how this basin is draining.

We provided additional spot elevations in Basin 4 for clarity.

5. Please use the updated (1996) FIRM maps in your report. If you do not have this, I can provide you one.

We corrected the FIRM map on the plans to reflect the 1996 updated version.

6. Please turn your overflow weir for pond F to the south.

We turned the overflow weir in pond F to the south as requested. The plans were updated to reflect this change.

A revised grading and drainage plan is attached for your review and records. Should you have any questions regarding this matter, please do not hesitate to contact me.

Sincerely,

Ronald R. Bohannan, PE

JN 990082 RRB/vpc/db

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42-382 100 SHEETS EYE-EASE® 5 SQUARE
42-389 200 SHEETS EYE-EASE® 5 SQUARE
42-392 100 RECYCLED WHITE 5 SQUARE
42-399 200 RECYCLED WHITE 5 SQUARE Made in U.S. A.

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City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

November 9, 2000

Ron Bohannan, PE Tierra West LLC 8509 Jefferson NE Albuquerque, NM 87113 From: Ed Adams
From: Brad B

Re:

Sage Mobile Home Park Drainage Report (L9/D25)

Engineer Stamp date 10-17-00

Dear Mr. Bohannan,

Based on information provided in your submittal dated 10-19-00, the above referenced report cannot be approved for Grading Permit until the following comments are addressed:

- Please define a swale along the eastern edge of Basins 12, 13 and 9. Please add calculations to size this swale.
- Please add elevations along 98th Street that are coordinated with SAD 222 design. The entrance to Tract I needs more detail. Are you using the area north of this entrance as part of your ROW pond also? Either way, show how you propose to grade this area.
- Please provide enough detail to ensure that all runoff will get to the pond at AP-2B and AP-1. I will need to see rundowns and a low flow channel in the design of your pond. This pond will be built by the
- Some contours in Basin 4 are incomplete so I cannot determine how this basin is draining.
- Please use the updated (1996) FIRM maps in your report. If you do not have this, I can provide you one.
- Please turn your overflow weir for pond F to the south.

As a reminder, your project's grading plan needs to be completely buildable, showings all details necessary to complete the work. If you have any questions, you can contact me at 924-3986.

Sincerely,

Bradley L. Bingham, PE

Sr. Engineer, Hydrology

C: file

DRAINAGE REPORT

For

Sage Mobile Home Park

Prepared by

Tierra West, LLC 8509 Jefferson NE Albuquerque, New Mexico 87113

Prepared for

Fred Seeley
Sage West, LLC
3511 Carlisle NE
Albuquerque, New Mexico 87110

October 13, 2800 NEVICE DO OCT 1 9 2000

Ronald R. Bohannam P.E. No. 7868

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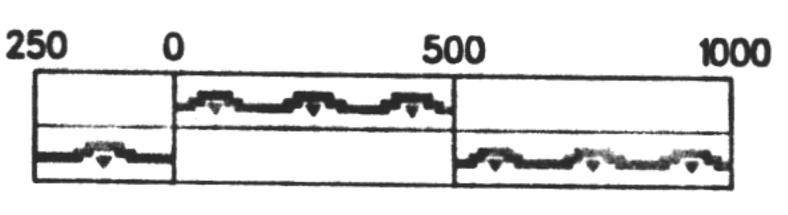
Albuquerque

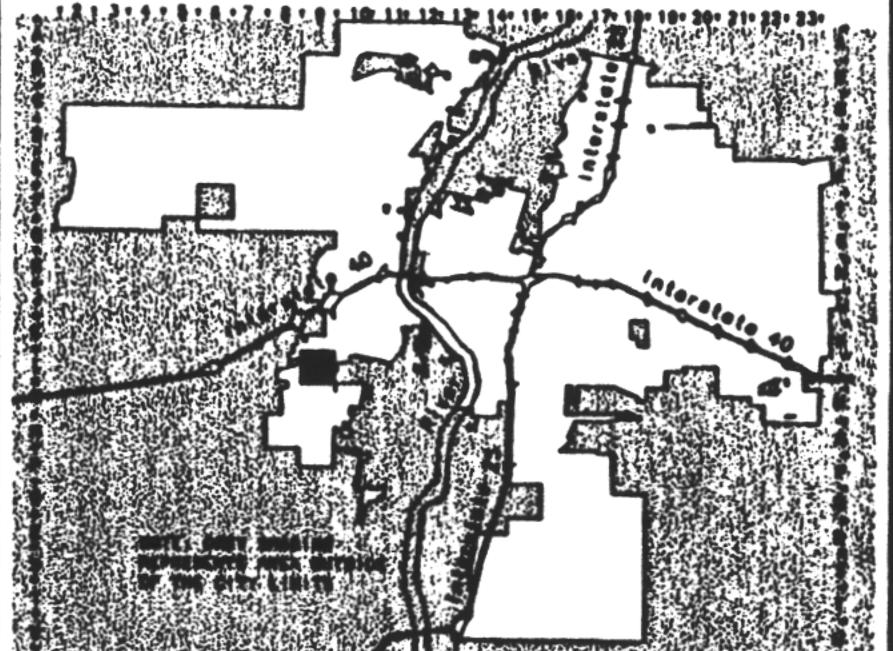
Abuqueque Geographic Information System
PLANNING DEPARTMENT

C Copyright 1998



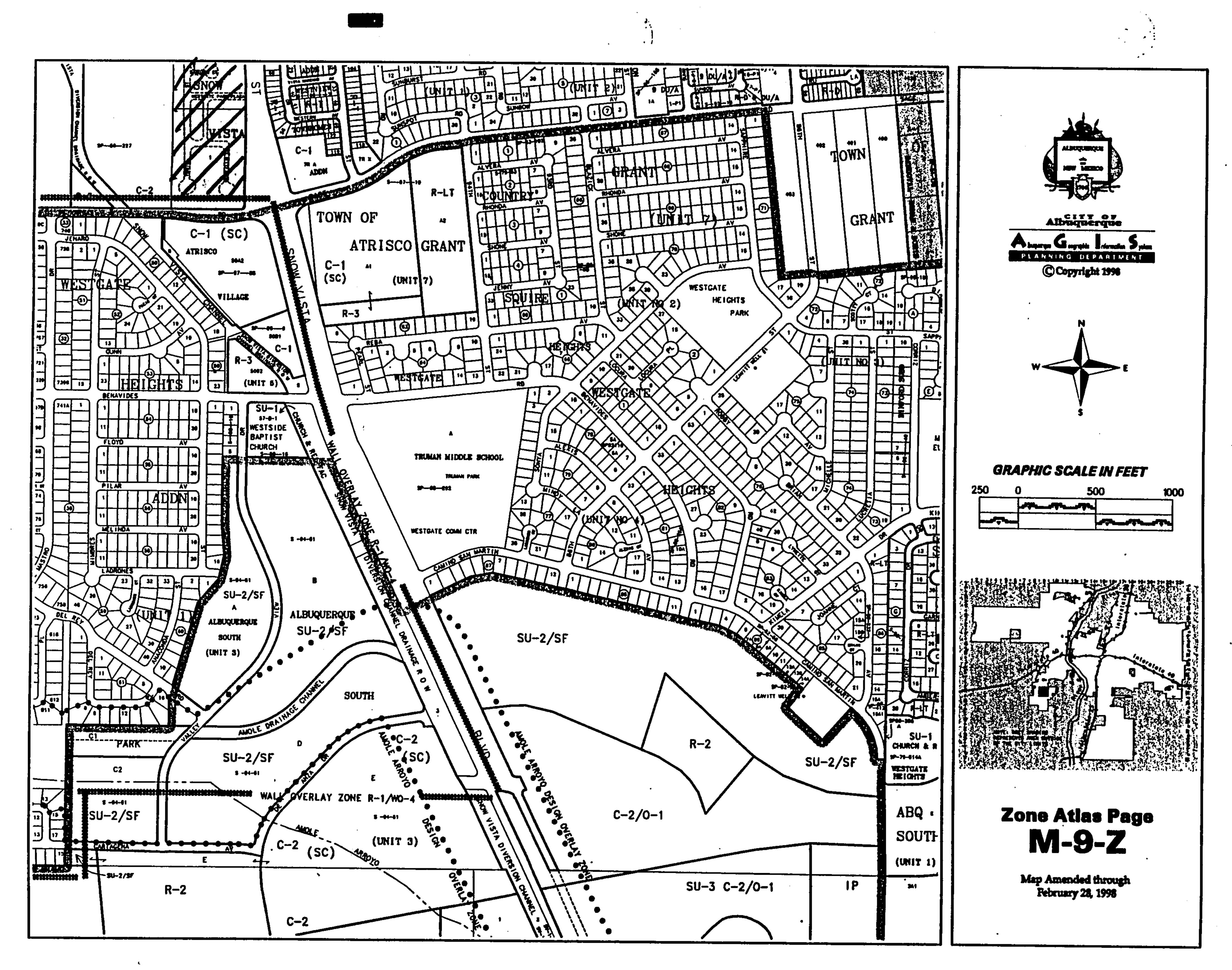
GRAPHIC SCALE IN FEET





Zone Atlas Page L-9-Z

> Map Amended through June 29, 1998



LOCATION:

Sage Mobile Home Park is an existing, established mobile home park. The 24.25-acre site is located in southwest Albuquerque immediately west of 98th Street, between Tower Road and Devargas Road. It encompasses Tracts 1, 2A and 3 as described in Exhibit A. The area is currently zoned R-T and C2. Sage West LLC, the owner of the mobile home park, and the City of Albuquerque entered into an a drainage covenant where by the owner agrees to capture a minimum of 80% of runoff generated by a 100-year event and detain it entire, and the City agrees to remove the owner and the property from the SAD 222 obligations. The purpose of this report is to provide the drainage analysis and management plan for fulfilling the owner's obligation under this drainage covenant.

DRAINAGE BASINS DESIGNATIONS:

For the purpose of this report the on-site and off-site drainage basins are designated as follows.

Off-Site Basins:

None

No off-site flows enter the site.

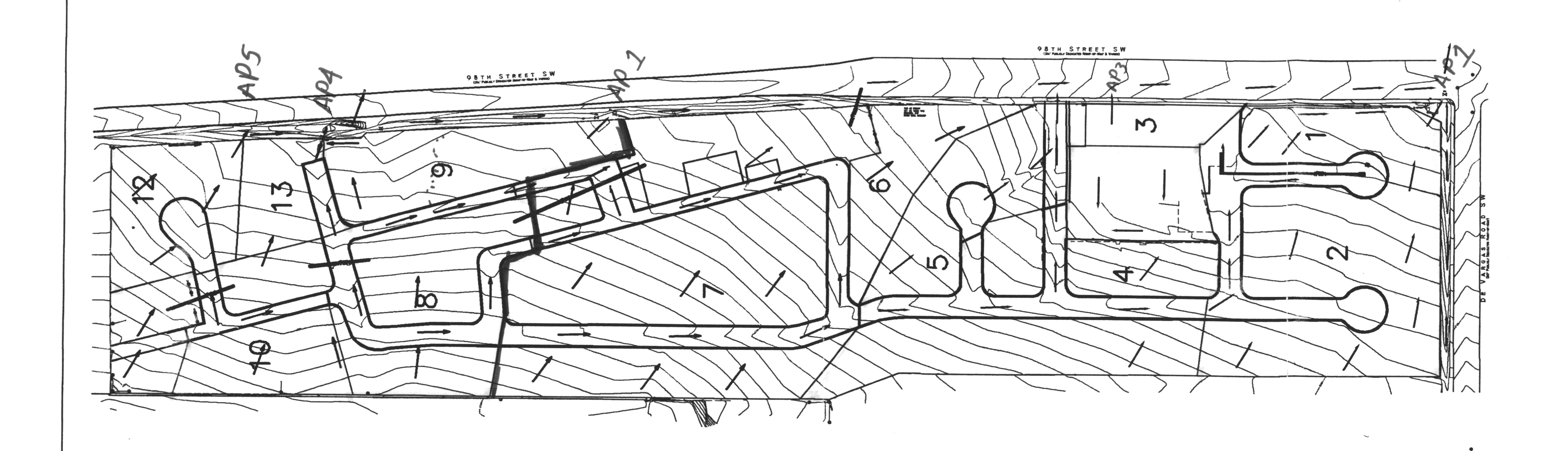
On-Site Basins:

BASIN	POND
DESIGNATION	DESIGNATION
Basin 1	No Pond. Flows are conveyed to ROW Pond.
Basin 2	No Pond. Flows are conveyed to ROW Pond.
Basin 3	No Pond. Flows are conveyed to ROW Pond.
Basin 4	No Pond. Flows are conveyed to ROW Pond.
Basin 5	No Pond. Flows are conveyed to ROW Pond.
Basin 6	No Pond. Flows are conveyed to ROW Pond.
Basin 7	Pond F
Basin 8	Pond G
Basin 9	No Pond. Flows are conveyed to Pond F.
Basin 10	Pond H
Basin 11	Pond I
Basin 12	No Pond. Flows are conveyed to Pond F.
Basin 13	No Pond. Flows are conveyed to Pond F.

EXISTING DRAINAGE CONDITIONS:

The site is currently developed as an existing mobile home park and a small commercial shopping center. The mobile home park is bordered by a perimeter CMU wall. No off-site flows enter the mobile home park. Off-site flow is intercepted by Devargas Road but generally flows south and east past the site. The existing streets within the park are paved and are bordered by a 2' flat concrete curb. The site was originally mass graded to drain generally south and east to the 98th existing to flow ay. The lots were not specifically graded to drain to the streets and cross lot drainage is prevalent. Please note, while storm waters flow across mobile home park spaces these are not platted lots. As such, the proposed or existing flow pattern does not violate City standards.

Runoff generated within the mobile home park is conveyed to breaks in the east perimeter fence or to the park entrances. The breaks in the perimeter fence are labeled as AP1, AP3, AP4 and AP5 on Exhibit 1 showing the existing drainage patterns. The park entrance is labeled as AP2 A&B. Once the flows exit the site to the 98th Street right of way they are conveyed south along the west side of the roadway. Currently, no significant on-site ponding occurs. A summary of existing flows and flow patterns is as follows.



LEGEND	
	BASIN LINE
4	EXISTING DRAWAGE FLOW
	NIDEX CONTOUR
	CONTOUR
	EXISTING DRIVE



GRAPHIC SCALE
80 40 0 40 80

SCALE: 1"=80"

EXHIBI	T #1
ENGINEER'S SEAL	SAGE MOBILE
	EXISTING DRAIN

	PARK	BATE
	EYIQTING DRANAGE	6-2-00
	EXISTING DRAINAGE	9982FLDVS2.DV
		SHEET #
	TIERRA WEST, LLC	
	8509 JEFFERSON NE ALBUQUERQUE, NEW MEXICO 87113	
DNALD R. BOHANNAN E. 47968	(505)858-3100	JCB # 990082

BASIN NO.	PEAK RUNOFF (CFS)	EXITS SITE @ POINT	CROSSES 98 TH @	NOTES
1	3.01	AP1	AP1	Channeled through break in fence to 98 th
2	12.87	AP1	AP1	Channeled through break in fence to 98 th
3	1.70	AP2A	AP1	Sheet flow from parking lot to 98 th
4	14.94	AP2B	AP1	Free discharge from entrance drive to 98 th
5	4.99	AP2B	AP1	Free discharge from entrance drive to 98 th
6	2.60	AP2B	AP1	Channeled by perimeter fence to entrance drive to 98 th
7	15.9	AP2B	AP1	Channeled by perimeter fence to entrance to 98th
	6.32	AP3	AP1	Channeled through break in fence to 98 th
8	7.66	AP3	AP1	Channeled through break in fence to 98 th
9	2.53	AP3	AP1	Channeled through break in fence to 98 th
	2.25	AP4	AP4	Channeled through break in fence to 98 th
10	4.45	AP4	AP4	Channeled through break in fence to 98 th
11	1.69	AP5	AP4	Channeled through break in fence to 98 th
12	4.31	AP5	AP4	Channeled through break in fence to 98 th
13	2.51	AP4	AP4	Channeled through break in fence to 98 th

Basins 1 and 2 currently drain to a point at the southeast corner of the park. This point has been marked as AP-1 on the attached basins map. Upon entering the street right of way, flows are combined with off-site flows being channeled down the Devargas right of way from the west and are then channeled across 98th Street via a battery of three 36" culvert pipes. Once across the road, the flows continue down the Devargas right of way. Basins 3, 4, 5, 6 and a portion of Basin 7 currently drain to existing park entrances or directly to 98th Street from the existing parking lot of the strip mall. The remaining portion of Basin 7 and Basins 8 and 9 drain to the break in the perimeter fence

indicated on the basin map as AP-3. Runoff from Basins 3 through 9 is carried in the 98th Street right of way south to Devargas Street. The flows are then channeled across 98th Street via the previously mentioned 36" culvert pipes. Basins 10 and 13 drain to a perimeter fence break indicated as AP-4, while basins 11 and 12 drain to another break in the same fence. This point is indicated on the basin map as AP-5. Runoff exiting the site from these two points is collected in a sump and conveyed across 98th Street in a single 36" culvert pipe. Once across 98th Street the flow continues east in a small, natural arroyo. Combined flows at AP-1 through AP-5 under existing conditions are as follows.

15.88 cfs
40.13 cfs
16.51 cfs
9.21 cfs)
9.21 cfs 7 6.00 cfs 5

Flows generated on site crossing 98th Street at AP-1 equal 72.52 cfs. Flows generated on site crossing 98th Street at AP-4 equal 15.21 cfs.

FIRM MAP AND SOIL CONDITIONS:

The site is located on Firm Map 350002, as shown on the attached excerpt. The map shows that the site does not lie within any 100-year flood plains. It does, however, lie adjacent to Flood Hazard Zone "A" that is shown on the attached applicable section of panel 33 of 50 of the National Flood Insurance Rate Maps, Bernalillo County, New Mexico and incorporated areas, dated October 14, 1993. Areas designated as Flood Hazard Zone "A" are areas of a 100-year flood, for which base flood elevations and flood hazard factors have not been determined. The proposed subdivision is separated from Zone AO by the mobile home park's perimeter fence.

The site predominately contains a soil type designated as Blue Point (BCC) by the Soil Conservation Service Soil Survey of Bernalillo County. Blue Point soil is present on nearly level to



et your insurance agent, or call the National Flood Insurance am at (800) 638-6620.



SCALE:1"=500' 500 500

NATIONAL FLOOD INSURANCE PROGRA

FLOOD INSURANCE RATE MAF
CITY OF
ALBUQUERQUE,
NEW MEXICO

BERNALILLO COUNTY

PANEL 33 OF 50

USE JAPATED MAP

COMMUNITY-PANEL NUMBER

350002 0033 C

EFFECTIVE DATE: OCTOBER 14, 1983



Federal Emergency Management Agency

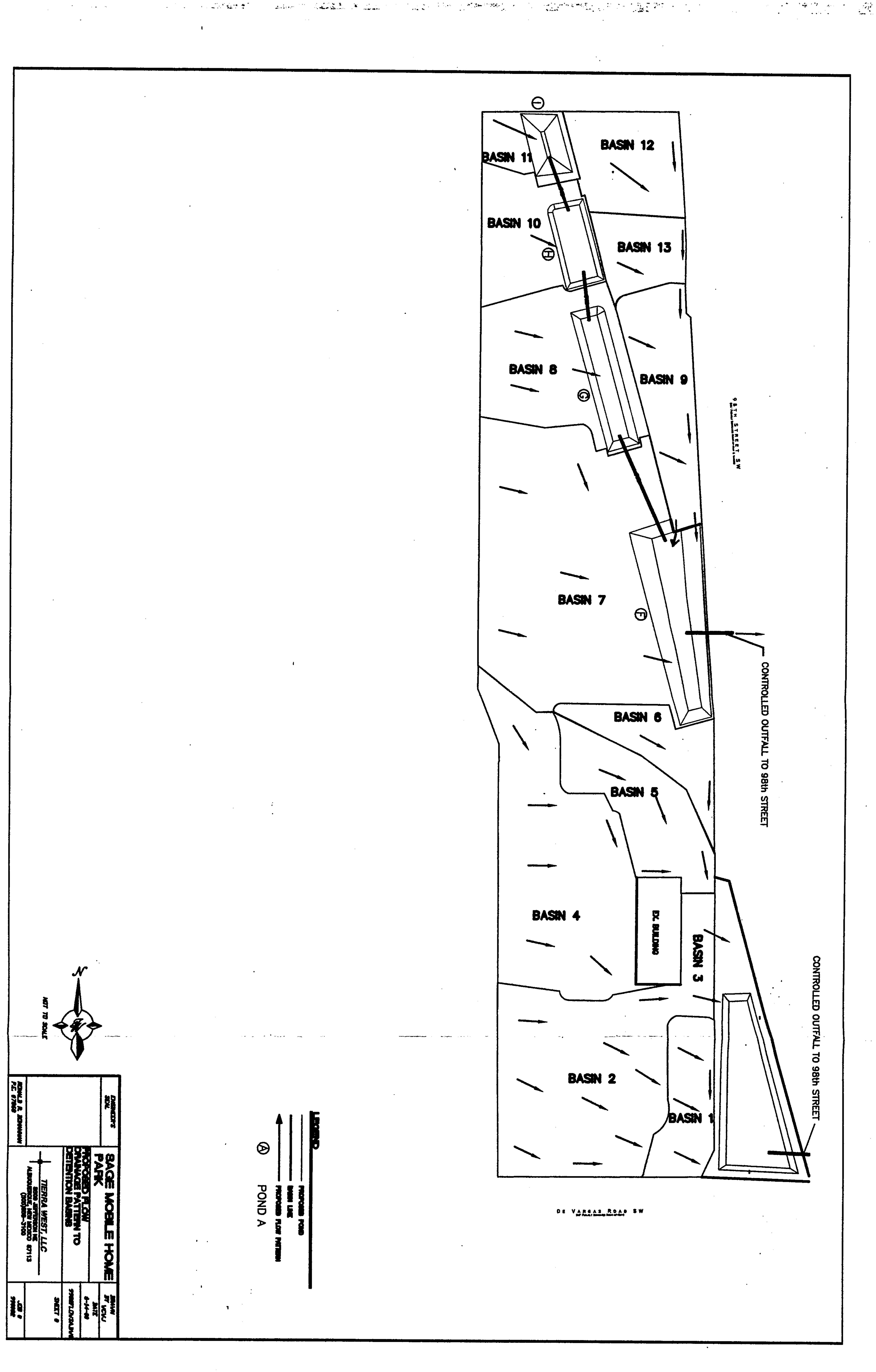
moderately sloping terrain and is formed in sandy alluvial and eolian sediments on alluvial fans and terraces. This deep, excessively drained soil is rapidly permeable and runoff is slow.

ONSITE DRAINAGE MANAGEMENT PLAN:

The mobile home park is a long established and fully occupied development. Capturing the runoff generated by a 100-year event while maintaining the integrity of the park was of primary consideration. Existing open areas within the site were selected as proposed ponding areas. The ponds are to be predominately shallow in depth containing relatively flat side slopes. Where possible, the bases of the ponds were designed to allow for the placement of parks and recreational facilities. All ponds will act as detention basins and are designed to fully drain either to an adjacent downstream pond or directly to the 98th Street storm drain in a twenty-four hour period. The 98th Street storm drain is to be constructed under SAD 222. The presence of the aforementioned, deep, excessively drained soils with high permeability rates will aid in the efforts to empty the pond. Speed humps and existing perimeter walls will serve as dikes to channel runoff in the appropriate directions.

As previously mentioned, the mobile home park was divided up into thirteen basins.

Drainage patterns under proposed conditions and pond locations are indicated on the attached exhibit. A summary of the proposed flows and pond capacities is as follows.



	PEAK	DRAIN	POND	EXCESS	<u> </u>	
BASIN	RUNOFF	ТО	CAPACITY	RUNOFF	POND	
NO.	(AC-FT)	POND	(AC-FT)	(AC-FT)	OUTFALL	NOTES
1	0.116	ROW	1.338	NONE	98 th	Primary outflow and
						emergency overflow to
						98 th
2	0.545	ROW	1.338	NONE	98 th	Primary outflow and
						emergency overflow to
						98 th
3	0.079	ROW	1.338	NONE	98 th	Primary outflow and
						emergency overflow to
	0.040					98 th
4	0.619	ROW	1.338	NONE	98 th	Primary outflow and
			,			emergency overflow to
	0.000		4 000		O O ID	98 th
) o	0.200	ROW	1.338	NONE	98 th	Primary outflow and
			,			emergency overflow to 98 th
6	0.091	ROW	1 220	NICALIE	Ooth	
	0.091	ROW	1.338	NONE	98 th	Primary outflow and
						emergency overflow to 98 th
7	1.376	F	1.403	NONE	98 th	Primary outflow and
			1.400	140141	30	emergency overflow to
				•		98 th
8	0.312	G	0.326	NONE	Pond F	Primary outflow to Pond
						F and emergency
						overflow to Basin 7
9	0.186	F	1.403	NONE	98 th	Emergency overflow to
						98 th
10	0.168	Н	0.169	NONE	Pond G	Primary outflow to Pond
						G and emergency
						overflow to Basin 8
11	0.059		0.085	NONE	Pond H	Primary outflow to Pond
						H and emergency
		 _	4 4 6 6			overflow to Basin 10
12	0.178	 -	1.403	NONE	1 98°	Emergency overflow to
12	0 40E		4 400			98 th
13	0.105	F	1.403	NONE	98 th	Emergency overflow to
<u> </u>			<u> </u>			98 th

Runoff from Basins 1 through 6 will continue to flow from the park as it has historically to 98th

Street. A pond will be constructed in the excess right-of-way created by the realignment of the 98th

Street \ Devargas Road intersection. Runoff from the excess right-of-way and Basins 1 through 6 will

be conveyed to and detained by this pond. The pond will discharge to the 98th Street storm drain that is to be constructed under SAD 222 via a 10-inch drain with a peak runoff of 1.76 cfs. Runoff in excess of a 100-year event and in excess of the pond's capacity will be channeled to 98th Street through the emergency overflow.

Ponds F, G, H and I will be constructed in Basins 7, 8, 10 and 11, respectively. These ponds will be placed in a 100-foot wide public utility easement running diagonally through the mobile home park in a southeasterly direction. The utility easement contains overhead electrical transmission lines. There are no dwelling units in the areas beneath the power lines. Some do contain recreational facilities such as an outdoor swimming pool and a basketball court. The swimming pool, which is no longer in use, and the basketball court, will be removed in the construction of Pond F. Ponds F, G, H and I are in series and flows from each pond will be routed to the adjacent downstream pond with the exception of Pond F which will outfall to the 98th Street storm drain to be constructed as part of SAD 222 at a peak runoff of 2.68 cfs. Flows in excess of a 100-year storm will discharge through emergency overflows to downstream basins with the exception of Pond F, which will contain an emergency overflow to 98th Street. The overflow widths for Ponds F thru I will be twenty feet wide and one foot deep. These pond overflows were calculated as broad crested weirs. The right-of-way pond will overflow to the 98th Street and DeVargas intersection for events larger than a 100-year storm.

Basins 9, 12 and 13 will contain no ponds, as no existing open space is available for their placement. Runoff from these basins is designed to be routed to Pond F, located in Basin 7. The existing breaks in the perimeter fence, which currently allow flows to exit the park to the 98th Street right of way, will be eliminated. Minor grading along the east perimeter fence, within the existing tenfoot public utility easement will allow flows generated in Basins 9, 12 and 13 to be channeled to Pond F. Pond F has been sized to handle these flows, as well as flows from Basin 7.

. .

With the runoff captured by ponds F through I and the right-of-way pond, 100 percent of the flows developed within the mobile home park are detained. The peak flows exiting the system to the 98th Street storm drain total 4.44 cfs. The peak occurs at 2.37 hours and is delayed from the runoff generated within 98th Street, which peaks at 1.5 hours.

FUTURE DEVELOPED CONDITIONS:

The mobile home park is currently developed and is operating at full capacity. It is not anticipated that conditions will arise that would increase the runoff from that used to develop this drainage plan and size the proposed ponds.

CRITERIA:

The site was analyzed using the procedures outlined in the Development Process Manual Volume 2, Chapter 22. The Weighted-E method was used in estimating volumes and flow rates of runoff from off-site basins. The AHYMO computer program was used to analyze on-site basins and the ponds. The existing and proposed conditions for on-site basins were analyzed for a 100-year, 24-hour rainfall event.

SUMMARY:

The site has a developed peak discharge of 87.73 cfs. A 100-year, 24-hour storm would generate 3.563 acre-feet of runoff. Ketention ponds will be constructed on-site and in the excess right-of-way created by the realignment of the 98th Street / Devargas intersection. The ponds will capture 100 percent of the flows generated within the mobile home park and the excess right-of-way. The detention ponds will be drained at a controlled release rate to the 98th Street storm drain that is to be built under SAD 222. The combined release rate will be 4.44 cfs. This will allow the ponds to be

completely drained in a 24-hour period. This is consistent with the policies and practices of the City of Albuquerque. The existing Devargas roadway and the west perimeter wall intercept all of the offsite flows. No offsite flows will enter the site.

Future redevelopment within the mobile home park is not anticipated and the conditions contributing to a 100-year event are not expected to change. This drainage plan was designed to intercept and detain 100 percent of the flows generated by a 100-year, 24-hour event. The detention ponds will either drain into an adjacent pond or will drain directly to the 98th Street storm drain. The rate at which these ponds will drain will be throttled to less than 2.7 cfs per outfall to insure that the detention ponds will be completely drained in a 24-hour period. The presence of deep, excessively drained soils with high permeability rates will aid in the efforts to empty the detention ponds. The ponds are designed to be shallow in depth with relatively flat side slopes, in an attempt to present as little of a hazard as possible to the residents of the mobile home park.

Sage Mobile Home Park

Emergency Overflows for Ponds:

	Q	Н	L (Required)	L (Provided)
Pond	(cfs)	(FT)	(FT)	(FT)
F	47.62	1	18	20
G	13.8	1	5.2	20
Н	6.14	1	2.3	20
	1.69	1	0.6	20
ROW	42.97	0.5	45.3	50

Q=CLH^3/2

C= 2.68 for broad crested weir

ON-SITE - DEVELOPED CONDITIONS

Design	A													100-Year		
Basin	Area	Area	Агеа	Treat	ment A	Treatm	ent B	Treat	tment C	Treat	ment D	Weighted E	V 360	V 1440	V 10 day	Flow
	(sf)	(acres)	(sq. miles)	<u>%</u>	(acres)	%	(acres)	%	_(acres)	%	(acres)	(ac-ft)	(ac-ft)	(ac-ft)	(ac-ft)	cfs
1	37612	0.863	0.001349145	0%	0	7%	0.057	49%	0.427014	44%	0.380	O Alliana de la California de la Califor	0.101	0.115	0.147	3.0
2	144554	3.319	0.005185161	0%	0	3%	0.092	28%		69%	2.282	1.655	0.458	0.545	0.737	12.8
_3	16901	0.388	0.00060624	0%	0	0%	0.000	0%	0	100%	0.388	1.970	0.064	0.079	0.737	
4	172194	3.953	0.00617661	0%	0	3%	0.133	34%	1.347997	63%	2.472		0.524	0.619	0.827	1.70
5	59704	1.371	0.002141586	0%	0	4%	0.055		0.583489	53%	0.732	1.501	0.171	0.199	0.027	14.9
6	36765	0.844	0.001318763	0%	0	23%	0.191		0.431263	26%	0.222		0.083	0.193		4.99
7	263739	6.055	0.009460335	0%	0	8%	0.488		2.063073	58%	3.503	1.531	0.773		0.110	2.60
8	94982	2.180	0.00340701	0%	0	23%	0.492		0.489323		1 199		0.265	0.907	1.202	22.22
9	57944	1.330	0.002078455	0%	0	0%	0.000	52%	0.69171	48%	0.639	1.460		0.311	0.412	7.04
10	59427	1.364	0.00213165	0%	0	24%	0.324		0.503747	39%	0.537	1.300	0.162	0.186	0.240	4.78
11	25521	0.586	0.00091544	0%	0	39%	0.228		0.221932	23%	0.136		0.148	0.168	0.214	4.45
12	49380	1.134	0.001771264	0%	0	0%	0.000		0.426506	62%		1.092	0.053	0.059	0.070	1.69
13	28439	-	0.001020109			0%	0.000		0.228504		0.707	1.601	0.151	0.178	0.238	4.31
14	60000		0.002152204	0%		0%	0.000		0.607441	65% 56%	0.424 0.770	1.627 1.538	0.089 0.176	0.105 0.206	0.141 0.271	2.51 5.11

Totals 1107162.00 25.42 0.039713972 3.769 4.980 92.80

Equations:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

V 360 = Weighted E * Total Area

V 1440 = V360 + Ad(P1440 - P 360)/12

Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

SAGE MOBILE HOME PARK 100-YEAR, 24-HR STORM (UNDER PROPOSED CONDITIONS) START TIME=0.0RAINFALL TYPE=2 RAIN QUARTER=0.0 IN RAIN ONE=1.87 IN RAIN SIX=2.20 IN RAIN DAY=2.66 IN DT=0.03333 HR * BASIN 1 COMPUTE NM HYD ID=1 HYD NO=100.1 AREA=0.0013491 SQ MI PER A=0.00 PER B=7.00 PER C=49.0 PER D=44.00 TP=-0.1333 HR MASS RAINFALL=-1 PRINT HYD ID=1 CODE=1 * BASIN 2 COMPUTE NM HYD ID=2 HYD NO=100.2 AREA=0.0051852 SQ MI PER A=0.00 PER B=3.00 PER C=28.00 PER D=69.00 TP=-0.1333 HR MASS RAINFALL=-1 PRINT HYD ID=2 CODE=1 * BASIN 3 COMPUTE NM HYD ID=3 HYD NO=100.3 AREA=0.0006062 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=-0.1333 HR MASS RAINFALL=-1 PRINT HYD ID=3 CODE=1 * BASIN 4 COMPUTE NM HYD ID=4 HYD NO=100.4 AREA=0.0061766 SQ MI PER A=0.00 PER B=3.00 PER C=34.00 PER D=63.00 TP=-0.1333 HR MASS RAINFALL=-1 PRINT HYD ID=4 CODE=1 * BASIN 5 COMPUTE NM HYD ID=5 HYD NO=100.5 AREA=0.0021416 SQ MI PER A=0.00 PER B=4.00 PER C=43.00 PER D=53.00 TP=-0.1333 HR MASS RAINFALL=-1 PRINT HYD ID=5 CODE=1 * BASIN 6 COMPUTE NM HYD ID=6 HYD NO=100.6 AREA=0.0013188 SQ MI PER A=0.00 PER B=23.00 PER C=51.00 PER D=26.00 TP=-0.1333 HR MASS RAINFALL=-1 PRINT HYD ID=6 CODE=1 BASIN 7 COMPUTE NM HYD ID=7 HYD NO=100.7 AREA=0.0094603 SQ MI PER A=0.00 PER B=8.00 PER C=34.0 PER D=58.00 TP=-0.1333 HR MASS RAINFALL=-1 PRINT HYD ID=7 CODE=1

```
* BASIN 8
 COMPUTE NM HYD
                     ID=8 HYD NO=100.8 AREA=0.003407 SQ MI
                     PER A=0.00 PER B=23.00 PER C=22.00 PER D=55.00
                     TP=-0.1333 HR MASS RAINFALL=-1
 PRINT HYD
                     ID=8 CODE=1
 * BASIN 9
 COMPUTE NM HYD
                     ID=9 HYD NO=100.9 AREA=0.0020785 SQ MI
                     PER A=0.00 PER B=.00 PER C=52.00 PER D=48.00
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                     ID=9 CODE=1
* BASIN 10
COMPUTE NM HYD
                     ID=10 HYD NO=100.10 AREA=0.0021317 SQ MI
                     PER A=0.00 PER B=24.00 PER C=37.00 PER D=39.00
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                    ID=10 CODE=1
* BASIN 11
COMPUTE NM HYD
                    ID=11 HYD NO=100.11 AREA=0.0009154 SQ MI
                    PER A=0.00 PER B=39.00 PER C=38.00 PER D=23.00
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                    ID=11 CODE=1
* BASIN 12
COMPUTE NM HYD
                    ID=12 HYD NO=100.12 AREA=0.0017713 SQ MI
                    PER A=0.00 PER B=0.00 PER C=38.00 PER D=62.00
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                    ID=12 CODE=1
* BASIN 13
                    ID=13 HYD NO=100.13 AREA=0.0010201 SQ MI
COMPUTE NM HYD
                    PER A=0.00 PER B=0.00 PER C=35.00 PER D=65.00
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                    ID=13 CODE=1
        ** *
* BASIN 14 EXCESS R.O.W.
*
COMPUTE NM HYD
                    ID=14 HYD NO=100.14 AREA=0.0021522 SQ MI
                    PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                    ID=14 CODE=1
*POND I
ROUTE RESERVOIR
                    ID=20 HYD NO=200.2 INFLOW ID=11 CODE=24
                    OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)
                    0
                                   .0000
                                                       72.80
                    0.686
                                   .0109
                                                       73.30
                    1.085
                                   .0276
                                                       73.55
```

1.372

Page 2

73.80

.0534

٠.

```
PRINT HYD
                     ID=20 CODE=1
 *ADD BASIN 10 TO POND I OUTFLOW
 ADD HYD
                     ID=44 HYD NO=102.1 ID=20 ID=10
 *POND H
 ROUTE RESERVOIR
                     ID=21 HYD NO=200.3 INFLOW ID=44 CODE=24
                     OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)
                     0
                                     .0000
                                                         70.00
                     0.686
                                     .0584
                                                         70.50
                     1.085
                                     .0970
                                                         70.75
                     1.372
                                     .1431
                                                         71.00
                     ID=21 CODE=1
 PRINT HYD
 *ADD BASIN TO POND I OUTFLOW
                     ID=44 HYD NO=102.2 ID=21 ID=8
ADD HYD
*POND G
ROUTE RESERVOIR
                     ID=22 HYD NO=200.4 INFLOW ID=44 CODE=24
                     OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)
                                     .0000
                                                         67.80
                     0.686
                                    .1061
                                                         68.30
                     1.085
                                    .1679
                                                         68.55
                     1.372
                                    .2365
                                                         68.80
                     1.565
                                     .2964
                                                         69.00
PRINT HYD
                     ID=22 CODE=1
*ADD BASINS 7,9,12&13 TO POND G OUTFLOW
ADD HYD
                     ID=44 HYD NO=102.3 ID=22 ID=7
ADD HYD
                     ID=44 HYD NO=102.4 ID=44 ID=9
ADD HYD
                     ID=44 HYD NO=102.5 ID=44 ID=12
ADD HYD
                     ID=44 HYD NO=102.6 ID=44 ID=13
PRINT HYD
                     ID=44 CODE=1
*POND F
ROUTE RESERVOIR
                     ID=23 HYD NO=200.5 INFLOW ID=44 CODE=24
                    OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)
                                    ..0000
                                                         59.00
                     1.372
                                    .2714
                                                         60.00
                     2.170
                                    .6012
                                                        61.00
                     2.745
                                    1.0479
                                                        62.00
                     2.796
                                    1.0989
                                                        62.10
PRINT HYD
                    ID=23 CODE=1
*ADD BASINS 1 THRU 6 AND BASIN 14 (EXCESS R.O.W.)
ADD HYD
                    ID=44 HYD NO=103.1 ID=1 ID=2
ADD HYD
                    ID=44 HYD NO=103.2 ID=44 ID=3
ADD HYD
                    ID=44 HYD NO=103.3 ID=44 ID=4
ADD HYD
                    ID=44 HYD NO=103.4 ID=44 ID=5
ADD HYD
                    ID=44 HYD NO=103.5 ID=44 ID=6
ADD HYD
                    ID=44 HYD NO=103.6 ID=44 ID=14
PRINT HYD
                    ID=44 CODE=1
*POND IN EXCESS ROW
ROUTE RESERVOIR
                    ID=25 HYD NO=200.7 INFLOW ID=44 CODE=24
                    OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)
                                   .0000
                                                     43.50
                    0.686
                                  0.3937
                                                     44.00
                    1.018
                                  0.5719
                                                     44.20
```

1.265	0.7554	44.40
1.472	0.9442	44.60
1.652	1.1382	44.80
1.815	1.3375	45.00

PRINT HYD

ID=25 CODE=1

*ADD POND IN EXCESS ROW OUTFLOW TO POND F OUTFLOW ADD HYD ID=44 HYD NO=200.8 ID=23 ID=25

+

PRINT HYD

ID=44 CODE=1

FINISH

COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR. DT =.033330 HOURS END TIME = 19.964670 HOURS .0000 .0016 .0033 .0050 .0067 .0085 .0103 .0122 .0141 .0160 .0180 .0201 .0222 .0243 .0266 .0289 .0312 .0337 .0362 .0388 .0415 .0443 .0472 .0502 .0534 .0567 .0601 .0637 .0675 .0715 .0758 .0809 .0865 .0924 .1050 .1334 .1771 .2398 .3254 .4379 .5814 .7600 .9780 1.1804 1.2649 1.3363 1.3997 1.4575 1.5106 1.5600 1.6061 1.6493 1.6900 1.7284 1.7646 1.7989 1.8314 1.8623 1.8915 1.9193 1.9456 1.9518 1.9576 1.9630 1.9682 1.9732 1.9780 1.9825 1.9869 1.9912 1.9953 1.9993 2.0031 2.0068 2.0104 2.0140 2.0174 2.0207 2.0240 2.0272 2.0303 2.0333 2.0363 2.0392 2.0420 2.0448 2.0475 2.0502 2.0528 2.0554 2.0580 2.0605 2.0629 2.0653 2.0677 2.0700 2.0723 2.0746 2.0768 2.0790 2.0812 2.0833 2.0855 2.0875 2.0916 2.0936 2.0956 2.0976 2.0995 2.1014 2.1033 2.1051 2.1070 2.1088 2.1106 2.1124 2.1141 2.1159 2.1176 2.1193 2.1210 2.1227 2.1244 2.1260 2.1292 2.1308 2.1324 2.1340 2.1355 2.1371 2.1386 2.1401 2.1416 2.1431 2.1446 2.1460 2.1475 2.1489 2.1504 2.1518 2.1532 2.1546 2.1560 2.1573 2.1587 2.1600 2.1614 2.1627 2.1640 2.1654 2.1667 2.1680 2.1692 2.1705 2.1718 2.1731 2.1743 2.1756 2.1780 2.1792 2.1804 2.1817 2.1829 2.1840 2.1852 2.1864 2.1876 2.1887 2.1899 2.1910 2.1922 2.1933 2.1944 2.1956 2.1967 2.1978 2.1989 2.2000 2.2026 2.2039 2.2065 2.2052 2.2078 2.2091 2.2116 2.2129 2.2142 2.2155 2.2167 2.2180 2.2193 2.2205 2.2218 2.2230 2.2243 2.2255 2.2268 2.2280 2.2293 2.2305 2.2318 2.2330 2.2342 2.2354 2.2367 2.2379 2.2391 2.2403 2.2415 2.2428 2.2440 2.2452 2.2464 2.2476 2.2488 2.2500 2.2512 2.2524 2.2536 2.2547 2.2559 2.2571 2.2583 2.2595 2.2606 2.2618 2.2630 2.2641 2.2653 2.2665 2.2676 2.2688 2.2699 2.2711 2.2722 2.2734 2.2745 2.2757 2.2768 2.2779 2.2791 2.2802 2.2813 2.2825 2.2836 2.2847 2.2858 2.2870 2.2881 2.2892 2.2903 2.2914 2.2925 2.2936 2.2947 2.2958 2.2969 2.2980 2.2991 2.3002 2.3013 2.3024 2.3035 2.3046 2.3057 2.3067 2.3078 2.3089 2.3100 2.3110 2.3121 2.3132 2.3142 2.3153 2.3164 2.3174 2.3185 2.3195 2.3206 2.3216 2.3227 2.3237 2.3248 2.3258 2.3269 2.3279 2.3290 2.3300 2.3310 2.3321 2.3331 2.3341 2.3352 2.3362 2.3372 2.3382 2.3392 2.3403 2.3413 2.3423 2.3433 2.3443 2.3453 2.3463 2.3473 2.3483 2.3493 2.3503 2.3513 2.3523

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2.3533
       2.3543 2.3553 2.3563 2.3573
                                     2.3583
                                             2.3593
2.3603
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                                      2.3651
2.3671
        2.3681
               2.3690
                       2.3700
                              2.3710
                                      2.3719
                                             2.3729
2.3738
        2.3748
               2.3758
                       2.3767
                              2.3777
                                      2.3786
                                             2.3796
2.3805
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               2.3824
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                              2.3843
                                      2.3852
                                             2.3862
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                              2.3909
                                      2.3918
2.3936
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                      2.3964
                              2.3973
                                      2.3983
                                             2.3992
2.4001
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                              2.4038
                                      2.4047
                                             2.4056
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               2.4083 2.4092
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2.4128
               2.4146 2.4155
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                              2.4164
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2.4190
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               2.4208
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                              2.4226
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                                     2.4296
2.4313
       2.4322
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                              2.4348
                                     2.4356 2.4365
2.4374 2.4382 2.4391 2.4399
                              2.4408
                                    2.4416 2.4425
2.4434 2.4442 2.4450 2.4459 2.4467 2.4476 2.4484
2.4493 2.4501 2.4510 2.4518
                             2.4526
                                     2.4535 2.4543
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                                     2.4593
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2.4609
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                              2.4642
                                     2.4651
2.4667 2.4675 2.4683 2.4691 2.4700
                                     2.4708
                                             2.4716
2.4724 2.4732 2.4740 2.4748
                              2.4756
                                     2.4764 2.4772
2.4780
       2.4788 2.4796 2.4804
                              2.4812
                                     2.4820
2.4836 2.4844 2.4852 2.4860
                             2.4868
                                     2.4876
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2.4892 2.4899
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                              2.4923
                                     2.4931
                                             2.4939
2.4946 2.4954 2.4962 2.4970
                              2.4978
                                     2.4985
                                             2.4993
2.5001 2.5008 2.5016 2.5024
                              2.5032
                                     2.5039
                                             2.5047
2.5055 2.5062 2.5070
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                                     2.5093
                                             2.5100
2.5108 2.5116 2.5123 2.5131
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                                     2.5146
                                            2.5153
2.5161 2.5168 2.5176 2.5183
                             2.5191 2.5198
                                             2.5206
2.5213 2.5221 2.5228 2.5236
                              2.5243
                                     2.5250
                                             2.5258
2.5265 2.5273 2.5280 2.5287
                             2.5295 2.5302 2.5309
2.5317 2.5324 2.5331 2.5339
                              2.5346 2.5353 2.5361
2.5368 2.5375 2.5382 2.5390
                              2.5397
                                     2.5404
                                            2.5411
2.5418 2.5426 2.5433
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                             2.5447
                                     2.5454 2.5462
2.5469 2.5476
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                      2.5490
                             2.5497
                                     2.5504
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2.5568
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                             2.5596 2.5603
                                            2.5610
2.5617 2.5624 2.5631 2.5638 2.5645 2.5652 2.5659
2.5665 2.5672 2.5679 2.5686 2.5693 2.5700 2.5707
2.5714 2.5721 2.5727 2.5734 2.5741 2.5748 2.5755
2.5761 2.5768 2.5775 2.5782
                             2.5789 2.5795 2.5802
2.5809 2.5816 2.5822 2.5829
                             2.5836
```

* BASIN 1

COMPUTE NM HYD

ID=1 HYD NO=100.1 AREA=0.0013491 SQ MI PER A=0.00 PER B=7.00 PER C=49.0 PER D=44.00 TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420 "

UNIT PEAK = 2.3436 CFS UNIT VOLUME = .9941 B = 526.28 P60 = 1.8700

AREA = .000594 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .109007 HR TP = .133300HR K/TP RATIO = .817760 SHAPE CONSTANT, N = 4.36

UNIT PEAK = 2.1487 CFS UNIT VOLUME = .9946 B = 379.11 P60 = 1.8700

AREA = .000755 SQ MI IA = .36875 INCHES INF = .88250 INCHES PER HOUR .
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .0333330

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 1.56168 INCHES = .1124 ACRE-FEET
PEAK DISCHARGE RATE = 3.01 CFS AT 1.500 HOURS BASIN AREA = .0013 SQ. MI.

* BASIN 2

COMPUTE NM HYD ID=2 HYD NO=100.2 AREA=0.0051852 SQ MI
PER A=0.00 PER B=3.00 PER C=28.00 PER D=69.00
TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = 14.125 CFS UNIT VOLUME = .9985 B = 526.28 P60 = 1.8700

AREA = .003578 SQ MI · IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .108298HR TP = .133300HR K/TP RATIO = .812440 SHAPE CONSTANT, N = 4.40 1502

UNIT PEAK = 4.5956 CFS UNIT VOLUME = .9977 B = 381.11 P60 = 1.8700

AREA = .001607 SQ MI IA = .36452 INCHES INF = .87065 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=2 CODE=1

PARTIAL HYDROGRAPH 100.20

RUNOFF VOLUME = 1.91473 INCHES = .5295 ACRE-FEET
PEAK DISCHARGE RATE = 12.89 CFS AT 1.500 HOURS BASIN AREA = .0052 SQ. MI.

* BASIN 3

COMPUTE NM HYD ID=3 HYD NO=100.3 AREA=0.0006062 SQ MI
PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00
TP=-0.1333 HR MASS RAINFALL=-1

K = .072649 HR TP = .133300 HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

"UNIT PEAK = 2.3933 CFS UNIT VOLUME = .9949 B = 526.28 P60 = 1.8700

AREA = .000606 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=3 CODE=1

PARTIAL HYDROGRAPH 100.30

RUNOFF VOLUME = 2.34522 INCHES = .0758 ACRE-FEET
PEAK DISCHARGE RATE = 1.70 CFS AT 1.500 HOURS BASIN AREA = .0006 SQ. MI.

* BASIN 4

COMPUTE NM HYD ID=4 HYD NO=100.4 AREA=0.0061766 SQ MI
PER A=0.00 PER B=3.00 PER C=34.00 PER D=63.00
TD=-0.1333 HB MACC BAINEALT 1

TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = 15.363 CFS UNIT VOLUME = .9987 B = 526.28 P60 = 1.8700

AREA = .003891 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .107904HR TP = .133300HR K/TP RATIO = .809482 SHAPE CONSTANT, N = 4.41 9397

UNIT PEAK = 6.5530 CFS UNIT VOLUME = .9984 B = 382.23 P60 = 1.8700

AREA = .002285 SQ MI IA = .36216 INCHES INF = .86405 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=4 CODE=1

PARTIAL HYDROGRAPH 100.40

RUNOFF VOLUME = 1.83366 INCHES = .6040 ACRE-FEET
PEAK DISCHARGE RATE = 14.99 CFS AT 1.500 HOURS BASIN AREA = .0062 SQ. MI.

* BASIN 5

*

COMPUTE NM HYD ID=5 HYD NO=100.5 AREA=0.0021416 SQ MI
PER A=0.00 PER B=4.00 PER C=43.00 PER D=53.00
TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = $\dot{}$.545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = 4.4812 CFS UNIT VOLUME = .9969 B = 526.28 P60 = 1.8700

AREA = .001135 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .108005HR TP = .133300HR K/TP RATIO = .810241 SHAPE CONSTANT, N = 4.41

UNIT PEAK = 2.8840 CFS UNIT VOLUME = .9959 B = 381.94 P60 = 1.8700

AREA = .001007 SQ MI IA = .36277 INCHES INF = .86574 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=5 CODE=1

PARTIAL HYDROGRAPH 100.50

RUNOFF VOLUME = 1.69472 INCHES = .1936 ACRE-FEET
PEAK DISCHARGE RATE = 4.99 CFS AT 1.500 HOURS BASIN AREA = .0021 SQ. MI.

* BASIN 6

COMPUTE NM HYD ID=6 HYD NO=100.6 AREA=0.0013188 SQ MI PER A=0.00 PER B=23.00 PER C=51.00 PER D=26.00

TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = 1.3537 CFS UNIT VOLUME = .9911 B = 526.28 P60 = 1.8700

AREA = .000343 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .113676HR TP = .133300HR K/TP RATIO = .852783 SHAPE CONSTANT, N = 4.17

2924

UNIT PEAK = 2.6835 CFS UNIT VOLUME = .9955 B = 366.54 P60 = 1.8700

AREA = .000976 SQ MI IA = .39662 INCHES INF = .96054 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=6 CODE=1

PARTIAL HYDROGRAPH 100.60

RUNOFF VOLUME = 1.26007 INCHES = .0886 ACRE-FEET

PEAK DISCHARGE RATE = 2.60 CFS AT 1.500 HOURS BASIN AREA = .0013 SQ. MI.

BASIN 7

COMPUTE NM HYD ID=7 HYD NO=100.7 AREA=0.0094603 SQ MI PER A=0.00 PER B=8.00 PER C=34.0 PER D=58.00 TP=-0.1333 HR MASS RAINFALL=-1

K'=.072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = 21.663 CFS UNIT VOLUME = .9988 B = 526.28 P60 = 1.8700

AREA = .005487 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .110653HR TP = .133300HR K/TP RATIO = .830101 SHAPE CONSTANT, N = 4.29

UNIT PEAK = 11.165 CFS UNIT VOLUME = .9992 B = 374.57 P60 = 1.8700

AREA = .003973 SQ MI IA = .37857 INCHES INF = .91000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=7 CODE=1

PARTIAL HYDROGRAPH 100.70

RUNOFF VOLUME = 1.74733 INCHES = .8816 ACRE-FEET

PEAK DISCHARGE RATE = 22.23 CFS AT 1.500 HOURS BASIN AREA = .0095 SQ. MI.

* BASIN 8

COMPUTE NM HYD

ID=8 HYD NO=100.8 AREA=0.003407 SQ MI PER A=0.00 PER B=23.00 PER C=22.00 PER D=55.00 TP=-0.1333 HR MASS RAINFALL=-1 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = 7.3981 CFS UNIT VOLUME = .9978 B = 526.28 P60 = 1.8700

AREA = .001874 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .118709HR TP = .133300HR K/TP RATIO = .890537 SHAPE CONSTANT, N = 3.98 2448

UNIT PEAK = 4.0715 CFS UNIT VOLUME = .9972 B = 354.00 P60 = 1.8700

AREA = .001533 SQ MI IA = .42667 INCHES INF = 1.04467 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=8 CODE=1

PARTIAL HYDROGRAPH 100.80

RUNOFF VOLUME = 1.65532 INCHES = .3008 ACRE-FEET
PEAK DISCHARGE RATE = 7.64 CFS AT 1.500 HOURS BASIN AREA = .0034 SQ. MI.

* BASIN 9

COMPUTE NM HYD ID=9 HYD NO=100.9 AREA=0.0020785 SQ MI PER A=0.00 PER B=.00 PER C=52.00 PER D=48.00 TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = .3.9389 CFS UNIT VOLUME = .9965 B = 526.28 P60 = 1.8700

AREA = .000998 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .105867HR TP = .133300HR K/TP RATIO = .794199 SHAPE CONSTANT, N = 4.51

UNIT PEAK = 3.1471 CFS UNIT VOLUME = .9963 B = 388.14 P60 = 1.8700

AREA = .001081 SQ MI IA = .35000 INCHES INF = .83000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=9 CODE=1

PARTIAL HYDROGRAPH 100.90

RUNOFF VOLUME = 1.64278 INCHES = .1821 ACRE-FEET
PEAK DISCHARGE RATE = 4.79 CFS AT 1.500 HOURS BASIN AREA = .0021 SQ. MI.

* BASIN 10

*

. .

COMPUTE NM HYD ID=10 HYD NO=100.10 AREA=0.0021317 SQ MI PER A=0.00 PER B=24.00 PER C=37.00 PER D=39.00 TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = 3.2823 CFS UNIT VOLUME = .9961 B = 526.28 P60 = 1.8700

AREA = .000831 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .115752HR TP = .133300HR K/TP RATIO = .868358 SHAPE CONSTANT, N = 4.09 1805

UNIT PEAK = 3.5239 CFS UNIT VOLUME = .9965 B = 361.24 P60 = 1.8700

AREA = .001300 SQ MI IA = .40902 INCHES INF = .99525 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=10 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 1.43351 INCHES = .1630 ACRE-FEET

PEAK DISCHARGE RATE = 4.44 CFS AT 1.500 HOURS BASIN AREA = .0021 SQ. MI.

* BASIN 11

*

COMPUTE NM HYD ID=11 HYD NO=100.11 AREA=0.0009154 SQ MI PER A=0.00 PER B=39.00 PER C=38.00 PER D=23.00 TP=-0.1333 HR MASS RAINFALL=-1

K = .072649 HR TP = .133300 HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = .83123 CFS UNIT VOLUME = .9862 B = 526.28 P60 = 1.8700

AREA = .000211 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .118593HR TP = .133300HR K/TP RATIO = .889666 SHAPE CONSTANT, N = 3.98 6608

UNIT PEAK = 1.8733 CFS UNIT VOLUME = .9936 B = 354.28 P60 = 1.8700

AREA = .000705 SQ MI IA = .42597 INCHES INF = 1.04273 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=11 CODE=1

PARTIAL HYDROGRAPH 100.11

RUNOFF VOLUME = 1.16593 INCHES = .0569 ACRE-FEET
PEAK DISCHARGE RATE = 1.70 CFS AT 1.500 HOURS BASIN AREA = .0009 SQ. MI.

* BASIN 12

. . .

COMPUTE NM HYD ID=12 HYD NO=100.12 AREA=0.0017713 SQ MI
PER A=0.00 PER B=0.00 PER C=38.00 PER D=62.00
TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420 UNIT PEAK = 4.3358 CFS UNIT VOLUME = .9969 B = 526.28 P60 = 1.8700 AREA = .001098 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .105867HR TP = .133300HR K/TP RATIO = .794199 SHAPE CONSTANT, N = 4.51 4851

UNIT PEAK = 1.9599 CFS UNIT VOLUME = .9941 B = 388.14 P60 = 1.8700

AREA = .000673 SQ MI IA = .35000 INCHES INF = .83000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=12 CODE=1

PARTIAL HYDROGRAPH 100.12

RUNOFF VOLUME = 1.83189 INCHES = .1731 ACRE-FEET
PEAK DISCHARGE RATE = 4.32 CFS AT 1.500 HOURS BASIN AREA = .0018 SQ. MI.

* BASIN 13

COMPUTE NM HYD ID=13 HYD NO=100.13 AREA=0.0010201 SQ MI PER A=0.00 PER B=0.00 PER C=35.00 PER D=65.00 TP=-0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.10 6420

UNIT PEAK = 2.6178 CFS UNIT VOLUME = .9949 B = 526.28 P60 = 1.8700

AREA = .000663 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .105867HR TP = .133300HR K/TP RATIO = .794199 SHAPE CONSTANT, N = 4.51

UNIT PEAK = 1.0396 CFS UNIT VOLUME = .9885 B = 388.14 P60 = 1.8700

AREA = .000357 SQ MI IA = .35000 INCHES INF = .83000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD ID=13 CODE=1

PARTIAL HYDROGRAPH 100.13

"RUNOFF VOLUME = 1.87244 INCHES = .1019 ACRE-FEET
PEAK DISCHARGE RATE = 2.53 CFS AT 1.500 HOURS BASIN AREA = .0010 SQ. MI.

* BASIN 14 EXCESS R.O.W.

COMPUTE NM HYD ID=14 HYD NO=100.14 AREA=0.0021522 SQ MI PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 . TP=-0.1333 HR MASS RAINFALL=-1

.133300HR K/TP RATIO = .130992HR TP =.982685 SHAPE CONSTANT, N = 3.593448 UNIT PEAK = 5.2810 CFS UNIT VOLUME = .9977 B = 327.09 P60 = 1.8700.002152 SQ MI IA = AREA = .50000 INCHES 1.25000 INCHES PER HOUR INF =

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

PRINT HYD

ID=14 CODE=1

PARTIAL HYDROGRAPH 100.14

RUNOFF VOLUME = .66738 INCHES = .0766 ACRE-FEET
PEAK DISCHARGE RATE = 2.80 CFS AT 1.533 HOURS BASIN AREA = .0022 SQ. MI.

*

*POND I

ROUTE RESERVOIR ID=20 HYD NO=200.2 INFLOW ID=11 CODE=24

OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)

0.000072.800.686.010973.301.085.027673.551.372.053473.80

* * * * * * * * * * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	72.80	.000	.00
.80	.00	72.80	.000	.00
1.60	1.27	73.39	.017	.83
2.40	.05	72.92	.003	.16
3.20	.00	72.81	.000	.01
4.00	.00	72.80	.000	.00

PEAK DISCHARGE = .857 CFS - PEAK OCCURS AT HOUR 1.67

MAXIMUM WATER SURFACE ELEVATION = 73.407

MAXIMUM STORAGE = .0181 AC-FT INCREMENTAL TIME= .033330HRS

*

PRINT HYD

ID=20 CODE=1

PARTIAL HYDROGRAPH 200.20

RUNOFF VOLUME = 1.16502 INCHES = .0569 ACRE-FEET
PEAK DISCHARGE RATE = .86 CFS AT 1.667 HOURS BASIN AREA = .0009 SQ. MI.

*
*ADD BASIN 10 TO POND I OUTFLOW

ADD HYD

ID=44 HYD NO=102.1 ID=20 ID=10

*POND H

ROUTE RESERVOIR ID=21 HYD NO=200.3 INFLOW ID=44 CODE=24

OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)

0.000070.000.686.058470.501.085.097070.751.372.143171.00

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	70.00	.000	.00
.80	.00	70.00	.000	.00

```
1.60
                4.05
                                       .075
                          70.60
                                                  .85
    2.40
                 .32
                                       .102
                          70.78
                                                 1.12
    3.20
                 .03
                          70.45
                                       .053
                                                  .62
    4.00
                                       .025
                 .02
                          70.22
                                                  .30
    4.80
                                       .012
                 .02
                          70.11
                                                  .14
    5.60
                 .02
                          70.06
                                       .007
                                                  .08
    6.40
                 .03
                          70.03
                                                  .05
                                       .004
    7.20
                 .02
                                       .003
                          70.03
                                                  .04
    8.00
                 .02
                                       .002
                          70.02
                                                  .03
    8.80
                 .02
                                       .002
                          70.02
                                                  .03
    9.60
                 .02
                                       .002
                          70.02
                                                  .02
   10.40
                                       .002
                 .02
                          70.02
                                                  .02
   11.20
                 .02
                                       .002
                          70.02
                                                  .02
                 .02
   12.00
                                       .002
                          70.01
                                                  .02
   12.80
                 .02
                          70.01
                                      .002
                                                  .02
   13.60
                 .02
                          70.01
                                      .002
                                                  .02
   14.40
                 .02
                         70.01
                                      .002
                                                  .02
   15.20
                 .02
                         70.01
                                      .001
                                                  .02
   16.00
                 .02
                         70.01
                                      .001
                                                  .02
   16.80
                 .02
                         70.01
                                      .001
                                                  .02
   17.60
                 .01
                         70.01
                                      .001
                                                  .02
   18.40
                 .01
                         70.01
                                      .001
                                                  .01
   19.20
                 .01
                         70.01
                                      .001
                                                  .01
PEAK DISCHARGE =
                         1.209 CFS - PEAK OCCURS AT HOUR
                                                              2.07
                                       (70.858)
MAXIMUM WATER SURFACE ELEVATION =
MAXIMUM STORAGE =
                                             INCREMENTAL TIME=
                           .1168 AC-FT
                                                                     .033330HRS
```

PRINT HYD

ID=21 CODE=1

PARTIAL HYDROGRAPH 200.30

RUNOFF VOLUME = 1.34544 INCHES = .2186 ACRE-FEET
PEAK DISCHARGE RATE = 1.21 CFS AT 2.066 HOURS BASIN AREA = .0030 SQ. MI.

*ADD BASIN TO POND I OUTFLOW

ADD HYD

ID=44 HYD NO=102.2 ID=21 ID=8

*POND G

ROUTE RESERVOIR ID=22 HYD NO=200.4 INFLOW ID=44 CODE=24 OUTFLOW (CFS) STORAGE(AC-FT) ELEVATION(FT)

0.000067.800.686.106168.301.085.167968.551.372.236568.801.565.296469.00

* * * * * * * * * * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	67.80	.000	.00
.80	.00	67.80	.000	.00
1.60	6.30	68.39	.129	.84
2.40	1.41	68.81	.239	1.38
3.20	.66	68.72	.215	1.28
4.00	.33	68.55	.168	1.09
4.80	.18	68.37	.122	.79
5.60	.11	68.21	.087	.56

```
6.40
                  .09
                         68.09
                                      .062
                                                 .40
      7.20
                  .08
                                      .045
                         68.01
                                                 .29
      8.00
                  .07
                                      .033
                         67.96
                                                 .22
      8.80
                  .07
                                      .025
                         67.92
                                                 .16
     9.60
                  .06
                                      .020
                         67.89
                                                 .13
    10.40
                  .06
                                      .016
                         67.88
                                                 .11
    11.20
                  .06
                         67.86
                                      .014
                                                 .09
    12.00
                  .05
                                      .012
                         67.86
                                                 .08
    12.80
                  .05
                                      .011
                         67.85
                                                 .07
    13.60 '
                  .05
                         67.85
                                      .010
                                                 .06
    14.40
                  .05
                                     .009
                         67.84
                                                .06
    15.20
                 .05
                                     .008
                         67.84
                                                .05
    16.00
                 .04
                         67.84
                                      .008
                                                .05
    16.80
                 .04
                         67.84
                                     .008
                                                .05
    17.60
                 .04
                         67.83
                                     .007
                                                .05
    18.40
            .04
                         67.83
                                     .007
                                                .04
    19.20
                 .04
                         67.83
                                     .007
                                                .04
 PEAK DISCHARGE =
                         1.381 CFS - PEAK OCCURS AT HOUR 2.43
 MAXIMUM WATER SURFACE ELEVATION =
                                         68.809
 MAXIMUM STORAGE =
                           .2392 AC-FT
                                            INCREMENTAL TIME=
                                                                 .033330HRS
*
PRINT HYD
                    ID=22 CODE=1
                                    PARTIAL HYDROGRAPH 200.40
    RUNOFF VOLUME =
                       1.49024 INCHES =
                                                    .5130 ACRE-FEET
    PEAK DISCHARGE RATE =
                                1.38 CFS AT 2.433 HOURS BASIN AREA =
                                                                            .0065 SQ. MI.
*ADD BASINS 7,9,12&13 TO POND G OUTFLOW
ADD HYD
                               ID=44 HYD NO=102.3 ID=22 ID=7 22. 2
ADD HYD
                               ID=44 HYD NO=102.4 ID=44 ID=9 4.8
ADD HYD
                               ID=44 HYD NO=102.5 ID=44 ID=12
ADD HYD
                               ID=44 HYD NO=102.6 ID=44 ID=13
PRINT HYD
                        ID=44 CODE=1
                                    PARTIAL HYDROGRAPH
                                                        102.60
    RUNOFF VOLUME =
                       1.67039 INCHES
                                                  1.8516 ACRE-FEET
    PEAK DISCHARGE RATE =
                              34.35 CFS AT
                                              1.500 HOURS BASIN AREA =
                                                                            .0208 SQ. MI.
*
*POND F
ROUTE RESERVOIR
                    ID=23 HYD NO=200.5 INFLOW ID=44 CODE=24
                   OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)
                                              .0000
                                                                  59.00
                              1.372
                                              .2714
                                                                  60.00
                              2.170
                                              .6012
                                                                 61.00
                              2.745
                                             1.0479
                                                                 62.00
                              2.796
                                             1.0989
                                                                 62.10
```

TIME INFLOW ELEV VOLUME OUTFLOW (HRS) (CFS) (FEET) (AC-FT) (CFS) .00 .00 59.00 .000 .00 .80 .00 59.00 .000 .00 1.60 24.79 60.87 .559 2.07 2.40 2.65 61.89 .998 2.68

```
3.20
                 1.50
                           61.77
                                       .947
                                                  2.61
      4.00
                                       .867
                 1.22
                           61.59
                                                  2.51
      4.80
                  .92
                           61.39
                                        .775
                                                  2.39
      5.60
                   .72
                           61.16
                                        .675
                                                  2.26
      6.40
                  .60
                           60.91
                                       .573
                                                  2.10
      7.20
                  .49
                           60.62
                                       .477
                                                  1.87
      8.00
                  .40
                           60.36
                                       .390
                                                  1.66
      8.80
                                       .311
                  .34
                           60.12
                                                  1.47
      9.60
                                       .242
                  .30
                           59.89
                                                 1.22
    10.40
                  .27
                                       .189
                           59.70
                                                   .96
    11.20
                  .24
                           59.55
                                       .150
                                                   .76
    12.00
                                       .120
                  .23
                           59.44
                                                   .61
    12.80
                                       .098
                  .21
                           59.36
                                                   .50
    13.60
                  .20
                           59.30
                                       .082
                                                   .41
    14.40
                  .19
                           59.26
                                       .070
                                                   .35
    15.20
                  .18
                           59.22
                                       .060
                                                   .30
    16.00
                  .18
                           59.20
                                       .053
                                                   .27
    16.80
                  .17
                          59.18
                                       .048
                                                   .24
    17.60
                  .16
                          59.16
                                       .044
                                                   .22
    18.40
                  .16
                          59.15
                                       .040
                                                   .20
    19.20
                  .15
                          59.14
                                       .038
                                                   .19
 PEAK DISCHARGE =
                          2.681 CFS - PEAK OCCURS AT HOUR
                                                               2.40
 MAXIMUM WATER SURFACE ELEVATION =
                                           61.888
                                           INCREMENTAL TIME=
 MAXIMUM STORAGE =
                             .9981 AC-FT
                                                                      .033330HRS
PRINT HYD
                         ID=23 CODE=1
                                     PARTIAL HYDROGRAPH
                                                            200.50
    RUNOFF VOLUME =
                         1.63841 INCHES
                                                     1.8162 ACRE-FEET
    PEAK DISCHARGE RATE =
                                 2.68 CFS
                                                 2.400 HOURS
                                                               BASIN AREA =
                                            \mathbf{AT}
                                                                                .0208 SQ. MI.
*ADD BASINS 1 THRU 6 AND BASIN 14 (EXCESS R.O.W.)
ADD HYD
                                ID=44 HYD NO=103.1 ID=1 ID=2
ADD HYD
                                ID=44 HYD NO=103.2 ID=44 ID=3
ADD HYD
                                ID=44 HYD NO=103.3 ID=44 ID=4
ADD HYD
                                ID=44 HYD NO=103.4 ID=44 ID=5
ADD HYD
                                ID=44 HYD NO=103.5 ID=44 ID=6
ADD HYD
                                ID=44 HYD NO=103.6 ID=44 ID=14
PRINT HYD
                         ID=44 CODE=1
  . .
                                     PARTIAL HYDROGRAPH
                                                          103.60
    RUNOFF VOLUME =
                         1.66458 INCHES
                                                     1.6805 ACRE-FEET
                                            =
    PEAK DISCHARGE RATE =
                                42.97 CFS
                                          AT
                                                 1.500 HOURS
                                                               BASIN AREA =
                                                                                .0189 SQ. MI.
*POND IN EXCESS ROW
ROUTE RESERVOIR
                         ID=25 HYD NO=200.7 INFLOW ID=44 CODE=24
                    OUTFLOW (CFS) STORAGE (AC-FT) ELEVATION (FT)
                                0
                                                .0000
                                                                  43.50
                                0.686
                                               0.3937
                                                                  44.00
                                1.018
                                               0.5719
                                                                  44.20
                                1.265
                                               0.7554
                                                                  44.40
                                1.472
                                               0.9442
                                                                  44.60
                                1.652
```

1.1382

44.80

1.3375

45.00

1.815

TIME INFLOW **ELEV** VOLUME OUTFLOW (CFS) (HRS) (FEET) (AC-FT) (CFS) .00 .00 43.50 .000 .00 .80 .00 .000 .00 43.50 1.60 30.57 44.35 .714 1.21 2.40 1.60 44.93 1.264 1.76 3.20 .26 44.86 1.195 1.70 4.00 .17 1.61 44.76 1.098 4.80 .16 44.66 1.005 1.53 .918 5.60 .19 44.57 1.44 6.40 .840 .25 1.36 44.49 7.20 .24 44.41 .769 1.28 8.00 .23 44.34 .703 1.19 8.80 .641 .22 44.28 1.11 9.60 .21 .584 44.21 1.03 10.40 .20 44.16 .532 .94 11.20 .19 44.10 .486 ...86 12.00 .18 .444 44.06 .78 12.80 .18 44.01 .406 .71 13.60 .17 43.97 .373 .65 14.40 .16 43.94 .343 .60 15.20 .16 .316 43.90 .55 16.00 .15 .291 43.87 .51 16.80 .15 .269 43.84 .47 17.60 .14 43.82 .249 .43 18.40 .14 43.79 .230 .40 19.20 .13 43.77 .37 . .214 PEAK DISCHARGE = 1.755 CFS -PEAK OCCURS AT HOUR 2.37 MAXIMUM WATER SURFACE ELEVATION = 44.927 1.2647 AC-FT MAXIMUM STORAGE = INCREMENTAL TIME= .033330HRS 1.3 ac. Fb with 2' of freeboad PRINT HYD ID=25 CODE=1 PARTIAL HYDROGRAPH 200.70 RUNOFF VOLUME = 1.46707 INCHES 1.4811 ACRE-FEET PEAK DISCHARGE RATE = 1.76 CFS AT 2.366 HOURS BASIN AREA = .0189 SQ. MI. *ADD POND IN EXCESS ROW OUTFLOW TO POND F OUTFLOW ADD HYD ID=44 HYD NO=200.8 ID=23 ID=25 PRINT HYD ID=44 CODE=1PARTIAL HYDROGRAPH 200.80 RUNOFF VOLUME = 1.55674 INCHES 3.2973 ACRE-FEET PEAK DISCHARGE RATE = 4.44 CFS \mathbf{AT} 2.366 HOURS BASIN AREA = .0397 SQ. MI. FINISH NORMAL PROGRAM FINISH END TIME (HR:MIN:SEC) = 10:30:54