

DRAINAGE INFORMATION SHEET

PROJECT TITLE: Crossland ZONE ATLAS/DRNG, FILE#: L-16/1023 F
DRB #: 97-533 EPC #: Z-97-126 WORK ORDER #: _____
LEGAL DESCRIPTION: Lot 3A, Cavan Sunport Addition
CITY ADDRESS: 2321 International
ENGINEERING FIRM: Mark Goodwin & Associates CONTACT: John MacKenzie
ADDRESS: P.O. Box 90606 PHONE: 828-2200
OWNER: ESA Management Inc. CONTACT: John Lidbury
ADDRESS: 5495 Beltline Rd, Suite 130, Dallas PHONE: 972-726-8568
ARCHITECT: Alliance Architects CONTACT: Robert Nieporte, Jr.
ADDRESS: 12750 Merit Dr., Suite 550 LBS2, Dallas PHONE: 972-233-0400
SURVEYOR: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CONTRACTOR: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____

TYPE OF SUBMITTAL:

CHECK TYPE OF APPROVAL SOUGHT:

☐ DRAINAGE REPORT
☐ DRAINAGE PLAN
☐ CONCEPTUAL GRADING & DRAINAGE PLAN
☐ GRADING PLAN
☐ EROSION CONTROL PLAN
☒ ENGINEER'S CERTIFICATION
☐ OTHER

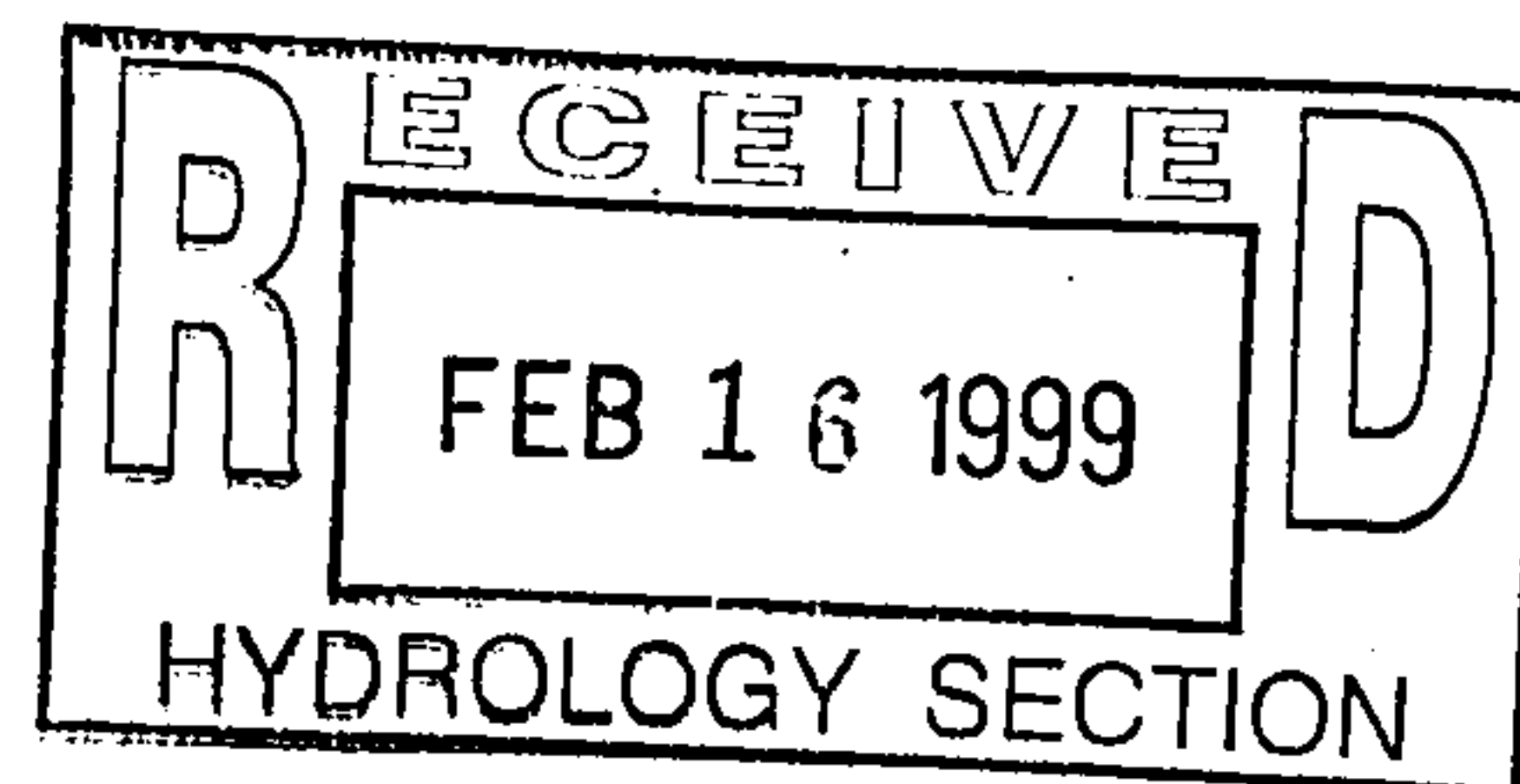
☐ SKETCH PLAT APPROVAL
☐ PRELIMINARY PLAT APPROVAL
☐ S. DEV. PLAN FOR SUB'D APPROVAL
☐ S. DEV. PLAN FOR BLDG PERMIT APPROVAL
☐ SECTOR PLAN APPROVAL
☐ FINAL PLAT APPROVAL
☐ FOUNDATION PERMIT APPROVAL
☐ BUILDING PERMIT APPROVAL
☒ CERTIFICATION OF OCCUPANCY APPROVAL
☐ GRADING PERMIT APPROVAL
☐ PAVING PERMIT APPROVAL
☐ S.A.D. DRAINAGE REPORT
☐ DRAINAGE REQUIREMENTS
☐ OTHER _____ (Specify)

PRE-DESIGN MEETING:

☐ YES
☒ NO
☐ COPY PROVIDED

DATE SUBMITTED: 2/15/99

BY: John MacKenzie
John MacKenzie





March 18, 1998

John MacKenzie, P.E.
Mark Goodwin & Associates
P.O. Box 90606
Albuquerque, NM 87199

(L-16/D23F)

RE: *CROSSLAND (~~L-16-D23~~). GRADING AND DRAINAGE PLAN FOR SITE DEVELOPMENT PLAN FOR BUILDING PERMIT, BUILDING PERMIT, AND SO #19 PERMIT APPROVALS. ENGINEER'S STAMP DATED 3-10-98.*

Dear Mr. MacKenzie:

Based on the information provided on your December 15, 1997 submittal, the above referenced project is approved for Site Development Plan for Building Permit, Building Permit and SO #19 Permit.

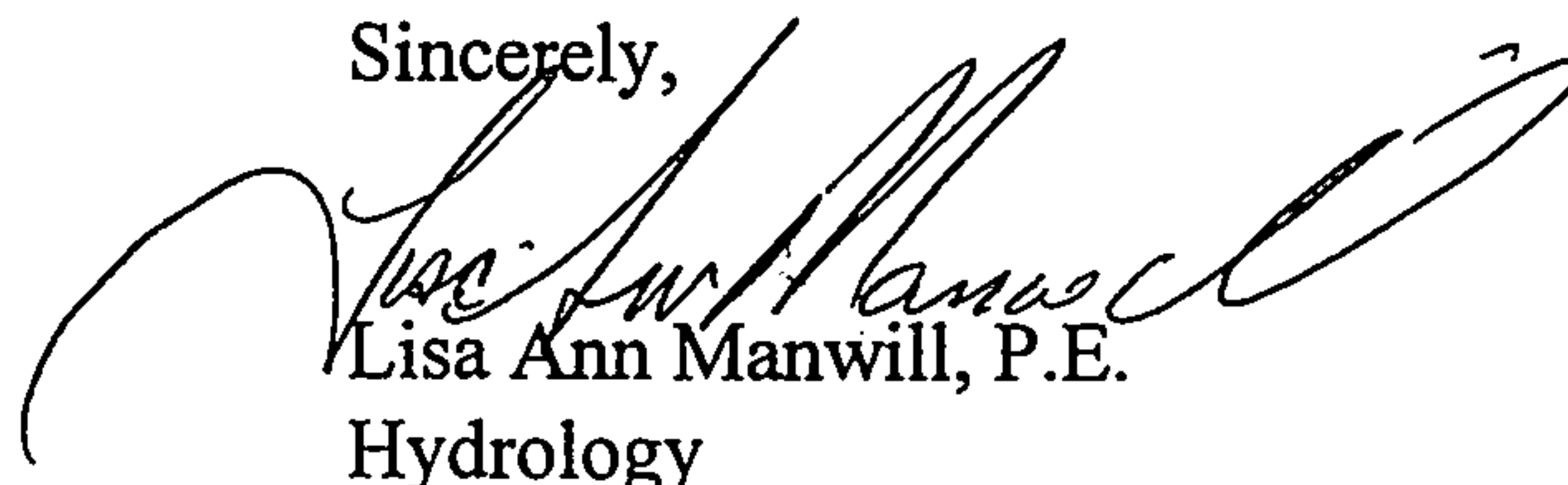
Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

A separate permit is required for construction within the City right-of-way. A copy of this approval letter must be on hand when applying for the excavation permit.

Prior to Certificate of Occupancy approval, an Engineer's Certification will be required.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,



Lisa Ann Manwill, P.E.
Hydrology

c: Arlene Portillo
Andrew Garcia
File

Good for You, Albuquerque!



CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

March 18, 1998

INTEROFFICE CORRESPONDENCE

HYDROLOGY DIVISION

TO: Desiderio Salas, Street Maintenance Division

FROM: Lisa Ann Manwill, P.E. Engineering Associate, PWD *LAM*

SUBJECT: **PRIVATE DRAINAGE FACILITIES WITHIN PUBLIC RIGHT-OF-WAY
DRAINAGE FILE NUMBER L16-D23F.**

Transmitted herewith, is a copy of the approved drainage plan for the referenced project incorporating the SO #19 design.

This plan is being submitted to you for permitting and inspection. Please provide this section with a signed-off copy per the signature block upon construction and acceptance by your office.

As you are aware, the signed off SO #19 is required by this office for Certificate of Occupancy release; therefore your expeditious processing of this plan would be greatly appreciated and would avoid any unnecessary delay in the release of the Certificate of Occupancy.

Thank you for your cooperation and if you should have any questions and/or comments, please feel free to call me at 924-3984.

Attachment

FILE

DRAINAGE INFORMATION SHEET

D 23F

PROJECT TITLE: Crossland ZONE ATLAS/DRNG, FILE#: L-16

DRB #: 97-533 EPC #: Z-97-126 WORK ORDER #: _____

LEGAL DESCRIPTION: Lot 3A, Cavan Sunport Addition

CITY ADDRESS: _____

ENGINEERING FIRM: Mark Goodwin & Associates CONTACT: John MacKenzie

ADDRESS: P.O. Box 90606 PHONE: 828-2200

OWNER: ESA Management Inc. CONTACT: John Lidbury

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ADDRESS: 12750 Merit Dr., Suite 550 LBS2, Dallas PHONE: 972-233-0400

SURVEYOR: _____ CONTACT: _____

ADDRESS: _____ PHONE: _____

CONTRACTOR: _____ CONTACT: _____

ADDRESS: _____ PHONE: _____

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CHECK TYPE OF APPROVAL SOUGHT:

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- ☒ DRAINAGE PLAN
- ☐ CONCEPTUAL GRADING & DRAINAGE PLAN
- ☒ GRADING PLAN
- ☐ EROSION CONTROL PLAN
- ☐ ENGINEER'S CERTIFICATION
- ☐ OTHER

- ☐ SKETCH PLAT APPROVAL
- ☐ PRELIMINARY PLAT APPROVAL
- ☐ S. DEV. PLAN FOR SUB'D APPROVAL
- ☒ S. DEV. PLAN FOR BLDG PERMIT APPROVAL
- ☐ SECTOR PLAN APPROVAL
- ☐ FINAL PLAT APPROVAL
- ☒ FOUNDATION PERMIT APPROVAL
- ☒ BUILDING PERMIT APPROVAL
- ☐ CERTIFICATION OF OCCUPANCY APPROVAL
- ☐ GRADING PERMIT APPROVAL
- ☐ PAVING PERMIT APPROVAL
- ☐ S.A.D. DRAINAGE REPORT
- ☐ DRAINAGE REQUIREMENTS
- ☒ OTHER SD #19 (Specify)

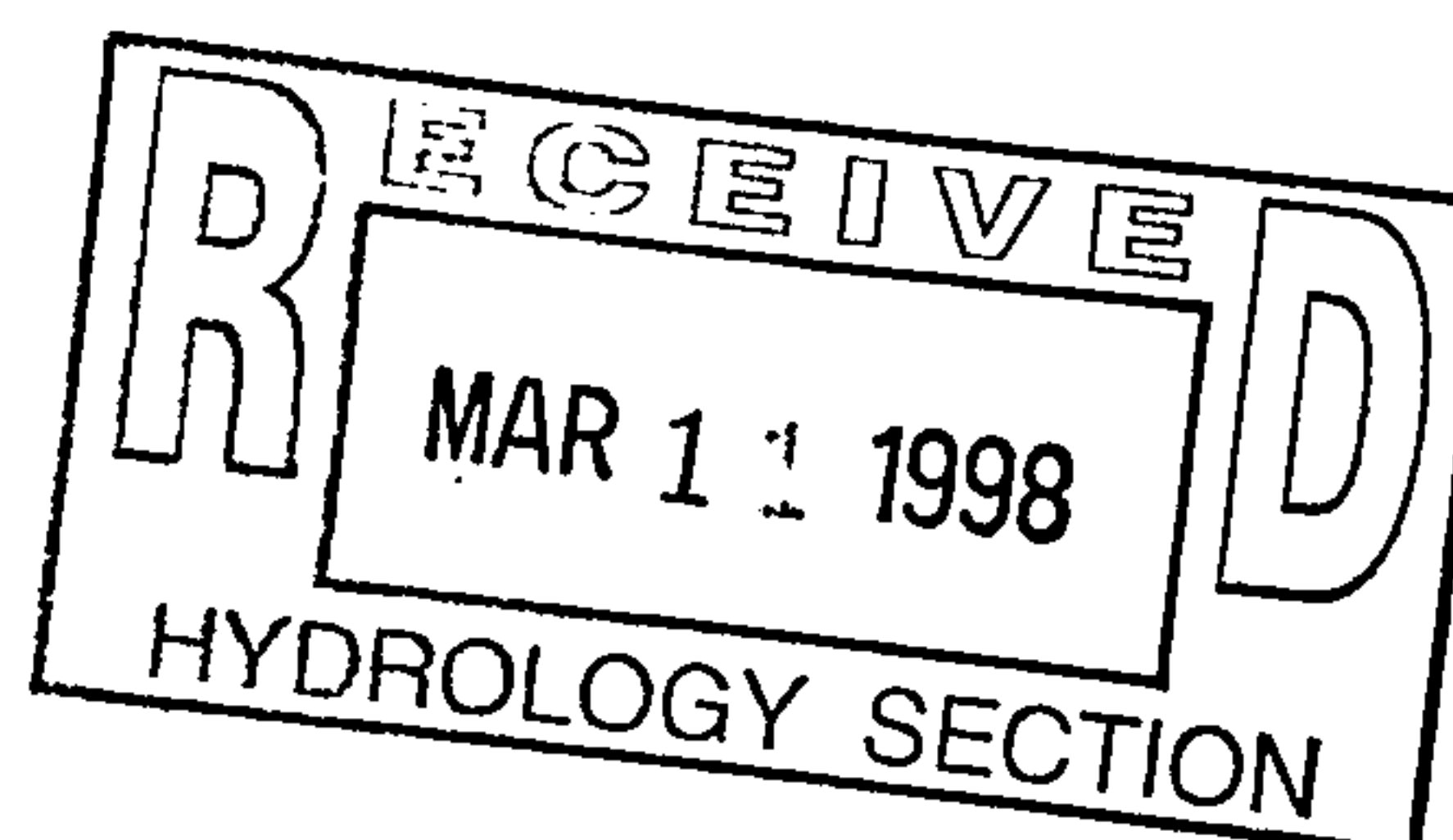
PRE-DESIGN MEETING:

- ☐ YES
- ☒ NO
- ☐ COPY PROVIDED

*Approved for SD Serv.
BP + SD #19
Stamped
3-10-98*

DATE SUBMITTED: 3/10/98

BY: John MacKenzie
John MacKenzie





D. Mark Goodwin & Associates, P.A.
Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199
(505) 828-2200 FAX 797-9539
e-mail: dmgs@swcp.com

March 10, 1998

Ms. Lisa Manwill, PE
Hydrology Division
City of Albuquerque
600 2nd Street NW
Albuquerque, NM 87103

Re: **Crossland Hotel (~~L16-D23~~) with Engineer's Stamp dated 3/10/98**

Dear Ms. Manwill:

(L16/D23F)

Pursuant to your previous comments, dated January 9, 1998, I have modified the grading and drainage plan as follows:

1. The finished floor elevation has been added to the third building. *Too steep*
2. The slope of the berm along the west property line is *2.5:1* within the site. It's intended to match up with existing grade. The previously displayed access easement has been eliminated.
3. The previously approved Yale Business Park Master Plan was obtained and the approved discharge of 12 cfs (old hydrology) for this particular tract is less than the 10 cfs discharge that is predicted by AHYMO using "new hydrology". From the master plan the allowable discharge is not a function of land treatment types but a function of downstream capacity. Most of the capacity (if not all) in Issacson & Arfman's master plan was derived from timing of the peak flows. Relative to the primary basin within which the subject site is located, the peak from this site's subbasin was significantly ahead of the peak from the overall basin. *OK*
4. Minor off-site flow from the east (0.78 cfs, see AHYMO) of this site presently crosses the east property line and impacts this site. The off-site basin comprises an unimproved city-owned alley and back yards of lots just east of the alley. There is a drainage basin boundary running north-south approximately half-way into the residential lots, meaning the fronts of these lots discharge into the street (Cornell) upon which they front. To the block wall along the east property line has been added a note instructing the contractor to allow for these off-site flows to be received by the subject site.

If you're not able to grant building permit approval on this submittal because there remain some outstanding issues, please consider granting conceptual plan approval (so the site plan can be signed-off) with the understanding that remaining issues will be addressed prior to release of building permit. Otherwise, if I can be of further assistance please contact me.

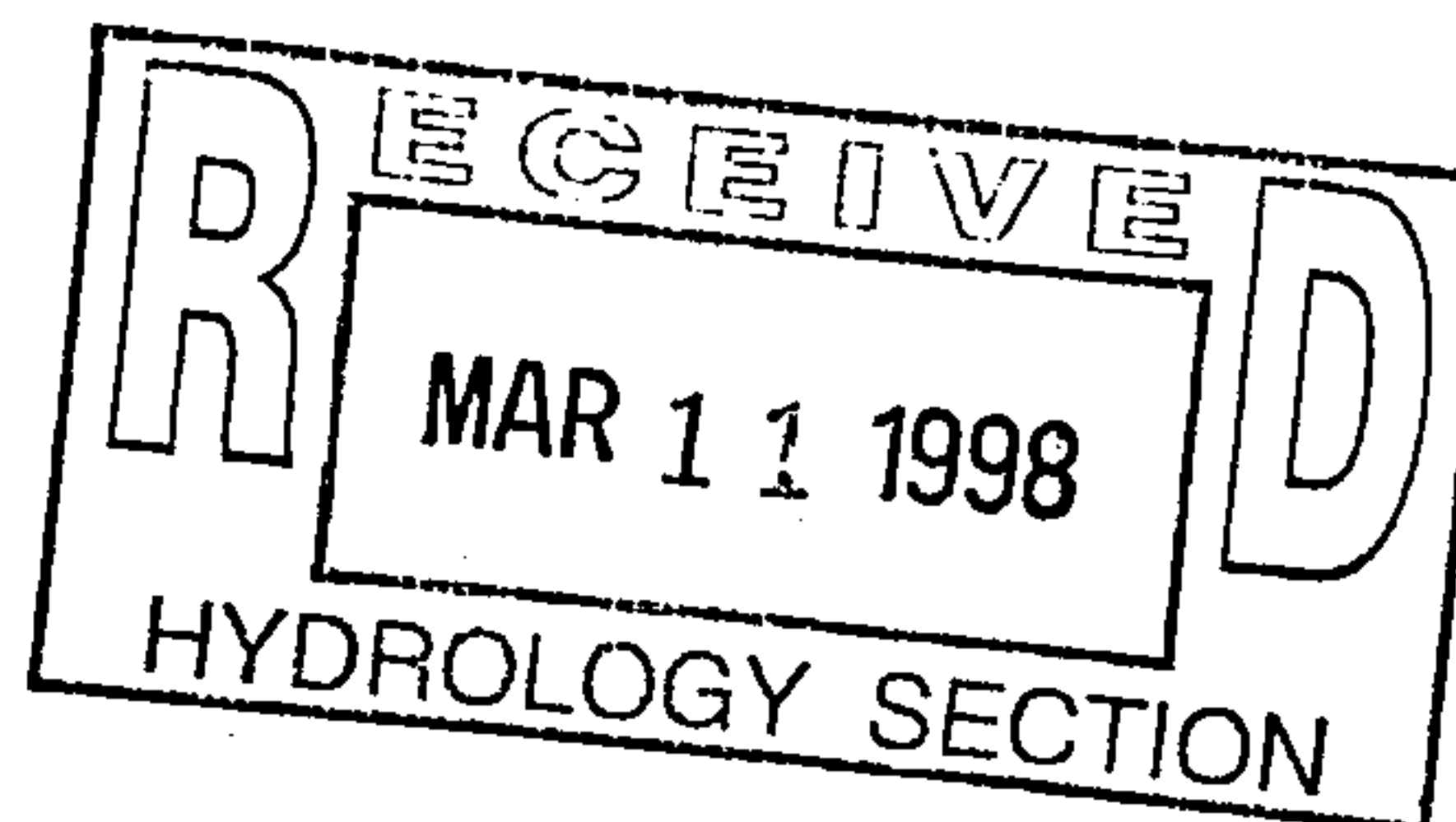
Sincerely,

MARK GOODWIN & ASSOCIATES, P.A.

John M. MacKenzie

John M. MacKenzie, PE

f:\cross\comments.no1



AHYMO PROGRAM (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
RUN DATE (MON/DAY/YR) = 03/09/1998
START TIME (HR:MIN:SEC) = 16:39:23 USER NO.= M_GOODWN.I01
INPUT FILE = CROSSLAN.DAT

START TIME=0.0

***** HYDROGRAPH FOR CROSSLAND HOTEL IN THE YALE BUSINESS PARK
***** ONLY DEVELOPED CONDITIONS WILL BE EVALUATED BECAUSE THE
***** SITE IS ALLOWED FREE DISCHARGE PER MASTER PLAN BY ISSACSON
***** & ARFMAN (L-16/D23)

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
RAIN ONE=2.02 IN RAIN SIX=2.34 IN
RAIN DAY=2.70 IN DT=0.033 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 H
DT = .033000 HOURS END TIME = 5.973000 HOURS

.0000	.0014	.0028	.0043	.0058	.0073	.0089
.0105	.0122	.0139	.0156	.0174	.0192	.0211
.0230	.0250	.0270	.0291	.0313	.0336	.0359
.0383	.0408	.0434	.0461	.0490	.0519	.0551
.0583	.0618	.0654	.0704	.0762	.0824	.0909
.1165	.1570	.2163	.2982	.4071	.5471	.7225
.9378	1.1975	1.3137	1.3951	1.4664	1.5307	1.5898
1.6444	1.6954	1.7431	1.7880	1.8303	1.8702	1.9080
1.9438	1.9778	2.0100	2.0406	2.0697	2.0891	2.0951
2.1009	2.1063	2.1115	2.1165	2.1212	2.1258	2.1301
2.1344	2.1384	2.1424	2.1462	2.1499	2.1535	2.1570
2.1604	2.1637	2.1670	2.1701	2.1732	2.1762	2.1791
2.1820	2.1848	2.1876	2.1903	2.1929	2.1955	2.1981
2.2006	2.2031	2.2055	2.2079	2.2102	2.2125	2.2148
2.2170	2.2192	2.2214	2.2235	2.2256	2.2277	2.2297
2.2317	2.2337	2.2357	2.2376	2.2395	2.2414	2.2433
2.2451	2.2470	2.2488	2.2506	2.2523	2.2541	2.2558
2.2575	2.2592	2.2609	2.2625	2.2641	2.2658	2.2674
2.2690	2.2705	2.2721	2.2736	2.2751	2.2767	2.2782
2.2797	2.2811	2.2826	2.2840	2.2855	2.2869	2.2883
2.2897	2.2911	2.2925	2.2938	2.2952	2.2965	2.2979
2.2992	2.3005	2.3018	2.3031	2.3044	2.3057	2.3069
2.3082	2.3094	2.3107	2.3119	2.3131	2.3143	2.3155
2.3167	2.3179	2.3191	2.3203	2.3214	2.3226	2.3238
2.3249	2.3260	2.3272	2.3283	2.3294	2.3305	2.3316
2.3327	2.3338	2.3349	2.3359	2.3370	2.3381	2.3391

*HYDROGRAPH FOR ON-SITE 2.24 AC DRAINAGE BASIN
*SITE WILL BE DIVIDED INTO 2 SUBBASINS

*BASIN A

COMPUTE NM HYD ID=1 HYD NO=101.1 AREA=0.0024 SQ MI
PER A=0.0 PER B=15.0 PER C=0.0 PER D=85.0
TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
UNIT PEAK = 8.0540 CFS UNIT VOLUME = .9981 B = 526.28 P60 = 2.02
AREA = .002040 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

K = .132208HR TP = .133300HR K/TP RATIO = .991810 SHAPE CONSTANT, N =
UNIT PEAK = .87683 CFS UNIT VOLUME = .9852 B = 324.67 P60 = 2.02
AREA = .000360 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 101.10

RUNOFF VOLUME = 1.90734 INCHES = .2441 ACRE-FEET
PEAK DISCHARGE RATE = 6.65 CFS AT 1.518 HOURS BASIN AREA = .0024 SQ. MI.

*BASIN B

COMPUTE NM HYD ID=2 HYD NO=101.2 AREA=0.0012 SQ MI
PER A=0.0 PER B=15.0 PER C=0.0 PER D=85.0
TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
UNIT PEAK = 4.0270 CFS UNIT VOLUME = .9965 B = 526.28 P60 = 2.02
AREA = .001020 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

K = .132208HR TP = .133300HR K/TP RATIO = .991810 SHAPE CONSTANT, N =
UNIT PEAK = .43841 CFS UNIT VOLUME = .9684 B = 324.67 P60 = 2.02
AREA = .000180 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

PRINT HYD ID=2 CODE=1

PARTIAL HYDROGRAPH 101.20

RUNOFF VOLUME = 1.90734 INCHES = .1221 ACRE-FEET
PEAK DISCHARGE RATE = 3.33 CFS AT 1.518 HOURS BASIN AREA = .0012 SQ. MI.

ADD HYD ID=2 HYD NO=102.1 ID=1 ID=2
PRINT HYD ID=2 CODE=1

PARTIAL HYDROGRAPH 102.10

RUNOFF VOLUME = 1.90718 INCHES = .3662 ACRE-FEET
PEAK DISCHARGE RATE = 9.98 CFS AT 1.518 HOURS BASIN AREA = .0036 SQ. MI.

*OFF-SITE BASIN 1

COMPUTE NM HYD ID=3 HYD NO=101.3 AREA=0.0006 SQ MI
PER A=40.0 PER B=60.0 PER C=0.0 PER D=0.0
TP=0.1333 HR MASS RAINFALL=-1

K = .143286HR TP = .133300HR K/TP RATIO = 1.074913 SHAPE CONSTANT, N =
UNIT PEAK = 1.3700 CFS UNIT VOLUME = .9896 B = 304.37 P60 = 2.02
AREA = .000600 SQ MI IA = .56000 INCHES INF = 1.41800 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

PRINT HYD

ID=3 CODE=1

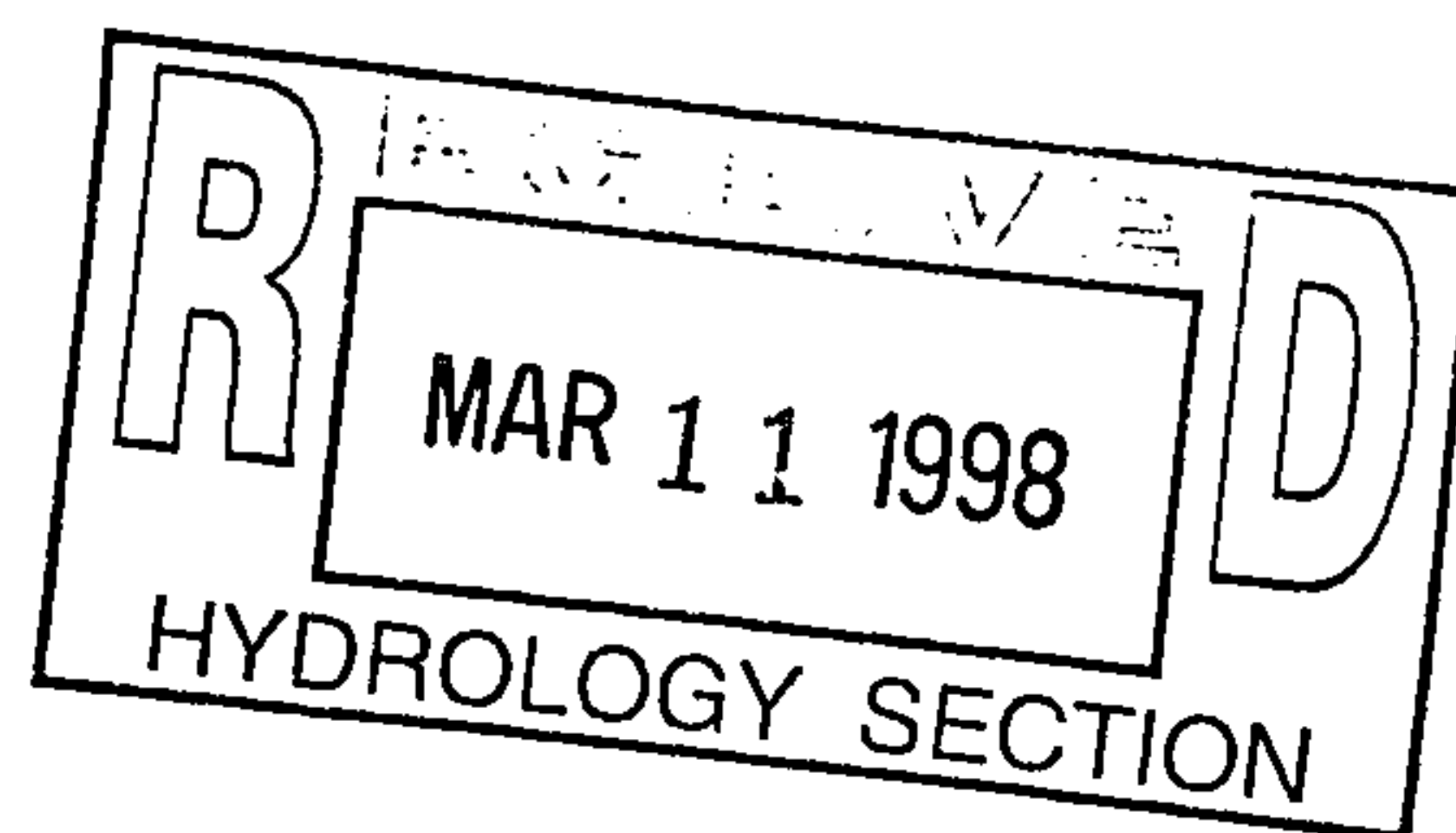
PARTIAL HYDROGRAPH 101.30

RUNOFF VOLUME = .67181 INCHES = .0215 ACRE-FEET
PEAK DISCHARGE RATE = .78 CFS AT 1.518 HOURS BASIN AREA = .0006 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 16:39:30





City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

January 9, 1998

John MacKenzie, P.E.
Mark Goodwin & Associates
P.O. Box 90606
Albuquerque, NM 87199

(L16/D23F)

RE: *CROSSLAND (L16-D23). GRADING AND DRAINAGE PLAN FOR SITE DEVELOPMENT PLAN FOR BUILDING PERMIT, BUILDING PERMIT, AND FOUNDATION PERMIT. ENGINEER'S STAMP DATED 12-15-97.*

Dear Mr. MacKenzie:

Based on the information provided on your December 15, 1997 submittal, City Hydrology has the following comments:

1. Provide finish floor elevation for all three proposed buildings.
2. What is the slope of the berm along the west property line? How will the access easement ever be used?
3. Provide applicable information showing allowable discharge. You may not have free discharge because the master plan you referred to used "old hydrology." Do the land treatment types you propose match those defined in the master plan?
4. Address off-site flow.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

Lisa Ann Manwill, P.E.

Hydrology

c: Andrew Garcia
File

M E M O R A N D U M

to: Hydrology Reviewer
from: John MacKenzie
subject: Crossland Grading and Drainage Plan
date: December 15, 1997

L16/D23F

I am seeking Site Development Plan and Building Permit Approval. If there are comments preventing building permit approval, please grant foundation permit approval so earthwork can begin while comments are in the process of being addressed.

DRAINAGE INFORMATION SHEET

D 23 F

PROJECT TITLE: Crossland ZONE ATLAS/DRNG, FILE#: L-16

DRB #: _____ EPC #: Z-97-126 WORK ORDER #: _____

LEGAL DESCRIPTION: Lot 3A, Cavan Sunport Addition

CITY ADDRESS: _____

ENGINEERING FIRM: Mark Goodwin & Associates CONTACT: John MacKenzie

ADDRESS: P.O. Box 90606 PHONE: 828-2200

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ARCHITECT: Alliance Architects CONTACT: Robert Nieporte, Jr.

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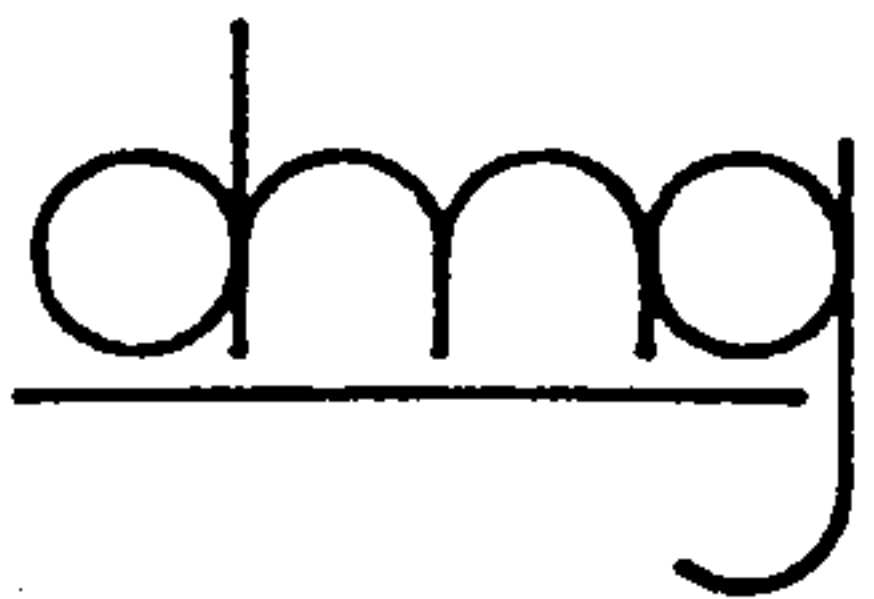
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- ☐ S.A.D. DRAINAGE REPORT
- ☐ DRAINAGE REQUIREMENTS
- ☐ OTHER _____ (Specify)

PRE-DESIGN MEETING:

- ☐ YES
- ☐ NO
- ☐ COPY PROVIDED

DATE SUBMITTED: 12-15-97

BY: John MacKenzie
John MacKenzie



D. MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS & SURVEYORS

PROJECT Crossland
SUBJECT Drainage
BY JMM DATE 12/10/97
CHECKED _____ DATE _____
SHEET _____ OF _____

Discharge from Basin A is via 24" sidewalk
culvert.

$Q = 6.65 \text{ cfs (from AHJMO) Corb Height = 3"$

$$L = \frac{Q}{2.9(H)^{1.5}} = \frac{6.65}{2.9(0.67)^{1.5}} = 4.18 \text{ ft}$$

Use 4.5 ft length

$$Q_{\text{Capacity}} = 2.9(4.5)(0.67)^{1.5} = 7.16 \text{ cfs}$$

Outfall channel is to International in
6" x 24" culvert

slope = 7% WP = 3' Area = 1 SF

$$Q = \frac{1.49}{0.014} \left(1 \left(\frac{1}{3} \right)^{0.67} \right) (0.07)^{0.5}$$

$$Q_{\text{Capacity}} = 13.5 \text{ cfs OK}$$

AHYMO PROGRAM (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
 RUN DATE (MON/DAY/YR) = 12/10/1997
 START TIME (HR:MIN:SEC) = 09:25:29 USER NO.= M_GOODWN.I01
 INPUT FILE = CROSSLAN.DAT

START TIME=0.0

***** HYDROGRAPH FOR CROSSLAND HOTEL IN THE YALE BUSINESS PARK
 ***** ONLY DEVELOPED CONDITIONS WILL BE EVALUATED BECAUSE THE
 ***** SITE IS ALLOWED FREE DISCHARGE PER MASTER PLAN BY ISSACSON
 ***** & ARFMAN (L-16/D23)

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 RAIN DAY=2.70 IN DT=0.033 HR

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.0105	.0122	.0139	.0156	.0174	.0192	.0211
.0230	.0250	.0270	.0291	.0313	.0336	.0359
.0383	.0408	.0434	.0461	.0490	.0519	.0551
.0583	.0618	.0654	.0704	.0762	.0824	.0909
.1165	.1570	.2163	.2982	.4071	.5471	.7225
.9378	1.1975	1.3137	1.3951	1.4664	1.5307	1.5898
1.6444	1.6954	1.7431	1.7880	1.8303	1.8702	1.9080
1.9438	1.9778	2.0100	2.0406	2.0697	2.0891	2.0951
2.1009	2.1063	2.1115	2.1165	2.1212	2.1258	2.1301
2.1344	2.1384	2.1424	2.1462	2.1499	2.1535	2.1570
2.1604	2.1637	2.1670	2.1701	2.1732	2.1762	2.1791
2.1820	2.1848	2.1876	2.1903	2.1929	2.1955	2.1981
2.2006	2.2031	2.2055	2.2079	2.2102	2.2125	2.2148
2.2170	2.2192	2.2214	2.2235	2.2256	2.2277	2.2297
2.2317	2.2337	2.2357	2.2376	2.2395	2.2414	2.2433
2.2451	2.2470	2.2488	2.2506	2.2523	2.2541	2.2558
2.2575	2.2592	2.2609	2.2625	2.2641	2.2658	2.2674
2.2690	2.2705	2.2721	2.2736	2.2751	2.2767	2.2782
2.2797	2.2811	2.2826	2.2840	2.2855	2.2869	2.2883
2.2897	2.2911	2.2925	2.2938	2.2952	2.2965	2.2979
2.2992	2.3005	2.3018	2.3031	2.3044	2.3057	2.3069
2.3082	2.3094	2.3107	2.3119	2.3131	2.3143	2.3155
2.3167	2.3179	2.3191	2.3203	2.3214	2.3226	2.3238
2.3249	2.3260	2.3272	2.3283	2.3294	2.3305	2.3316
2.3327	2.3338	2.3349	2.3359	2.3370	2.3381	2.3391

*HYDROGRAPH FOR ON-SITE 2.24 AC DRAINAGE BASIN
 *SITE WILL BE DIVIDED INTO 2 SUBBASINS

*BASIN A

COMPUTE NM HYD ID=1 HYD NO=101.1 AREA=0.0024 SQ MI
 PER A=0.0 PER B=15.0 PER C=0.0 PER D=85.0
 TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
 UNIT PEAK = 8.0540 CFS UNIT VOLUME = .9981 B = 526.28 P60 = 2.02
 AREA = .002040 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

K = .132208HR TP = .133300HR K/TP RATIO = .991810 SHAPE CONSTANT, N =
UNIT PEAK = .87683 CFS UNIT VOLUME = .9852 B = 324.67 P60 = 2.02
AREA = .000360 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 101.10

RUNOFF VOLUME = 1.90734 INCHES = .2441 ACRE-FEET
PEAK DISCHARGE RATE = 6.65 CFS AT 1.518 HOURS BASIN AREA = .0024 SQ. MI.

*BASIN B

COMPUTE NM HYD ID=2 HYD NO=101.2 AREA=0.0012 SQ MI
PER A=0.0 PER B=15.0 PER C=0.0 PER D=85.0
TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
UNIT PEAK = 4.0270 CFS UNIT VOLUME = .9965 B = 526.28 P60 = 2.02
AREA = .001020 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

K = .132208HR TP = .133300HR K/TP RATIO = .991810 SHAPE CONSTANT, N =
UNIT PEAK = .43841 CFS UNIT VOLUME = .9684 B = 324.67 P60 = 2.02
AREA = .000180 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033000

PRINT HYD ID=2 CODE=1

PARTIAL HYDROGRAPH 101.20

RUNOFF VOLUME = 1.90734 INCHES = .1221 ACRE-FEET
PEAK DISCHARGE RATE = 3.33 CFS AT 1.518 HOURS BASIN AREA = .0012 SQ. MI.

ADD HYD ID=2 HYD NO=102.1 ID=1 ID=2
PRINT HYD ID=2 CODE=1

PARTIAL HYDROGRAPH 102.10

RUNOFF VOLUME = 1.90718 INCHES = .3662 ACRE-FEET
PEAK DISCHARGE RATE = 9.98 CFS AT 1.518 HOURS BASIN AREA = .0036 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 09:25:35