CITY OF ALBUQUERQUE



Timothy M. Keller, Mayor

January 19, 2018

G. Roberts Adams, P.E. Adams Engineering 8951 Cypress Waters Blvd., Suite 150 Dallas, TX 75019

RE: McDonald's - 5001 Gibson SE

Grading and Drainage Plan Engineer Stamp Date: 1/9/18 Hydrology File: L18D053

Dear Mr. Adams:

PO Box 1293 Based upon the information provided in your resubmittal received 01/16/2018, the Grading

Plan is approved for Building Permit.

Albuquerque Please attach a copy of this approved plan in the construction sets for Building Permit

processing. Prior to Certificate of Occupancy release, Engineer Certification per the DPM

checklist will be required

NM 87103 If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

www.cabq.gov

Renée C. Brissette, P.E. CFM

Renée C Brissette

Senior Engineer, Hydrology

Planning Department

THIS PERMIT IS UNDER FASTRAX REVIEW



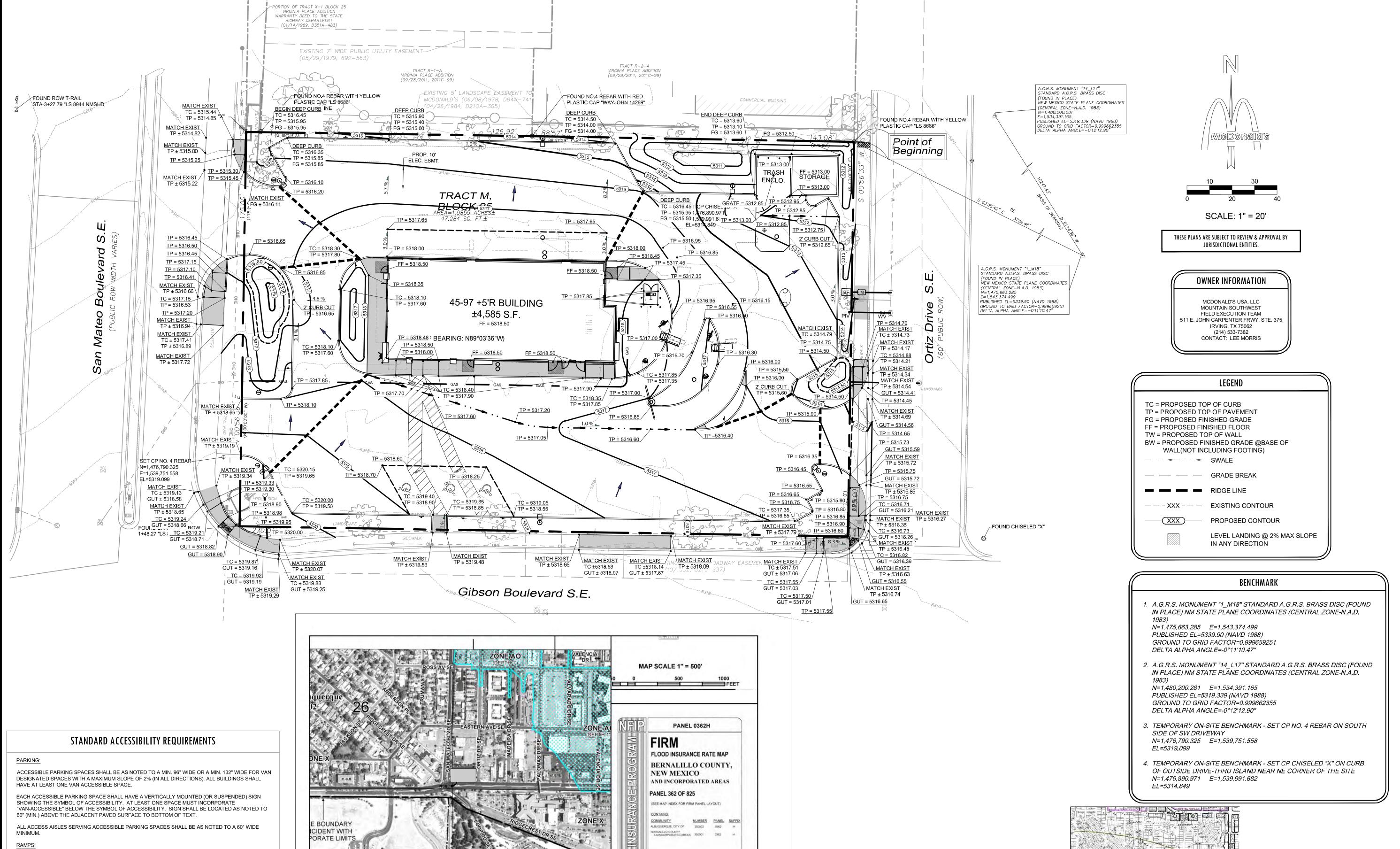
City of Albuquerque

Planning Department
Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 11/2016)

M E N	FASTRAX	X REVIEW PERMIT
Project Title:	Building Pe	ermit #: Hydrology File #:
DRB#:	EPC#:	Work Order#:
Legal Description:		
City Address:		
Applicants		Contact:
Address:		
Phone#:	Fax#:	E-mail:
Other Contact:		Contact:
Address:		
		E-mail:
Check all that Apply:		TYPE OF APPROVAL/ACCEPTANCE SOUGHT
DEPARTMENT:		BUILDING PERMIT APPROVAL
HYDROLOGY/ DRAINAGE		CERTIFICATE OF OCCUPANCY
TRAFFIC/ TRANSPORTATION		
TYPE OF SUBMITTAL:		PRELIMINARY PLAT APPROVAL
ENGINEER/ARCHITECT CERTIFICAT	ION	SITE PLAN FOR SUB'D APPROVAL
		SITE PLAN FOR BLDG. PERMIT APPROVA
CONCEPTUAL G & D PLAN		FINAL PLAT APPROVAL
GRADING PLAN		
DRAINAGE MASTER PLAN		SIA/ RELEASE OF FINANCIAL GUARANTE
DRAINAGE REPORT		FOUNDATION PERMIT APPROVAL
CLOMR/LOMR		GRADING PERMIT APPROVAL
		SO-19 APPROVAL
TRAFFIC CIRCULATION LAYOUT (TO	CL)	PAVING PERMIT APPROVAL
TRAFFIC IMPACT STUDY (TIS)		GRADING/ PAD CERTIFICATION
		WORK ORDER APPROVAL
OTHER (SPECIFY)		CLOMR/LOMR
PRE-DESIGN MEETING?		
IS THIS A RESUBMITTAL?: Yes	_No	OTHER (SPECIFY)
DATE SUBMITTED:	By:	Cepar fegoria
		Ju ju
COA STAFF	ELECTRONIC	C SUBMITTAL RECEIVED:

FEE PAID:_



Notice to User. The Map Number shown below should be used when placing map orders, the Community Number shown above should be used on insurance applications for the subject community.

Federal Emergency Management Agency

is is an official copy of a portion of the above referenced flood map. It is sextracted using F-MIT On-Line. This map does not reflect changes amendments which may have been made subsequent to the date on the block. For the latest product information about National Flood Insurance gram flood maps check the FEMA Flood Map Store at www.msc.fema.go

MAP NUMBER

MAP REVISED

AUGUST 16, 2012

35001C0362H

RAMPS EXCEEDING 6" IN RISE (EXCLUDING CURB RAMPS) SHALL HAVE APPROPRIATE EDGE PROTECTION WITH HANDRAILS ON EACH SIDE AT BETWEEN 34" AND 38", AND EXTEND 12" BEYOND THE TOP AND BOTTOM OF RAMP. HANDRAIL SHALL NOT DIMINISH THE CLEAR AREA REQUIRED FOR

IF REQUIRED BY LOCAL OR STATE JURISDICTION, RAMPS SHALL CONTAIN A TRUNCATED DOME SURFACE ARRANGED SO THAT WATER WILL NOT ACCUMULATE. COLOR OF RAMP FINISH MATERIAL (INCLUDING CONCRETE) SHALL HAVE A LIGHT AND REFLECTIVE VALUE AND MUST CONTRAST SIGNIFICANTLY TO DISTINGUISH IT FROM ADJACENT SURFACES - (OR PAINT STRIPE).

LANDINGS FOR RAMPS SHALL BE AS WIDE AS THE RAMP AND 60" LONG MINIMUM (36" MINIMUM FOR CURB RAMPS).

RAMPS SHALL NOT EXCEED A 1:12 RUNNING SLOPE OR 30" VERTICAL RISE. RAMPS AND LANDING SHALL NOT EXCEED 1:48 (2% CROSS SLOPE).

SIDEWALKS AND ACCESSIBLE ROUTES:

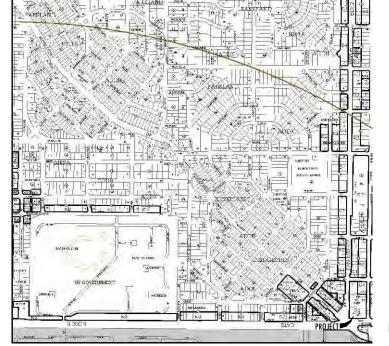
TOP AND BOTTOM LANDINGS SERVING THE RAMPS.

SIDEWALKS MUST BE AT LEAST 36" WIDE WITH A CROSS SLOPE THAT SHALL NOT EXCEED 1:48 (2%). LONGITUDINAL SLOPE OF ANY SIDEWALK (ACCESSIBLE ROUTE) SHALL NOT EXCEED 1:20 (5%).

FLOOD STATEMENT

According to Map No. 35001C0362H, Dated AUGUST 16, 2012, of the Federal Emergency Management Agency, National Flood Insurance Program Map, this property is within flood zone "X", areas determined to be outside the 0.2% annual chance floodplain. If this site is not within an be free from flooding or flood damage. On rare occasions, greater floods can and will occur and flood heights may be increased by man-made or natural causes. This statement shall not create liability on the part of Adams Engineering.

identified special flood hazard area, this flood statement does not imply that the property and/or the structures thereon will



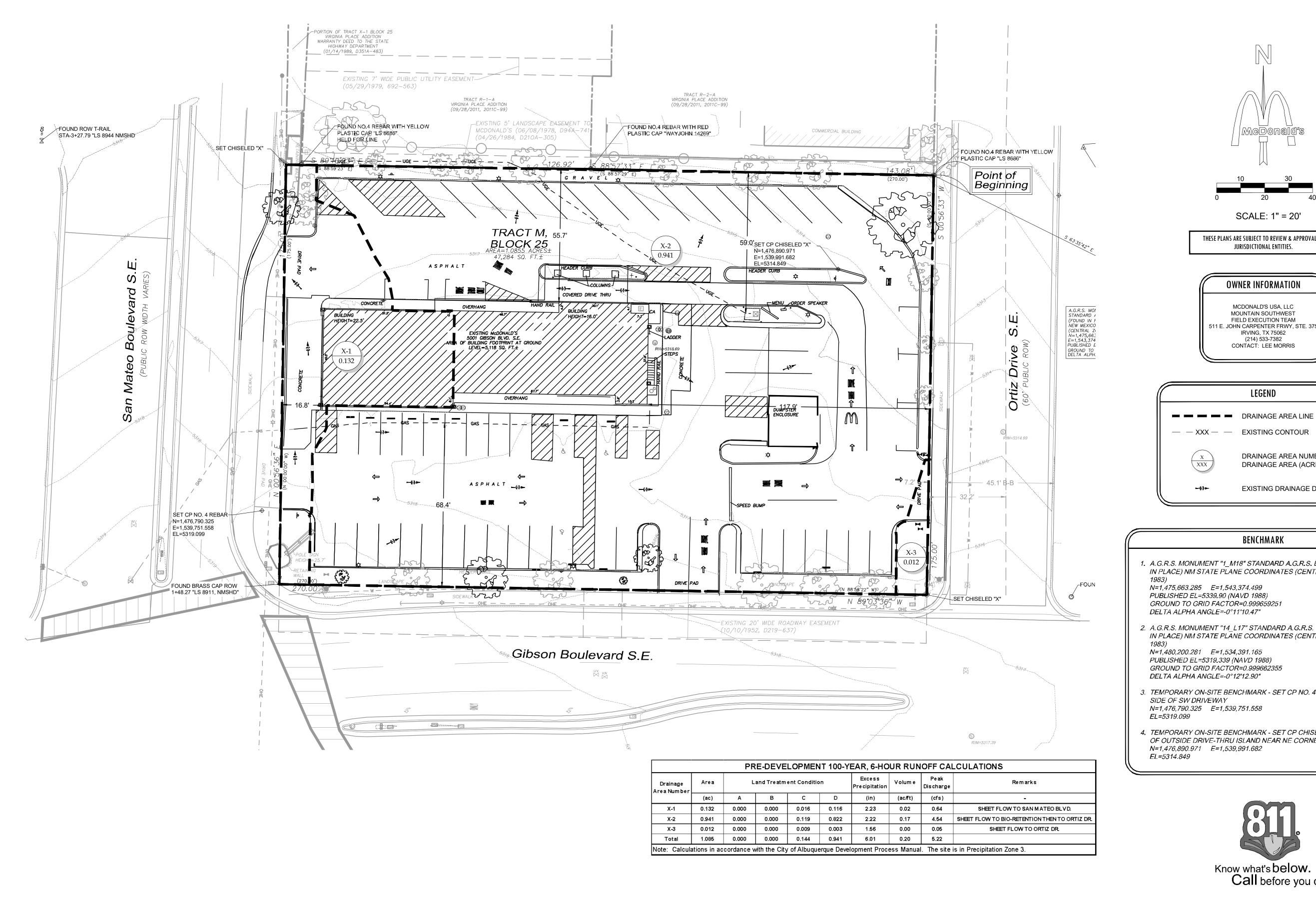
Know what's below. Call before you dig.

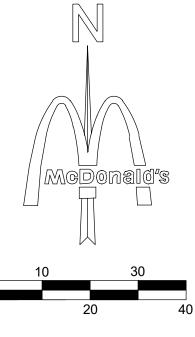
VICINITY MAP ZONE ATLAS PAGE: L-17-Z

	E O		
		DATE	BY
	DESIGNED	NOV. 2017	НЈМ
	DRAWN	NOV. 2017	НЈМ
	CHECKED	11/20/17	DWL
	AS-BUILT		
below	GRAI	DING	7

PLAN







SCALE: 1" = 20'

THESE PLANS ARE SUBJECT TO REVIEW & APPROVAL BY JURISDICTIONAL ENTITIES.

OWNER INFORMATION MCDONALD'S USA, LLC MOUNTAIN SOUTHWEST FIELD EXECUTION TEAM 511 E. JOHN CARPENTER FRWY, STE. 375 IRVING, TX 75062

(214) 533-7382 CONTACT: LEE MORRIS LEGEND

-- XXX -- EXISTING CONTOUR

DRAINAGE AREA NUMBER DRAINAGE AREA (ACRES)

EXISTING DRAINAGE DIRECTION

BENCHMARK

1. A.G.R.S. MONUMENT "1 M18" STANDARD A.G.R.S. BRASS DISC (FOUND IN PLACE) NM STATE PLANE COORDINATES (CENTRAL ZONE-N.A.D.

N=1,475,663.285 E=1,543,374.499 PUBLISHED EL=5339.90 (NAVD 1988) GROUND TO GRID FACTOR=0.999659251 DELTA ALPHA ANGLE=-0°11'10.47"

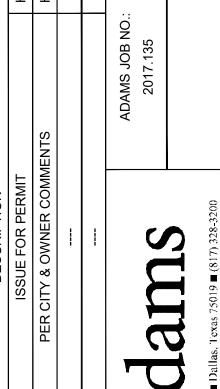
2. A.G.R.S. MONUMENT "14 L17" STANDARD A.G.R.S. BRASS DISC (FOUND IN PLACE) NM STATE PLANE COORDINATES (CENTRAL ZONE-N.A.D.

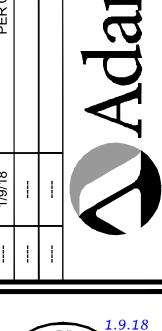
N=1,480,200.281 E=1,534,391.165 PUBLISHED EL=5319.339 (NAVD 1988) GROUND TO GRID FACTOR=0.999662355 DELTA ALPHA ANGLE=-0°12'12.90"

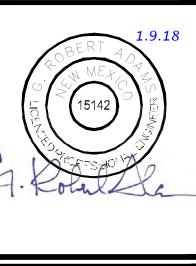
3. TEMPORARY ON-SITE BENCHMARK - SET CP NO. 4 REBAR ON SOUTH SIDE OF SW DRIVEWAY N=1,476,790.325 E=1,539,751.558

4. TEMPORARY ON-SITE BENCHMARK - SET CP CHISELED "X" ON CURB OF OUTSIDE DRIVE-THRU ISLAND NEAR NE CORNER OF THE SITE N=1,476,890.971 E=1,539,991.682





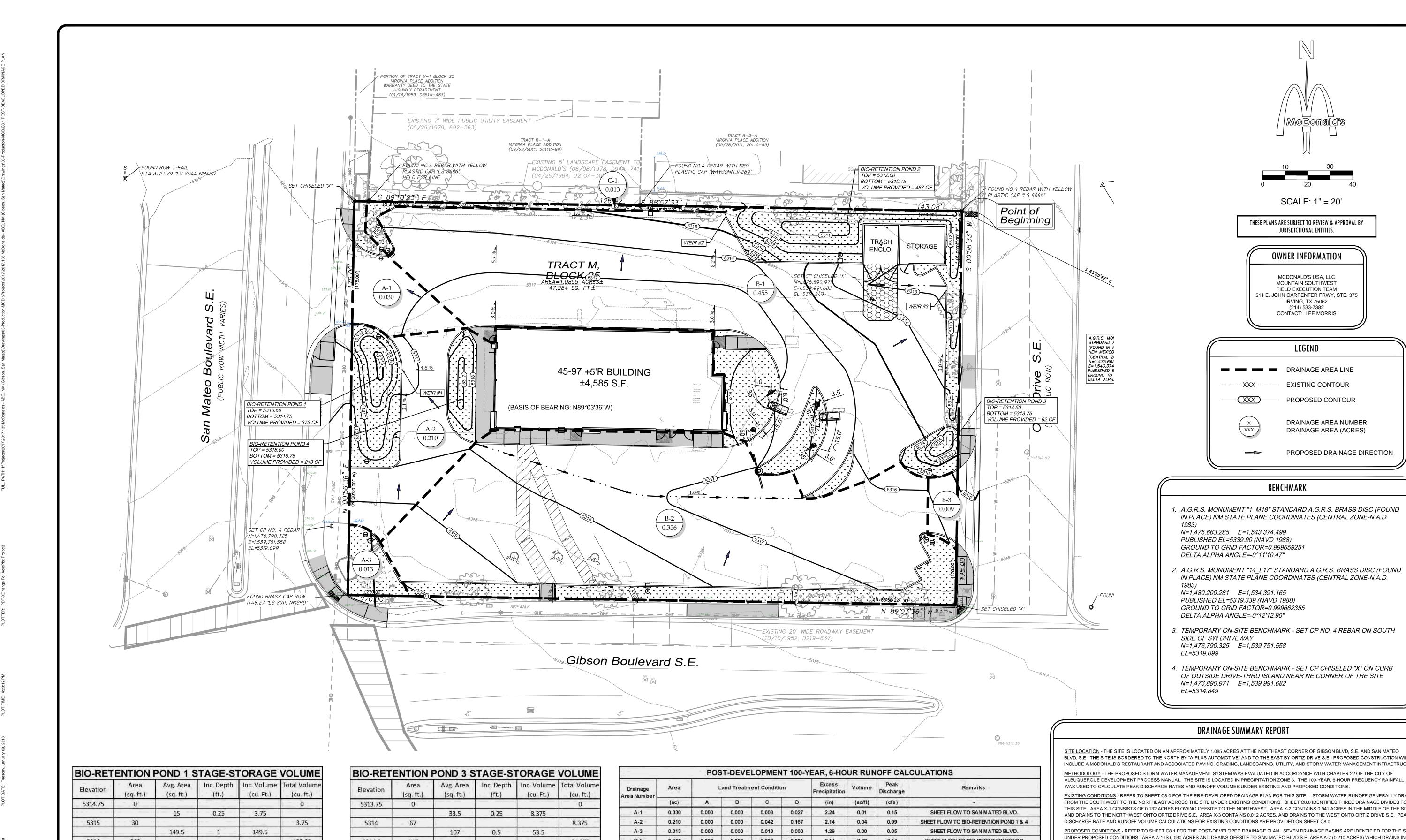




ADDRESS KROC DRIVE - OAK BROOK, ILLINOIS			These drawings and specifications are the confidential and proprietary not be copied or reproduced without written authorization. The contraction	specific site in conjunction with its issue date and are not suitable for u of these drawings for reference or example on another project requires	and engineers. Reproduction of the contract documents for reuse on a	7 5001 GIBSON S	ALBUQUERQUE,	
DATE								
SIGNATURE (2 REQUIRED)					CO-SIGN SIGNATURES			
	REGIONAL MGR.	CONST. MGR.	OPERATIONS DEPT.	REAL ESTATE DEPT.)	CONTRACTOR	OWNER	

DATE BY DESIGNED NOV. 2017 HJM DRAWN NOV. 2017 HJM CHECKED NOV 2017 | MDK AS-BUILT

PRE-DEVELOPED DRAINAGE PLAN



Elevation	Area (sq. ft.)	Avg. Area (sq. ft.)	Inc. Depth (ft.)	Inc. Volume (cu. Ft.)	Total Volume (cu. ft.)
5314.75	0	F - 4	2		0
		15	0.25	3.75	
5315	30				3.75
		149.5	1	149.5	
5316	269				153,25
		367	0.6	220.2	
5316.6	465				373.45

Elevation	Area (sq. ft.)	Avg. Area (sq. ft.)	Inc. Depth (ft.)	Inc. Volume (cu. Ft.)	Total Volume (cu. ft.)
5310.75	- 0		-		0
-		83.5	0.25	20.875	
5311	167		-		20.875
		466.5	1	466.5	
5312	766		-		487.375
					13

Elevation	Area (sq. ft.)	Avg. Area (sq. ft.)	Inc. Depth (ft.)	Inc. Volume (cu. Ft.)	Total Volume (cu. ft.)
5313.75	0				0
		33.5	0.25	8.375	
5314	67				8.375
		107	0.5	53.5	j* . •
5314.5	147				61.875

Elevation	Area (sq. ft.)	Avg. Area (sq. ft.)	Inc. Depth (ft.)	Inc. Volume (cu. Ft.)	Total Volume (cu. ft.)
5317	58				0
		213	1 -	213	
5318	368				213
		184	0	0	
5318	0				213
7,000 mg or	red Volume = en Pond 1 & 4				

Drainage Area Number	Area	a Land Treatment Condition			Excess Precipitation	Volume	Peak Discharge	Remarks	
area (variabe)	(ac)	Α	В	C	D	(in)	(ac/ft)	(cfs)	
A-1	0.030	0,000	0.000	0.003	0.027	2.24	0.01	0.15	SHEET FLOW TO SAN MATEO BLVD.
A-2	0.210	0.000	0.000	0.042	0.167	2.14	0.04	0.99	SHEET FLOW TO BIO-RETENTION POND 1 & 4
A-3	0.013	0.000	0.000	0.013	0.000	1.29	0.00	0.05	SHEET FLOW TO SAN MATEO BLVD.
B-1	0.455	0.000	0.000	0.094	0.361	2.14	0.08	2.14	SHEET FLOW TO BIO-RETENTION POND 2
B-2	0.356	0.000	0.000	0.042	0.315	2.23	0.07	1.72	SHEET FLOW TO BIO-RETENTION POND 3
B-3	0.009	0.000	0.000	0.000	0,009	2.36	0.00	0.04	SHEET FLOW TO ORTIZ DR.
C-1	0.012	0.000	0.000	0.012	0.000	1.29	0.00	0.04	SHEET FLOW OFF-SITE TO NORTH
Total	1.085	0.000	0.000	0.207	0.878	13.69	0.19	5.12	

BIORETENTION VOLUME CAL	.CULATIONS
Total Impervious Area =	SF ac 38259,85 0.878
Req'd Retention Volume (0.34"/acre) =	1084 cf
Retention Volume Provided =	1136 cf

Pond #	Drainage Areas	Impervious Area (Ac)	FF Required Volume (cf)	FF Provided Volume (cf)
1	A-2	0.167	206	586
2	B-1	0.361	445	487
3	B-2	0.315	388	62
Off-Site	A-1, A-3, B-3, C-1	0.036	44	4
Total	All	0.878	1084	1136

SCALE: 1" = 20'

JURISDICTIONAL ENTITIES.

OWNER INFORMATION

MCDONALD'S USA, LLC MOUNTAIN SOUTHWEST

FIELD EXECUTION TEAM

IRVING, TX 75062

CONTACT: LEE MORRIS

LEGEND

DRAINAGE AREA NUMBER

DRAINAGE AREA (ACRES)

PROPOSED DRAINAGE DIRECTION

SITE LOCATION - THE SITE IS LOCATED ON AN APPROXIMATELY 1.085 ACRES AT THE NORTHEAST CORNER OF GIBSON BLVD, S.E. AND SAN MATEO BLVD, S.E. THE SITE IS BORDERED TO THE NORTH BY "A-PLUS AUTOMOTIVE" AND TO THE EAST BY ORTIZ DRIVE S.E. PROPOSED CONSTRUCTION WILL INCLUDE A MCDONALD'S RESTAURANT AND ASSOCIATED PAVING, GRADING, LANDSCAPING, UTILITY, AND STORM WATER MANAGEMENT INFRASTRUCTURE.

METHODOLOGY - THE PROPOSED STORM WATER MANAGEMENT SYSTEM WAS EVALUATED IN ACCORDANCE WITH CHAPTER 22 OF THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL. THE SITE IS LOCATED IN PRECIPITATION ZONE 3. THE 100-YEAR, 6-HOUR FREQUENCY RAINFALL EVENT WAS USED TO CALCULATE PEAK DISCHARGE RATES AND RUNOFF VOLUMES UNDER EXISTING AND PROPOSED CONDITIONS. EXISTING CONDITIONS - REFER TO SHEET C8.0 FOR THE PRE-DEVELOPED DRAINAGE PLAN FOR THIS SITE. STORM WATER RUNOFF GENERALLY DRAINS

FROM THE SOUTHWEST TO THE NORTHEAST ACROSS THE SITE UNDER EXISTING CONDITIONS. SHEET C8.0 IDENTIFIES THREE DRAINAGE DIVIDES FOR THIS SITE. AREA X-1 CONSISTS OF 0.132 ACRES FLOWING OFFSITE TO THE NORTHWEST. AREA X-2 CONTAINS 0.941 ACRES IN THE MIDDLE OF THE SITE, AND DRAINS TO THE NORTHWEST ONTO ORTIZ DRIVE S.E. AREA X-3 CONTAINS 0.012 ACRES, AND DRAINS TO THE WEST ONTO ORTIZ DRIVE S.E. PEAK DISCHARGE RATE AND RUNOFF VOLUME CALCULATIONS FOR EXISTING CONDITIONS ARE PROVIDED ON SHEET C8.0.

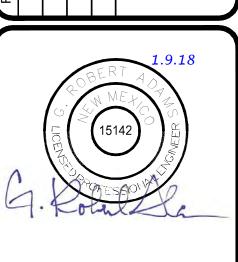
PROPOSED CONDITIONS - REFER TO SHEET C8.1 FOR THE POST-DEVELOPED DRAINAGE PLAN. SEVEN DRAINAGE BASINS ARE IDENTIFIED FOR THE SITE UNDER PROPOSED CONDITIONS. AREA A-1 IS 0.030 ACRES AND DRAINS OFFSITE TO SAN MATEO BLVD S.E. AREA A-2 (0.210 ACRES) WHICH DRAINS INTO BIO-RETENTION POND 4 WITH AN OVERFLOW TO BIO-RETENTION POND 1. AFTER THE FIRST 0.34" RUNOFF IS CAPTURED, ADDITIONAL OVERFLOW DISCHARGES TO SAN MATEO BLVD S.E. MINOR NUISANCE WATER FROM AREAS A-1 (0.030 ACRES) AND A-3 (0.013 ACRES) DRAIN DIRECTLY OFFSITE TO SAN MATEO BLVD S.E. AREA B-1 (0.455 ACRES) FLOWS NORTHEAST TO BIO-RETENTION POND 2, OVERFLOWING TO ORTIZ DRIVE S.E. AFTER CONTAINING FIRST FLUSH. AREA B-2 (0.356 ACRES) FLOWS NORTHEAST TO BIO-RETENTION POND 3. WATER FROM BIO-RETENTION POND 3 OVERFLOWS TO BIO-RETENTION POND 2 AND THEN DISCHARGES TO ORTIZ DRIVE S.E. AREAS B-3 (0.009 ACRES) AND C-1 (0.013 ACRES) BOTH FLOW OFFSITE TO ORTIZ DRIVE S.E. AND TO THE NORTH PROPERTY "A-PLUS AUTOMOTIVE", RESPECTFULLY. THE TOTAL FLOW OFF-SITE AT THE 100-YEAR STORM EVENT IS LESS THAN THE EXISTING FLOW OF 5.22 CFS TOTAL. TOTAL REQUIRED BIO-RETENTION (0.34") IS 1084 CUBIC FEET WITH AN OVERALL TOTAL PROPOSED STORAGE OF 1136 CUBIC

CONCLUSIONS - THE OVERALL AMOUNT OF IMPERVIOUS AREA AT THIS SITE WILL BE SIMILAR UNDER THE PROPOSED CONDITIONS AS THE EXISTING SITE; HOWEVER, RATHER THAN A LARGE PORTION OF FLOW DRAINING DIRECTLY INTO THE R.O.W., 1.021 ACRES DRAIN TO BIO-RETENTION PONDS BEFORE EXITING THE SITE. FURTHER, UNDER SMALLER STORM EVENTS, NEARLY ALL FLOW IS RETAINED AND INFILTRATED ON-SITE WHILE EVEN THE 100-YEAR STORM FLOWS OFF THE SITE ARE KEPT BELOW EXISTING VOLUMETRIC FLOW RATE. AS A RESULT OF THE IMPROVEMENTS, QUALITY AND QUANTITY OF FLOW SHOULD BE IMPROVED WHEN COMPARED TO THE EXISTING SITE.

<u> </u>			WEIR	CALCULAT	IONS			
	ALLOWABLE		VABLE	- Lr			CAPACITY	ACTUAL
CURBCUT	D.A.	Peak Q	HEAD	90% eff.	L	% FULL	90 % eff.	HEAD
#	#	(cfs)	H (ft)	(ft)	(ft)	Lr.7L	(cfs)	H (ft)
7	100% A-2	0.88	0.5	0.83	2.00	42%	2.12	0.26
2	31% B-1	0.87	0.5	0.82	2.00	41%	2.12	0.26
3	37% B-1	1.04	0.5	0.98	2.00	49%	2.12	0.29
			Q	= 3.33 * L * H^	1,5		,	

now what's below. Call before you dig

ВУ	MCH	MCH	 	0::	
				ADAMS JOB NO.: 2017.135	
z	MIT	OMMENTS			



Ŏ

DATE DESIGNED NOV. 2017 HJM DRAWN NOV. 2017 HJM CHECKED NOV 2017 | MDK **AS-BUILT**

POST-DEVELOPED DRAINAGE

Copyright 2017, Adams Engineering