HYDROLOGY L20/D14 EUBANK AND ACOMA S.E.

L20/D14

KUBANK AND
ACOMA S.E.

1 PLAN

MAX Q CFS	POND SIZE CF	POND SIZE SF	REDUCTION IN LEASABLE AREA SF	EST FILL CY	EST FILL COST $	RETAINING WALL LF	EST WALL COST $
1.47	23961	7987	1300	4901	19604	424	15179
4.07	19845	6615	600	4977	19908	368	13174
6.00	15582	5194	-0-	5056	20224	310	11098

PROJECT NAME MINI WAREHOUSES SHEET ____ OF ____

PROJECT 89709 BY JCH DATE 7/14/83

SUBJECT ____ CH'D ____ DATE ____

SOIL TYPE - B

AREA CALCULATIONS

AREA #	READING	CONVERSION	AREA
1	2492	9	22428
2	4355	9	39195
3	4099	9	36891
4	2873	9	25857
5	2016	9	18144

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APR 15 1983
ENGINEERING

TOTAL 192515 SF

= 3.27 ACRES

6 HR RAINFALL VOLUME 100 YR FREQUENCY 2.92"
(1973 NOAA ATLAS 2, VOL IV)

UNDEVELOPED FLOW CALCULATIONS (NATURAL STATE)

ASSUME 5% IMPERVIOUS $C = 0.36$

Δ ELEVATION = 9.9 FT LENGTH = 566 FT SLOPE = 0.017 FT/FT

VELOCITY = 0.77 FT/SEC PLATE 22.2 B1

$T_c = 566 / 0.77 = 12.25$ MIN.

$I =$ (6HR RAINFALL VOLUME) $6.84 (t_c^{-0.51})$

$I = 2.92 (6.84)(12.25^{-0.51}) = 4.61$ "/HR

$Q = CIA$

$Q = 0.36 (4.61)(3.27) = 5.43$ CFS

DEVELOPED FLOW CALCULATIONS (NEW DEVELOPMENT)

ASSUME 95% IMPERVIOUS $C = 0.92$

DEVELOPED FLOW CALCULATIONS (CONT)

THERE WILL BE 2 REACHES IN WHICH THE WATER HAS TO TRAVEL

REACH 1

LENGTH = 529' Δ ELEVATION = 6.6' SLOPE = 0.012 FT/FT

VELOCITY = 1.5 FT/SEC.

T_{c_1} = 529/1.5 = 5.88 MIN

REACH 2

LENGTH = 270' Δ ELEVATION = 2.3' SLOPE = 0.009 FT/FT

VELOCITY = 1.3 FT/SEC

T_{c_2} = 270/1.3 = 3.46 MIN

$T_{c\,TOTAL}$ = 5.88 + 3.46 = 9.34 MIN USE 10 MIN

$I = 2.42(6.84)(10^{-0.51}) = 5.12''/HR$

$Q = CiA = 0.92(5.12)(3.27) = 15.40\,CFS$

$V = CiA = 0.93(2.42)(142512)/12 = 26728\,CF$

STORM DURATION $T = 2V/Q_p = 26728(2)/15.4(60)$

$T = 58$ MIN

PROJECT NAME MINI WAREHOUSES SHEET 2 OF 7

PROJECT NO 39909 BY TCH DATE 9/6/85

SUBJECT CH'D DATE

EXISTING ON-SITE FLOW CALCULATIONS

AREA CALCULATIONS

AREA	READING	CONVERSION	AREA
A1	4946	8	39568
A2	1400	8	11200
A3	1630	8	13040
B	3443	8	27544
C	0041	8	328
D	2749	8	21992
E	1807	8	14456
F	1241	8	9928
G	0837	8	6696
		TOTAL	144752 SF = 3.32 ACRES

FLOW CALCULATIONS

A2

80% IMPERVIOUS SOIL TYPE-B C = 0.76

Δ ELEVATION = 6.9' LENGTH = 374' SLOPE = 0.018 FT/FT

VELOCITY = 0.80 FT/SEC

T_c = 374/0.80 = 7.8 MIN USE 10 MIN

$I = 2.42(6.84)(10^{-0.61}) = 5.12$ "/HR

$Q = 0.76(5.12)(0.91) =$ 3.54 CFS

A2 + A3 + B

85% IMPERVIOUS C = 0.81

Δ ELEVATION = 9.9' LENGTH = 566' SLOPE = 0.019 FT/FT

VELOCITY = 0.77 FT/S

T_c = 566/0.77 = 12.25 MIN

$I = 2.42(6.84)(12.25^{-0.61}) = 4.61$ "/HR

$Q = 0.81(4.61)(1.19) = 4.44$ CFS

PROJECT NAME MINI WAREHOUSES SHEET 3 OF 7

PROJECT NO 39409 BY TCH DATE 1/3/83

SUBJECT CH'D DATE

FLOW CALCULATIONS (CON'T)

C+E

$C = 0.98$ Δ ELEVATION = 3.3 LENGTH = 210'

SLOPE = 0.016

VELOCITY = 1.73 FT/SEC

$T_c = 210/1.73 = 2.02$ MIN USE 10

$I = 2.42(6.84)(10^{-0.51}) = 5.12''/HR$

$Q = 0.98(5.12)(0.34) = 1.71$ CFS

F

$C = 0.48$ 30% IMPERVIOUS

Δ ELEVATION = 1.1' LENGTH = 98' SLOPE = 0.014

VELOCITY = 0.70 FT/SEC

$T_c = 98/0.70 = 1.86$ MIN USE 10 MIN

$I = 5.12''/HR$

$Q = 0.48(5.12)(0.23) = 0.57$ CFS

G

$C = 0.98$ Δ ELEVATION = 1.2' LENGTH = 99' SLOPE = 0.024

VELOCITY = 2.0 FT/SEC

$T_c = 99/2 = 0.91$ MIN USE 10 MIN

$I = 5.12''/HR$

$Q = 0.98(5.12)(0.15) = 0.75$ CFS

$Q_T = 11.01$ CFS

STORM DURATION FOR DEVELOPED FLOW 58 MIN

REQ'D POND SIZE $49 \times 100 \times 3 = 14700$ CF

$\frac{1}{2} Q_P (58)(60) = 14700$ $Q_P = 8.45$ CFS

$Q_{DISCHARGE} = 15.40 - 8.45 = 6.95$ CFS

PROJECT NAME MINI WAREHOUSES SHEET 7 OF 7

PROJECT NO 99409 BY TCH DATE 1/7/93

SUBJECT CH'D DATE

PROPOSAL 1

3' HEAD MAXIMUM 6" PIPE

TIME MIN	Q_{IN} CFS	VOLUME CF	HEAD FT	Q_{OUT} CFS	VOLUME CF	VOLUME CF DET
1	1.54	46	0.005	0	0	46
5	7.70	1109	0.11	0.32	19	1136
10	15.40	3465	0.47	0.65	136	4465
15	13.80	4860	0.95	0.92	276	9049
20	12.19	3899	1.32	1.08	326	12623
25	10.59	3417	1.64	1.21	362	15678
30	8.98	2934	1.90	1.30	390	18222
35	7.38	2454	2.11	1.37	411	20265
40	5.77	1973	2.27	1.42	426	21812
45	4.17	1491	2.38	1.45	436	22867
50	2.57	1011	2.44	1.47	441	23437
55	0.96	628	2.45	1.47	442	23523
58	0	86	2.41	1.46	263	23346

TIME TO EMPTY TANK FROM END OF STORM

$$t = \frac{2(7987)}{0.119(64.4)}(2.45) = 11.58 \text{ HRS}$$

SF OF UNITS LOST = 4000 SF

REQUIRED FILL = 4901 CY

POND AREA = 49 x 163 = 7987 SF

PROJECT NAME MINI WAREHOUSES SHEET 5 OF 7
PROJECT NO 39409 BY FCH DATE 4/11/83
SUBJECT CH'D DATE

6.0 CFS MAX 3' HEAD MAX 11.5"Ø ORIFICE PROPOSAL 2

TIME MIN	Q_{IN} CFS	VOLUME CF	HEAD FT	Q_{OUT} CFS	VOLUME CF	$VOLUME_{DET}$ CF
1	1.54	46	0.008	0	0	46
5	7.70	1109	0.21	1.60	96	1059
10	15.40	3465	0.87	3.22	725	3801
15	13.80	4860	1.66	4.46	1152	7509
20	12.19	3899	2.19	5.12	1437	9971
25	10.59	5417	2.56	5.55	1600	11788
30	8.98	2934	2.82	5.81	1705	13017
35	7.38	2954	2.96	5.96	1766	13705
40	5.77	1973	3.00	6.00	1795	13883
45	4.17	1491	2.95	5.94	1791	13583
50	2.57	1011	2.80	5.79	1760	12834
55	0.96	528	2.56	5.54	1700	11662
58	0	86	2.25	5.20	966	10782

TIME TO EMPTY FROM END OF STORM

$$t = \frac{2A_T}{cA_0\sqrt{2g}}\left(h_1^{1/2} - h_2^{1/2}\right) \qquad h_2 = 0$$

$$t = \frac{2(5220)}{0.598(0.72)\sqrt{2(32.2)}}(2.25) = 1.88 \text{ HRS}$$

SF OF UNITS LOST = 2700 SF

REQUIRED FILL = 5056 CY

POND AREA = 106 x 49 = 5194 SF

PROJECT NAME MINI WAREHOUSES SHEET 6 OF 7
PROJECT NO. 99409 BY JCH DATE 4/11/83
SUBJECT CH'D DATE

9.1" ORIFICE 3' HEAD MAX. PROPOSAL 3

TIME MIN	Q_{IN} CFS	VOLUME CF	HEAD FT	Q_{OUT} CFS	VOLUME CF	VOLUME CF DET
1	1.59	96	0.027	0	0	96
5	7.70	1109	0.17	1.00	60	1095
10	15.70	3465	0.69	1.99	444	4061
15	13.80	4860	1.35	2.79	716	8205
20	12.19	3899	1.83	3.25	905	11199
25	10.59	3417	2.21	3.57	1023	13593
30	8.98	2934	2.50	3.79	1105	15422
35	7.38	2454	2.70	3.94	1160	16716
40	5.77	1973	2.83	4.03	1196	17493
45	4.17	1491	2.87	4.06	1214	17770
50	2.57	1011	2.84	4.04	1215	17566
55	0.96	528	2.74	3.97	1201	16893
58	0	86	2.57	3.84	1172	15807

TIME TO EMPTY TANK FROM END OF STORM:

$$t = \frac{2(6615)}{0.299\sqrt{64.4}}(2.57) = 3.94 \text{ HRS}$$

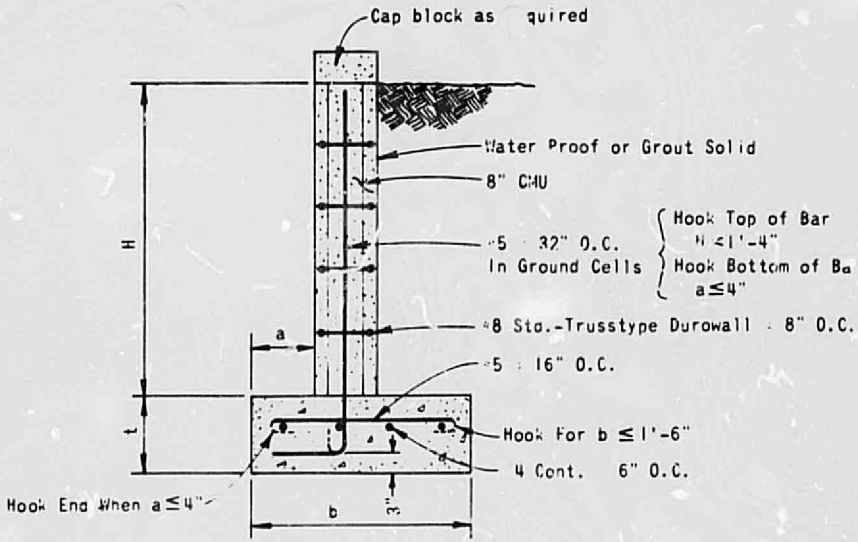
SF OF UNITS LOST = 3300 SF

REQUIRED FILL = 4977 CY

POND SIZE = 49 x 135 = 6615 SF

PROJECT NAME MINI WAREHOUSES SHEET 7 OF 7
PROJECT NO 39409 BY TCH DATE 7/12/83
SUBJECT CH'D DATE

EUBANK BLVD.	CEN[illegible] AVE.	TIJERAS ARROYO	EARTH CHANNEL	9200	.0015	340	L-21 M-21	CONSTRUCT
MOON ST.	SOUTHERN AVE.	SANTA CLARA AVE.	60"	350	.0025	113	L-20	CONSTRUCT
MOON ST.	SANTA CLARA AVE.	COCHITI RD.	54"	1350	.004	108	L-20	PARALLEL
MOON ST.	COCHITI RD.	CENTRAL AVE.	54"	350	.004	108	L-20	PARALLEL
MOON ST.	CENTRAL AVE.	DOMINGO RD.	60"	1800	.007	189	K-20	CONSTRUCT
DOMINGO RD.	MOON ST.	GEN. SOMERVELL ST.	60"	250	.007	189	K-20	CONSTRUCT
GEN. SOMERVELL ST.	DOMINGO RD.	COPPER AVE.	66"	750	.007	244	K-20	CONSTRUCT
LOS ALTOS GOLF COURSE	COPPER AVE. & GEN. SOMERVELL ST.	N.E. TO I-40 CHANNEL	72"	2250	.005	260	K-20	CONSTRUCT
WYOMING BLVD.	CENTRAL AVE.	CHICO RD.	48"	925	.0045	84	K-20	CONSTRUCT
WYOMING BLVD.	CHICO RD.	COPPER AVE.	42"	1350	.0045	58	K-20	PARALLEL
WYOMING BLVD.	COPPER AVE.	MARQUETTE AVE.	42"	1270	.0081	78	K-20	PARALLEL



H	a	b	t
3'-4"	8"	2'-4"	10"
2'-8"	[illegible]	2'-0"	10"
2'-0"	4"	1'-6"	10"
1'-4"	4"	1'-6"	10"
0'-8"	4"	1'-4"	10"
3'-4"	0	2'-4"	10"
2'-8"	0	2'-0"	10"
2'-0"	0	1'-6"	10"
1'-4"	0	1'-6"	10"
0'-8"	0	1'-4"	10"

REINFORCED CONCRETE MASONRY WALL
0 to 3'-4"

NOTE: IF STEEL IS PLACED 2" FROM THE LOADED FACE OF THE WALL AND THIS PRACTICE IS CAREFULLY CONTROLLED. 6" C.M.U. BLOCKS MAY BE USED IN LIEU OF 8" C.M.U. BLOCKS.

us Concrete
f Wall

BOHANNAN-HUSTON INC.
4125 CARLISLE BLVD., N.E. ALBUQUERQUE, NEW MEXICO 87107 505 881-2000
6713 VISCOUNT BLVD., EL PASO, TEXAS 79925 915 778-4491
[illegible]30 C[illegible]FIELD, SANTA FE, NEW MEXICO 87501 505 988-7671

L20-D14

April 14, 1983

RECEIVED
APR 15 1983
ENGINEERING

Mr. Brian Burnett
City Engineering – Hydrology
City of Albuquerque
P. O. Box 1293
Albuquerque, NM 87103

Re: Preliminary Drainage Plan for Site at Eubank and Acoma, S.E.

Dear Brian:

As you are aware, there is limited or no downstream capacity for the above-referenced site. Improvements to the downstream facilities are recommended in the AMDS. While these facilities are not yet in place, they are classified as an "A" priority. The recommended improvements include a new 54" line with a design capacity of 108 cfs.

The present site is divided into two sections. The east section is deteriorated asphalt and hard pack earth while the west section is hard pack earth. There are no detention or storm drainage facilities on the site at present. The current peak flow rate generated on-site is approximately 11.0 cfs. The hard pack earth was considered to be 85% impervious with a "C" value of 0.81. This "C" value for hard pack soil is in accordance with page 22.2 of the DPM which states, "Dirt and gravel parking lots, trails and play areas are generally considered impervious for the purposes of "C" factor determinations." Calculations of the on-site flows are enclosed.

Proposed development calls for construction of self storage units with an asphalt driving surface in between the units. A storm detention pond will be located in the southwest corner. The developed peak flow generated on-site is 15.4 cfs. The total runoff volume is 26,728 cfs (see enclosed calculations).

A strict interpretation of the drainage ordinance results in the detention basin discharge being minimized, due to limited downstream capacity. An 8,000 square foot pond three feet deep with a peak discharge of 1.47 cfs (through a 6" orifice) should satisfy the above requirements. The time required to empty the pond from the end of the storm is 11.6 hours. Section "K" of the drainage ordinance states, "Detention basins with longer than six hour evacuation times shall use a 24 hour or longer storm volume and distribution." The 24 hour, 100-year rainfall amount is estimated to be 3.03". Use of the 24 hour, 100-year storm would necessitate an even larger detention basin. A reduction of surface area of the detention basin, while increasing the depth to reduce the evacuation time, is not practical, due to topographical limitations (calculations are enclosed).

PRINCIPALS
JERRY R. BOHANNAN, P. E. & L.S.
LARRY W. HUSTON
DONALD T. CREMANS, P. E.
C. J. (SKIP) MEAD, P. E.
MICHAL M. EMERY, P. E.

ASSOCIATES
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RAMON LARA, P. E.
DARRELL L. WADE, P. E.
PATRICK L. STIEHR, P. E.
DAVID M. Y. MILLIKAN, P. E.

We propose to discharge from the site at approximately 6 cfs. This is a reduction of approximately 45% from the present condition. This will be done through the use of a detention pond with controlled discharge. This proposal results in a pond size of 5,200 square feet with a maximum three-foot depth and a peak discharge of 6.0 cfs. The evacuation time for the detention pond is 1.9 hours. This proposal improves the on-site discharge from the present condition and therefore reduces its impact on the system downstream (calculations are enclosed).

An intermediate solution was also investigated. The average volume of the first two proposals was used in the determination of the pond volume. A pond size of 6,615 square feet, with a maximum three-foot depth, is required. The peak discharge from the pond would be 4.06 cfs. The resulting evacuation time would be 3.9 hours (calculations are enclosed).

The possibility of using a phased solution was investigated. This would allow a higher discharge when the future downstream facilities are built. In this case, it does not appear feasible to construct a pond of the greater size now then come back in the future and fill in a portion of the pond and build additional storage units.

All three drainage solutions investigated represent an improvement of existing on-site and downstream conditions. The major item which must be resolved is to determine the extent to which this property owner must improve the present downstream conditions. We feel that a 5,200 square-foot pond with a maximum discharge of 6.0 cfs is the most reasonable solution.

A complete drainage report will be submitted as soon as the drainage requirements, which will be placed upon development of this property, are known.

If you have any questions, please call Jim Hoagland, Dave Millikan or me, at this office.

Sincerely,

Michial M. Emery, P.E.
Vice President

cc: Mr. Nick Wallin

Enclosures

JCH/dlh
Job No. 3 940 9

City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

April 20, 1983

Mr. Michial M. Emery
Bohannan-Huston, Inc.
4125 Carlisle N.E.
Albuquerque, N.M. 87110

RE: PRELIMINARY DRAINAGE PLAN FOR SITE AT EUBANK & ACOMA SE - YOUR LETTER OF APRIL 14, 1983

Dear Mike:

My comments regarding the referenced submittal:

1. You are correct in interpreting that new development, irrespective of the existing condition of the site, must meet current criteria outlined in the Drainage Ordinance and DPM. We do, however, try and evaluate each case on its own merit and at times, criteria is adjusted to match the situation.

2. Based on the information supplied a 100-year release rate of 6 cfs maybe used to design the pond and outlet works. The following items should be addressed in the drainage plan:

a. Outlet structure and connection to Acoma Rd. N.E.

b. Approved copies of "Construction of Private Drainage Facilities in City Right-of-Way" will be required for any work in City right-of-way.

c. Fencing of the pond since depth exceeds 18".

I suggest that a Drainage Plan be prepared for this site in-leau-of a drainage report. Much of the information required for a development of this site can be placed on one 24" X 36" sheet (see enclosed example). Drainage Plan requirements are contained on pages 106-109 of the DPM.

I appreciate the professional manner in which the material was persented. Please contact me if you have any questions.

Very truly yours,

Brian B. Burnett
Civil Engineer/Hydrology

BGB/tsl

BOHANNAN-HUSTON INC.
4125 CARLISLE BLVD., N.E. ALBUQUERQUE, NEW MEXICO 87107 505 881-2000
6719 VISCOUNT BLVD., EL PASO, TEXAS 79925 915 778-4491
330 GARFIELD, SANTA FE, NEW MEXICO 87501 505 988-7671

April 21, 1983

RECEIVED
APR 25 1983
CITY ENGINEER

Mr. Brian Burnett
Civil Engineer/Hydrology
City of Albuquerque
P.O. Box 1293
Albuquerque, NM 87103

Re: Your Response to the Preliminary Drainage Plan for Site at Eubank and Acoma, Dated April 20, 1983

Dear Brian:

I would like to express my sincere appreciation for your rapid response to our above referenced submittal. Considering the workload you have inherited, the response time was nearly instantaneous.

I would like to also comment upon your judgement regarding this case. You took a very constrained site and, based on the facts, made a decision that did not greatly harm the interests of the developer, or in our opinion, jeopardize the position of the city or its citizens with respect to the allowable discharge which you approved for the area. I would like to very much thank you for your consideration of the facts and for the rational decision I think we can both live with.

We appreciate your extra effort and deliberation in this matter.

Sincerely yours,

Michial M. Emery, P.E.
Vice President

cc: Mr. Nick Wallin
Mr. Jim McClintic
Mr. Jim Miller

MME/blm
Job No. 3 940 9

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BOHANNAN-HUSTON INC.

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6713 VISCOUNT BLVD., EL PASO, TEXAS 79925 915 778-4491
330 GARFIELD, SANTA FE, NEW MEXICO 87501 505 988-7671

May 3, 1983

Mr. Brian Burnett
City Engineering – Hydrology
City of Albuquerque
P. O. Box 1293
Albuquerque, NM 87103

Re: Drainage and Grading Plan at Eubank and Acoma, S.E.

Dear Brian:

Submitted herewith is a copy of the Drainage and Grading Plan for the above-referenced site. This plan has been prepared in accordance with your letter of April 20, 1983.

If you have any questions, please call Jim Hoagland, Dave Millikan or me at this office.

Sincerely,

Michial M. Emery

Michial M. Emery, P.E.
Vice President

Enclosure

JH/dlh
Job No. 2 154 0

RECEIVED
MAY 04 1983
ENGINEERING

PRINCIPALS
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DAVID M. Y. MILLIKAN, P. E.

INFORMATION SHEET

PROJECT TITLE Self-storage Units @ Eubank & Acoma, SE TYPE OF SUBMITTAL Drainage & Grading Plan

ZONE ATLAS PAGE NO. L-20-Z CITY ADDRESS

LEGAL DESCRIPTION Blocks 10 & 11 of Skyline Heights Addition

ENGINEERING FIRM Bohannan–Huston, Inc. CONTACT Dave Millikan

ADDRESS 4125 Carlisle Blvd., NE PHONE 881-2000

OWNER Wallin Construction Co., Inc. CONTACT Nick Wallin

ADDRESS P. O. Box 13596, Albuq., NM 87192 PHONE

ARCHITECT Miller & Associates CONTACT Jim Miller

ADDRESS 3225 Candelaria, NE PHONE 884-1255

SURVEYOR Bohannan–Huston, Inc. CONTACT Dave Millikan

ADDRESS 4125 Carlisle Blvd., NE PHONE 881-2000

CONTRACTOR Wallin Construction Co., Inc. CONTACT Nick Wallin

ADDRESS P. O. Box 13596, Albuq., NM 87192 PHONE

DATE SUBMITTED May 4, 1983

BY David M. G. Millikan

RECEIVED
MAY 04 1983
ENGINEERING

City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

L20 - D14

May 23, 1983

Mr. Dave Millikan
Bohannan-Huston, Inc.
4125 Carlisle Blvd. N.E.
Albuquerque, N.M. 87110

RE: SELF STORAGE UNITS DRAINAGE PLAN (EUBANK & ACOMA)

Dear Dave:

The referenced plan is approved in concept. Per our telephone conversation of 5/19/83:

a. You may want to indicate that the orifice opening in the retaining wall will be located at the sidewalk culvert in the SW corner of the pond.

b. Please provide a scale on the drawing.

c. Approved copies of "Construction of Private Drainage Facilities in Public Right-of-Way" should be placed in the City construction sets prior to Hydrology sign-off.

Please see that any revisions to the drainage plan are accompanied by a revision date by your PE stamp and seal. I will forward a formal approval upon final submittal.

If you have any questions, please call.

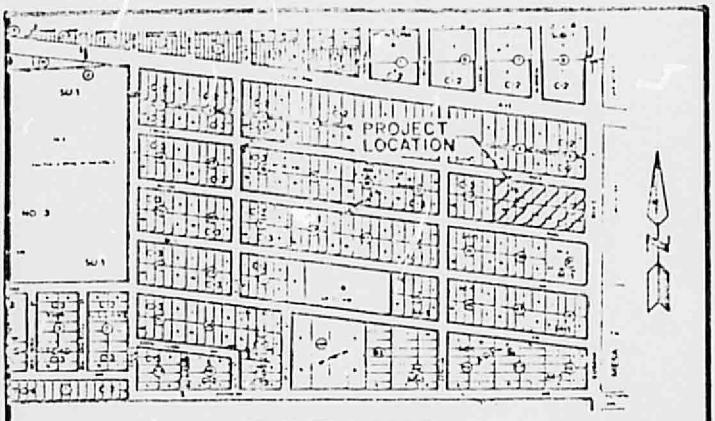
Very truly yours,

Brian G. Burnett
Civil Engineer/Hydrology

BGB/tsl

CITY OF ALBUQUERQUE

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY



VICINITY MAP
NO SCALE

LEGAL DESCRIPTION:
BLOCKS 10 AND 11 OF SKYLINE HEIGHTS ADDITION

L20-014

David M. Gallegos
5-26-83
PROFESSIONAL ENGINEER

NOTICE TO CONTRACTOR

1. An excavation/construction permit will be required before beginning any work within City right-of-way. An approved copy of these plans must be submitted at the time of application for this permit.

2. All work detailed on these plans to be performed, except as otherwise stated or provided hereon, shall be constructed in accordance with "Contract Documents for City-Wide Utilities and Cash Paving No. 31 "

3. Two working days prior to any excavation, contractor must contact Line Locating Service, 765-1234, for location of existing utilities.

4. Prior to construction, the contractor shall excavate and verify the horizontal and vertical locations of all obstructions. Should a conflict exist, the contractor shall notify the engineer so that the conflict can be resolved with a minimum amount of delay.

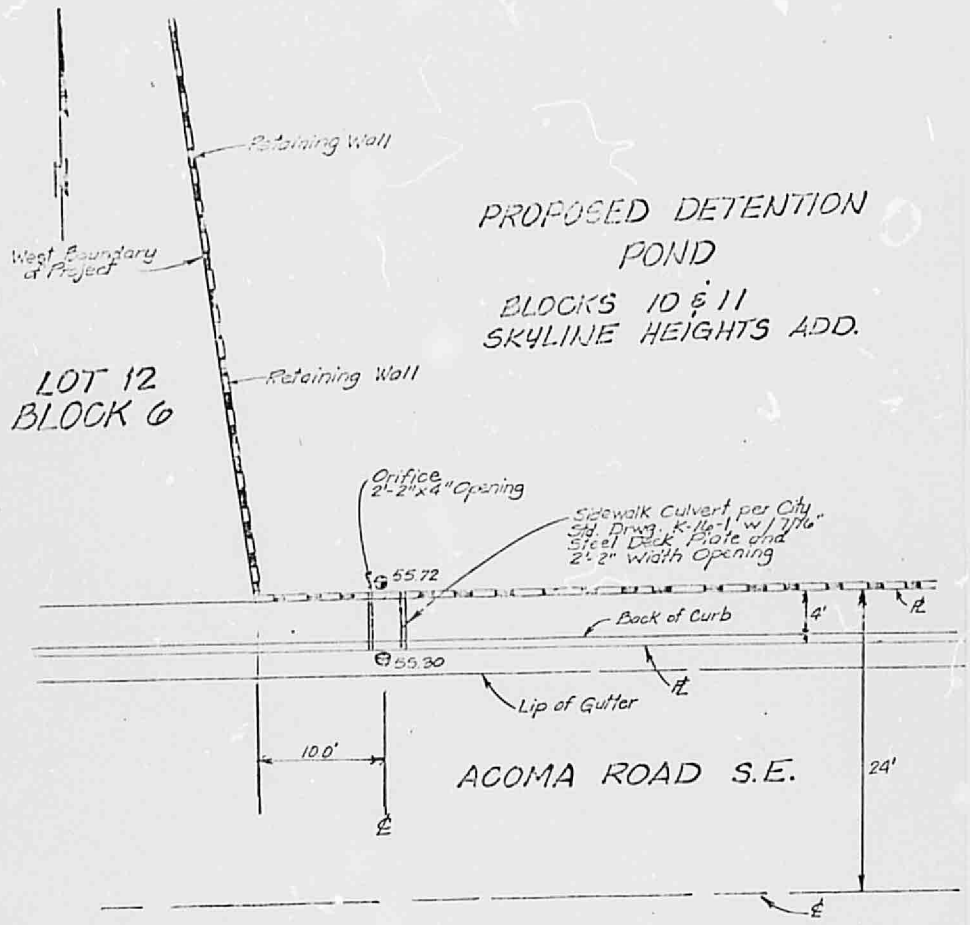
5. Backfill compaction shall be according to Residential street use.

P-09516 Lowry Wright Const C-62

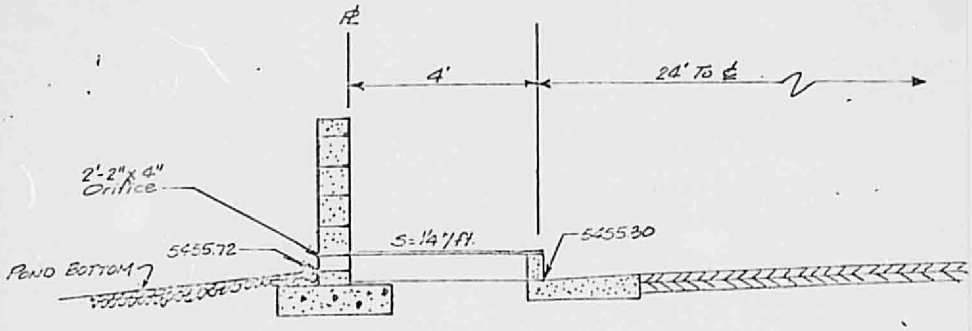
APPROVALS	NAME	DATE	TITLE: 201 Eubank SE SELF STORAGE UNITS at EUBANK & ACOMA, S.E.
A.C.E./DESIGN	[illegible]	26 [illegible] 83	
INSPECTOR	[illegible]	1/16/84	MAP No. L-20

CITY OF ALBUQUERQUE

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY



PLAN VIEW (1"=10')



PROFILE (3/8"=1')

C-62

APPROVALS	NAME	DATE
A.C.E./DESIGN	[illegible]	[illegible]
INSPECTOR	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]

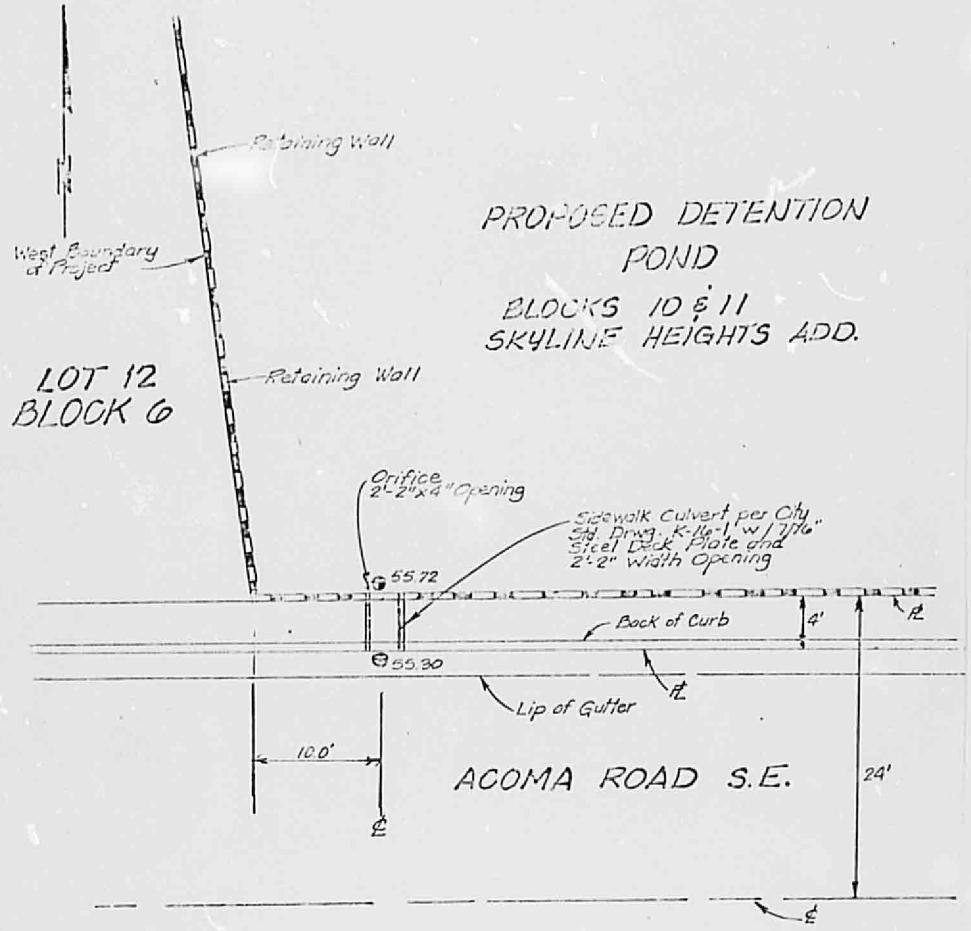
TITLE: SELF STORAGE UNITS on EUBANK & ACOMA, S.E.
201 Eubank SE

PERMIT NO. [illegible] SHEET 2 OF 2

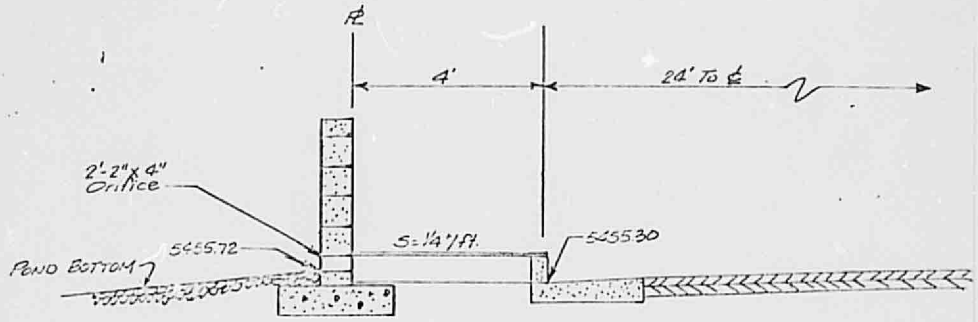
MAP NO. [illegible]

CITY OF ALBUQUERQUE

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY



PLAN VIEW (1"=10')



PROFILE (3/8"=1')

C-62

APPROVALS	NAME	DATE	TITLE: SELF STORAGE UNITS on EUBANK & ACOMA, S.E. 201 Eubank SE
A.C.E./DESIGN	[illegible]	[illegible]	
INSPECTOR	[illegible]	[illegible]	PERMIT NO.
	[illegible]	[illegible]	MAP NO. [illegible]