











PANEL 36

200 to 0 200 400 VICINITY MAP L-20

EXISTING CONDITIONS

The site is located on Trumbuli Avenue 1,200 feet west of Eubank Boulevard, S.E. The area in which the site is located is presently developed as light industrial. The terrain slopes gently from east to west. Trumbull Avenue is paved with standard curb and gutter. The lot to the east, Lot A, Block 52, is developed with all runoff leaving the lot by a 4" PVC pipe through the curb into Trumbull Avenue, so there is no off-site flow from this lot. The site does not lie within a designated flood hazard zone. There is, however, a flood zone downstream from the site which is a result of an inadequately-sized storm drain line in Moon Street. A parallel 54" diameter wash line was constructed to drain the City of Albuquerque reservoir at Garcia and Southern. This line also has inlets in Moon Street which increases the capacity of the Moon Street storm drain. However, no map revision has been done so the flood hazard area in Moon Street is still considered to exist.

PROPOSED CONDITIONS:

It is proposed to develope the site as shown on the grading plan.

DRAINAGE CRITERIA:

The calculations shown on this plan were prepared in accordance with Section 22.2, Hydrology, of the Development Process Manual, Volume 2, Design Criteria, for the City of Albuquerque in cooperation with Bernalillo County, New Mexico and the Albuquerque Metropolitan Arroyo Flood Control Authority, January 1993.

PRECIPITATION ZONE:

The site is east of San Mateo Boulevard but west of Eubank Boulevard and is, therefore, in Precipitation Zone 3.

PEAK DISCHARGE PER ACRE, EXCESS PRECIPITATION AND AREAS:

The existing site is in an industrial area and has been graded at one time. It is considered to be Land Treatment "C" The developed land treatment areas are shown in the following table: q E Existing Site Areas Developed Site Areas Treatment 100-yr.10-yr. 100-yr 10-year % Sq. Ft. Acres % Sq. Ft Acres. A 1.87 0.58 0.66 0.19 0.0 0, 0.0 0.0 0, 0.0 B 2.60 1.19 0.92 0.36 0.0 0, 0.0 12.9 5,813 0.133 C 3.45 2.00 1.29 0.62 95.4 43,007 0.987 18.5 8,344 0.192 D 5.02 3.39 2.36 1.50 <u>4.6</u> <u>2.075</u> <u>0.048</u> <u>68.6</u> <u>30.925</u> <u>0.710</u> 100.0 45,082 1.035 100.0 45,082 1.035

WEIGHTED UNIT PEAK DISCHARGE VALUES:

Existing $q_{W100} = 0.954 \times 3.45 + 0.046 \times 5.02 = 3.52 \text{ cfs/acre}$ $q_{W10} = 0.954 \times 2.00 + 0.046 \times 3.39 = 2.06 \text{ cfs/acre}$

Developed $q_{W100} = 0.129 \times 2.60 + 0.185 \times 3.45 + 0.686 \times 5.02 = 4.42 \text{ cfs/acre}$ $q_{W10} = 0.129 \times 1.19 + 0.185 \times 2.00 + 0.686 \times 3.39 = 2.85 cfs/acre$

CONSTRUCTION NOTES:

- 1. THIS PLAN SHOWS ELEVATIONS AND DISPOSITION OF STORM WATER RUNOFF ONLY. NO DIMENSIONS ARE SHOWN ON THE PLAN. FOR DIMENSIONS OF
- BUILDING AND PARKING LAYOUT, SEE ARCHITECTURAL SITE PLAN. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
- 3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.

LEGAL DESCRIPTION:

Present Legal Description: All of Lots 11 through 16, Block 53 of Skyline Heights Addition together with the west half (W 1/2) of vacated EK Street per quit claim deed filed march 04, 1960 in book D530, Page 551

Legal Description Upon Completion of Replat:

Lot 16-A, Block 53 of Skyline Heights Addition

FRANK D. LOVELADY, P. E 300 ALAMOSA ROAD N.W. ALBUQUERQUE, N. M. 87107 (505) 345-2267

WEIGHTED EXCESS PRECIPITATION:

. E_{W100} = 0.959 X 1.29 + 0.046 X 2.36 = 1.34 in. $E_{W10} = 0.959 \times 0.62 + 0.046 \times 1.50 = 0.66$ in.

 $E_{W100} = 0.129 \times 0.92 + 0.185 \times 1.29 + 0.686 \times 2.36 = 1.98$ in. Developed $E_{W10} = 0.129 \times 0.36 + 0.185 \times 0.62 + 0.686 \times 1.50 = 1.19$ in.

SUMMARY OF VOLUMES AND PEAK DISCHARGE RATES

	<u>V100</u>	V10	Q100	Q10
EXISTING	5,034	2,480	3.64	2.13
DEVELOPED	7,439	4,471	4.57	2.95
NCREASE	2,405	1,991	0.93	0.82

DOWNSTREAM CAPACITY:

The site will drain into Trumbull Avenue. Approximately 750' west of that point, at Altez Street, there are inlets on both sides of Trumbull Avenue. The inlet on the south side of the street will intercept flow from the site. Any flow passing these inlets will continue down Trumbull Avenue to Moon Street where there are 6 inlets in the intersection. This intersection is not actually in the Moon Street flood zone which begins just south of Bell Street. A 54" RCP storm drain line has been installed in Moon Street parallel to the existing 36" RCP storm drain line and the two lines are interconnected at manholes. The new line is a "wash" line for the City Reservoirs at Garcia and Trumbull. The distance from Altez Street to Eubank Boulevard is roughly 2,100 feet. The site is, therefore, in the lower 1/3 of the watershed and runoff from the site will have entered the storm sewer and passed the flood zone before peak flows arrive. Any continuation of the watershed east of Eubank Boulevard that may have existed will now be eliminated by the storm drain under construction in Eubank Blvd. The site is an infill site. For these reasons, unrestricted discharge is warranted for this site.

OFF-SITE FLOW:

The property south of the site is almost flat but does have some slope in a northwesterly direction. However, it appears to pass south of the SW corner of the site. Lot A to the east drains to Trumbull Street. An area of approximately 45' X 180' = 8,100 sf, or 0.186 acre, may drain into the site just north of the SE corner. It is assumed that the peak discharge per acre is the same q_W as was used for this site, or 4.42 cfs / acre. $Q_{100} = 4.42 \times 0.186 = 0.82$ cfs.

DRAINAGE CHANNEL CAPACITY:

Design Q = 4.57 cfs + 0.82 cfs - 0.45 cfs = 4.94 cfs (0.45cfs drains directly to the street through the driveway. Use gravel-lined channel, 5.0' wide flat bottom with a concrete containment curb on the west side and a 2:1 gravel-lined side slope on the east side. Depth of flow = 5" $A = 5.0 \times 0.41 + (0.41 \times 0.82)1/2 = 2.22 \text{ sf}$

 $P = 0.41 + 5.0 + (0.41^2 + 0.82^2)^{1/2} = 6.33 \text{ ft. } R = A/P = 2.22 / 6.33 = 0.3507$ N = 0.012 for concrete and 0.023 for gravel $Nw = (0.41 \times 0.013 + 5.92 \times 0.023)/6.33 = 0.022$ $V = (1.486 / 0.022) (0.3507)^{2/3} (0.0050)^{1/2} = 2.37 \text{ fps } Q = AV = 2.22 \times 2.37 = 5.26 \text{ cfs}$ 5.26 cfs > 4.94 cfs (Adequate)

SIDEWALK CULVERT CAPACITY:

Design Q = 4.94 cfs Use 2 ea 2'-0" sidewalk culverts. Use Weir Equation Q = CLH3/2 $C = 3.0 L = 4 H = 0.58' (7") Q = 3.0 \times 4 \times (0.58)3/2 = 5.30 cfs > 4.94 cfs (Adequate)$

EROSION CONTROL REQUIREMENTS:

The contractor shall be responsible for compliance with the following:

- 1. No sediment-bearing water shall be allowed to discharge from the site during construction.
- During grading operations and until the project has been completed, all adjacent property, rights-of-way, and easements shall be protected from flooding by runoff
- 3. Should the contractor fail to prevent sediment-bearing water from entering public right-of-way, he shall promptly remove from the public right-of-way any and all sedimentation originating from the site.
- Control of sediment-bearing waters will be accomplished by use of a compacted earth berm of adequate height. The berm shall be located along the downstream perimeter of the property.

BENCH MARK:

Elevations shown are based on ACS Station "SUSAN", with and Elevation of 5444.49 Feet.

GRADING AND DRAINAGE PLAN OFFICE/WAREHOUSE COMPLEX FOR ALL-AMERICAN FOAM

SHEET

Designed: F.D.L. Drawn: STAFF Checked: F.D.L. Scale: 1"=20' Date: 9/95 Job No. 500