DRAINAGE PLAN

I. INTRODUCTION AND EXECUTIVE SUMMARY

THIS PROJECT, LOCATED IN THE SOUTHEAST HEIGHTS IMMEDIATELY EAST OF ISOTOPES PARK AT THE NORTHWEST CORNER OF THE INTERSECTION OF AVENIDA CESAR CHAVEZ SE AND BUENA VISTA DRIVE SE, REPRESENTS A MODIFICATION TO AN EXISTING SITE WITHIN AN INFILL AREA. THE PURPOSE OF THIS PROJECT IS THREEFOLD:

- TO REMOVE THE EXISTING PARKING LOT ALONG BUENA VISTA DRIVE SE AND CONSTRUCT A SOUND WALL AND LANDSCAPING AT THE EAST EDGE OF THE SITE BETWEEN THE EXISTING CONCESSIONS BUILDING AND BUENA VISTA DRIVE SE
- TO CLOSE THE EXISTING DRIVEPAD SERVING THE BUENA VISTA PARKING LOT BEING REMOVED TO CONSTRUCT THE SOUND WALL AND
- CONSTRUCT A NEW PARKING LOT ENTRANCE (DRIVEPAD) AT THE SOUTHEAST CORNER OF THE SITE
- CONSTRUCT A PUBLIC SIDEWALK ALONG THE SOUTH EDGE OF THE SIDE ON THE NORTH SIDE OF AVENIDA CESAR CHAVEZ SE WITHIN APPARENT PUBLIC RIGHT-OF-WAY

THIS SUBMITTAL IS MADE IN SUPPORT OF BUILDING PERMIT APPROVAL WITHIN THE JURISDICTION OF THE CITY OF ALBUQUERQUE.

II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP, THE SITE IS LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF AVENIDA CESAR CHAVEZ SE AND BUENA VISTA DRIVE SE. THE CURRENT LEGAL DESCRIPTION IS TRACT CA, ALBUQUERQUE SPORTS COMPLEX. AS SHOWN BY PANELS 334 AND 353 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, 2012, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE. FURTHERMORE, THE LARGE DIAMETER STORM DRAIN WITHIN AVENIDA CESAR CHAVEZ SE ALLOWS FOR THE FREE DISCHARGE OF DEVELOPED RUNOFF FROM THIS SITE, THE CONCEPT ESTABLISHED BY PRIOR SUBMITTALS.

III. BACKGROUND DOCUMENTS

THE PREPARATION OF THIS PLAN RELIED UPON THE FOLLOWING DOCUMENTS AND ACTIVITIES:

- RECORD GRADING AND DRAINAGE PLAN (PHASE 1) PREPARED BY THIS OFFICE (NMPE 8547) DATED 11-14-2005 AND 01-16-2006 AND CERTIFIED 05-08-2007. THIS REFERENCED PLAN PROVIDES THE BASIS FOR THE FREE DISCHARGE OF DEVELOPED RUNOFF FROM THIS SITE TO AVENIDA CESAR CHAVEZ SE. THE RECORD PLAN ALSO DEFINED THE CURRENT ONSITE DRAINAGE BASINS WITHIN THE SITE, QUANTIFIED THE EXISTING HYDROLOGY FOR THOSE BASINS AND ESTABLISHED THE AS-CONSTRUCTED LOCATIONS, SIZES AND CAPACITIES OF THE EXISTING ONSITE PRIVATE STORM DRAIN SYSTEM CONSTRUCTED AS PART OF PHASE 1
- TOPOGRAPHIC SURVEY PREPARED BY BOHANNAN HUSTON (NMPS 18331) DATED 07-08-12. THIS REFERENCED SURVEY PROVIDES THE EXISTING CONDITIONS FOR PORTIONS OF THE SITE
- SITE VISIT ON 09-03-2012 TO VERIFY EXISTING CONDITIONS BY VISUAL SITE INSPECTION.

IV. EXISTING CONDITIONS

THE SITE PRESENTLY CONSISTS OF TWO DRAINAGE BASINS. BASIN A DRAINS TO THE EXISTING ONSITE PRIVATE STORM DRAIN SYSTEM. IT CONSISTS OF SEVEN (7) SUB-BASINS AND DISCHARGES TO THE EXISTING PUBLIC STORM DRAIN IN AVENIDA CESAR CHAVEZ SE. BASIN B DISCHARGES BY SURFACE MEANS TO AVENIDA CESAR CHAVEZ SE. BASIN B CONSISTS OF THREE (3) SUB-BASINS.

NO APPARENT OFFSITE FLOWS ENTER THE SITE CONSISTENT WITH THE RECORD GRADING AND DRAINAGE PLAN FOR PHASE 1 REFERENCED

V. DEVELOPED CONDITIONS

THE PROPOSED CONSTRUCTION CONSISTS OF THE FOLLOWING:

- REMOVAL OF THE EXISTING PARKING LOT AT THE NORTHEAST CORNER OF THE SITE AND REPLACING THOSE IMPROVEMENTS WITH LANDSCAPING AND A SOUND WALL. THIS PROPOSED WORK WILL IMPACT SUB-BASIN B-1. SUB-BASIN B-1 DECREASE IN IMPERVIOUSNESS.
- CONSTRUCTION OF A NEW PARKING LOT ENTRANCE AT THE SOUTHEAST CORNER OF THE SITE. THIS WILL ALSO OCCUR IN SUB-BASIN B-1. SUB-BASIN B-1 WILL EXPEREINCE A NET DECREASE IN IMPERVIOUSNES AND THEREFORE A NET DECREASE IN DEVELOPED RUNOFF. . CONSTRUCTION OF A SIDEWALK AT THE SOUTH EDGE OF THE SITE ALONG THE NORTH SIDE OF AVENIDA CESAR CHAVEZ WITHIN APPARENT PUBLIC RIGHT-OF-WAY. THIS WILL HAVE NO IMPACT ON THE ONSITE DRAINAGE BASINS. TO FACILITATE THE CONSTRUCTION OF THE SIDEWALK, MINOR GRADING OF THE AREA BEHIND THE CURB PLUS THE EXISTING UNDEVELOPED EARTHEN SLOPE ARE NECESSARY, THE GRADE SLOPE WILL BE STABILIZED WITH CRUSHED GRAVEL MULCH. THE GRADE SLOPE LIES WITHIN SUB-BASIN B-2.
- NO IMPROVEMENTS OR CONSTRUCTION ARE PROPOSED WITHIN BASIN A OR SUB-BASIN B-3.

THE PROPOSED GRADING AND DRAINAGE WILL NOT CHANGE THE CONDITION OF OFFSITE FLOWS; NO APPARENT OFFSITE FLOWS ENTER THE SITE

VI. GRADING PLAN

THE GRADING PLAN SHOWS 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS TAKEN FROM THE SURVEY BY BOHANNAN HUSTON (NMPS 18331), DATED 07-18-12, 2.) EXISTING STORM DRAIN IMPROVEMENTS TAKEN FROM THE RECORD GRADING AND DRAINAGE PALN CERTIFIED 05-08-2007 (NMPE 8547), 3.) PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS, 4.) THE LIMIT AND CHARACTER OF THE EXISTING AND PROPOSED IMPROVEMENTS, AND 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE PROPOSED GRADING WILL CONTINUE THE PATTERN OF DISCHARGE TO THE EXISTING ONSITE PRIVATE STORM DRAIN SYSTEM WITHIN BASIN A AND WILL ALSO CONTINUE THE FREE DISCHARGE OF DEVELOPED RUNOFF FROM BASIN B BY SURFACE METHODS TO AVENIDA CESAR CHAVEZ SE. THE GRADING AND DRAINAGE CONCEPT PROPOSED BY THIS PLAN IS CONSISTENT WITH PREVIOUSLY APPROVED PLANS FOR THIS SITE.

VII. CALCULATIONS

THE CALCULATIONS CONTAINED HEREON ANALYZE THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. AS DEMONSTRATED BY THESE CALCULATIONS, THE PROPOSED PROJECT WILL RESULT IN THE FOLLOWING:

- NO CHANGE IN THE AMOUNT OF RUNOFF GENERATED BY BASIN A OR ITS SUB-BASINS
- DECREASE IN THE AMOUNT OF RUNOFF GENERATED BY SUB-BASIN B-1
- NO CHANGE IN THE AMOUNT OF RUNOFF GENERATED BY SUB-BASIN B-2 • NO CHANGE TO SUB-BASIN B-3 (NOT PART OF THIS PROJECT)
- NET DECREASE IN THE AMOUNT OF DEVELOPED RUNOFF GENERATED BY THE OVERALL SITE

VIII. CONCLUSIONS

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVALUATIONS CONTAINED HEREIN:

- 1. THE PROPOSED IMPROVEMENTS REPRESENT MODIFICATIONS TO AN EXISTING SITE WITHIN AN INFILL AREA
- 2. THE PROPOSED IMPROVEMENTS WILL MAINTAIN EXISTING DRAINAGE PATTERNS AND CONCEPTS 3. THE PROPOSED DRAINAGE PATTERNS ARE CONSISTENT WITH PREVIOUSLY APPROVED DRAINAGE SUBMITTALS FOR THIS SITE
- 4. THE PROPOSED IMPROVEMENTS WILL RESULT IN A MINOR DECREASE IN TOTAL RUNOFF GENERATED BY THE OVERALL SITE 5. THE PROPOSED IMPROVEMENTS WILL NOT ADVERSELY IMPACT DOWNSTREAM PROPERTIES OR DOWNSTREAM DRAINAGE CONDITIONS
- 6. THE FREE DISCHARGE OF DEVELOPED RUNOFF IS APPROPRIATE BASED UPON THE PRECEDENT ESTABLISHED BY PRIOR SUBMITTALS

CALCULATIONS

2.35 IN

SITE CHARACTERISTICS

A. PRECIPITATION ZONE =

B. $P_{6,100} = P_{360} =$

C. TOTAL PROJECT AREA (A_T) = 289,650 SF 6.65 AC

II. HYDROLOGY

A. METHODOLOGY

VOLUME EQUATION

 $E_{W} = (E_{A}A_{A} + E_{B}A_{B} + E_{C}A_{C} + E_{D}A_{D})/A_{T}$ $V_{100} = (E_W/12)A_T$

PEAK DISCHARGE EQUATION $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

B. EXISTING CONDITION

	BASIN	A _T (SF/AC)	A _B (SF/AC)	A _C (SF/AC)	A _D (SF/AC)	V ₁₀₀ (AC-FT)	Q ₁₀₀ (CFS)	Q ₁₀₀ (CFS) CUM
	A-1	26,154 / 0.60	300 / 0.01	0	25,854 / 0.59	0.10	2.80	2.80
Ī	A-2	11,475 / 0.26	0	0	11,475 / 0.26	0.05	1.22	4.02
	A-3	11,475 / 0.26	0	0	11,475 / 0.26	0.05	1.22	5.24
[A-4	13,939 / 0.32	0	0	13,939 / 0.32	0.06	1.50	6.47
I	A-5	25,560 / 0.59	0	0	25,560 / 0.59	0.10	2.77	9.51
	A-6	73,927 / 1.70	0	0	73,927 / 1.70	0.30	7.99	17.50
	A -7	37,629 / 0.86	0	0	37,629 / 0.86	0.15	4.04	21.54
	B-1	78,408 / 1.80	8,690 / 0.20	0	69,718 / 1.60	0.30	7.98	N/A
	B-2	110,209 / 2.53	0	101,819 / 2.34	8,390 / 0.19	0.24	8.15	N/A
[B-3	113,256 / 2.60	0	113,256 / 2.60	0	0.24	8.16	N/A
I	TOTAL	502,029 / 11.5	8,990 / 0.2	223,465 / 5.1	269,580 / 6.2	1.6	45.8	N/A

C. DEVELOPED CONDITION

BASIN	A _T (SF/AC)	A _B (SF/AC)	A _C (SF/AC)	A _D (SF/AC)	V ₁₀₀ (AC-FT)	Q ₁₀₀ (CFS)	Q ₁₀₀ (CFS) CUM
A-1			NO CHANGE	O CHANGE			2.80
A-2			NO CHANGE			1.22	4.02
A-3			NO CHANGE			1.22	5.24
A-4			NO CHANGE			1.50	6.47
A-5			NO CHANGE			2.77 9	9.51
A-6			NO CHANGE			7.99	17.50
A-7			NO CHANGE			4.04	21.54
B-1	78,408 / 1.80	16,350 / 0.38	0	62,058 / 1.42	0.27	7.54	N/A
B-2	-	NO CHANGE					N/A
B-3		THIS PROJECT					

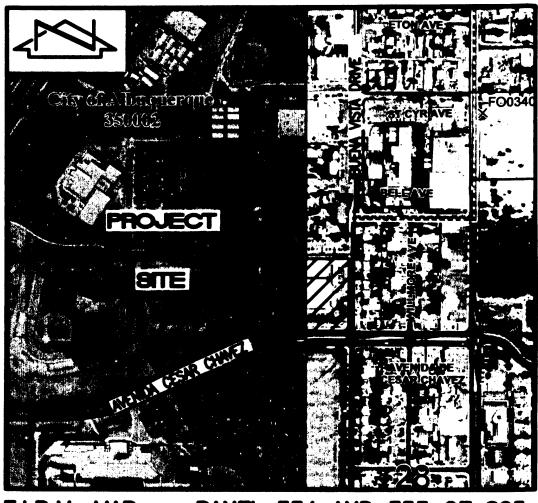
D. COMPARISON

BASIN	V	OLUME (AC-F	Γ)	PEAK DISCHARGE (CFS)					
1 .	EXIST	DEV	Δ	EXIST	DEV	Δ			
A-1			NO C	HANGE	· · · · · · · · · · · · · · · · · · ·				
A-2			NO C	HANGE					
A-3			NO C	HANGE					
A-4			NO C	HANGE					
A-5			NO C	HANGE	· · · · · · · · · · · · · · · · · · ·				
A-6			NO C	HANGE					
A-7	NO CHANGE								
B-1	0.30	0.27	-0.03	7.98	7.54	-0.44			
B-2			NO C	HANGE	·				
B-3		BASIN NOT INCLUDED IN THIS PROJECT							
TOTAL -0.03									

NOTE:

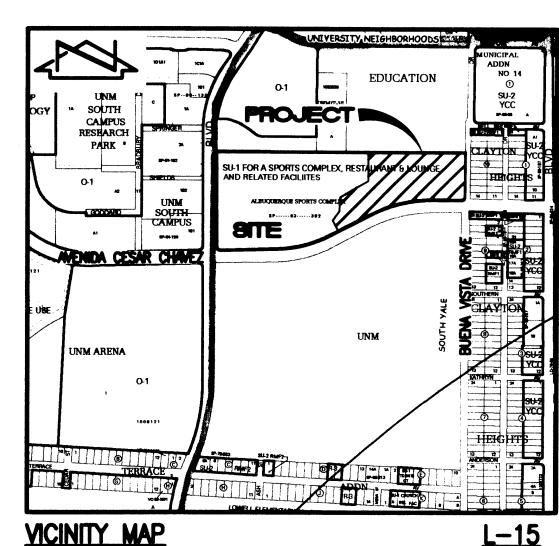
REFER TO SHEET C-6 FOR DRAINAGE BASIN BOUNDARIES

FEDERAL EMERGENCY MANAGEMENT AGENCY



SCALE: 1'' = 500'

PANEL 334 AND 353 OF 825 DATED 8/16/2012 AND 9/26/2012



CHERRY/SEE/REAMES

ARCHITECTS, LLP

220 gold avenue sw albuquerque, nm 87102 505 - 842 - 1278 fax 505 - 766 - 9269

SCALE: 1" = 750'

CONSTRUCTION NOTES:

- 1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM 260-1990 (ALBUQUERQUE AREA), 1-800-321-ALERT(2537) (STATEWIDE), FOR LOCATION OF EXISTING
- 2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM WRITING SO THAT THE WRITING SO THAT THE WRITING SO THAT THE WRITING SO THE WRITING SO THAT THE WRITING SO THE WRI AMOUNT OF DELAY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL INTERPRETATIONS IT MAKES WITHOUT FIRST CONTACTING THE ENGINEER AS REQUIRED ABOVE.
- 3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH
- 4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND
- 5. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE. MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES. PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
- 6. THE DESIGN OF PLANTERS AND LANDSCAPED AREAS IS NOT PART OF THIS PLAN. ALL PLANTERS AND LANDSCAPED AREAS ADJACENT TO THE BUILDING(S) SHALL BE PROVIDED WITH POSITIVE DRAINAGE TO AVOID ANY PONDING ADJACENT TO THE STRUCTURE. FOR CONSTRUCTION DETAILS, REFER TO LANDSCAPING PLAN.

EROSION CONTROL MEASURES:

- 1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY.
- 2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
- 3. WHEN APPLICABLE. CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" FROM THE CITY AND/OR FILE A NOTICE OF INTENT (N.O.I.) WITH THE EPA PRIOR TO BEGINNING CONSTRUCTION.
- 4. UNLESS FINAL STABILIZATION IS OTHERWISE PROVIDED FOR, ANY AREAS OF EXCESS DISTURBANCE (TRAFFIC ACCESS, STORAGE YARD, EXCAVATED MATERIAL, ETC.) SHALL BE RE-SEEDED ACCORDING TO C.O.A. SPECIFICATION 1012 "NATIVE GRASS SEEDING". THIS WILL BE CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT WILL BE MADE.

LEGAL DESCRIPTION:

TRACT CA, ALBUQUERQUE SPORTS COMPLEX.

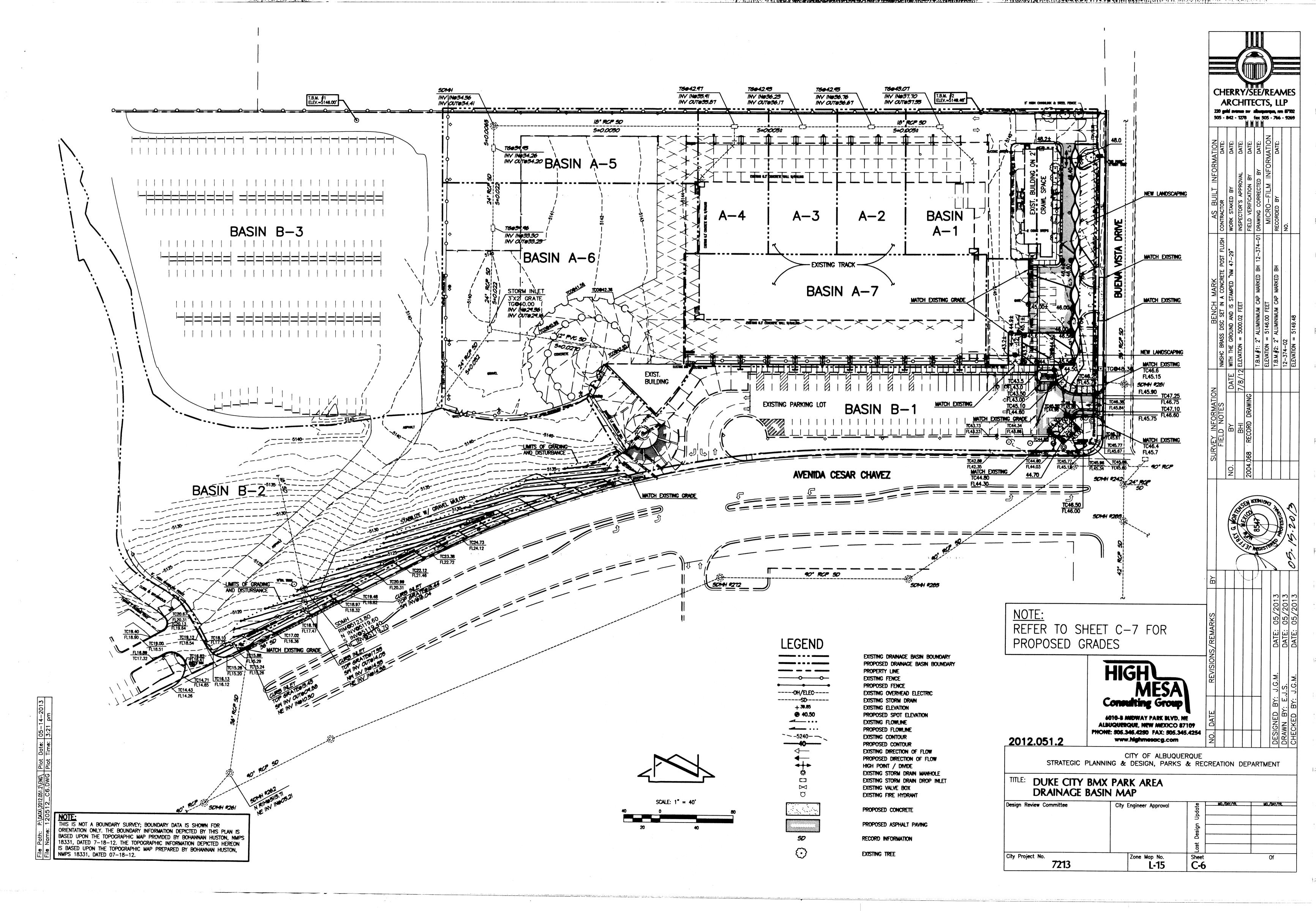


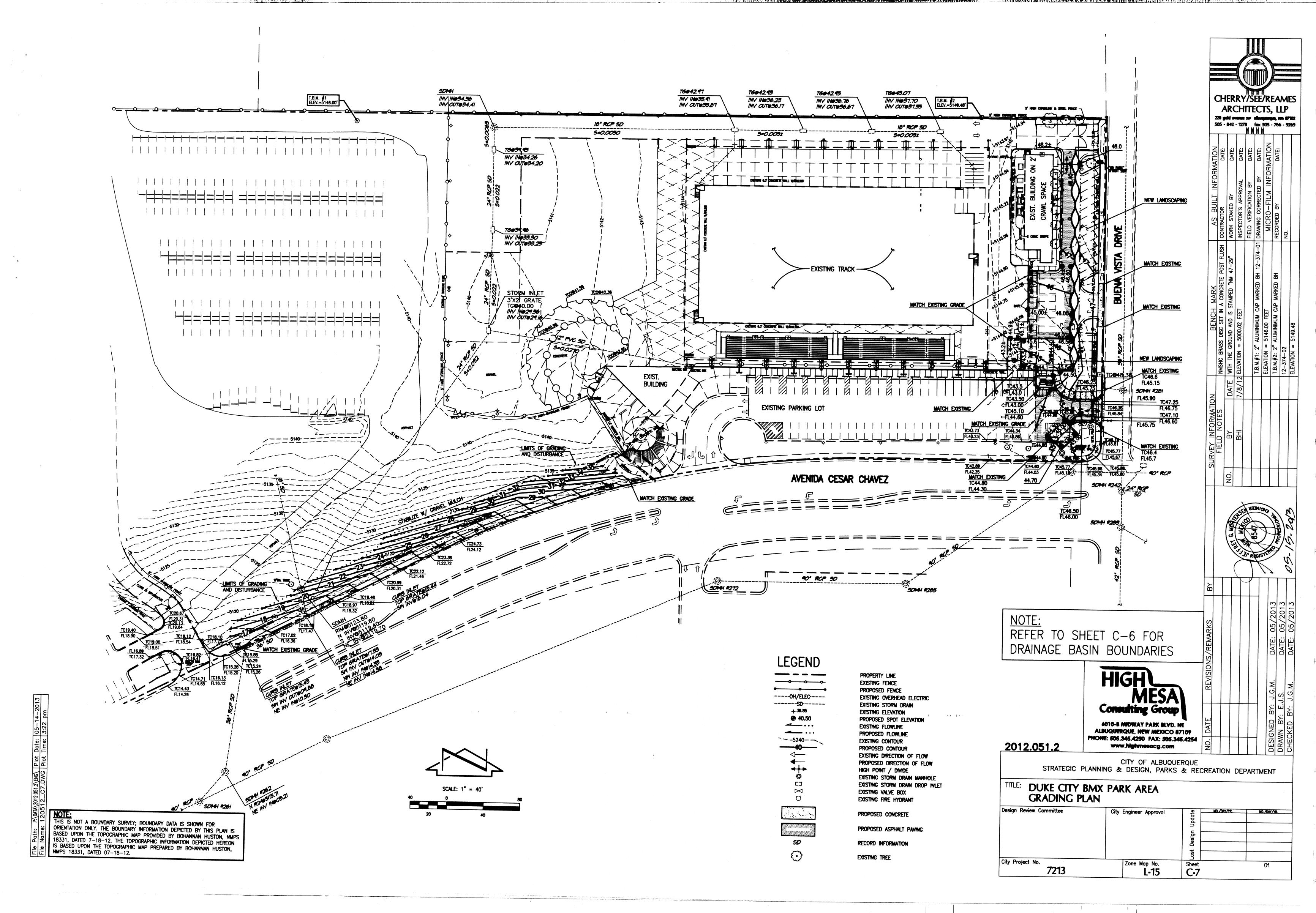
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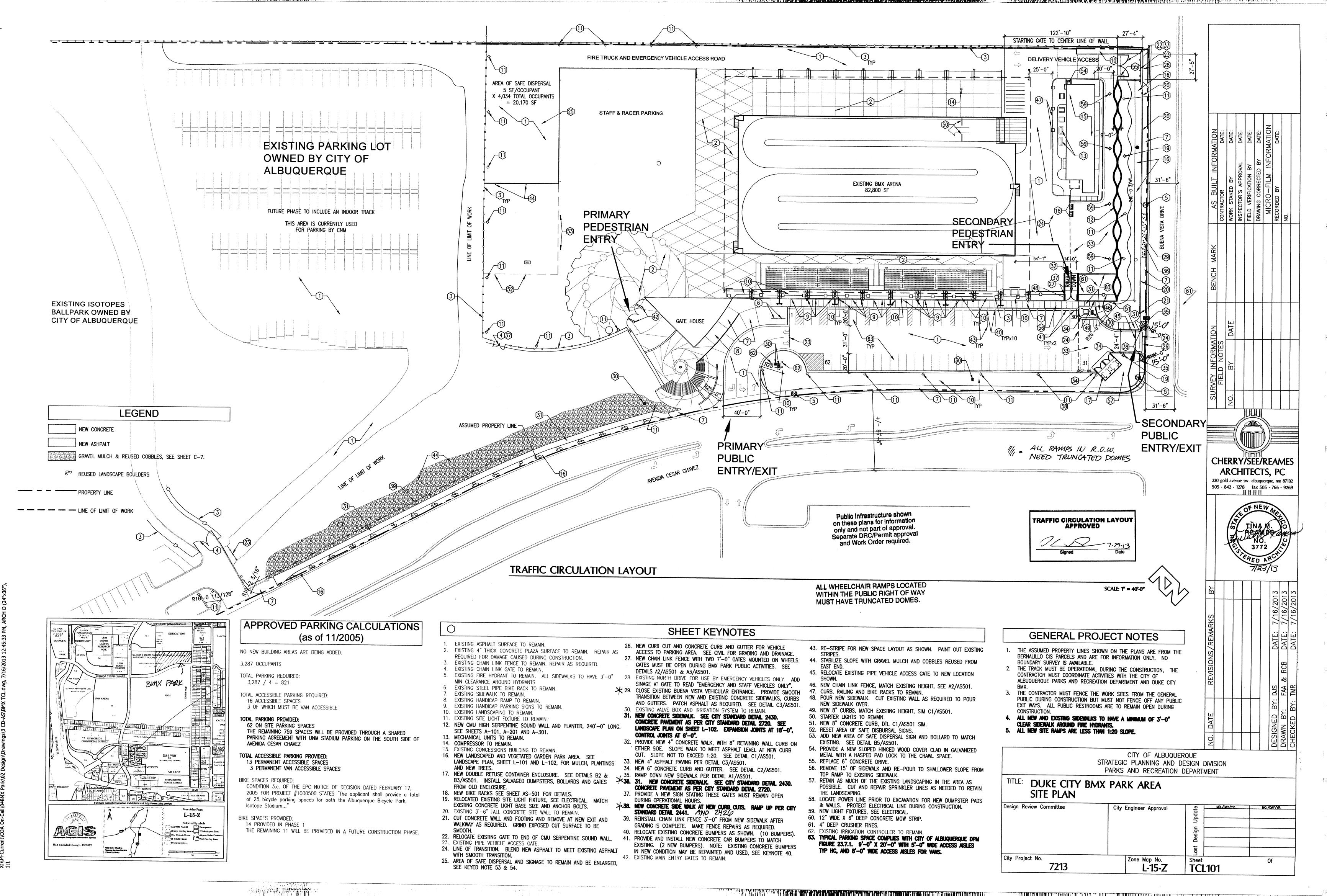
CITY OF ALBUQUERQUE STRATEGIC PLANNING & DESIGN, PARKS & RECREATION DEPARTMENT

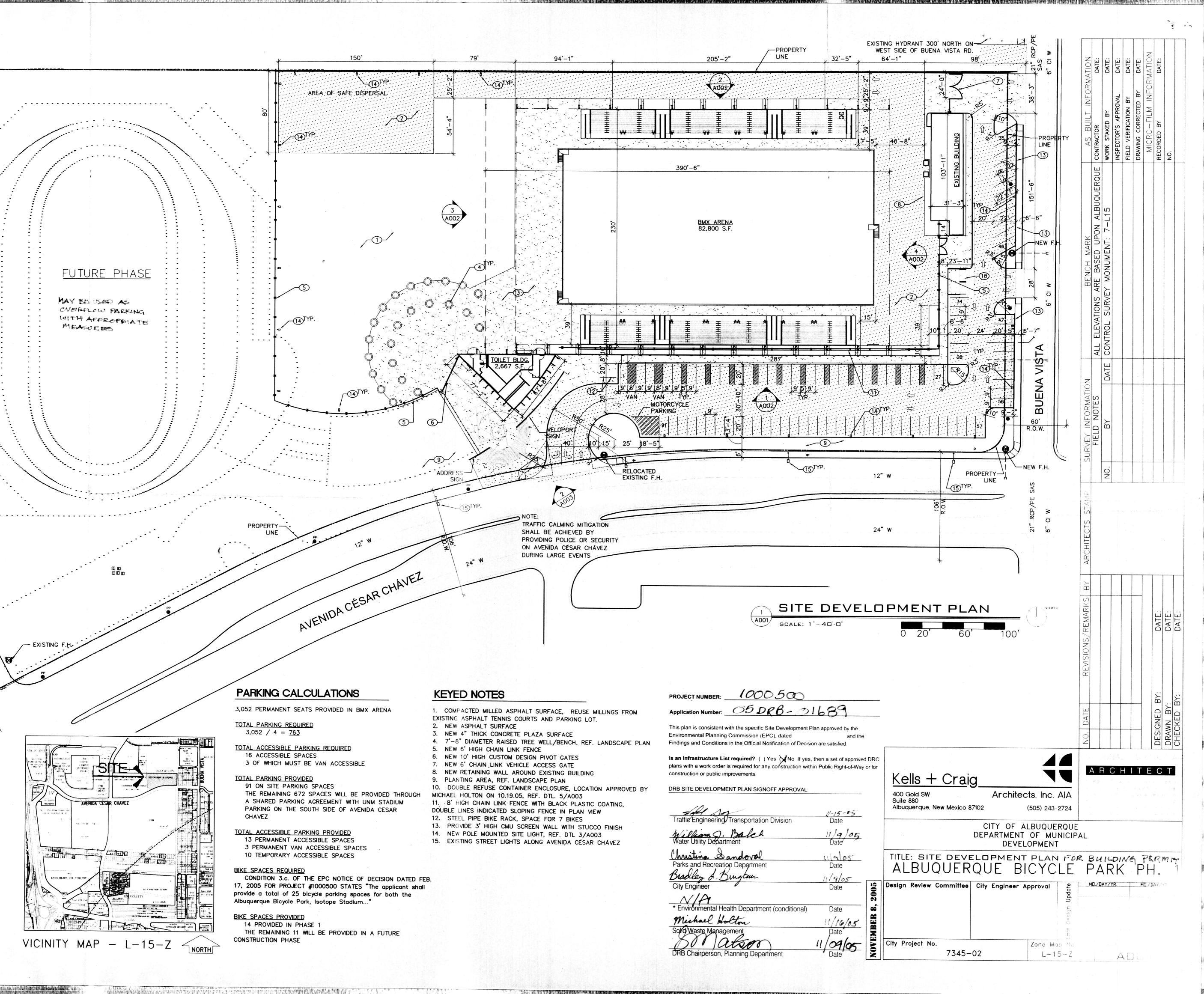
DUKE CITY BMX PARK AREA

esign Review Committee	City Engineer Approval	ast Design Update	MO./DAY/YR.	MO./DAY/YR
ity Project No. 7213	Zone Map No. L-15	Sheet C-5		Of









THIS SUBMITTAL IS MADE IN SUPPORT OF WORK ORDER AND BUILDING PERMIT WITHIN THE JURISDICTION OF THE CITY OF ALBUQUERQUE. THIS PLAN SUPERCEDES THE PRIOR CONCEPTUAL PLAN DATED 12-17-2004.

II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP, THE SITE IS LOCATED ON THE NORTH SIDE OF AVENIDA CESAR CHAVEZ BETWEEN UNIVERSITY BLVD. SE AND BUENA VISTA DRIVE SE. AT PRESENT, THE SITE IS UNDEVELOPED. THE LAND IMMEDIATELY TO THE WEST IS DEVELOPED AS ISOTOPES BALL PARK. THE LAND TO THE NORTH IS INSTITUTIONAL (TVI AND UNM). THE LAND TO THE EAST IS DEVELOPED AS CITY TENNIS COURTS. AS SHOWN BY PANEL 334 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, AND INCORPORATED AREAS DATED SEPTEMBER 20, 1996, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE (ZONE AO). AN AO ZONE IS MAPPED DOWNSTREAM, HOWEVER, IT IS BELIEVED THAT THE MORE RECENT STORM DRAIN IMPROVEMENTS WITHIN AVENIDA CESAR CHAVEZ (FORMERLY STADIUM BLVD.) HAVE ALLEVIATED THE DOWNSTREAM FLOODING.

M. BACKGROUND DOCUMENTS & RESEARCH

THE FOLLOWING ITEMS WERE REVIEWED IN THE PREPARATION OF THIS SUBMITTAL: A. TOPOGRAPHIC SURVEY OF THE EXISTING SITE PREPARED BY WILSON & CO. DATED 10/21/2004. THE SUBJECT SURVEY SHOWS THE EXISTING IMPROVEMENTS.

B. PRE-DESIGN CONFERENCE WITH THE CITY HYDROLOGIST 08/24/2004. THE OUTCOME OF THAT RECAP CONFIRMED THAT FREE DISCHARGE TO AVENIDA CESAR CHAVEZ IS APPROPRIATE IN LIGHT OF THE RECENT DRAINAGE IMPROVEMENTS TO UNIVERSITY BLVD SE AND AVENIDA CESAR CHAVEZ SE. C. CONCEPTUAL GRADING AND DRAINAGE PLAN (L15/D13A) PREPARED BY THIS OFFICE DATED 12-17-2004. THIS PRIOR SUBMITTAL ESTABLISHED THE DRAINAGE CRITERIA (FREE DISCHARGE) FOR THIS SITE.

IV. EXISTING CONDITIONS

THE TOPOGRAPHIC DATA PRESENTED HEREWITH, TAKEN FROM THE ABOVE REFERENCED SURVEY BY WILSON & CO., DEMONSTRATES THE EXISTING CONDITIONS OF THE PROJECT SITE. AT PRESENT, THE SITE IS PARTIALLY DEVELOPED WITH CITY TENNIS COURTS AND LIMITED ASPHALT PAVING. THE SITE CURRENTLY DRAINS FROM EAST TO WEST WITH MUCH OF THE DEVELOPED FLOW BEING DIRECTED INTO A TEMPORARY DETENTION POND. IT IS BELIEVED THAT THE TEMPORARY POND WAS CREATED TO MITIGATE THE DISCHARGE OF EXCESS RUNOFF ONTO THE ISOTOPES BALL PARK. THE SITE IS SITUATED TOPOGRAPHICALLY HIGHER THAN AVENIDA CESAR CHAVEZ SE AND AS SUCH DRAINS TO THAT PUBLIC ROADWAY. ISOTOPES BALL PARK IS TOPOGRAPHICALLY LOWER AN HENCE DOES NOT CONTRIBUTE OFFSITE FLOWS. THE INSTITUTIONAL DEVELOPMENT TO THE NORTH HAS BEEN GRADED SUCH THAT IT DOES NOT DRAIN ONTO THE SUBJECT SITE. ITS FLOWS ARE DIRECTED WEST AND EVENTUALLY DISCHARGE TO UNIVERSITY BLVD SE. BUENA VISTA DRIVE LIES TO THE EAST AND APPEARS TO CONTAIN ITS FLOWS AND AS SUCH IS NOT BELIEVED TO CONTRIBUTE OFFSITE FLOWS.

V. PROPOSED CONDITIONS (PHASE 1)

THE PROPOSED IMPROVEMENTS CONSIST OF A BMX TRACK, A NEW BUILDING, PARKING IMPROVEMENTS, PEDESTRIAN PAVING AND ASSOCIATED LANDSCAPING. IN ORDER TO LOCATE THE ABOVE REFERENCED IMPROVEMENTS ON THE SITE, THE EXISTING TENNIS COURTS AND RELATED IMPROVEMENTS MUST BE DEMOLISHED. THE TEMPORARY DETENTION POND WILL REMAIN. THE EXISTING BUILDING AT THE NORTHEAST CORNER WILL BE FOR THE PURPOSES OF ANALYSIS, THE SITE HAS BEEN DIVIDED INTO TWO (2) BASIC DRAINAGE AREAS. BASIN

A DRAINS INTERNALLY TO A PRIVATE STORM DRAIN SYSTEM THAT ULTIMATELY DISCHARGES TO THE AVENIDA CESAR CHAVEZ STORM DRAIN. PRESENTLY, THE TEMPORARY DETENTION POND OUTLETS TO THE AVENIDA CESAR CHAVEZ STORM DRAIN VIA AN 18-INCH STORM DRAIN CONNECTION TO THE BACK OF A EXISTING STORM INLET IN THE NORTH CURB LINE OF THE ROADWAY. THAT CONNECTION WILL BE UTILIZED FOR THE DISCHARGE OF RUNOFF FROM BASIN A AND ITS SUB-BASINS A-1 THROUGH A-6. THE RUNOFF WILL FLOW A CROSS THE NEW PARKING LOT TO EVENTUALLY DISCHARGE TO AVENIDA CESAR CHAVEZ. BASIN B IS AN EXISTING CONDITION WHERE NO CHANGES ARE PROPOSED. BASIN B INCLUDES THE EXISTING DETENTION POND.

VI. GRADING PLAN

THE GRADING PLAN SHOWS 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 2.) PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS, 3) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, AND 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE SITE WILL DRAIN BY VARIOUS MEANS. BASIN A, AS INDICATED ABOVE, WILL DRAIN VIA AN INTERNAL PRIVATE STORM DRAIN SYSTEM INTEGRATED WITH A WATER HARVESTING FEATURE. BASIN B WILL CONTINUE TO DRAIN IN THE EXISTING CONDITION UNTIL A SUBSEQUENT PHASE DEVELOPS ON THIS REMAINING PORTION OF THE SITE.

VII. CALCULATIONS

THE CALCULATIONS THAT APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS, FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. THE RESULTS OF THE CALCULATIONS FOR THE DEVELOPED CONDITION ARE PRESENTED IN TABLE FORM. KEY ANALYSIS POINTS, SHOWN ON THE DRAINAGE BASIN MAP PLAN, ARE FURTHER ANALYZED IN ORDER THAT THE PROPOSED PRIVATE STORM DRAIN BE SIZED APPROPRIATELY. THE MANNING EQUATION WAS USED TO EVALUATE PIPE SIZES AND RELATED CAPACITIES. AS A RESULT OF THIS ANALYSIS, THE TOTAL PEAK DISCHARGE CALCULATED FOR BASIN A (16.95 CFS) WILL NOT EXCEED THE CAPACITY (44.4 CFS) OF THE EXISTING 18-INCH STORM DRAIN CONNECTION TO AVENIDA CESAR CHAVEZ.

VIII. CONCLUSIONS

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVALUATIONS CONTAINED

- 1. THIS SUBMITTAL FOLLOWS THE DRAINAGE CONCEPTS ESTABLISHED BY THE PREVIOUSLY APPROVED
- CONCEPTUAL PLAN
- 2. THE SITE DOES NOT LIE WITHIN A DESIGNATED 100-YEAR FLOODPLAIN 3. THIS SITE IS NOT RESTRICTED BY LIMITED OR INADEQUATE DOWNSTREAM CAPACITY
- 4. THE INCREASED RUNOFF FROM THIS SITE WILL BE HANDLED BY THE EXISTING DOWNSTREAM PUBLIC DRAINAGE IMPROVEMENTS THAT ULTIMATELY DISCHARGE TO THE SOUTH DIVERSION CHANNEL
- 5. THIS SUBMITTAL IS FOR BUILDING PERMIT APPROVAL.
- 6. SITE SPECIFIC DRAINAGE SUBMITTALS WILL BE REQUIRED FOR SUBSEQUENT PHASES OF CONSTRUCTION 7. OFFSITE FLOWS DO NOT IMPACT THIS SITE

CALCULATIONS

- I SITE CHARACTERISTICS
- A) PRECIPITATION ZONE = 2
- B) P6,100 = P360 = 2.35 INCHES
- C) TOTAL AREA (AT) = 50,1900 SF/11.52 ACRES
- D) EXISTING LAND TREATMENT

TREATMENT AREA (SF/AC) %

a) TREATMENT B = 7,525/0.17 02 b) TREATMENT C = 316.573/7.2763c) TREATMENT D = 177,802/4.08 35

II EXISTING CONDITION

- WEIGHTED E (EW) = (EAAA+EBAB+ECAC+EDAD)/AT
- \ddot{i}) EW = [0.78(0.17)+1.13(7.27)+2.12(4.08)]/11.52 = 1.48
- iii) V360 = EW * AT /12iv) V360 = 1.48 * 11.52/12 = 1.42 ACRE-FEET = 61,890 CUBIC FEET

B) PEAK DISCHARGE

TOTAL QP = QPAAA+QPBAB+ QPCAC+QPDAD ii) TOTAL QP = 2.28(0.17)+3.14(7.27)+4.70(4.08) = 42.4 CFS

III DEVELOPED CONDITION

THE DEVELOPED CONDITION HAS BEEN INCORPORATED INTO A TABLE FORMAT AS PRESENTED BELOW.

			PHASE 1	BASIN A	NALYSIS		
BASINS	A _T (SFIAC)	A _B (sf/AC)	A _C (SF/AC)	A _D (SFIAC)	V ₁₀₀ (AC-FT)	Q ₁₀₀ (CFS)	Q ₁₀₀ (crs) CUM
A-1	26,154/0.60	300/0.01		25,854/0.59	0.10	2.80	2.80
A-2	11,475/0.26			11,475/0.26	0.05	1.22	4.02
A-3	11,475/0.26			11,475/0.26	0.05	1.22	5.24
A-4	13,939/0.32			13,939/0.32	0.06	1.50	6.74
A5	25,560/0.59			25,560/0.59	0.10	2.77	9.51
A-6	73,927/1.70			73,927/1.70	0.30	7.99	17.50
A-7	37,629/0.86			37,629/0.86	0.15	4.04	21.54
B-1	113,256/2.60		113,256/2.60		0.24	8.16	
B-2	110,209/2.53		110,209/2.53		0.24	8.15	
B-3	78,406/1.80	8,690./0.20		69,718/1.60	0.30	7.98	
TOTAL	502,029/11.5	8,990/0.2	223,465/5.1	269,580/6.2	1.6	45.8	

IV COMPARISON

a) VOLUME

 \triangle V100 = 1.57-1.42 = 0.15 AC-FT

b) PEAK DISCHARGE

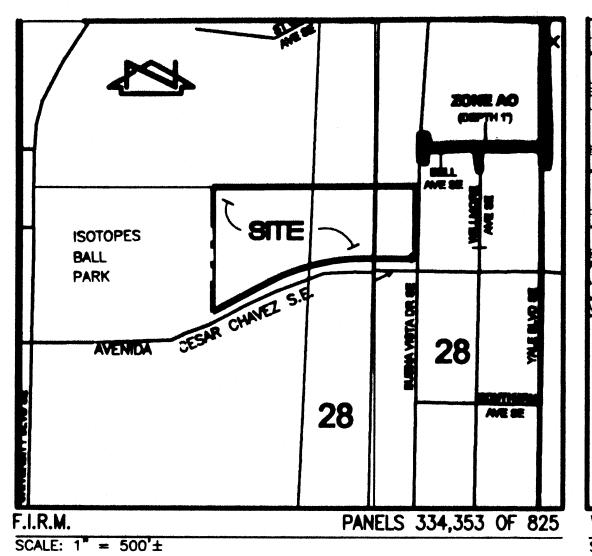
 \triangle Q100 = 45.2-42.4 = 2.8 CFS

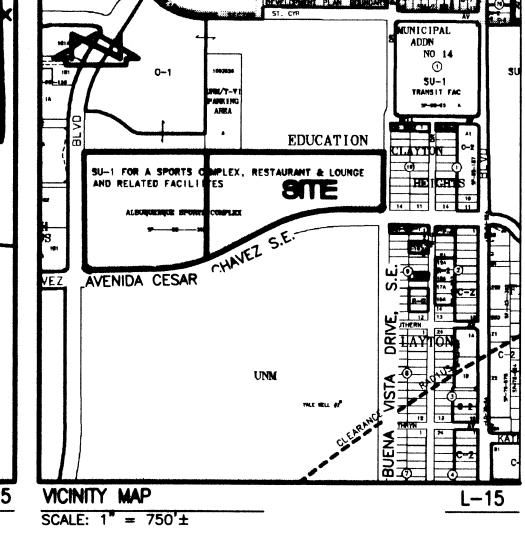
			DRAINAGE	STRUCTURE SUMM	ARY TABLE	PHASE.	1
		KEY *	AP	STRUCTURE	RIM/TG	INV. IN	INV. OU
		1	# 1	EXIST. SDMH	5123.09'	04 5119.90	18.54 5119.74
		2	# 2	SINGLE 'D' INLET * *	5140.00'	29.56 51 34:0 5'	24.16 51 30.9 5
	A	3	N/A	SINGLE 'D' INLET * *	39.96 514 0.00 '	55.50 5132.81	55.2 51 32.71
	A	4	# 3	SINGLE 'D' INLET * *	<i>34.4</i> 5 514 0.80 '	26 5134. 0 5'	54.2 51 33.9 5
	A	5	# 4	4' SDMH ***	. 9 / 5140. 78 '	56 5134 ,3 5	<i>41</i> 5134. 95
7	ß	6	# 5	SINGLE 'D' INLET * *	<i>42.</i> 97 514 3.00 '	<i>9</i> 5.41 51.36.75'	5135.05
7	A	7	# 6	SINGLE 'D' INLET * *	<i>42.98</i> 51 43.80 '	<i>25</i> 5136.26'	77 5136.45
١	A	8	\$ 7	SINGLE 'D' INLET * *	<i>42.4</i> 5 51 43.00 '	76 5136. 2 5'	<i>67</i> 5136.68
7	A	9	#8	SINGLE 'D' INLET * *	. <i>01</i> 5143 .06 '	<i>10</i> 5137. 25	<i>55</i> 5137. <i>3</i> 5
•		10	\$ 9	DELETE SOMH & REPLACE W/ 12" 11 1/4" ELL 4 SOMH **	N/A -5143.70'	5136.70 5130.70	5136.70 5130.60
		11	N/A	12" CAP	N/A	N/A	5142.00

- * REFER TO SHEETS C102 & C103
- * * CONSTRUCT PER C.O.A. STD. DWG. 2206 * * CONSTRUCT PER C.O.A. STD. DWG. 2101

AREA DRAIN KEYED NOTES:

- 12. INSTALL 1-3" 45deg PVC ELL WITH WATERTIGHT GASKETS, INV 5136.40
- 13. INSTALL 1-3" 45deg PVC ELL WITH WATERTIGHT GASKETS, INV @ 5137.95 14. INSTALL 1-3" 45deg PVC ELL WITH WATERTIGHT GASKETS, INV @ 5137.75
- 15. INSTALL 3" PVC (SDR-35) DRAIN LINE @ S=0.0320
- 16. INSTALL 3" PVC (SDR-35) DRAIN LINE @ S=0.0100
- 17. FOR CONTINUATION, SEE SHEET M101; INV @ 5138.00





			F	PHASE 1	STORM	DRAIN HYD	DRAULICS		
		AP TO AP	PIPE SIZE	LENGTH	SLOPE	Q100 (CFS)	QCAP (CFS)	VEL (FPS)	PIPE INVERTS
		CURB INLET TO AP1	18"	30'	0.179	37.6	44.4	25.2	5 14.53' TO 5119.90'
		AP1 TO AP2	24"	350'	0.032	28.36	40.5	12.9	5119390' TO 5171.00'
		AP2 TO AP3	24"	191-210		\$	27.7 34	9.1	5131.00' TO 5/34.00'
_		AP3 TO AP4	24"	-17-25	D812 0.0	6.74	24.8 18.5	7.9	5134.00\ TO 5134.20'
lack	_	AP4 TO AP5	18"	260'	0.0050	6.74	7.5 🗸	4.2	5134.20' (7 5137.00'
Λ		AP5 TO AP6	18"	-73' 74	D.0055 O.	<i>000</i> 5 5.24	20 6.4	4.5	5137.00' 70 5137.70'
$\bigwedge_{i=1}^{n} \bigwedge_{j=1}^{n}$		AP6 TO AP7	.18"	75 TS	0.0055 🗸	4.02	8.0 🗸	4.5	5137.70'/ TO \5138.40'
$\triangle \Lambda$		AP7 TO AP8	18"	73' 76	D.0065	2.80	20 10.8	4.5	5138.40 TO 5139.10'
					0.010				
	Æ	AP2 TO AP9	12"	210°	0.0270 -0.041	4.04	5.9 7.1	N/A 12	51,51.00' TO 5139,60
	A	AP9 TO AP10	12" 18 "	330'	0.0130 -0.007	4.04	4.1 -9.0	N/A -5	5136.70 5141.00 5139.70' TO 5142.0'

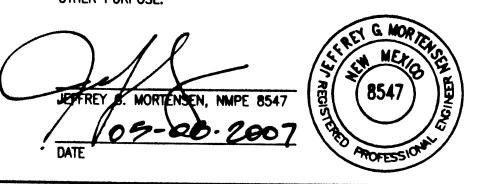
NOTES: 1. ALL LENGTHS ABOVE DETERMINED FOR HYDRAULIC ANALYSIS PURPOSES. AS-BUILT DISTANCES MAY VARY. 2. INLETS RECEIVING ROOF RUNOFF AND CONNECTING TO NEW 12" SD BETWEEN AP-9 AND AP-10 SHALL CONNECT TO TOP OF 12" PIPE AND SERVE AS CLEANOUTS FOR FUTURE MAINTENANCE

REFER TO SUMMARY TABLE @ LEFT

DRAINAGE CERTIFICATION

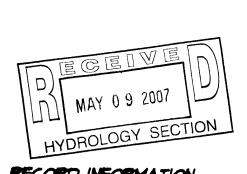
I, JEFFREY G. MORTENSEN. NMPE 8547. OF THE FIRM JEFF MORTENSEN & ASSOCIATES, INC., HEREBY CERTIFY THAT THOSE PORTIONS OF THIS PROJECT COMPLETED AS OF THE DATE OF THE AS-BUILT SURVEY HAVE BEEN GRADED AND DRAINED IN SUBSTANTIAL COMPLIANCE WITH AND IN ACCORDANCE WITH THE DESIGN INTENT OF THE APPROVED PLAN DATED 11-14-2005. 01-16-2006 AND 08-24-2006. THE RECORD INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY JOAQUIN ARGUELLES, NMPS 7472, DATED 04-17-07. BASED UPON MY VISUAL INSPECTION OF THE SITE AND REVIEW OF THE AS-BUILT SURVEY DATA, THE INFORMATION PRESENTED HEREON IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR PERMANENT CERTIFICATE OF OCCUPANCY.

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE WHICH IS BEYOND THE SCOPE OF GRADING AND DRAINAGE. THOSE RELYING ON THIS RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE.



RECORD DRAWING





RECORD INFORMATION RECORD INFORMATION FOR TEMP C.O.

RECORD INFORMATION FOR FINAL C.O. RECORD DATA . AS-DESIGNED

Kelle	4	Crain	

VCIIO T CI OIM Architects, Inc. AIA 400 Gold SW Sulle SSD Albuquerque, New Mexico 87102

TEL 8 0 8 . 2 4 3 . 8 1 0 0 (505) 243-2724 FAX 508.2 4 3.8101

CITY OF ALBUQUERQUE DEPARTMENT OF MUNICIPAL DEVELOPMENT

TITLE: PHASE 1 DRAINAGE PLAN AND CALCULATIONS ALBUQUERQUE BICYCLE PARK PH.

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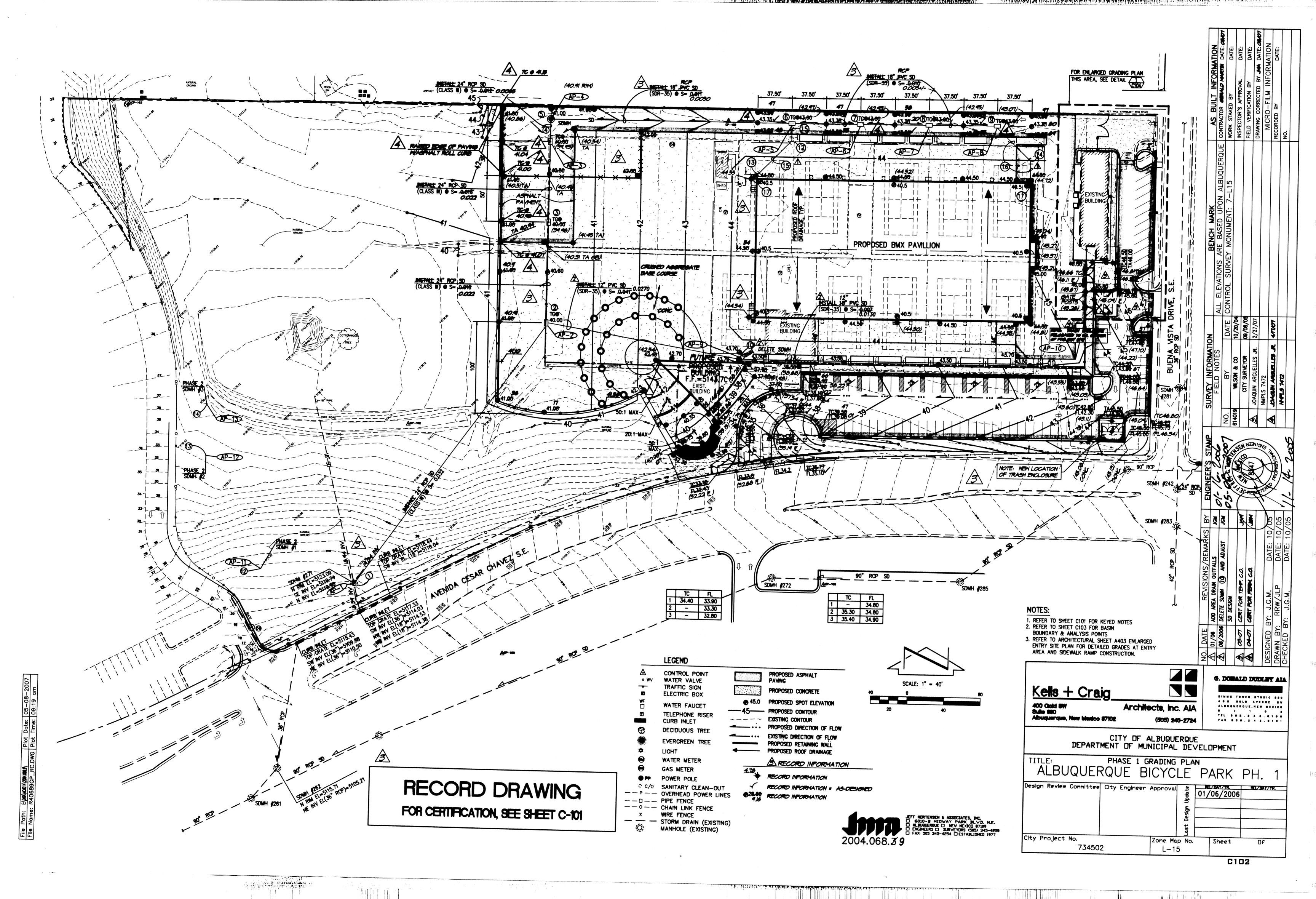
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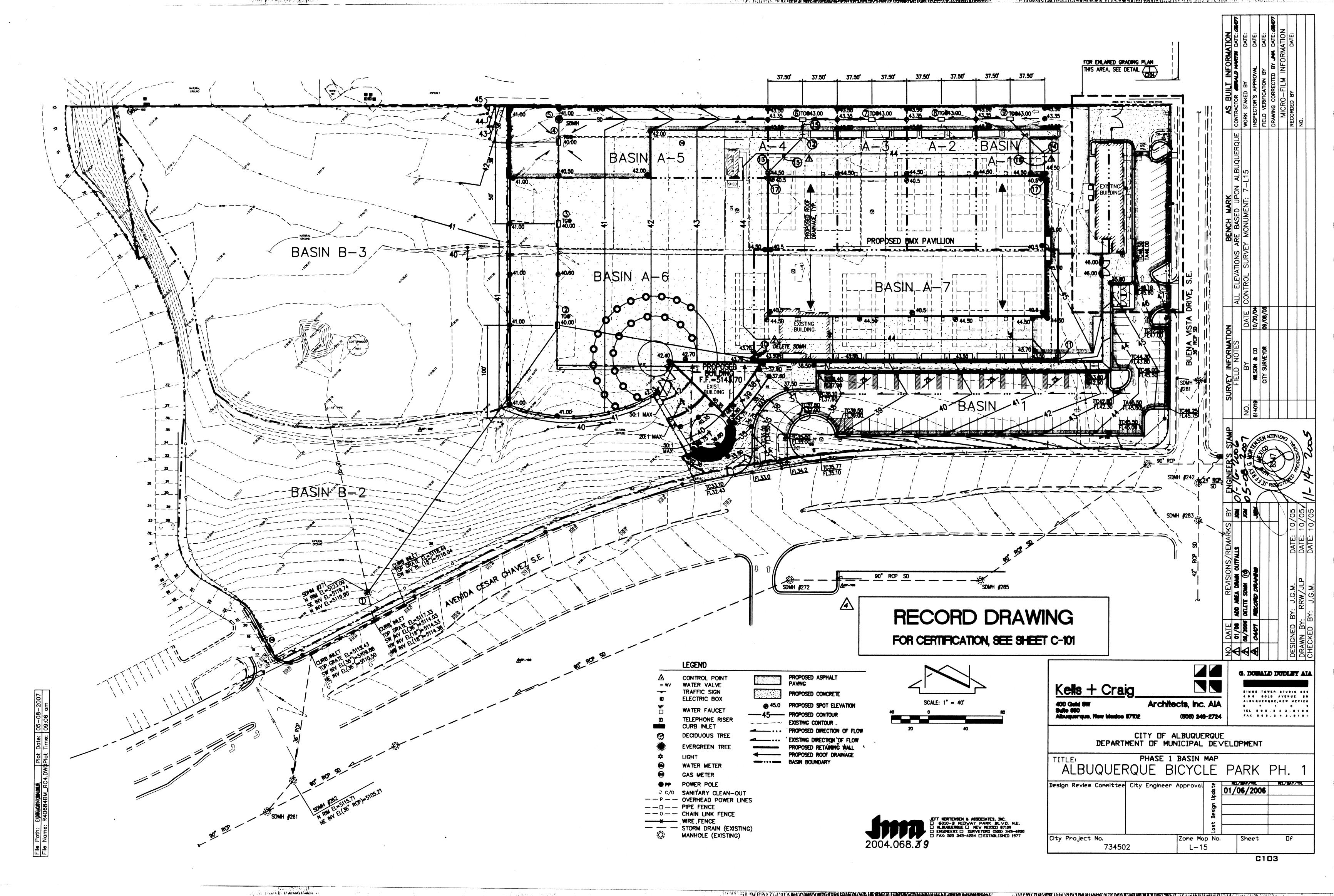
G. DOWALD DUDLEY ALA

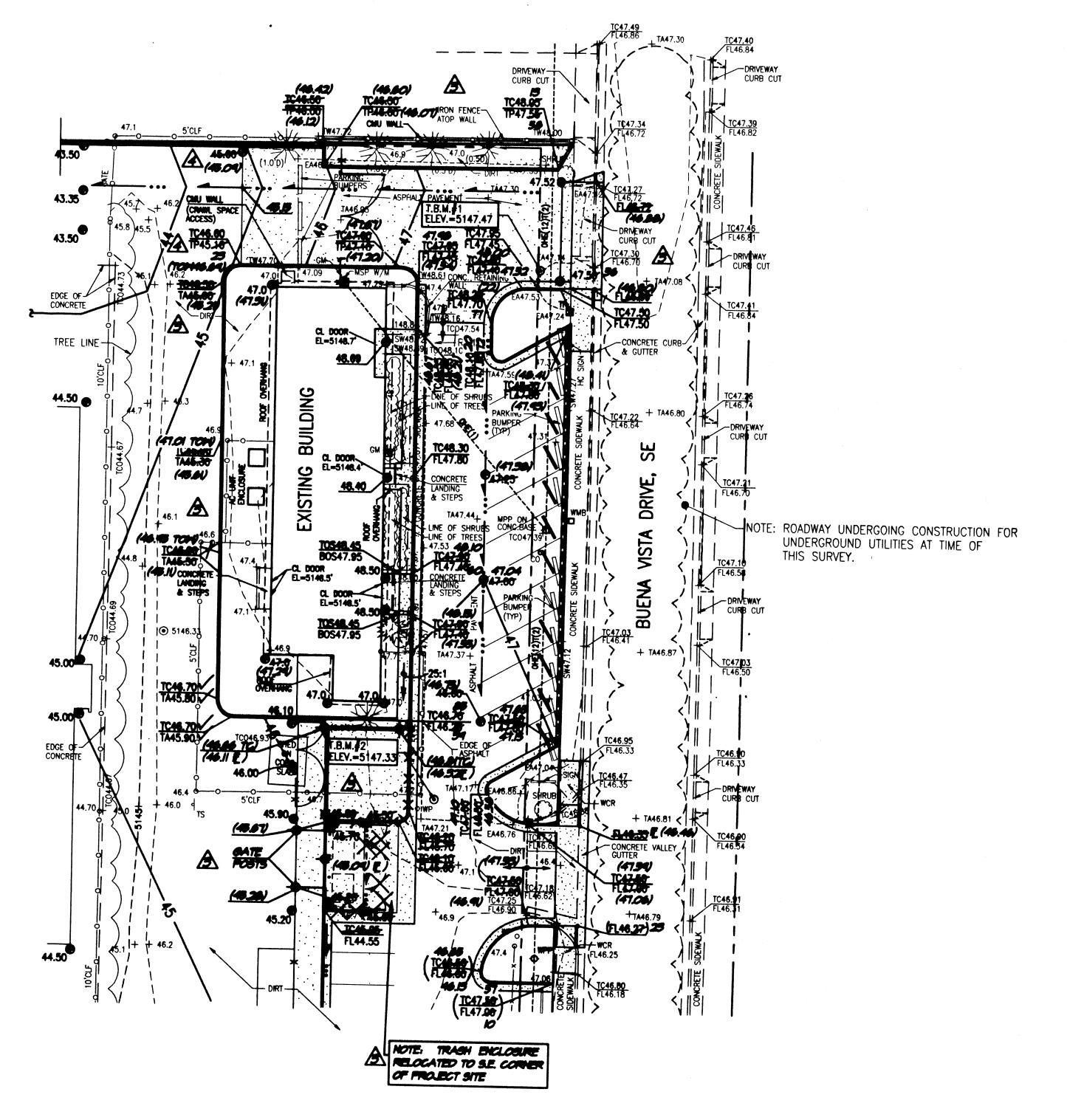
SIMMS TOWER STUDIO 850

ALBUQUERQUE, NEW MEXICO

400 BOLD AVENUE SW







RECORD DRAWING FOR CERTIFICATION, SEE SHEET C-101

2004.068.39

BOTTOM OF STEP
CENTER LINE
CHAIN LINK FENCE
CONCRETE MASONRY
SANITARY SEWER CLEAN—OUT
EDGE OF ASPHALT
ELECTRIC METER
FLOW LINE (OF GUTTER)
GAS METER FL GM MPP MSP W/M OHE(1) OHT(1) PWF SDMH SW METAL POWER POLE METAL POWER POLE
METAL SERVICE POLE WITH METER
OVERHEAD ELECTRIC (NO. OF LINES)
OVERHEAD TELEPHONE (NO. OF LINES)
POST & WIRE FENCE
STORM DRAIN MAN HOLE
EDGE OF SIDEWALK
TOP OF ASPHALT
TOP OF CURB
TOP OF CONCRETE
TOP OF PAVEMENT
TOP OF STEP
TRAFFIC SIGN
TOP OF WALL TOP OF WALL WCR WMB WP WPP WHEEL CHAIR RAMP WATER METER BOX WOOD POST WOOD POWER POLE **EXISTING SPOT ELEVATION** PROPOSED SPOT ELEVATION PROPOSED CONTOUR **EXISTING CONTOUR** PROPOSED DIRECTION OF FLOW PROPOSED RETAINING WALL PROPOSED HIGHPOINT PROPOSED CONCRETE

A RECORD INFORMATION

LEGEND

RECORD INFORMATION RECORD INFORMATION . AS-DESIGNED RECORD INFORMATION

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ENLARGED GRADING PLAN

THIS SUBMITTAL IS MADE IN SUPPORT OF WORK ORDER AND BUILDING PERMIT WITHIN THE JURISDICTION OF THE CITY OF ALBUQUERQUE. THIS PLAN SUPERCEDES THE PRIOR CONCEPTUAL PLAN DATED 12-17-2004.

II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP, THE SITE IS LOCATED ON THE NORTH SIDE OF AVENIDA CESAR CHAVEZ BETWEEN UNIVERSITY BLVD. SE AND BUENA VISTA DRIVE SE. AT PRESENT, THE SITE IS UNDEVELOPED. THE LAND IMMEDIATELY TO THE WEST IS DEVELOPED AS ISOTOPES BALL PARK. THE LAND TO THE NORTH IS INSTITUTIONAL (TVI AND UNM). THE LAND TO THE EAST IS DEVELOPED AS CITY TENNIS COURTS. AS SHOWN BY PANEL 334 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, AND INCORPORATED AREAS DATED SEPTEMBER 20, 1996, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE (ZONE AO). AN AO ZONE IS MAPPED DOWNSTREAM, HOWEVER, IT IS BELIEVED THAT THE MORE RECENT STORM DRAIN IMPROVEMENTS WITHIN AVENIDA CESAR CHAVEZ (FORMERLY STADIUM BLVD.) HAVE ALLEVIATED THE DOWNSTREAM FLOODING.

III. BACKGROUND DOCUMENTS & RESEARCH

THE FOLLOWING ITEMS WERE REVIEWED IN THE PREPARATION OF THIS SUBMITTAL: A. TOPOGRAPHIC SURVEY OF THE EXISTING SITE PREPARED BY WILSON & CO. DATED 10/21/2004. THE SUBJECT SURVEY SHOWS THE EXISTING IMPROVEMENTS. B. PRE-DESIGN CONFERENCE WITH THE CITY HYDROLOGIST 08/24/2004. THE OUTCOME OF THAT RECAP CONFIRMED THAT FREE DISCHARGE TO AVENIDA CESAR CHAVEZ IS APPROPRIATE IN LIGHT OF THE RECENT DRAINAGE IMPROVEMENTS TO UNIVERSITY BLVD SE AND AVENIDA CESAR CHAVEZ SE. C. CONCEPTUAL GRADING AND DRAINAGE PLAN (L15/D13A) PREPARED BY THIS OFFICE DATED 12-17-2004. THIS PRIOR SUBMITTAL ESTABLISHED THE DRAINAGE CRITERIA (FREE DISCHARGE) FOR THIS SITE.

IV. EXISTING CONDITIONS

THE TOPOGRAPHIC DATA PRESENTED HEREWITH, TAKEN FROM THE ABOVE REFERENCED SURVEY BY WILSON & CO., DEMONSTRATES THE EXISTING CONDITIONS OF THE PROJECT SITE. AT PRESENT, THE SITE IS PARTIALLY DEVELOPED WITH CITY TENNIS COURTS AND LIMITED ASPHALT PAVING. THE SITE CURRENTLY DRAINS FROM EAST TO WEST WITH MUCH OF THE DEVELOPED FLOW BEING DIRECTED INTO A TEMPORARY DETENTION POND IT IS BELIEVED THAT THE TEMPORARY POND WAS CREATED TO MITIGATE THE DISCHARGE OF EXCESS RUNOFF ONTO THE ISOTOPES BALL PARK. THE SITE IS SITUATED TOPOGRAPHICALLY HIGHER THAN AVENIDA CESAR CHAVEZ SE AND AS SUCH DRAINS TO THAT PUBLIC ROADWAY. ISOTOPES BALL PARK IS TOPOGRAPHICALLY LOWER AN HENCE DOES NOT CONTRIBUTE OFFSITE FLOWS. THE INSTITUTIONAL DEVELOPMENT TO THE NORTH HAS BEEN GRADED SUCH THAT IT DOES NOT DRAIN ONTO THE SUBJECT SITE. ITS FLOWS ARE DIRECTED WEST AND EVENTUALLY DISCHARGE TO UNIVERSITY BLVD SE. BUENA VISTA DRIVE LIES TO THE EAST AND APPEARS TO CONTAIN ITS FLOWS AND AS SUCH IS NOT BELIEVED TO CONTRIBUTE OFFSITE FLOWS.

V. PROPOSED CONDITIONS (PHASE 1)

THE PROPOSED IMPROVEMENTS CONSIST OF A BMX TRACK, A NEW BUILDING, PARKING IMPROVEMENTS, PEDESTRIAN PAVING AND ASSOCIATED LANDSCAPING. IN ORDER TO LOCATE THE ABOVE REFERENCED IMPROVEMENTS ON THE SITE, THE EXISTING TENNIS COURTS AND RELATED IMPROVEMENTS MUST BE DEMOLISHED. THE TEMPORARY DETENTION POND WILL REMAIN. THE EXISTING BUILDING AT THE NORTHEAST CORNER WILL BE

FOR THE PURPOSES OF ANALYSIS, THE SITE HAS BEEN DIVIDED INTO TWO (2) BASIC DRAINAGE AREAS. BASIN A DRAINS INTERNALLY TO A PRIVATE STORM DRAIN SYSTEM THAT ULTIMATELY DISCHARGES TO THE AVENIDA CESAR CHAVEZ STORM DRAIN. PRESENTLY, THE TEMPORARY DETENTION POND OUTLETS TO THE AVENIDA CESAR CHAVEZ STORM DRAIN VIA AN 18-INCH STORM DRAIN CONNECTION TO THE BACK OF A EXISTING STORM INLET IN THE NORTH CURB LINE OF THE ROADWAY. THAT CONNECTION WILL BE UTILIZED FOR THE DISCHARGE OF RUNOFF FROM BASIN A AND ITS SUB-BASINS A-1 THROUGH A-6. THE RUNOFF WILL FLOW A CROSS THE NEW PARKING LOT TO EVENTUALLY DISCHARGE TO AVENIDA CESAR CHAVEZ. BASIN B IS AN EXISTING CONDITION WHERE NO CHANGES ARE PROPOSED. BASIN B INCLUDES THE EXISTING

VI. GRADING PLAN

THE GRADING PLAN SHOWS 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 2.) PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS, 3) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, AND 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE SITE WILL DRAIN BY VARIOUS MEANS. BASIN A. AS INDICATED ABOVE, WILL DRAIN VIA AN INTERNAL PRIVATE STORM DRAIN SYSTEM INTEGRATED WITH A WATER HARVESTING FEATURE. BASIN B WILL CONTINUE TO DRAIN IN THE EXISTING CONDITION UNTIL A SUBSEQUENT PHASE DEVELOPS ON THIS REMAINING PORTION OF THE SITE.

VII. CALCULATIONS

THE CALCULATIONS THAT APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS, FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. THE RESULTS OF THE CALCULATIONS FOR THE DEVELOPED CONDITION ARE PRESENTED IN TABLE FORM. KEY ANALYSIS POINTS, SHOWN ON THE DRAINAGE BASIN MAP PLAN, ARE FURTHER ANALYZED IN ORDER THAT THE PROPOSED PRIVATE STORM DRAIN BE SIZED APPROPRIATELY. THE MANNING EQUATION WAS USED TO EVALUATE PIPE SIZES AND RELATED CAPACITIES. AS A RESULT OF THIS ANALYSIS, THE TOTAL PEAK DISCHARGE CALCULATED FOR BASIN A (16.95 CFS) WILL NOT EXCEED THE CAPACITY (44.4 CFS) OF THE EXISTING 18-INCH STORM DRAIN CONNECTION TO AVENIDA CESAR CHAVEZ.

VIII. CONCLUSIONS

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVALUATIONS CONTAINED

- 1. THIS SUBMITTAL FOLLOWS THE DRAINAGE CONCEPTS ESTABLISHED BY THE PREVIOUSLY APPROVED
- CONCEPTUAL PLAN 2. THE SITE DOES NOT LIE WITHIN A DESIGNATED 100-YEAR FLOODPLAIN
- 3. THIS SITE IS NOT RESTRICTED BY LIMITED OR INADEQUATE DOWNSTREAM CAPACITY
- 4. THE INCREASED RUNOFF FROM THIS SITE WILL BE HANDLED BY THE EXISTING DOWNSTREAM PUBLIC DRAINAGE IMPROVEMENTS THAT ULTIMATELY DISCHARGE TO THE SOUTH DIVERSION CHANNEL
- 5. THIS SUBMITTAL IS FOR BUILDING PERMIT APPROVAL.
- 6. SITE SPECIFIC DRAINAGE SUBMITTALS WILL BE REQUIRED FOR SUBSEQUENT PHASES OF CONSTRUCTION 7. OFFSITE FLOWS DO NOT IMPACT THIS SITE

CALCULATIONS

I SITE CHARACTERISTICS

- A) PRECIPITATION ZONE = 2
- B) P6,100 = P360 = 2.35 INCHES
- C) TOTAL AREA (AT) = 50,1900 SF/11.52 ACRES
- D) EXISTING LAND TREATMENT

TREATMENT AREA (SF/AC) %

a) TREATMENT B = 7,525/0.17 02 TREATMENT C = 316,573/7.27 63c) TREATMENT D = 177,802/4.08 35

II EXISTING CONDITION

WEIGHTED E (EW) = (EAAA+EBAB+ECAC+EDAD)/ATii) EW = [0.78(0.17) + 1.13(7.27) + 2.12(4.08)]/11.52 = 1.48

iii) V360 = EW * AT /12iv) V360 = 1.48 * 11.52/12 = 1.42 ACRE-FEET = 61,890 CUBIC FEET

B) PEAK DISCHARGE

TOTAL QP = QPAAA+QPBAB+ QPCAC+QPDADii) TOTAL QP = 2.28(0.17)+3.14(7.27)+4.70(4.08) = 42.4 CFS

III DEVELOPED CONDITION

THE DEVELOPED CONDITION HAS BEEN INCORPORATED INTO A TABLE FORMAT AS

			PHASE 1	BASIN A	NALYSIS		
BASINS	A _T (SF/AC)	A _B (SF/AC)	A _C (SF/AC)	A _D (SF/AC)	V ₁₀₀ (AC-FT)	Q ₁₀₀ (cFs)	Q ₁₀₀ (cfs) CUM
A-1	26,154/0.60	300/0.01		25,854/0.59	0.10	2.80	2.80
A-2	11,475/0.26			11,475/0.26	0.05	1.22 •	4.02
A-3	11,475/0.26			11,475/0.26	0.05	1.22	5.24
' A-4	13,939/0.32			13,939/0.32	0.06	1.50	6.74
A-5	25,560/0.59			25,560/0.59	0.10	2.77	9.51
A-6	73,927/1.70			73,927/1.70	0.30	7.99	17.50
A-7	37,629/0.86			37,629/0.86	0.15	4.04	21.54
B-1	113,256/2.60		113,256/2.60		0.24	8.16	
B-2	110,209/2.53		110,209/2.53		0.24	8.15	
B-3	78,408/1.80	8,690./0.20		69,718/1.60	0.30	7.98	
TOTAL	502,029/11.5	8,990/0.2	223,465/5.1	269,580/6.2	1.6	45.8	

IV COMPARISON

a) VOLUME

 \triangle V100 = 1.57-1.42 = 0.15 AC-FT

b) PEAK DISCHARGE

 \triangle Q100 = 45.2-42.4 = 2.8 CFS

		DRAINAGE	STRUCTURE SUMM	ARY TABL	E PHASE	1
	KEY *	AP	STRUCTURE	RIM/TG	INV. IN	INV. OUT
3	1	#1	EXIST. SDMH	5123.09'	5119.98	18.59 5119.74
3	2	#2	SINGLE 'D' INLET * *	5140.00'	29.56 51 31.8 5	29.14 5130.95
A	3	N/A	SINGLE 'D' INLET * *	39.96 51 40.00 '	35.30 5132.81'	33.25 5132-71
A	4	#3	SINGLE 'D' INLET * *	34.45 514 0.00 '	2.4 5134,05	34.20 51 33.9 5
A	5	#4	4' SDMH ***	.91 5140 .76 °	54 5134,38	41 5134.08
A A	6	# 5	SINGLE 'D' INLET * *	42.97 514 3.00 '	35.91 = 5135.75	57 5135,68°
<u> </u>	7	#6	SINGLE 'D' INLET * *	42.98 514 3.00 '	23 · 5136,25	17 5136.18°
	8	# 7	SINGLE 'D' INLET * *	42.95 514 3.00 '	74 5136.75	47 5136.65
<u> </u>	9	#8	SINGLE 'D' INLET * *	<i>.01</i> 5143 <i>.</i> 00'	*** * ' 5137.25'	失 火火火 5137.15'
2	10	#9	DELETE SDMH & REPLACE W/ 12" 11 1/4" ELL 4' SDMH ***	N/A -5143.70'-	51 36.70 5139.70	5136.70 5139.60'
	11	N/A	12" CAP	N/A	N/A	5142.00

* REFER TO SHEETS C102 & C103

* * CONSTRUCT PER C.O.A. STD. DWG. 2206

** CONSTRUCT PER C.O.A. STD. DWG. 2101 3 *** NOT VERIFIED BY CONSTRUCTION PHASE SURVEYOR

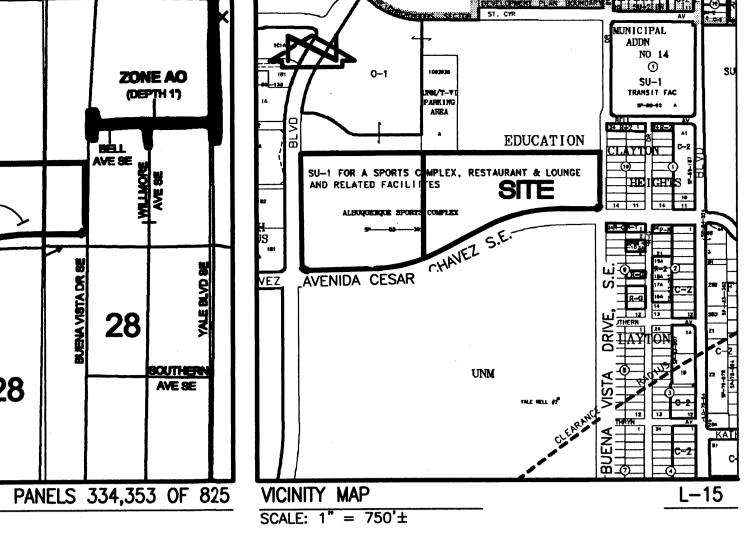
AREA DRAIN KEYED NOTES:

- 12. INSTALL 1-3" 45deg PVC ELL WITH WATERTIGHT GASKETS, INV 5136.40
- 13. INSTALL 1-3" 45deg PVC ELL WITH WATERTIGHT GASKETS, INV 5137.95
- 14. INSTALL 1-3" 45deg PVC ELL WITH WATERTIGHT GASKETS, INV © 5137.75 15. INSTALL 3" PVC (SDR-35) DRAIN LINE • S=0.0320
- 16. INSTALL 3" PVC (SDR-35) DRAIN LINE S=0.0100 17. FOR CONTINUATION, SEE SHEET M101; INV • 5138.00



F.I.R.M.

SCALE: $1" = 500' \pm$



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			PHASE 1	STORM	DRAIN HYD	PRAULICS		
	AP TO AP	PIPE SIZE	LENGTH	SLOPE	Q100 (CFS)	QCAP (CFS)	VEL (FPS)	PIPE INVERTS
	CURB INLET TO AP1	18"	30°	0.179	37.6	44.4	25.2	5114.53' TO 5119.90'
	AP1 TO AP2	24"	350'	0.032	28.36	40.5	12.9	51 9.90' TO 5131,00'
A	AP2 TO AP3	24*	191-210	201600	22 24.32	27.734	9.1	5131,00' TO 5134.00'
	AP3 TO AP4	24*	-17-23		9065 6.74	24.818.5	7.9	5134.00' TO 5/34.20'
	AP4 TO AP5	18"	260'	0.0050 V	6.74	7.5	4.2	5134.20 TO 5137.00'
	AP5 TO AP6	18"	-73 74	0.0055 0.	9035 5.24	4.4 20	4.5	5137.00' 5137.70'
Ā	AP6 TO AP7	18"	-73-75	0.0055 🗸	4.02	8.0	4.5	5137.70' 70 5138.40'
	AP7 TO AP8	18"	-73-76	0.0055	2.80	8.0 🕂	4.5	5138.40 TO 5139.10'
A	AP2 TO AP9	12"	210'	0.0270 -0.041	4.04	5.9 -7.1	N/A 12	51 3 1.00' TO 5136.70
A	AP9 TO AP10	12" -18"	330'	0.0130 -0.007	4.04	4.1 -9.0	N/A -5	5136.70 5141.00 5139.70' TO 5142.0'

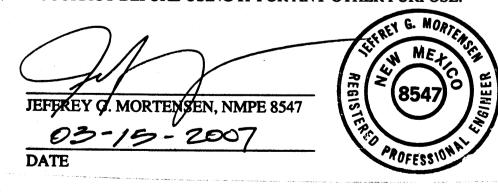
NOTES: 1. ALL LENGTHS ABOVE DETERMINED FOR HYDRAULIC ANALYSIS PURPOSES. AS-BUILT DISTANCES MAY VARY. 2. INLETS RECEIVING ROOF RUNOFF AND CONNECTING TO NEW 12" SD BETWEEN AP-9 AND AP-10 SHALL CONNECT TO TOP OF 12" PIPE AND SERVE AS CLEANOUTS FOR FUTURE MAINTENANCE

REFER TO SUMMARY TAPOLIC & LEFT (* COULD NOT VERIFY; INVEST DATA NOT PROVIDED BY CONSTRUCTION PHASE SURVEYOR.)

DRAINAGE CERTIFICATION

I, JEFFREY G. MORTENSEN, NMPE 8547, OF THE FIRM JEFF MORTENSEN & ASSOCIATES, INC., HEREBY CERTIFY THAT THOSE PORTIONS OF THIS PROJECT COMPLETED AS OF THE DATE OF THE AS-BUILT SURVEY HAVE BEEN GRADED AND DRAINED IN SUBSTANTIAL COMPLIANCE WITH AND IN ACCORDANCE WITH THE DESIGN INTENT OF THE APPROVED PLAN DATED 11-14-2005, 01-16-2006 AND 08-24-2006. THE RECORD INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY JOAQUIN ARGUELLES, NMPS 7472, DATED 2-27-07 ALONG WITH ADDITIONAL AS-BUILT INFORMATION PROVIDED BY JOAQUIN ARGUELLES. NMPS 7472, NO DATE. BASED UPON MY VISUAL INSPECTION OF THE SITE AND REVIEW OF THE AS-BUILT SURVEY DATA, THE INFORMATION PRESENTED HEREON IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR TEMPORARY CERTIFICATE OF OCCUPANCY.

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE ASPECTS OF THIS PROJECT. THIS CERTIFICATION DOES NOT ADDRESS ADA COMPLIANCE WHICH IS BEYOND THE SCOPE OF GRADING AND DRAINAGE. THOSE RELYING ON THIS RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE.



RECORD DRAWING

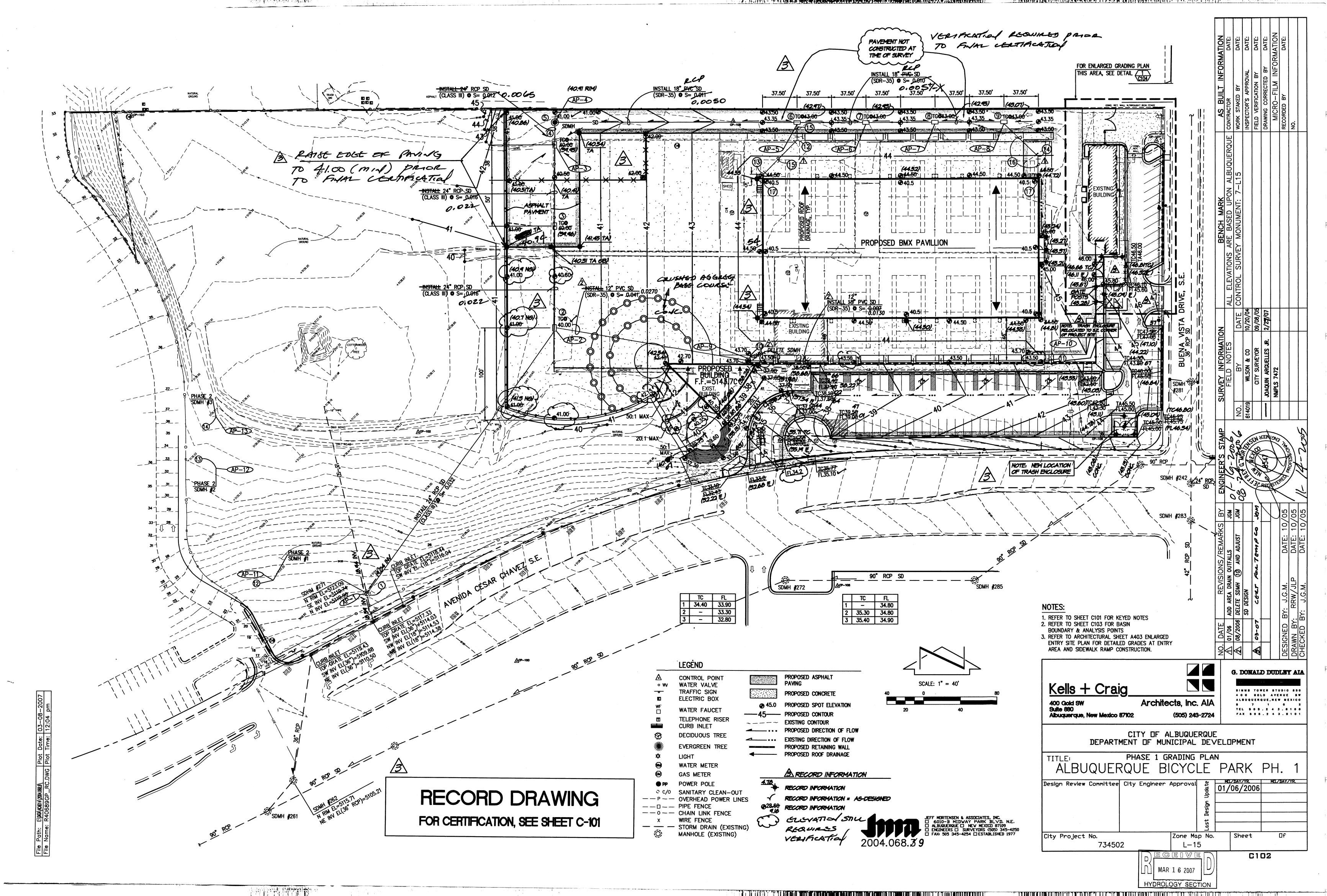


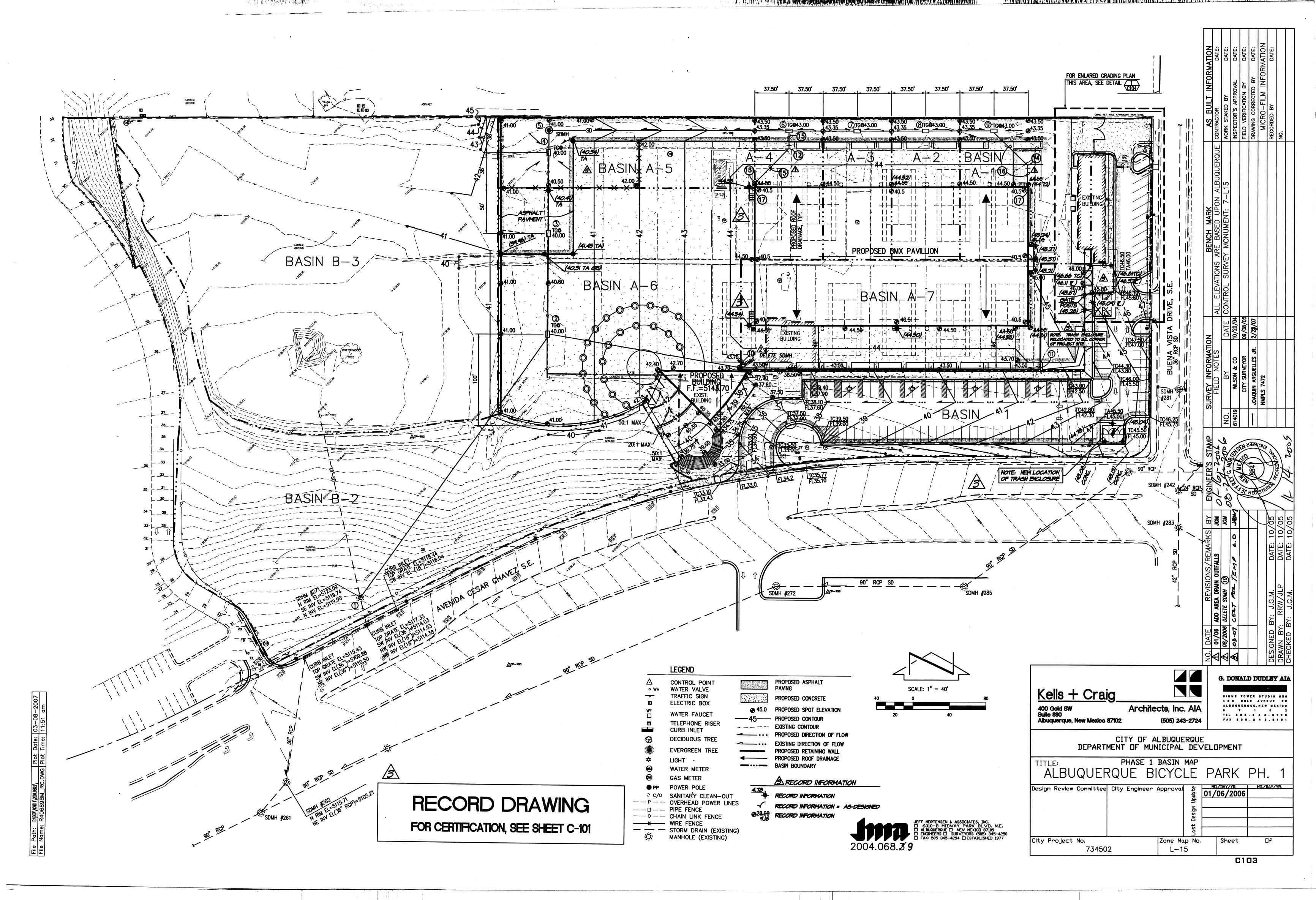
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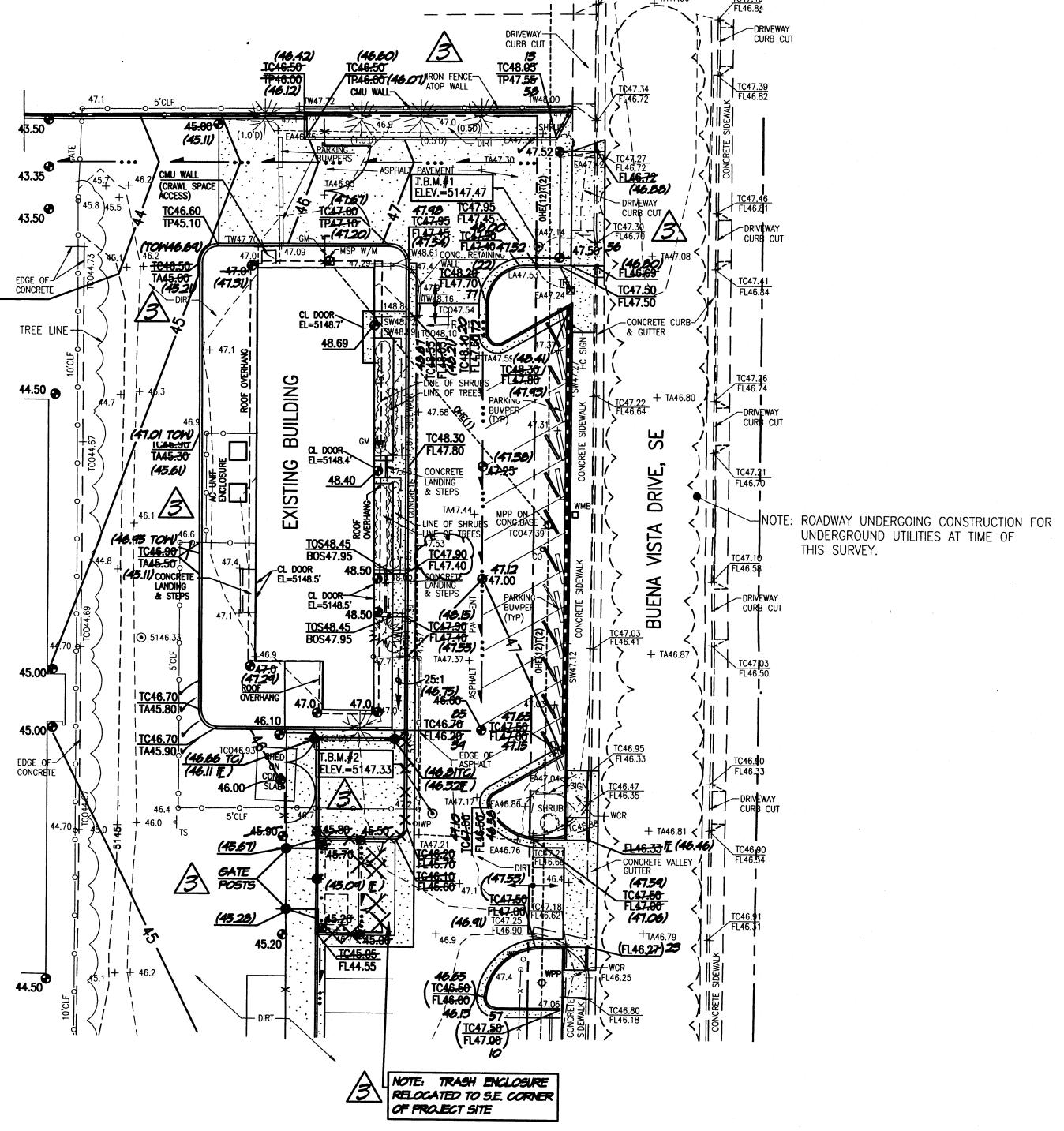
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MAR 1 6 2007 HYDROLOGY SECTION







RECORD DRAWING

FOR CERTIFICATION, SEE SHEET C-101

LEGEND

MSP W/M

OHE(1)
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PWF
SDMH
SW

BOTTOM OF STEP CENTER LINE CHAIN LINK FENCE CONCRETE MASONRY

TRAFFIC SIGN TOP OF WALL

WHEEL CHAIR RAMP WATER METER BOX WOOD POST WOOD POWER POLE

EXISTING SPOT ELEVATION PROPOSED SPOT ELEVATION

PROPOSED DIRECTION OF FLOW PROPOSED RETAINING WALL

B RECORD INFORMATION

RECORD INFORMATION = AS-DESIGNED

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PROPOSED HIGHPOINT

PROPOSED CONCRETE

RECORD INFORMATION

RECORD INFORMATION

SANITARY SEWER CLEAN-OUT

SANITARY SEWER CLEAN—OUT
EDGE OF ASPHALT
ELECTRIC METER
FLOW LINE (OF GUTTER)
GAS METER
METAL POWER POLE
METAL SERVICE POLE WITH METER
OVERHEAD ELECTRIC (NO. OF LINES)
OVERHEAD TELEPHONE (NO. OF LINES)
POST & WIRE FENCE
STORM DRAIN MAN HOLE
EDGE OF SIDEWALK
TOP OF ASPHALT
TOP OF CURB
TOP OF CONCRETE
TOP OF PAVEMENT
TOP OF STEP
TRAFFIC SIGN

Kells + Crail 400 Gold SW Suite 880 Albuquerque, New Mexico	Archite	ects, Inc. (505) 243		(SIMMS TADONA SIMMS TADO SALBUQUE TEL 5 OF FAX 5 O	OWER STUDE OWER STUDE OLD AVENUER OLD AVENUER OLD STUDE	0 850 E 8W 4EXICO 2 8 1 0 0 8 1 0 1
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ENLARGED GRADING PLAN

PROPERTY OWNED BY THE CITY OF ALBUQUERQUE. THE SITE LIES IMMEDIATELY EAST OF THE ISOTOPES BALL PARK. THE DRAINAGE CONCEPT FOR THIS PROJECT WILL BE THE FREE DISCHARGE OF DEVELOPED RUNOFF TO AVENIDA CESAR CHAVEZ SE WHERE A MAJOR PUBLIC STORM DRAIN LIES. THE EXISTING PUBLIC STORM DRAIN COLLECTS RUNOFF CARRIED BY THE STREET AND CONVEYS THAT RUNOFF WEST TO THE SOUTH DIVERSION CHANNEL.

THIS SUBMITTAL IS MADE IN SUPPORT OF WORK ORDER AND BUILDING PERMIT WITHIN THE JURISDICTION OF THE CITY OF ALBUQUERQUE. THIS PLAN SUPERCEDES THE PRIOR CONCEPTUAL PLAN DATED 12-17-2004.

II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP, THE SITE IS LOCATED ON THE NORTH SIDE OF AVENIDA CESAR CHAVEZ BETWEEN UNIVERSITY BLVD. SE AND BUENA VISTA DRIVE SE. AT PRESENT, THE SITE IS UNDEVELOPED. THE LAND IMMEDIATELY TO THE WEST IS DEVELOPED AS ISOTOPES BALL PARK. THE LAND TO THE NORTH IS INSTITUTIONAL (TVI AND UNM). THE LAND TO THE EAST IS DEVELOPED AS CITY TENNIS COURTS. AS SHOWN BY PANEL 334 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, AND INCORPORATED AREAS DATED SEPTEMBER 20, 1996, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE (ZONE AO). AN AO ZONE IS MAPPED DOWNSTREAM, HOWEVER, IT IS BELIEVED THAT THE MORE RECENT STORM DRAIN IMPROVEMENTS WITHIN AVENIDA CESAR CHAVEZ (FORMERLY STADIUM BLVD.) HAVE ALLEVIATED THE DOWNSTREAM FLOODING.

III. BACKGROUND DOCUMENTS & RESEARCH

THE FOLLOWING ITEMS WERE REVIEWED IN THE PREPARATION OF THIS SUBMITTAL:

A. TOPOGRAPHIC SURVEY OF THE EXISTING SITE PREPARED BY WILSON & CO. DATED 10/21/2004. THE SUBJECT SURVEY SHOWS THE EXISTING IMPROVEMENTS.

B. PRE-DESIGN CONFERENCE WITH THE CITY HYDROLOGIST 08/24/2004. THE OUTCOME OF THAT RECAP CONFIRMED THAT FREE DISCHARGE TO AVENIDA CESAR CHAVEZ IS APPROPRIATE IN LIGHT OF THE

CONFIRMED THAT FREE DISCHARGE TO AVENIDA CESAR CHAVEZ IS APPROPRIATE IN LIGHT OF THE RECENT DRAINAGE IMPROVEMENTS TO UNIVERSITY BLVD SE AND AVENIDA CESAR CHAVEZ SE.

C. CONCEPTUAL GRADING AND DRAINAGE PLAN (L15/D13A) PREPARED BY THIS OFFICE DATED 12-17-2004. THIS PRIOR SUBMITTAL ESTABLISHED THE DRAINAGE CRITERIA (FREE DISCHARGE) FOR THIS SITE.

W. EXISTING CONDITIONS

THE TOPOGRAPHIC DATA PRESENTED HEREWITH, TAKEN FROM THE ABOVE REFERENCED SURVEY BY WILSON & CO., DEMONSTRATES THE EXISTING CONDITIONS OF THE PROJECT SITE. AT PRESENT, THE SITE IS PARTIALLY DEVELOPED WITH CITY TENNIS COURTS AND LIMITED ASPHALT PAVING. THE SITE CURRENTLY DRAINS FROM EAST TO WEST WITH MUCH OF THE DEVELOPED FLOW BEING DIRECTED INTO A TEMPORARY DETENTION POND. IT IS BELIEVED THAT THE TEMPORARY POND WAS CREATED TO MITIGATE THE DISCHARGE OF EXCESS RUNOFF ONTO THE ISOTOPES BALL PARK. THE SITE IS SITUATED TOPOGRAPHICALLY HIGHER THAN AVENIDA CESAR CHAVEZ SE AND AS SUCH DRAINS TO THAT PUBLIC ROADWAY. ISOTOPES BALL PARK IS TOPOGRAPHICALLY LOWER AN HENCE DOES NOT CONTRIBUTE OFFSITE FLOWS. THE INSTITUTIONAL DEVELOPMENT TO THE NORTH HAS BEEN GRADED SUCH THAT IT DOES NOT DRAIN ONTO THE SUBJECT SITE. ITS FLOWS ARE DIRECTED WEST AND EVENTUALLY DISCHARGE TO UNIVERSITY BLVD SE. BUENA VISTA DRIVE LIES TO THE EAST AND APPEARS TO CONTAIN ITS FLOWS AND AS SUCH IS NOT BELIEVED TO CONTRIBUTE OFFSITE FLOWS.

V. PROPOSED CONDITIONS (PHASE 1)

THE PROPOSED IMPROVEMENTS CONSIST OF A BMX TRACK, A NEW BUILDING, PARKING IMPROVEMENTS, PEDESTRIAN PAVING AND ASSOCIATED LANDSCAPING. IN ORDER TO LOCATE THE ABOVE REFERENCED IMPROVEMENTS ON THE SITE, THE EXISTING TENNIS COURTS AND RELATED IMPROVEMENTS MUST BE DEMOLISHED. THE TEMPORARY DETENTION POND WILL REMAIN. THE EXISTING BUILDING AT THE NORTHEAST CORNER WILL BE RENOVATED.

FOR THE PURPOSES OF ANALYSIS, THE SITE HAS BEEN DIVIDED INTO TWO (2) BASIC DRAINAGE AREAS. BASIN A DRAINS INTERNALLY TO A PRIVATE STORM DRAIN SYSTEM THAT ULTIMATELY DISCHARGES TO THE AVENIDA CESAR CHAVEZ STORM DRAIN. PRESENTLY, THE TEMPORARY DETENTION POND OUTLETS TO THE AVENIDA CESAR CHAVEZ STORM DRAIN VIA AN 18-INCH STORM DRAIN CONNECTION TO THE BACK OF A EXISTING STORM INLET IN THE NORTH CURB LINE OF THE ROADWAY. THAT CONNECTION WILL BE UTILIZED FOR THE DISCHARGE OF RUNOFF FROM BASIN A AND ITS SUB-BASINS A-1 THROUGH A-6. THE RUNOFF WILL FLOW A CROSS THE NEW PARKING LOT TO EVENTUALLY DISCHARGE TO AVENIDA CESAR CHAVEZ.

BASIN B IS AN EXISTING CONDITION WHERE NO CHANGES ARE PROPOSED. BASIN B INCLUDES THE EXISTING

VI. GRADING PLAN

DETENTION POND.

THE GRADING PLAN SHOWS 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 2.) PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS, 3) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, AND 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE SITE WILL DRAIN BY VARIOUS MEANS. BASIN A, AS INDICATED ABOVE, WILL DRAIN VIA AN INTERNAL PRIVATE STORM DRAIN SYSTEM INTEGRATED WITH A WATER HARVESTING FEATURE. BASIN B WILL CONTINUE TO DRAIN IN THE EXISTING CONDITION UNTIL A SUBSEQUENT PHASE DEVELOPS ON THIS REMAINING PORTION OF THE SITE.

VII. CALCULATIONS

THE CALCULATIONS THAT APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS, FOR THE 100—YEAR, 6—HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. THE RESULTS OF THE CALCULATIONS FOR THE DEVELOPED CONDITION ARE PRESENTED IN TABLE FORM. KEY ANALYSIS POINTS, SHOWN ON THE DRAINAGE BASIN MAP PLAN, ARE FURTHER ANALYZED IN ORDER THAT THE PROPOSED PRIVATE STORM DRAIN BE SIZED APPROPRIATELY. THE MANNING EQUATION WAS USED TO EVALUATE PIPE SIZES AND RELATED CAPACITIES. AS A RESULT OF THIS ANALYSIS, THE TOTAL PEAK DISCHARGE CALCULATED FOR BASIN A (16.95 CFS) WILL NOT EXCEED THE CAPACITY (44.4 CFS) OF THE EXISTING 18—INCH STORM DRAIN CONNECTION TO AVENIDA CESAR CHAVEZ.

VIII. CONCLUSIONS

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVALUATIONS CONTAINED

- 1. THIS SUBMITTAL FOLLOWS THE DRAINAGE CONCEPTS ESTABLISHED BY THE PREVIOUSLY APPROVED
- CONCEPTUAL PLAN

 2. THE SITE DOES NOT LIE WITHIN A DESIGNATED 100-YEAR FLOODPLAIN
- 3. THIS SITE IS NOT RESTRICTED BY LIMITED OR INADEQUATE DOWNSTREAM CAPACITY
 4. THE INCREASED RUNOFF FROM THIS SITE WILL BE HANDLED BY THE EXISTING DOWNSTREAM PUBLIC
- DRAINAGE IMPROVEMENTS THAT ULTIMATELY DISCHARGE TO THE SOUTH DIVERSION CHANNEL
- 5. THIS SUBMITTAL IS FOR BUILDING PERMIT APPROVAL.
 6. SITE SPECIFIC DRAINAGE SUBMITTALS WILL BE REQUIRED FOR SUBSEQUENT PHASES OF CONSTRUCTION
- 7. OFFSITE FLOWS DO NOT IMPACT THIS SITE

CALCULATIONS

I SITE CHARACTERISTICS

- A) PRECIPITATION ZONE = 2
- B) P6,100 = P360 = 2.35 INCHES
- C) TOTAL AREA (AT) = 50,1900 SF/11.52 ACRES
- D) EXISTING LAND TREATMENT

TREATMENT AREA (SF/AC) %

a) TREATMENT B = 7,525/0.17 02 b) TREATMENT C = 316,573/7.27 63

c) TREATMENT D = 177,802/4.08 35

II EXISTING CONDITION

A) VOLUME

- WEIGHTED E (EW) = (EAAA + EBAB + ECAC + EDAD)/AT
- ii) EW = [0.78(0.17) + 1.13(7.27) + 2.12(4.08)]/11.52 = 1.48
- iii) V360 = EW * AT /12 iv) V360 = 1.48 * 11.52/12 = 1.42 ACRE-FEET = 61,890 CUBIC FEET
- B) PEAK DISCHARGE
 i) TOTAL QP = QPAAA+QPBAB+ QPCAC+QPDAD
- ii) TOTAL QP = 2.28(0.17)+3.14(7.27)+4.70(4.08) = 42.4 CFS

III DEVELOPED CONDITION
THE DEVELOPED CONDITION HAS BEEN INCORPORATED INTO A TABLE FORMAT AS PRESENTED BELOW.

			PHASE 1	BASIN A	NALYSIS	•	
BASINS	A _T (SF/AC)	A _B (SF/AC)	A _C (SF/AC)	A _D (8F/AC)	V ₁₀₀ (AC-FT)	Q ₁₀₀ (CF3)	Q ₁₀₀ (cfs) CUM
A-1	26,154/0.60	300/0.01		25,854/0.59	0.10	2.80	2.80
A -2	11,475/0.26			11,475/0.26	0.05	1.22	4.02
A-3	11,475/0.26			11,475/0.26	0.05	1.22	5.24
A-4	13,939/0.32			13,939/0.32	0.06	1.50	6.74
A-5	25,560/0.59			25,560/0.59	0.10	2.77	9.51
A-6	73,927/1.70			73,927/1.70	0.30	7.99	17.50
A 7	37,629/0.86			37,629/0.86	0.15	4.04	21.54
B-1	113,256/2.60		113,256/2.60		0.24	8.16	
B-2	110,209/2.53		110,209/2.53		0.24	8.15	
B-3	78,408/1.80	8,690./0.20		69,718/1.60	0.30	7.98	
TOTAL	502,029/11.5	8,990/0.2	223,465/5.1	269,580/6.2	1.6	45.8	

IV COMPARISON

a) VOLUME

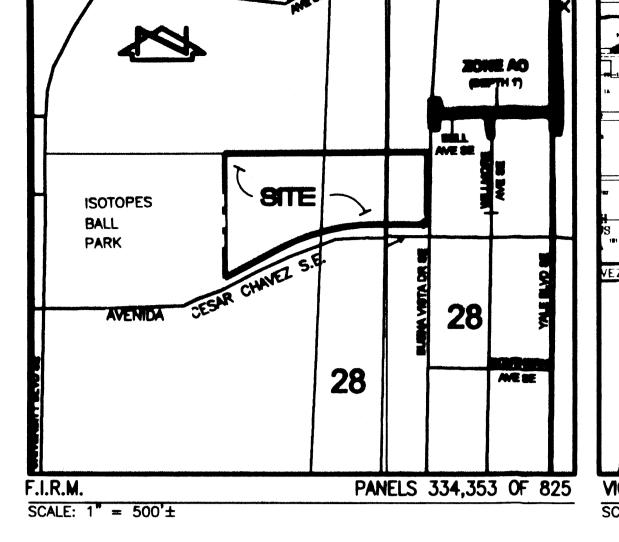
 \triangle V100 = 1.57-1.42 = 0.15 AC-FT

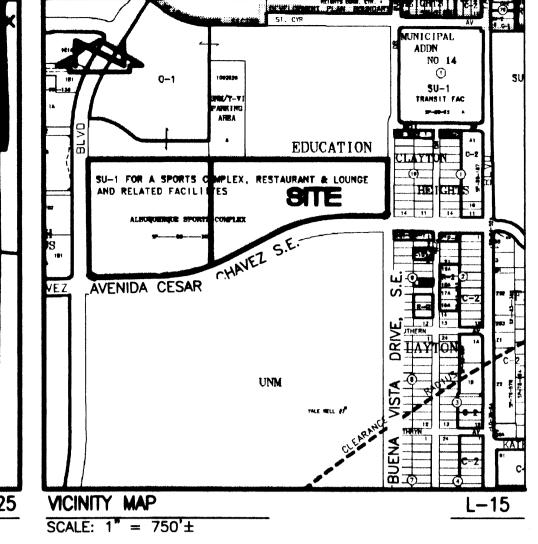
b) PEAK DISCHARGE

 \triangle Q100 = 45.2-42.4 = 2.8 CFS

	DRAINAGE	STRUCTURE SUMM	ARY TABL	E PHASE	1
KEY	AP	STRUCTURE	RIM/TG	INV. IN	INV. OUT
1	#1	EXIST. SDMH	5123.09'	5119.90'	5119.74'
2	#2	SINGLE 'D' INLET * *	5140.00'	5131.05'	5130.95'
3	N/A	SINGLE 'D' INLET * *	5140.00'	5132.81'	5132.71'
4	#3	SINGLE 'D' INLET * *	5140.00'	5134.05'	5133.95
5	#4	4' SDMH ***	5140.70'	5134.35'	5134.05'
6	# 5	SINGLE 'D' INLET * *	5143.00'	5137.05'	5136.95'
7	#6	SINGLE 'D' INLET * *	5143.00'	5137.75'	5137.65'
8	# 7	SINGLE 'D' INLET * *	5143.00'	5138.45'	5138.35'
9	#8	SINGLE 'D' INLET * *	5143.00'	N/A ·	5139.10'
10	#9	4' SDMH ***	5143.70'	5139.70	5139.60'
11	N/A	12" CAP	N/A	N/A	5142.00

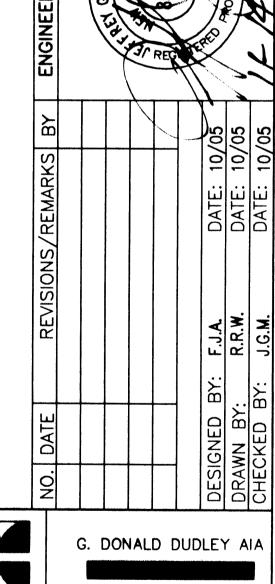
- * REFER TO SHEETS C102 & C103
- * * CONSTRUCT PER C.O.A. STD. DWG. 2206 * * CONSTRUCT PER C.O.A. STD. DWG. 2101





PHASE 1 STORM DRAIN HYDRAULICS											
AP TO AP	PIPE SIZE	LENGTH	SLOPE	Q100 (CFS)	QCAP (CFS)	VEL (FPS)	PIPE INVERTS				
CURB INLET TO AP1	18"	30'	0.179	37.6	44.4	25.2	5114.53' TO 5119.90'				
AP1 TO AP2	24"	350'	0.032	28.36	40.5	12.9	5119.90' TO 5131.00'				
AP2 TO AP3	24"	191'	0.016	24.32	27.7	9.1	5131.00' TO 5134.00'				
AP3 TO AP4	24"	17'	0.012	6.74	24.8	7.9	5134.00' TO 5134.20'				
AP4 TO AP5	18"	255'	0.011	6.74	11.0	6.2	5134.20' TO 5137.00'				
AP5 TO AP6	18"	68'	0.010	5.24	10.5	5.9	5137.00' TO 5137.70'				
AP6 TO AP7	18"	68'	0.010	4.02	10.5	5.9	5137.70' TO 5138.40'				
AP7 TO AP8	18"	68'	0.010	2.80	10.5	5.9	5138.40' TO 5139.10'				
AP2 TO AP9	12°	210'	0.041	4.04	7.1	12	5131.00' TO 5139.60				
AP9 TO AP10	18"	330'	0.007	4.04	9.0	5	5139.70' TO 5142.0'				

NOTE: ALL LENGHTS ABOVE DETERMINED FOR HYDRAULIC ANALYSIS PURPOSES. AS-BUILT DISTANCES MAY VARY



Kells + Craig

Architects Inc.

Sulle 880

Albuquerque, New Mesdoo 87102

Architects, Inc. AIA

(505) 249-2724

G. DONALD DUDLEY AIA

SINMS TOWER STUDIO 850

ALSUQUERQUE, NEW MEXICO

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HYDROLOGY SECTION

CITY OF ALBUQUERQUE

DEPARTMENT OF MUNICIPAL DEVELOPMENT

City Project No.

734502

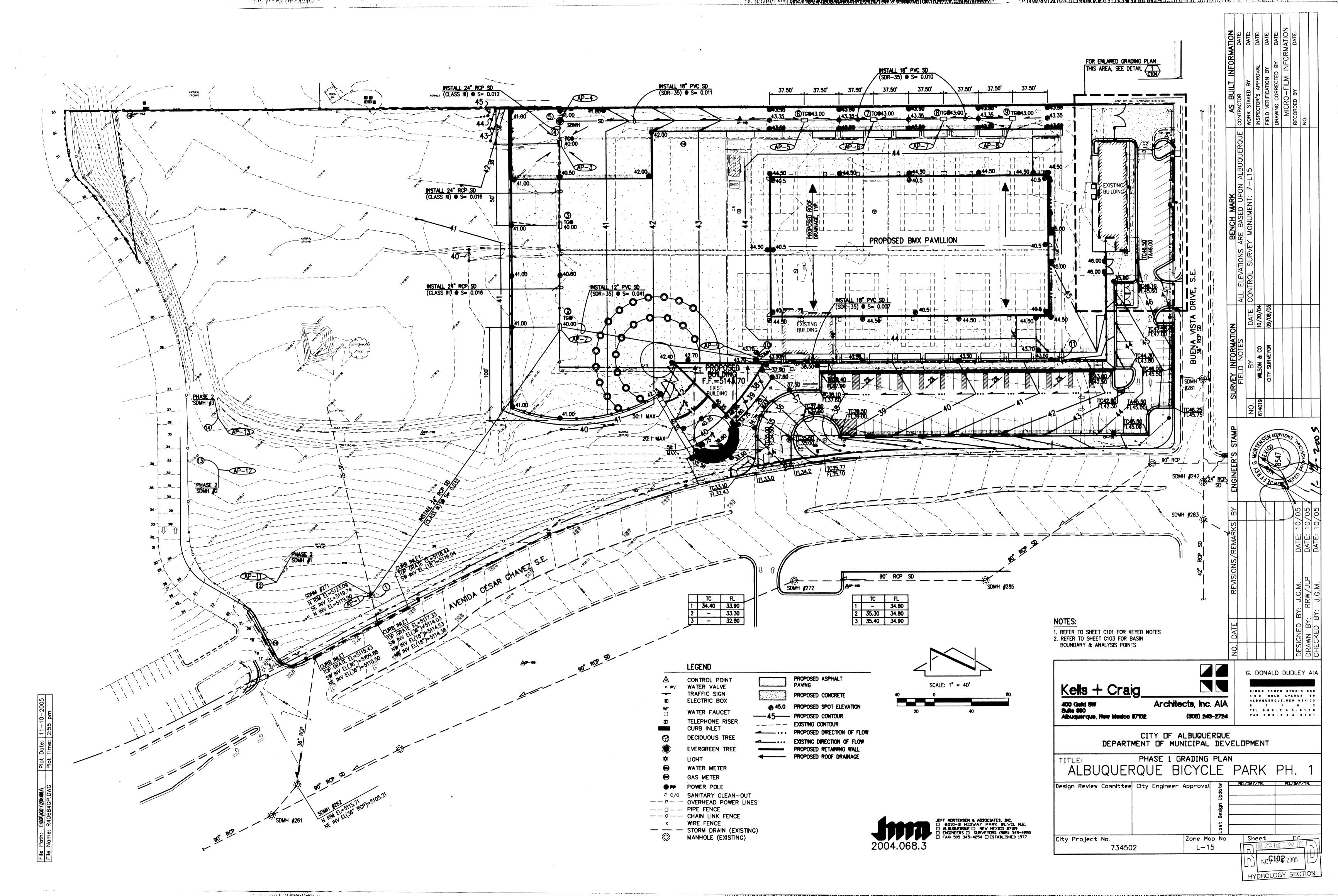
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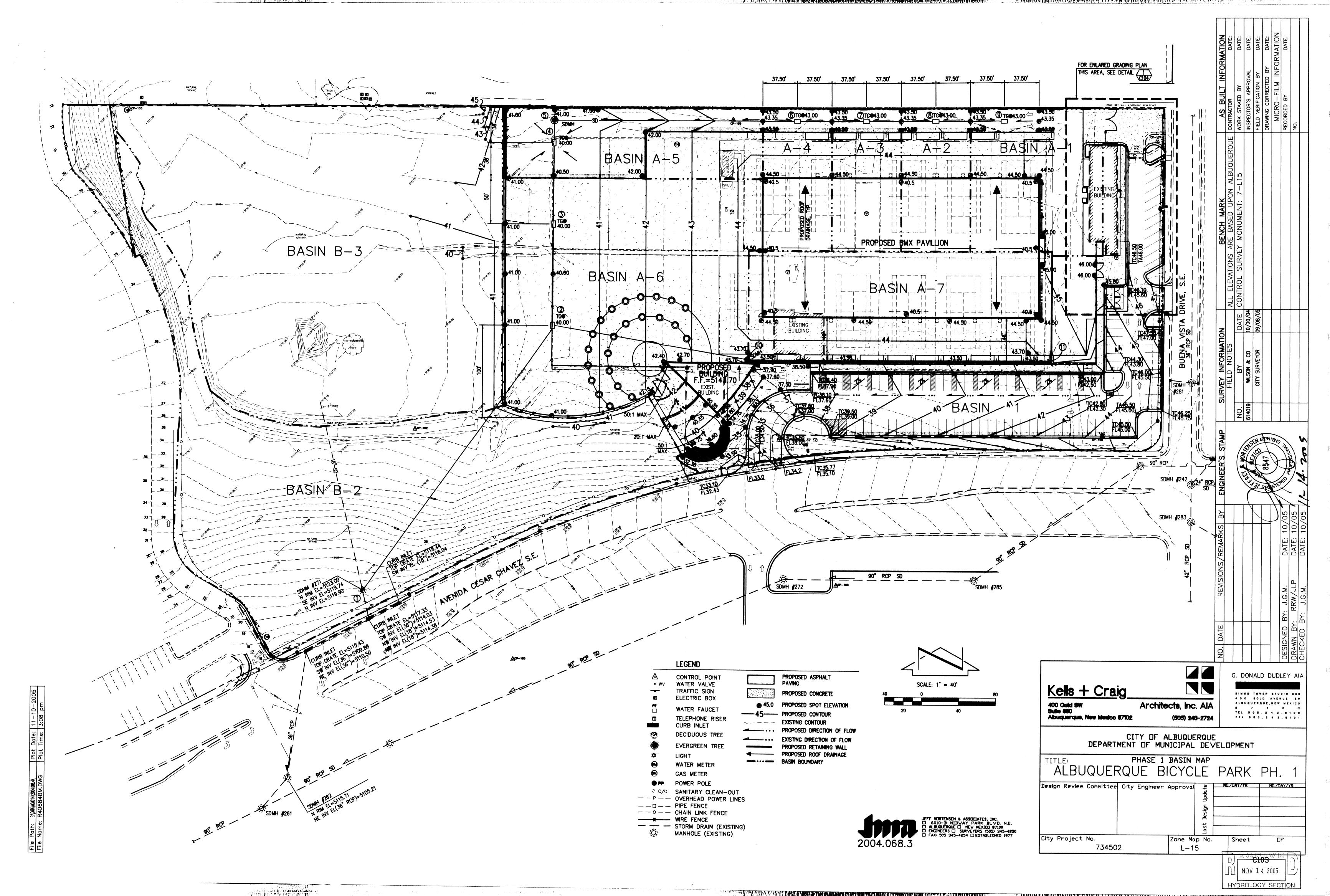
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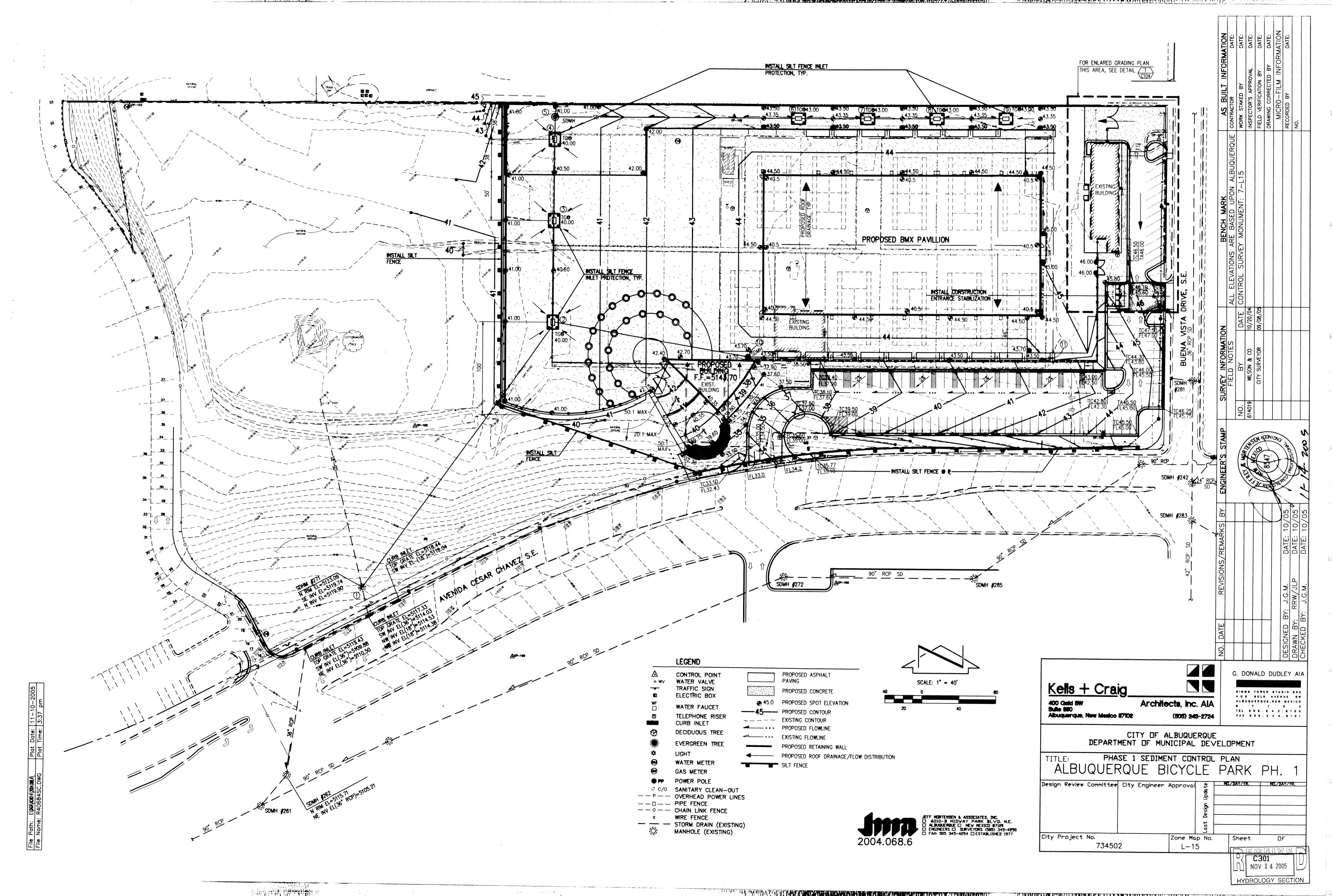
JEFF HORTENSEN & ASSOCIATES, INC.

| 6010-3 MIDWAY PARK 3LVD, N.E.
| ALBUQUEERS | NEVEXICO 87109 |
| ENGINEERS | SURVEYORS (505) 345-4250 |
| FAXI 505 345-4254 | ESTABLISHED 1977

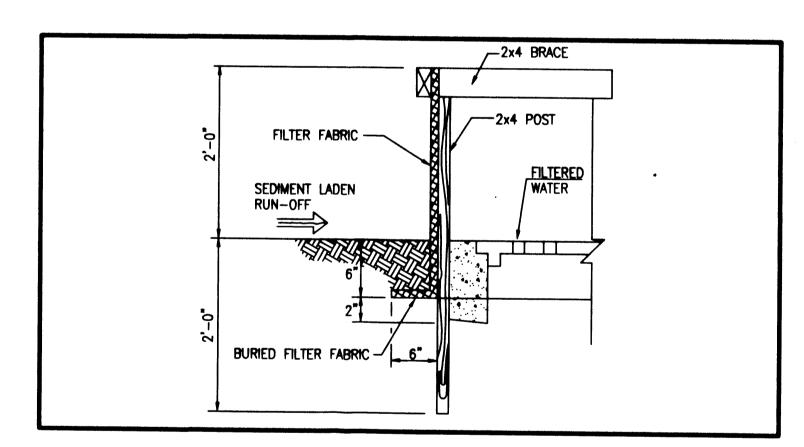
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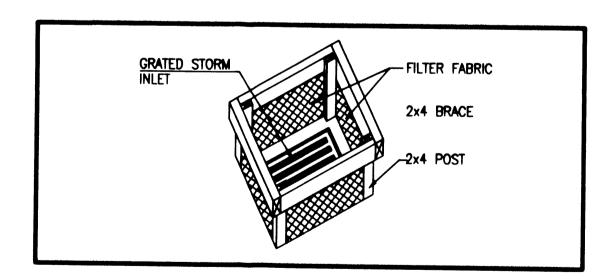




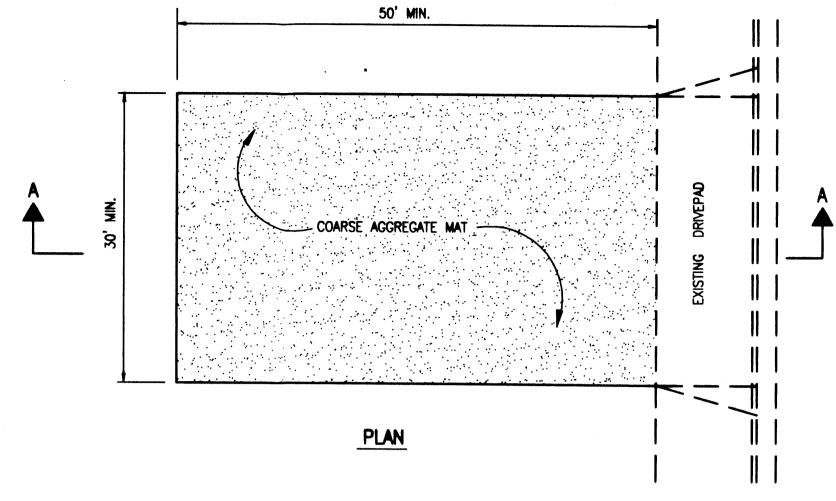


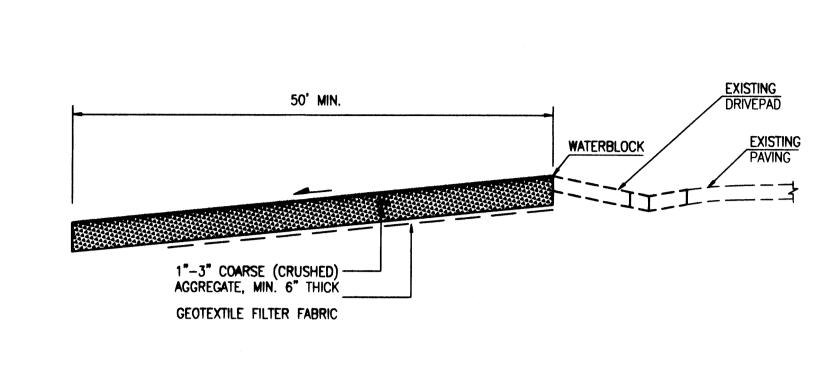


SILT FENCE INLET PROTECTION SECTION NOT TO SCALE



SILT FENCE INLET PROTECTION NOT TO SCALE

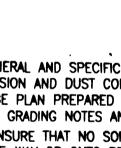




AND INTERNAL INTERNAL DESIGNATION OF A STATE OF THE PROPERTY O

CONSTRUCTION ENTRANCE STABILIZATION SCALE: 1" = 10' HORIZ. 1" = 2' VERT

SECTION A-A



1. THIS PLAN ADDRESSES GENERAL AND SPECIFIC MEASURES FOR CONSTRUCTION PHASE EROSION AND DUST CONTROL. REFER TO THE GRADING AND DRAINAGE PLAN PREPARED BY JEFF MORTENSEN AND ASSOCIATES, INC. FOR GRADING NOTES AND INFORMATION. 2. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE

SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. 3. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN

4. CONCRETE TRUCKS SHALL BE SENT BACK TO PLANT FOR WASHING. THE WASHING OF CONCRETE TRUCKS SHALL NOT BE PERMITTED ON THIS SITE.

5. THE CONTRACTOR SHALL SECURE ALL REQUIRED PERMITS PRIOR TO BEGINNING CONSTRUCTION. 6. THE CONTRACTOR SHALL PROMPTLY REMOVE SEDIMENT ACCUMULATION

FROM SILT FENCES WITHIN 48 HOURS OF A RAINFALL EVENT. 7. THE CONTRACTOR SHALL PICK UP LITTER AND CONSTRUCTION DEBRIS ON A DAILY BASIS.

8. OFFSITE MATERIAL STORAGE AREAS USED BY THIS PROJECT ARE CONSIDERED PART OF THE PROJECT AND ARE SUBJECT TO THE REQUIREMENTS OF THIS EROSION CONTROL PLAN.

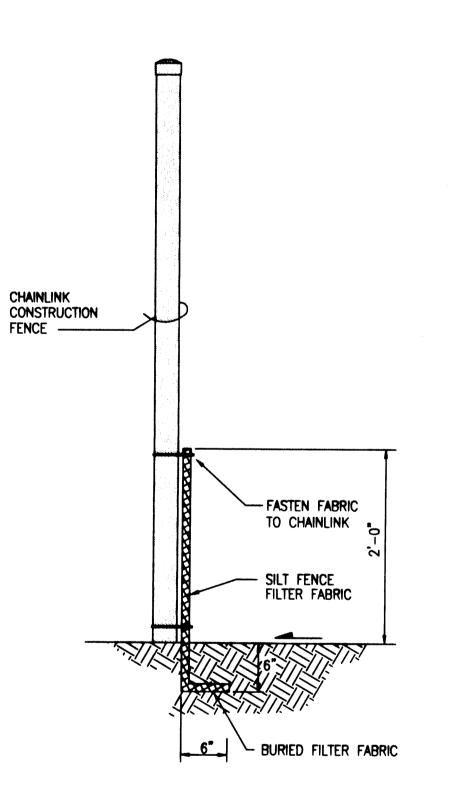
9. THE CONTRACTOR SHALL IMPLEMENT ON-SITE STRUCTURAL EROSION CONTROL PRACTICES AS REQUIRED TO COMPLY WITH THE EROSION CONTROL PLAN. THESE PRACTICES MAY INCLUDE BUT ARE NOT LIMITED TO: SILT FENCES, EARTHEN DIKES, DRAINAGE SWALES. SEDIMENT TRAPS, CHECK DAMS, SUBSURFACE DRAINS, PIPE SLOPE DRAINS, LEVEL SPREADERS, STORM RETAINING SYSTEM, GABIONS AND TEMPORARY OR PERMANENT SEDIMENT BASINS.

10. THE CONTRACTOR SHALL MINIMIZE OFFSITE VEHICLE TRACKING OF SEDIMENT AND DUST GENERATION.

11. UPON COMPLETION OF MASS GRADING, ALL DISTURBED AREAS SHALL BE STABILIZED WITH PERMANENT CONSTRUCTION, LANDSCAPING, VEGETATION AND/OR GRAVEL MULCH SILT FENCING CAN BE REMOVED UPON SUCCESSFUL ESTABLISHMENT OF VEGETATION.

12. REFER TO STORM WATER POLLUTION PREVENTION PLAN FOR PROJECT SPECIFIC PHASING AND INFORMATION. THIS PROJECT SHALL BE IMPLEMENTED IN PHASES TO MINIMIZE THE EXTENT AND DURATION OF

POLLUTANT DISCHARGE ELIMINATION SYSTEM MANUAL - STORM WATER MANAGEMENT GUIDELINES FOR CONSTRUCTION AND INDUSTRIAL ACTIVITIES, NOVEMBER 2002. COPIES ARE INCLUDED IN THE APPENDIX OF THE PROJECT SWPPP FOR EASE OF REFERENCE.



CONSTRUCTION FENCE/SILT FENCE SECTION SCALE: 1'' = 2'

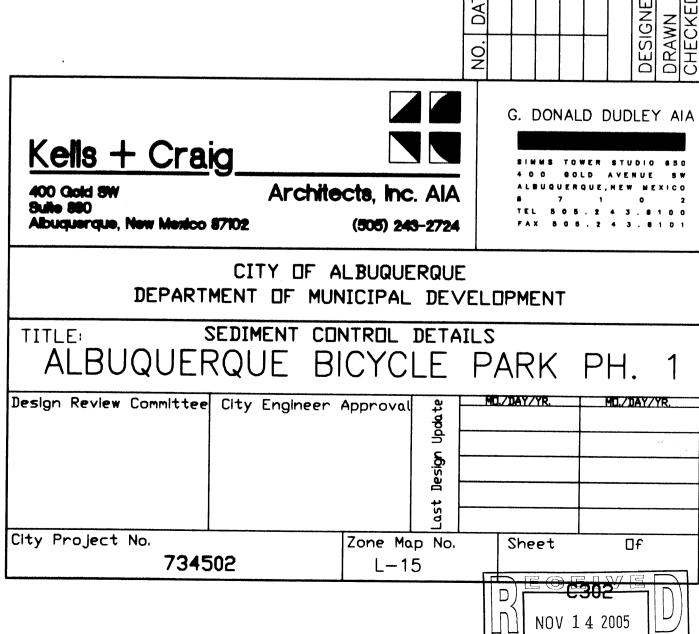
JEFF MORTENSEN & ASSOCIATES, INC.

| 6010-3 MIDWAY PARK BLVD, N.E.

| ALBUQUERQUE | NEV MEXICO 87109

| ENGINEERS | SURVEYORS (505) 345-4250

| FAX: 505 345-4254 | ESTABLISHED 1977 2004.068.6



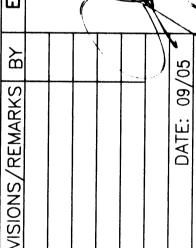
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HYDROLOGY SECTION

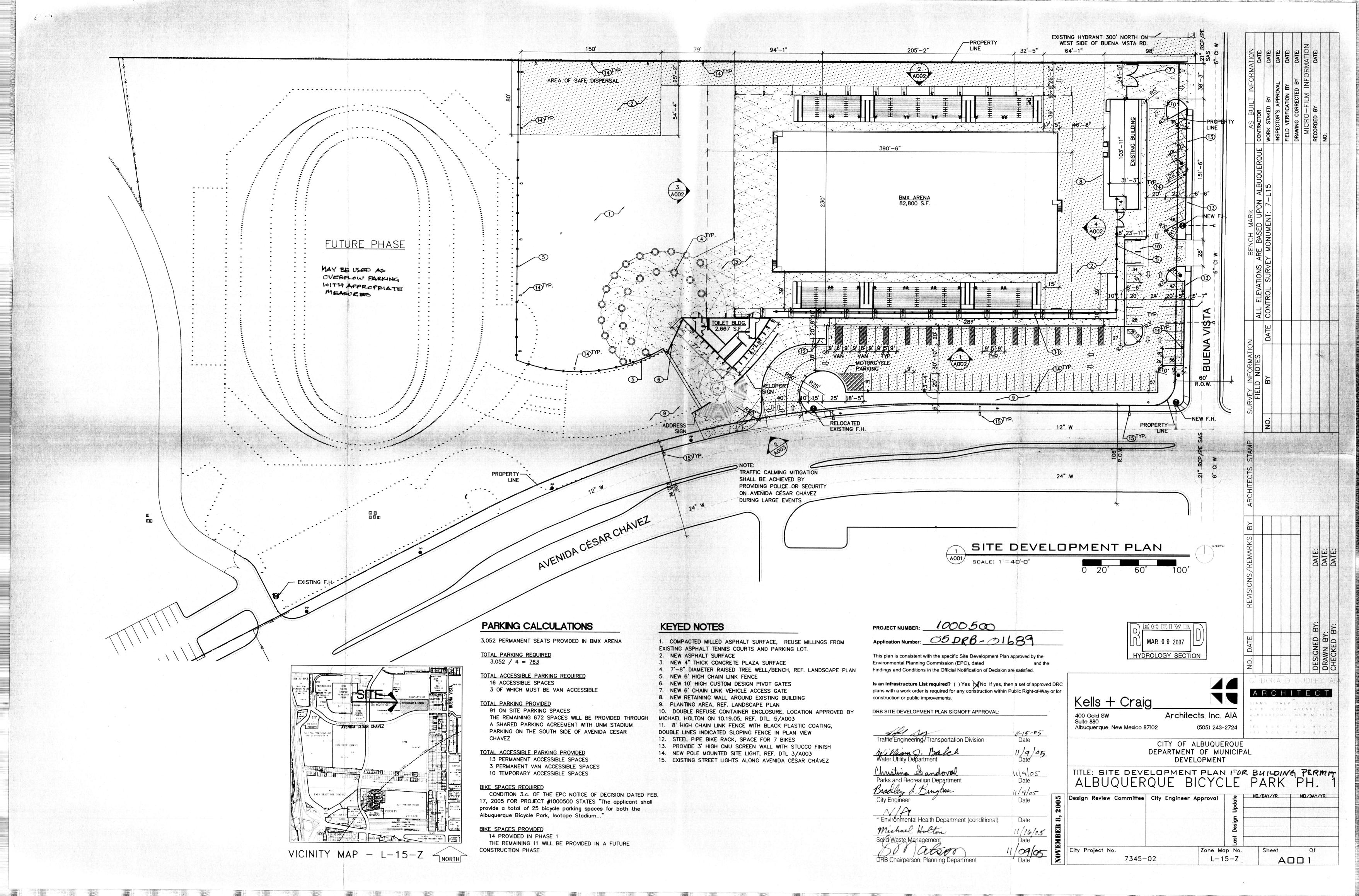
SURFACE DISTURBANCE.

13. FOR EXAMPLES OF ADDITIONAL BMP'S, REFER TO THE NATIONAL

(ALTERNATE INSTALLATION METHOD)



B (: B): B (: B)



THIS SUBMITTAL IS MADE IN SUPPORT OF SITE PLAN FOR BUILDING PERMIT (EPC AND DRB) WITHIN THE JURISDICTION OF THE CITY OF ALBUQUERUQE. THIS PLAN SUPERCEDES THE PRIOR PLAN DATED 12-17-2004.

II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP, THE SITE IS LOCATED ON THE NORTH SIDE OF AVENIDA CESAR CHAVEZ BETWEEN UNIVERSITY BLVD. SE AND BUENA VISTA DRIVE SE. AT PRESENT, THE SITE IS UNDEVELOPED. THE LAND IMMEDIATELY TO THE WEST IS DEVELOPED AS ISOTOPES BALL PARK. THE LAND TO THE NORTH IS INSTITUTIONAL (TVI AND UNM). THE LAND TO THE EAST IS DEVELOPED AS CITY TENNIS COURTS. AS SHOWN BY PANEL 334 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, AND INCORPORATED AREAS DATED SEPTEMBER 20, 1996, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE (ZONE AO). AN AO ZONE IS MAPPED DOWNSTREAM, HOWEVER, IT IS BELIEVED THAT THE MORE RECENT STORM DRAIN IMPROVEMENTS WITHIN AVENIDA CESAR CHAVEZ (FORMERLY STADIUM BLVD.) HAVE ALLEVIATED THE DOWNSTREAM FLOODING.

III. BACKGROUND DOCUMENTS & RESEARCH

THE FOLLOWING ITEMS WERE REVIEWED IN THE PREPARATION OF THIS SUBMITTAL: A. TOPOGRAPHIC SURVEY OF THE EXISTING SITE PREPARED BY WILSON & CO. DATED 10/21/2004. THE SUBJECT SURVEY SHOWS THE EXISTING IMPROVEMENTS. B. PRE-DESIGN CONFERENCE WITH THE CITY HYDROLOGIST 08/24/2004. THE OUTCOME OF THAT RECAP

CONFIRMED THAT FREE DISCHARGE TO AVENIDA CESAR CHAVEZ IS APPROPRIATE IN LIGHT OF THE RECNET DRAINAGE IMPROVEMENTS TO UNIVERSITY BLVD SE AND AVENIDA CESAR CHAVEZ SE. C. CONCEPTUAL GRADING AND DRAINAGE PLAN (LIS/D-) PREPARED BY THIS OFFICE DATED 12-17-2004. THIS PRIOR SUBMITTAL ESTABLISHED THE DRAINAGE CRITERIA (FREE DISCHARGE) FOR THIS SITE.

IV. EXISTING CONDITIONS

THE TOPOGRAPHIC DATA PRESENTED HEREWITH, TAKEN FROM THE ABOVE REFERENCED SURVEY BY WILSON & CO., DEMONSTRATES THE EXISTING CONDITIONS OF THE PROJECT SITE. AT PRESENT, THE SITE IS PARTIALLY DEVELOPED WITH CITY TENNIS COURTS AND LIMITED ASPHALT PAVING. THE SITE CURRENTLY DRAINS FROM EAST TO WEST WITH MUCH OF THE DEVELOPED FLOW BEING DIRECTED INTO A TEMPORARY DETENTION POND. IT IS BELIEVED THAT THE TEMPORARY POND WAS CREATED TO MITIGATE THE DISHARGE OF EXCESS RUNOFF ONTO THE ISOTOPES BALL PARK. THE SITE IS SITUATED TOPOGRAPHICALLY HIGHER THAN AVENIDA CESAR CHAVEZ SE AND AS SUCH DRAINS TO THAT PUBLIC ROADWAY. ISOTOPES BALL PARK IS TOPOGRAPHICALLY LOWER AN HENCE DOES NOT CONTRIBUTE OFFSITE FLOWS. THE INSTITUTIONAL DEVELOPMENT TO THE NORTH HAS BEEN GRADED SUCH THAT IT DOES NOT DRAIN ONTO THE SUBJECT SITE. ITS FLOWS ARE DIRECTED WEST AND EVENTUALLY DISCHARGE TO UNIVERSITY BLVD SE. BUENA VISTA DRIVE LIES TO THE EAST AND APPEARS TO CONTAIN ITS FLOWS AND AS SUCH IS NOT BELIEVED TO CONTRIBUTE OFFSITE FLOWS.

V. PROPOSED CONDITIONS

THE PROPOSED IMPROVEMENTS CONSIST OF A VELODROME, A BMX TRACK, SEVERAL BUILDINGS, PARKING IMPROVEMENTS, PEDESTRIAN PAVING AND ASSOCIATED LANDSCAPING. IN ORDER TO LOCATE THE ABOVE REFERENCED IMPROVMENTS ON THE SITE, SEVERAL OF THE EXISTING TENNIS COURTS AND RELATED IMPROVMENTS WILL REQUIRE DEMOLITION. SIMILARLY, THE TEMPORARY DETENTION POND WILL BE

FOR THE PURPOSES OF ANALYSIS, THE SITE HAS BEEN DIVIDED INTO FOUR (4) BASIC DRAINAGE AREAS. BASIN A DRAINS INTERNALLY TO A PRIVATE STORM DRAIN SYSTEM THAT ULTIMATELY DISCHARGES TO THE AVENIDA CESAR CHAVEZ STORM DRAIN. IN AN EFFORT TO CONSERVE WATER RESOURCES, IT IS PROPOSED TO INTERCEPT THE MAJORITY OF THE STORM WATER IN AN UNDERGROUND STORAGE FACILITY TO LATER USE FOR SITE IRRIGATION. ONCE THE UNDERGROUND STORAGE IS FILLED, THE STORM WATER WILL BYPASS THE WATER HARVESTING FEATURE AND RELEASE TO THE AVENIDA CESAR CHAVEZ STORM DRAIN. PRESENTLY. THE TEMPORARY DETENTION POND OUTLETS TO THE AVENIDA CESAR CHAVEZ STORM DRAIN VIA AN 18-INCH STORM DRAIN CONNECTION TO THE BACK OF A EXISTING STORM INLET IN THE NORTH CURB LINE OF THE ROADWAY. THAT CONNECTION WILL BE UTILIZED FOR THE DISCHARGE OF RUNOFF FROM BASIN A AND ITS SUB-BASINS A-1 THROUGH A-9. IN THE EVENT THAT THE SYSTEM BECOMES OVERTAXED, THE EXISTING STORM DRAIN MANHOLE (AP-1) WILL ACT AS A BUBBLER TO RELEASE THE EXCESS VIA A GRATED LID. FROM THIS POINT, THE RUNOFF WILL FLOW ACROSS THE NEW PARKING LOT TO EVENTUALLY DISCHARGE TO AVENIDA CESAR CHAVEZ VIA AN EXISTING PRIVATE ENTRANCE.

BASIN B IS AN EXISTING CONDITION WHERE EXISTING PAVING AND BUILDING IMPROVEMENTS SHEET DRAIN TO BUENA VISTA DRIVE SE. THIS PATTERN WILL REMAIN UNALTERED. BASIN C REPRESENTS THE MAJORITY OF THE PARKING LOT IMPROVEMENTS THAT WILL FREE DISCHARGE TO AVENIDA CESAR CHAVEZ. DISCHARGE WILL OCCUR VIA EXISTING AND PROPOSED PRIVATE ENTRANCES. BASIN I IS A VERY SMALL AREA THAT WILL CONTINUE TO DRAIN ONTO THE EXISTING SERVICE ROAD THAT LIES EAST OF AND ABOVE ISOTOPES PARK. THE LIMITED TOPOGRAPHIC DATA PROVIDED DOES NOT ALLOW FOR THE DETERMINATION OF THE ULTIMATE DISCHARGE POINT FOR BASIN I, HOWEVER, THE AREA AND ASSOCIATED RUNOFF ARE SMALL.

VI. GRADING PLAN

THE GRADING PLAN SHOWS 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 2.) PROPOSED GRADES INCICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS, 3) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, AND 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED grades. As shown by this plan, the site will drain by various means. Basin a, as indicated ABOVE, WILL DRAIN VIA AN INTERNAL PRIVATE STORM DRAIN SYSTEM INTEGRATED WITH A WATER HARVESTING FEATURE. BASIN B WILL CONTINUE TO SHEET DRAIN TO BUENA VISTA DRIVE SE. BASIN C WILL SURFACE DRAIN TO AVENIDA CESAR CHAVEZ VIA EXISTING AND PROPOSED PRIVATE ENTRANCES.

VIII. CALCULATIONS

THE CALCULATIONS THAT APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS, FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. THE RESULTS OF THE CALCULATIONS FOR THE DEVELOPED CONDITION ARE PRESENTED IN TABLE FORM. KEY ANALYSIS POINTS, SHOWN ON THE DRAINAGE BASIN MAP PLAN, ARE FURTHER ANALYZED IN ORDER THAT THE PROPOSED PRIVATE STORM DRAIN BE SIZED APPROPRIATELY. THE MANNING EQUATION WAS USED TO EVALUATE PIPE SIZES AND RELATED CAPACITIES. AS A RESULT OF THIS ANALYSIS, THE TOTAL PEAK DISCHARGE CALCULATED FOR BASIN A (37.6 CFS) WILL NOT EXCEED THE CAPACITY (45.8 CFS) OF THE EXISTING 18-INCH STORM DRAIN CONNECTION TO AVENIDA CESAR CHAVEZ. OVERFLOW RUNOFF WILL ESCAPE AT AP-1 WHERE A GRATED LID WILL BE INCORPORATED INTO THE EXISTING STORM DRAIN MANHOLE WITHIN THE BASIN C-2 PARKING LOT AS FURTHER DESCRIBED ABOVE.

VIII. CONCLUSIONS

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVALUATIONS CONTAINED

- 1. THIS SUBMITTAL ESTABLISHES THE DRAINAGE CONCEPTS BY WHICH THIS PROJECT WILL DEVELOP
- 2. THE SITE DOES NOT LIE WITHIN A DESIGNATED 100-YEAR FLOODPLAIN
- 3. THIS SITE IS NOT RESTRICTED BY LIMITED OR INADEQUATE DOWNSTREAM CAPACITY 4. THE INCREASED RUNOFF FROM THIS SITE WILL BE HANDLED BY THE EXISTING DOWNSTREAM PUBLIC
- DRAINAGE IMPROVEMENTS THAT ULTIMATELY DISCHARGE TO THE SOUTH DIVERSION CHANNEL 5. THIS SUBMITTAL IS FOR SITE PLAN FOR BUILDING PERMIT (EPC AND DRB) APPROVAL AND AS SUCH
- REPRESENTS A MASTER DRAINAGE PLAN FOR THE PHASED DEVELOPMENT OF THIS SITE
- 6. SITE SPECIFIC DRAINAGE SUBMITTALS WILL BE REQUIRED FOR INDIVIDUAL PHASES OF CONSTRUCTION
- 7. OFFSITE FLOWS DO NOT IMPACT THIS SITE 8. ALL ROOF RUNOFF WILL BE PIPED DIRECTLY INTO THE PRIVATE STORM DRAIN SYSTEM.

CALCULATIONS

I. SITE CHARACTERISTICS

A. PRECIPITATION ZONE = 2

B. P_{6,100} = P₃₆₀ = 2.35 N.

D. EXISTING LAND TREATMENT

C. TOTAL AREA $(A_{T}) = 501900 \text{ SF}/11.5 \text{ AC}$

7525/0.17 316,573/7.27

177,802/4.08

II. EXISTING CONDITION

A. VOLUME

 $E^{\mathbf{M}} = (E^{\mathbf{A}}^{\mathbf{A}} + E^{\mathbf{B}}^{\mathbf{B}} + E^{\mathbf{C}}^{\mathbf{A}} C^{\mathbf{E}}^{\mathbf{D}}^{\mathbf{A}}) / A^{\mathbf{T}}$

 $E_{W} = [0.78(0.17) + 1.13(7.27) + 2.12(4.08]/11.52 = 1.48 \text{ IN}$

 $V_{100} = (E_{W}/12)A_{T}$

 $V_{100} = (1.48/12)11.52 = 1.42 \text{ oc-ft} = 61890 \text{ CF } (1.42 \text{ AC/FT.})$

B. PEAK DISCHARGE

 $d^{b} = d^{bA}V^{A} + d^{bB}V^{B} + d^{bC}V^{C} + d^{bD}V^{D}$

 $Q_p = Q_{100} = 2.28(0.17) + 3.14(7.27) + 4.70(4.08) = 42.4 cfs$

M. DEVELOPED CONDITION

THE DEVELOPED CONDITION HYDROLOGY HAS BEEN INCORPORATED INTO TABLE FORMAT AS PRESENTED BELOW.

	MASTER PLAN BASIN ANALYSIS											
BASINS	A _T (SFIAC)	A _B (SF/AC)	A _C (sfiac)	A _D (SFIAC)	V ₁₀₀ (CF8)	Q ₁₀₀ (CF8)	Q _{100(CF8)} CUN					
A-1	26,154/0.60	300/0.01		25,854/0.59	0.10	2.80	2.80					
A-2	11,475/0.26		X X	11,475/0.26	0.05	1.22	4.02					
A-3	11,475/0.26			11,475/0.26	0.05	1.22	5.24					
A4	13,939/0.32			13,939/0.32	0.06	1.50	6.74					
A-6	25,560/0.59			25,560/0.59	0.10	2.77	9.51					
A-6	73,927/1.70			73,927/1.70	0.30	7.99	17.50					
A-7	37,629/0.86			37,629/0.86	0.15	4.04	21.54					
A-6	63,162/1.45			63,162/1.45	0.26	6.82	28.36					
B-1	65,340/1.50	27,663/0.64		37,677/0.86	0.19	5.50 /						
B-2	64,469/1.48			64,469/1.48	0.26	6.96						
B-3	78,408/1.80	8,690./0.20		69,718/1.60	0.30	7.98						
NW.	30,492/0.70			30,492/0.70	0.18	3.29	V					
TOTAL	502,029/11.52	36,353/0.85		465,377/10.67	1.84	52.09 🗸						

IV. COMPARISON

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A. VOLUME

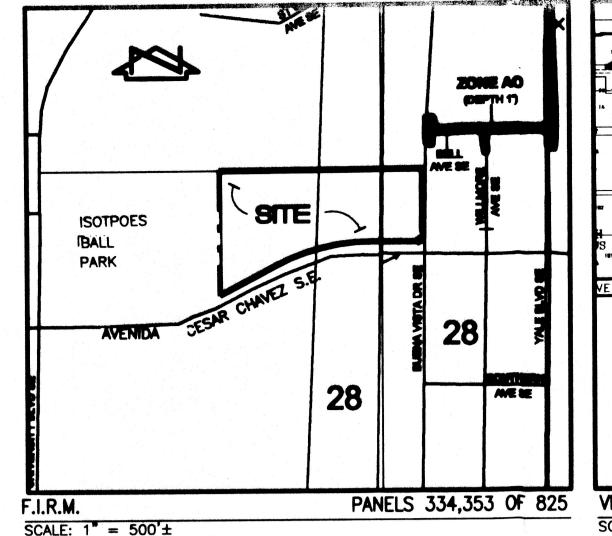
 $\Delta V_{100} = 1.84 - 1.42 = 0.42$ AC. FT.

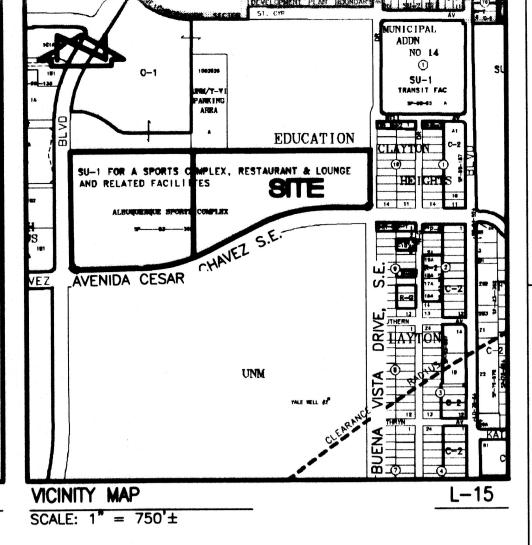
= 18,300 CF.

B. PEAK DISCHARGE

 $\Delta Q_{100} = 52.1-42.4 = 9.7$ CFS

KEY	AP	STRUCTURE	RIM/TG	INV. IN	INV. OUT
1	#1	EXIST SDMH	5123.09'	5119.90'	5119.74'
2	# 2	SINGLE 'D' INLET	5140.00'	5131.05'	5130.95
3	NA	SINGLE 'D' INLET	5140.00'	5132.81	5132.71
4	# 3	SINGLE 'D' INLET	5140.00'	5134.05	5133.95'
5	#4	4' SDMH	5140.70'	5134.35'	5134.05
6	# 5	SINGLE 'D' INLET	5143.00'	5137.05	5136.95
7	# 6	SINGLE 'D' INLET	5143.00'	5137.75'	5137.65
8	# 7	SINGLE 'D' INLET	5143.00'	5138.45	5138.35
9	# 8	SINGLE 'D' INLET	5143.00'	N/A	5139.10
10	# 9	4' SDMH	5143.70'	N/A	5137.65
11	# 10	EXIST CURB INLET	5115.43'	5110.5	5109.88
12	# 11	4' SDMH	5123.00'	5117.10	5117.00'
13	# 12	4' SDMH	5140.00'	5133.90'	5133.80'
14	#13	4' SDMH	5140.50	N/A	5134.50





	MASTER PLAN STORM DRAIN HYDRAULICS										
AP TO AP	PIPE SIZE	LENGTH	SLOPE	Q ₁₀₀ (CFS)	Q _{CAP} (CFS)	VEL (FPS)	PIPE INVERTS				
SDMH 262 TO AP10	36"	140'	0.033	38.1	121.1	17.1	5105.21' TO 5109.88'				
AP10 TO CURB INLET	36"	80'	0.058	37.5	160.6	22.7	5109.88' TO 5114.53'				
CURB INLET TO AP1	18"	30'	0.179	37.6	44.4	25.2	5114.53' TO 5119.90'				
AP1 TO AP2	24"	350'	0.032	28.36	40.5	12.9	5119.90' TO 5131.00'				
AP2 TO AP3	24"	191'	0.016	24.32	27.7	9.1	5131.00' TO 5134.00'				
AP3 TO AP4	24"	17'	0.012	6.74	24.8	7.9	5134.00' TO 5134.20'				
AP4 TO AP5	18"	255'	0.011	6.74	11.0	6.2	5134.20' TO 5137.00'				
AP5 TO AP6	18"	68'	0.010	5.24	10.5	5.9	5137.00' TO 5137.70'				
AP6 TO AP7	18"	68'	0.010	4.02	10.5	5.9	5137.70' TO 5138.40'				
AP7 TO AP8	18"	68'	0.010	2.80	10.5	5.9	5138.40' TO 5139.10'				
AP2 TO AP9	12"	210'	0.032	4.04	6.4	10.6	5131.00 TO 5137.70'				
				, , , , , , ,							
AP10 TO AP11	18"	90'	0.072	6.96	28.2	16.0	5110.5' TO 5117.0'				
AP11 TO AP12	18"	140'	0.119	6.96	36.2	20.5	5117.1' TO 5133.8'				
AP12 TO AP13	18"	60'	0.010	6.96	10.5	5.9	5133.9' TO 5134.5'				

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Kells + Craig

Abuquerque, New Mexico 87102

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Architects, Inc. AIA

G. DONALD DUDLEY AI SIMMS TOWER STUDIO 850 4 0 0 BOLD AVENUE SW ALBUQUERQUE, NEW MEXICO TEL 808.2 4 3.8100 FAX 8 0 8 . 2 4 3 . 0 1 0 1

Sheet

CITY OF ALBUQUERQUE DEPARTMENT OF MUNICIPAL DEVELOPMENT

(505) 243-2724

Zone Map No.

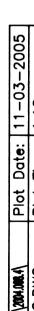
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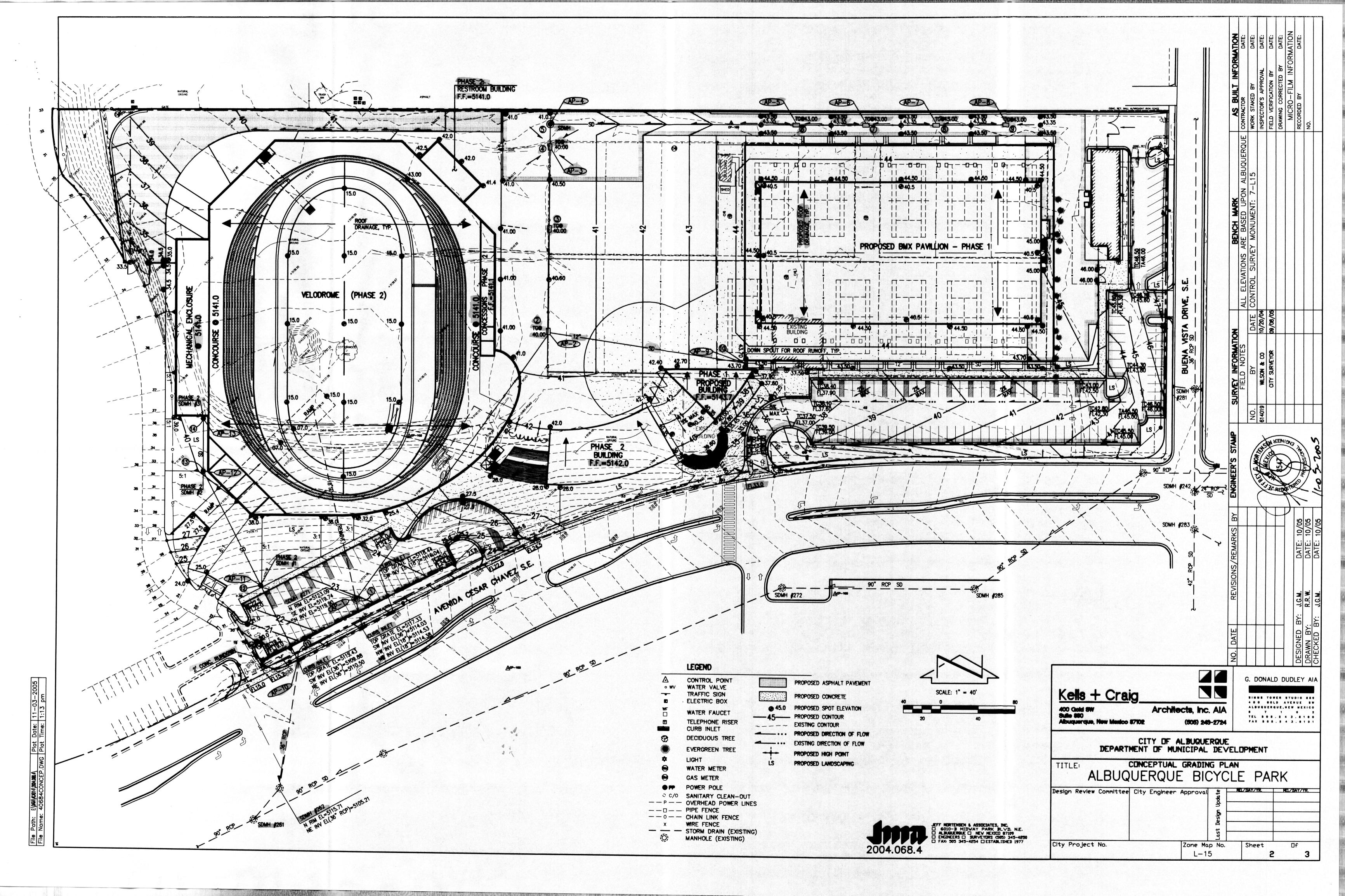
TITLE: CONCEPTUAL DRAINAGE PLAN AND CALCULATIONS ALBUQUERQUE BICYCLE PARK

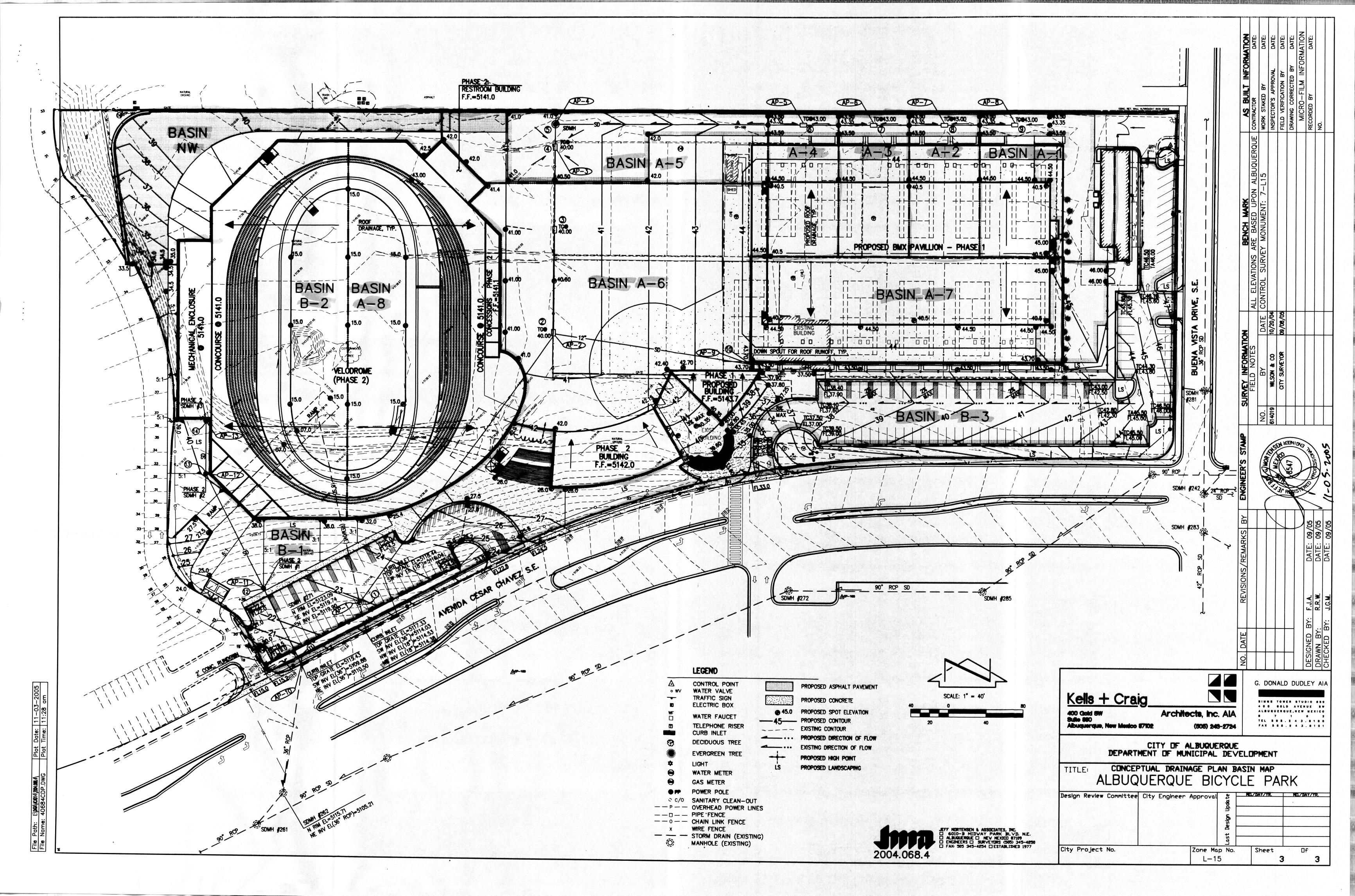
Design Review Committee City Engineer Approval

FF NURTENSEN & ASSOCIATES, INC.
6010-3 MIDWAY PARK BLVD. N.E.
ALBUQUERQUE | NEV MEXICO 87189
ENGINEERS | SURVEYORS (505) 345-4250 FAX: 505 345-4254 ESTABLISHED 1977

City Project No. 2004.068.4







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RECNET DRAINAGE IMPROVEMENTS TO UNIVERSITY BLVD SE AND AVENIDA CESAR CHAVEZ SE.

IV. EXISTING CONDITIONS

THE TOPOGRAPHIC DATA PRESENTED HEREWITH, TAKEN FROM THE ABOVE REFERENCED SURVEY BY WILSON & CO., DEMONSTRATES THE EXISTING CONDITIONS OF THE PROJECT SITE. AT PRESENT, THE SITE IS PARTIALLY DEVELOPED WITH CITY TENNIS COURTS AND LIMITED ASPHALT PAVING. THE SITE CURRENTLY DRAINS FROM EAST TO WEST WITH MUCH OF THE DEVELOPED FLOW BEING DIRECTED INTO A TEMPORARY DETENTION POND. IT IS BELIEVED THAT THE TEMPORARY POND WAS CREATED TO MITIGATE THE DISHARGE OF EXCESS RUNOFF ONTO THE ISOTOPES BALL PARK. THE SITE IS SITUATED TOPOGRAPHICALLY HIGHER THAN AVENIDA CESAR CHAVEZ SE AND AS SUCH DRAINS TO THAT PUBLIC ROADWAY. ISOTOPES BALL PARK IS TOPOGRAPHICALLY LOWER AN HENCE DOES NOT CONTRIBUTE OFFSITE FLOWS. THE INSTITUTIONAL DEVELOPMENT TO THE NORTH HAS BEEN GRADED SUCH THAT IT DOES NOT DRAIN ONTO THE SUBJECT SITE. ITS FLOWS ARE DIRECTED WEST AND EVENTUALLY DISCHARGE TO UNIVERSITY BLVD SE. BUENA VISTA DRIVE LIES TO THE EAST AND APPEARS TO CONTAIN ITS FLOWS AND AS SUCH IS NOT BELIEVED TO CONTRIBUTE OFFSITE FLOWS.

V. PROPOSED CONDITIONS

THE PROPOSED IMPROVEMENTS CONSIST OF A VELODROME, A BMX TRACK, SEVERAL BUILDINGS, PARKING IMPROVEMENTS, PEDESTRIAN PAVING AND ASSOCIATED LANDSCAPING. IN ORDER TO LOCATE THE ABOVE REFERENCED IMPROVMENTS ON THE SITE, SEVERAL OF THE EXISTING TENNIS COURTS AND RELATED IMPROVMENTS WILL REQUIRE DEMOLITION. SIMILARLY, THE TEMPORARY DETENTION POND WILL BE REMOVED.

FOR THE PURPOSES OF ANALYSIS, THE SITE HAS BEEN DIVIDED INTO FOUR (4) BASIC DRAINIGE AREAS. BASIN A DRAINS INTERNALLY TO A PRIVATE STORM DRAIN SYSTEM THAT ULTIMATELY DISCHARGES TO THE AVENIDA CESAR CHAVEZ STORM DRAIN. IN AN EFFORT TO CONSERVE WATER RESOURCES, IT IS PROPOSED TO INTERCEPT THE MAJORITY OF THE STORM WATER IN AN UNDERGROUND STORAGE FACILITY TO LATER USE FOR SITE IRRIGATION. ONCE THE UNDERGROUND STORAGE IS FILLED, THE STORM WATER WILL BYPASS THE WATER HARVESTING FEATURE AND RELEASE TO THE AVENIDA CESAR CHAVEZ STORM DRAIN. PRESENTLY. THE TEMPORARY DETENTION POND OUTLETS TO THE AVENIDA CESAR CHAVEZ STORM DRAIN VIA AN 18-INCH STORM DRAIN CONNECTION TO THE BACK OF A EXISTING STORM INLET IN THE NORTH CURB LINE OF THE ROADWAY. THAT CONNECTION WILL BE UTILIZED FOR THE DISCHARGE OF RUNOFF FROM BASIN A AND ITS SUB-BASINS A-1 THROUGH A-9. IN THE EVENT THAT THE SYSTEM BECOMES OVERTAXED, THE EXISTING STORM DRAIN MANHOLE (AP-1) WILL ACT AS A BUBBLER TO RELEASE THE EXCESS VIA A GRATED LID. FROM THIS POINT, THE RUNOFF WILL FLOW ACROSS THE NEW PARKING LOT TO EVENTUALLY DISCHARGE TO AVENIDA CESAR CHAVEZ VIA AN EXISTING PRIVATE ENTRANCE.

BASIN B IS AN EXISTING CONDITION WHERE EXISTING PAVING AND BUILDING IMPROVEMENTS SHEET DRAIN TO BUENA VISTA DRIVE SE. THIS PATTERN WILL REMAIN UNALTERED. BASIN C REPRESENTS THE MAJORITY OF THE PARKING LOT IMPROVEMENTS THAT WILL FREE DISCHARGE TO AVENIDA CESAR CHAVEZ. DISCHARGE WILL OCCUR VIA EXISTING AND PROPOSED PRIVATE ENTRANCES. BASIN I IS A VERY SMALL AREA THAT WILL CONTINUE TO DRAIN ONTO THE EXISTING SERVICE ROAD THAT LIES EAST OF AND ABOVE ISOTOPES PARK. THE LIMITED TOPOGRAPHIC DATA PROVIDED DOES NOT ALLOW FOR THE DETERMINATION OF THE ULTIMATE DISCHARGE POINT FOR BASIN I, HOWEVER, THE AREA AND ASSOCIATED RUNOFF ARE SMALL.

VI. GRADING PLAN

THE GRADING PLAN SHOWS 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 2.) PROPOSED GRADES INCICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS, 3) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS TAKEN FROM THE SURVEY BY WILSON & CO. REFERENCED ABOVE, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, AND 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE SITE WILL DRAIN BY VARIOUS MEANS. BASIN A, AS INDICATED ABOVE, WILL DRAIN VIA AN INTERNAL PRIVATE STORM DRAIN SYSTEM INTEGRATED WITH A WATER HARVESTING FEATURE. BASIN B WILL CONTINUE TO SHEET DRAIN TO BUENA VISTA DRIVE SE. BASIN C WILL SURFACE DRAIN TO AVENIDA CESAR CHAVEZ VIA EXISTING AND PROPOSED PRIVATE ENTRANCES.

VII. CALCULATIONS

THE CALCULATIONS THAT APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS, FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. THE RESULTS OF THE CALCULATIONS FOR THE DEVELOPED CONDITION ARE PRESENTED IN TABLE FORM. KEY ANALYSIS POINTS, SHOWN ON THE DRAINAGE BASIN MAP PLAN, ARE FURTHER ANALYZED IN ORDER THAT THE PROPOSED PRIVATE STORM DRAIN BE SIZED APPROPRIATELY. THE MANNING EQUATION WAS USED TO EVALUATE PIPE SIZES AND RELATED CAPACITIES. AS A RESULT OF THIS ANALYSIS, THE TOTAL PEAK DISCHARGE CALCULATED FOR BASIN A (37.6 CFS) WILL NOT EXCEED THE CAPACITY (45.8 CFS) OF THE EXISTING 18-INCH STORM DRAIN CONNECTION TO AVENIDA CESAR CHAVEZ. OVERFLOW RUNOFF WILL ESCAPE AT AP-1 WHERE A GRATED LID WILL BE INCORPORATED INTO THE EXISTING STORM DRAIN MANHOLE WITHIN THE BASIN C-2 PARKING LOT AS FURTHER DESCRIBED ABOVE.

VIII. CONCLUSIONS

7. OFFSITE FLOWS DO NOT IMPACT THIS SITE

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVALUATIONS CONTAINED

1. THIS SUBMITTAL ESTABLISHES THE DRAINAGE CONCEPTS BY WHICH THIS PROJECT WILL DEVELOP

2. THE SITE DOES NOT LIE WITHIN A DESIGNATED 100-YEAR FLOODPLAIN 3. THIS SITE IS NOT RESTRICTED BY LIMITED OR INADEQUATE DOWNSTREAM CAPACITY

4. THE INCREASED RUNOFF FROM THIS SITE WILL BE HANDLED BY THE EXISTING DOWNSTREAM PUBLIC DRAINAGE IMPROVEMENTS THAT ULTIMATELY DISCHARGE TO THE SOUTH DIVERSION CHANNEL

5. THIS SUBMITTAL IS FOR SITE PLAN FOR BUILDING PERMIT (EPC AND DRB) APPROVAL AND AS SUCH REPRESENTS A MASTER DRAINAGE PLAN FOR THE PHASED DEVELOPMENT OF THIS SITE 6. SITE SPECIFIC DRAINAGE SUBMITTALS WILL BE REQUIRED FOR INDIVIDUAL PHASES OF CONSTRUCTION

8. WATER HARVESTING WILL BE INCORPORATED INTO THE CONSTRUCTION PLANS FOR THE INTERNAL PRIVATE STORM DRAIN SYSTEM SERVING BASIN A

CALCULATIONS

I. PRECIPITATION ZONE = 2

IV. EXISTING LAND TREATMENT

II. $P_{6,100} = P_{360} = 2.35$ IN.

III. TOTAL AREA $(A_T) = 501900 \text{ SF}/11.5 \text{ AC}$

7525/0.17 316,573/7.27

AREA (SF/AC)

177,802/4.08

V. EXISTING CONDITION

A. VOLUME

 $E_{\mathbf{W}} = (E_{\mathbf{A}} A_{\mathbf{A}} + E_{\mathbf{B}} A_{\mathbf{B}} + E_{\mathbf{C}} A_{\mathbf{C}} E_{\mathbf{D}} A_{\mathbf{D}}) / A_{\mathbf{T}}$

 $E_W = [0.78(0.17) + 1.13(7.27) + 2.12(4.08]/11.52 = 1.48 \text{ IN}$

 $V_{100} = (E_W/12)A_T$

 $V_{100} = (1.48/12)11.52 = 1.42 \text{ ac-ft} = 61890 \text{ CF } (1.42 \text{ AC/FT.})$

B. PEAK DISCHARGE

 $Q_{P} = Q_{PA}^{A} + Q_{PB}^{A} + Q_{PC}^{A} + Q_{PD}^{A}$

 $Q_{\rm p} = Q_{100} = 2.28(0.17) + 3.14(7.27) + 4.70(4.08) = 42.4 cfs 1

VI. DEVELOPED CONDITION

THE DEVELOPED CONDITION HYDROLOGY HAS BEEN INCORPORATED INTO TABLE FORMAT AS PRESENTED BELOW.

BASIN ANALYSIS										
BASINS	A _T (SF/AC)	A _B (SF/AC)	A _C (SF/AC)	A _C (SF/AC)	A _D (SF/AC)	Q ₁₀₀ (CFS)				
A-1	104,919/2.41	0	20,306/0.47	84,613/1.94	0.39	10.59				
A-2	10,036/0.23	5,887/0.14	0	4,149/0.09	0.03	0.74				
A-3	24,809/0.57	0	0	24,809/0.57	0.10	2.68				
A4	3,900/0.09	0	0	3,900/0.09	0.02	0.42				
A-5	9,962/0.23	77/0.003	0	9,885/0.227	0.04	1.07				
A-6	77,044/1.77	693/0.02	0	76,351/1.75	0.31	8.27				
A-7	36,570/0.84	0	0	36,570/0.84	0.15	3.95				
A-8	34,490/0.79	154/0.01	12,821/0.29	21,515/0.49	0.11	3.24				
A-9	66,217/1.52	10,627/0.24	386/0.009	55,204/1.27	0.24	6.54				
A-10	5,047/0.12	0	0	5,047/0.12	0.02	0.56				
В	11,213/0.26	930/0.02	0	10,283/0.24	0.04	1.17				
C-1	80,179/1.84	16,134/0.37	-0	64,045/1.47	0.28	7.75				
C-2	21,886/0.50	2,360/0.05	Ö	19,526/0.45	0.08	2.23				
	12,315/0.28	0	1,758/0.04	10,557/0.24	0.05	1.25				
TOTAL	498,587/11.5	36,862/0.85	35,271/0.81	426,454/9.79	1.86					

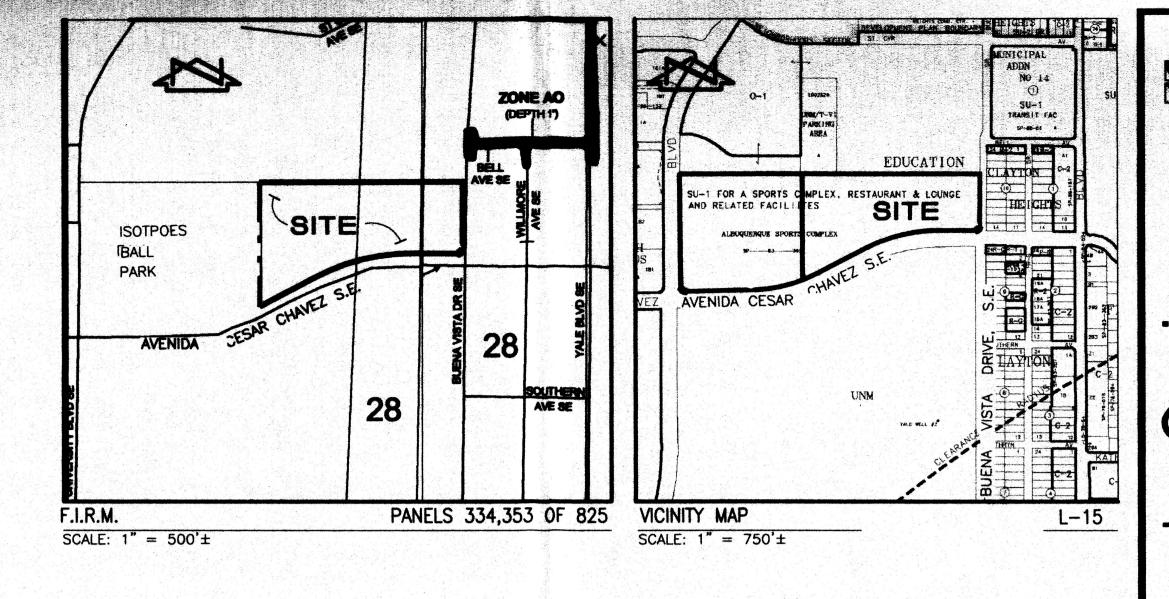
VII. COMPARISON

A. VOLUME

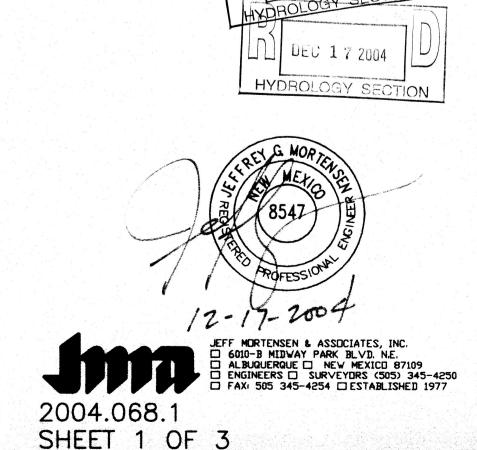
 $\Delta V_{100} = 1.86 - 1.42 = 0.44$ AC. FT.

PEAK DISCHARGE

 $\Delta Q_{100} = 50.5 - 42.4 = 8.1 \text{ CFS}$



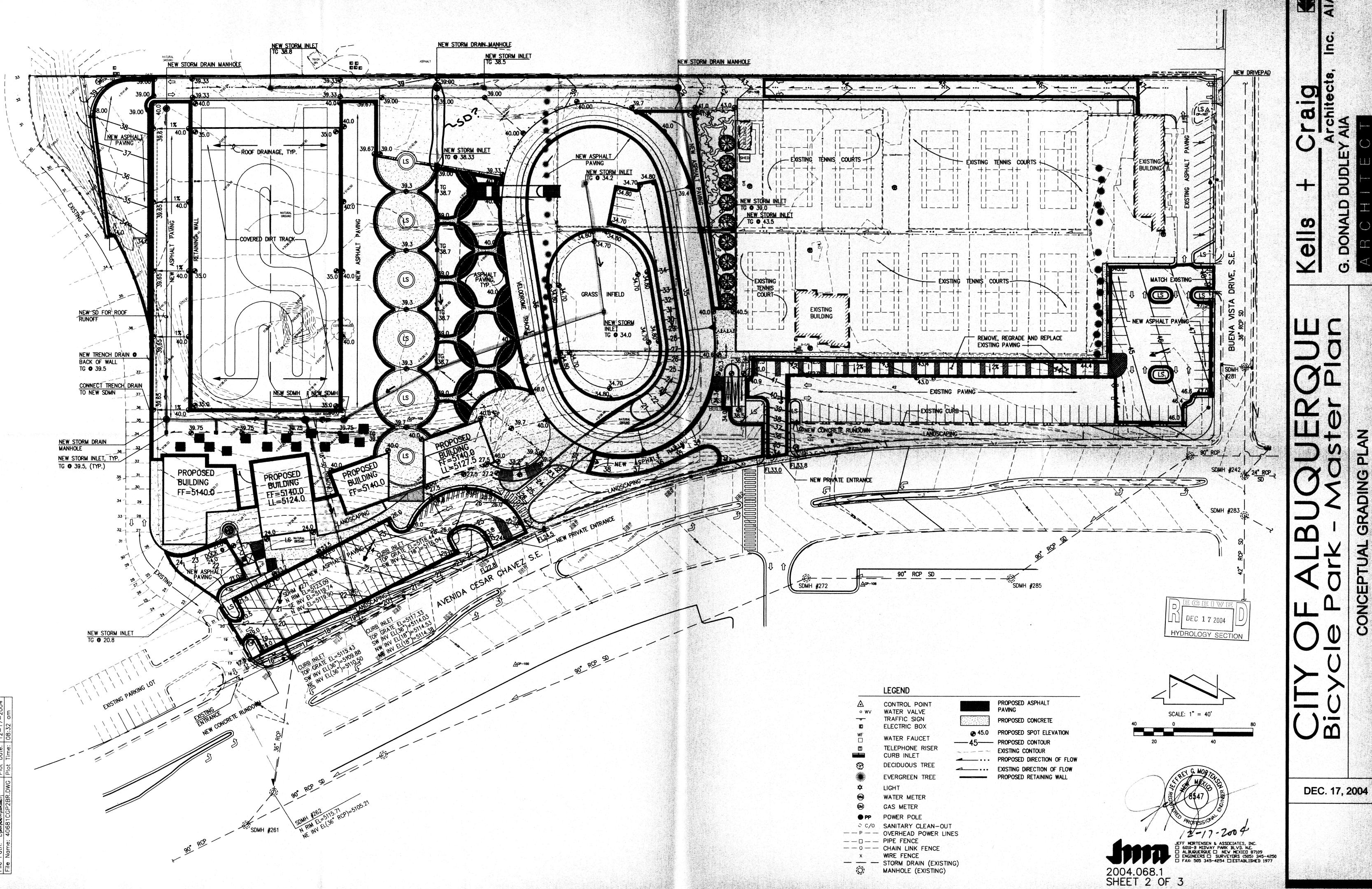
AP TO AP	PIPE SIZE	LENGTH	SLOPE	Q100 (CFS)	QCAP (CFS)	PIPE INVERTS
SDMH 262 TO AP13	36*	140'	0.033	38.1	121.1	5105.21 (SDMH262)
AP13 TO CURB	36°	80'	0.052	37.5	152.0	5109.88' (AP13)
CURB TOI AP1	18"	30'	0.190	37.6	45.8	5114.53' (CURB)
AP1 TO AP2	36"	185'	0.044	37.6	139.9	5119.9' (AP1)
AP2 TP AP3	36°	40'	0.005	33.6	47.1	5128.1' (AP2)
AP3 TO AP4	36"	110'	0.005	30.3	47.1	5128.3' (AP3)
AP4 TP AP5	36 *	207'	0.006	23.8	51.6	5128.8' (AP4)
AP5 TO AP6	24"	80'	0.006	14.4	17.5	5130.0' (AP5)
AP6 TO AP6A	24"	48'	0.010	14.0	22.6	5130.5' (AP6)
AP6A TO AP7	24"	195'	0.010	11.3	22.6	5131.0' (AP6A)
AP7 TO AP8	18"	147'	0.014	11.3	12.4	5133.0' (AP7)
AP8 TO AP9	18"	33 '	0.014	10,6	12.4	5135.0' (AP8)
						5139.5' (AP9)
AP4 TO AP15	18*	180'	0.004	6.54	6.6	5128.8' (AP4)
AP15 TO AP16	18"	130'	0.005	6.54	6.6	5129.5' (AP15)
						5130.2' (AP16)
AP13 TO AP14	12"	110'	0.040	0.56	7.1	5110.5' (AP13)
						5114.8' (AP14)
AP2 TO AP11	18"	140'	0.016	7.2	13.3	5128.1' (AP2)
AP11 TO AP12	18"	320'	0.005	3.95	7.4	5130.3' (AP11)
						5131.9' (AP12)
AP6 TO AP10	12"	170'	0.014	0.42	4.2	5130.5' (AP6)
						5132.8' (AP10)

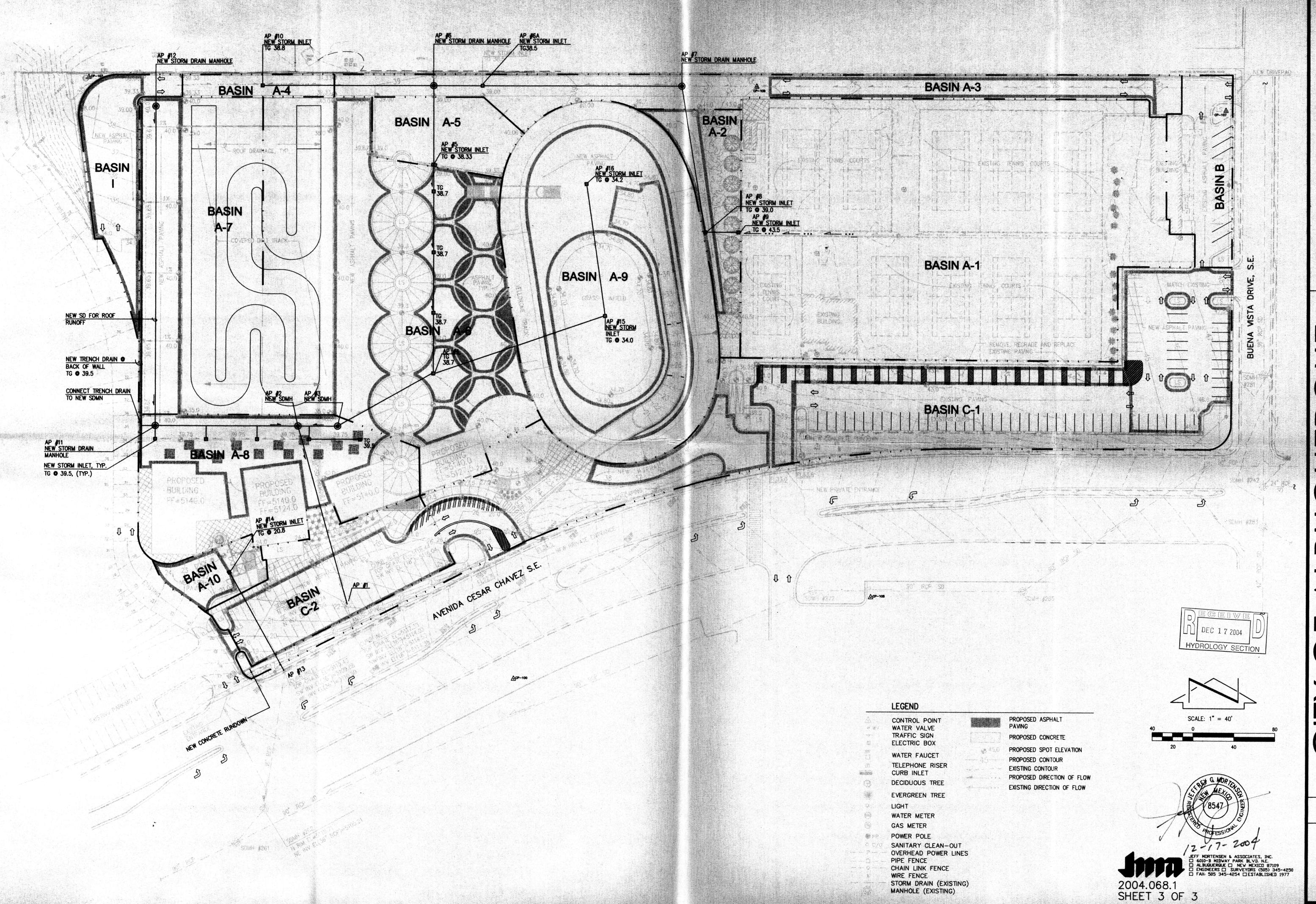


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