

GRADING & DRAINAGE PLAN

AVENUE. THE GRADING AND DRAINAGE SCHEME HEREON IS IN COMPLIANCE WITH THE BERNALILLO PLAN IS REQUIRED IN ORDER TO FACILITATE THE OWNER'S REQUEST FOR BUILDING PERMIT. THE

- 1. EXISTING CONTOURS, AND SPOT ELEVATIONS AND EXISTING DRAINAGE PATTERNS, AND
- 2. PROPOSED IMPROVEMENTS: 1500 SF WAREHOUSE BUILDING, CONCRETE FLAT WORK, NEW GRADE ELEVATIONS, AND LANDSCAPING.
- 3. CONTINUITY BETWEEN EXISTING AND PROPOSED ELEVATIONS. 4. QUANTIFICATION OF DEVELOPED FLOWS GENERATED BY THE IMPROVEMENTS WHICH

CONTRIBUTE TO THE EXISTING FLOWS. PRESENTLY. THE SITE A DIRT, "HARD-PAN" SURFACE PRIMARILY WITHOUT VEGETATION. THE SITE

IS BOUNDED BY DEVELOPED THOUGH VACANT COMMERCIALLY ZONED PROPERTY. THE SITE FALLS APPROXIMATELY 1% FROM SOUTHEAST TO NORTHWEST. PRIMARY ACCESS TO THE SITE IS FROM COCHITI ROAD AND WILL REMAIN SO. SITE RUNOFF WILL BE ALLOWED TO DRAIN THROUGH THE SITE, OR PONDED IN DEPRESSED LANDSCAPE AREAS. THE

HISTORICAL DOWNSTREAM OUTFALL LOCATIONS WILL REMAIN UNCHANGED WITH DEVELOPMENT. FREE DISCHARGE OF SITE RUNOFF IS ACCEPTABLE SINCE DOWNSTREAM CAPACITY EXISTS WITH THE MINIMAL INCREASE DUE TO DEVELOPMENT. A PORTION OF SITE RUNOFF IS ROUTED THROUGH PROPOSED LANDSCAPING.

CALCULATIONS

SITE HAS HISTORICALLY DRAINED TO THE NORTHWEST.

DESIGN_CRITERIA

HYDROLOGIC METHODS PER SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL (DPM)
REVISED JANUARY 1993 FOR CITY OF ALBUQUERQUE, ADOPTED BY THE COUNTY OF BERNALILLO DISCHARGE RATE: Q=QPEAK x AREA.."Peak Discharge Rates For Small Watersheds" VOLUMETRIC DISCHARGE: VOLUME = EWeighted x AREA

P100 = 2.90 Inches, Zone 4 Time of Concentration, TC = 10 Minutes

DESIGN STORM: 100-YEAR/6-HOUR, 10-YEAR/6-HOUR [] = 10 YEAR VALUES

100% C
LOT AREA = 0.16 ACRES, WHERE EXCESS PRECIP. 'Composite' =1.46 In. [0.73],
PEAK DISCHARGE, Q100 = 0.6 CFS [0.4], WHERE UNIT PEAK DISCHARGE 'C' = 3.73 CFS/AC. [2.26]
THEREFORE: VOLUME 100 = 848 CF [424]

DEVELOPED CONDITIONS

DETERMINE LAND TREATMENTS, PEAK DISCHARGE AND VOLUMETRIC DISCHARGE FOR STUDY AREA

UNDEVELOPED LANDSCAPING — COMPACTED SOIL & Slopes > ROOF — PAVEMENT	AREA Ac. 0.035 Ac. 0.035 Ac. 0.09 Ac. 0.16 Ac.	LAND_TREATM'T A B C D	Q Peak 2.20[0.87] 2.92[1.45] 3.73[2.26] 5.25[3.57]	E 0.80[0.28] 1.08[0.46] 1.46[0.73] 2.64[1.69]
THEREFORE: E _{Weighted} = 2.04 Q100 = 0.71 CFS Q10 = 0.45 CFS	In.[1.21] 8	VOLUME 100 = VOLUME 10 =		

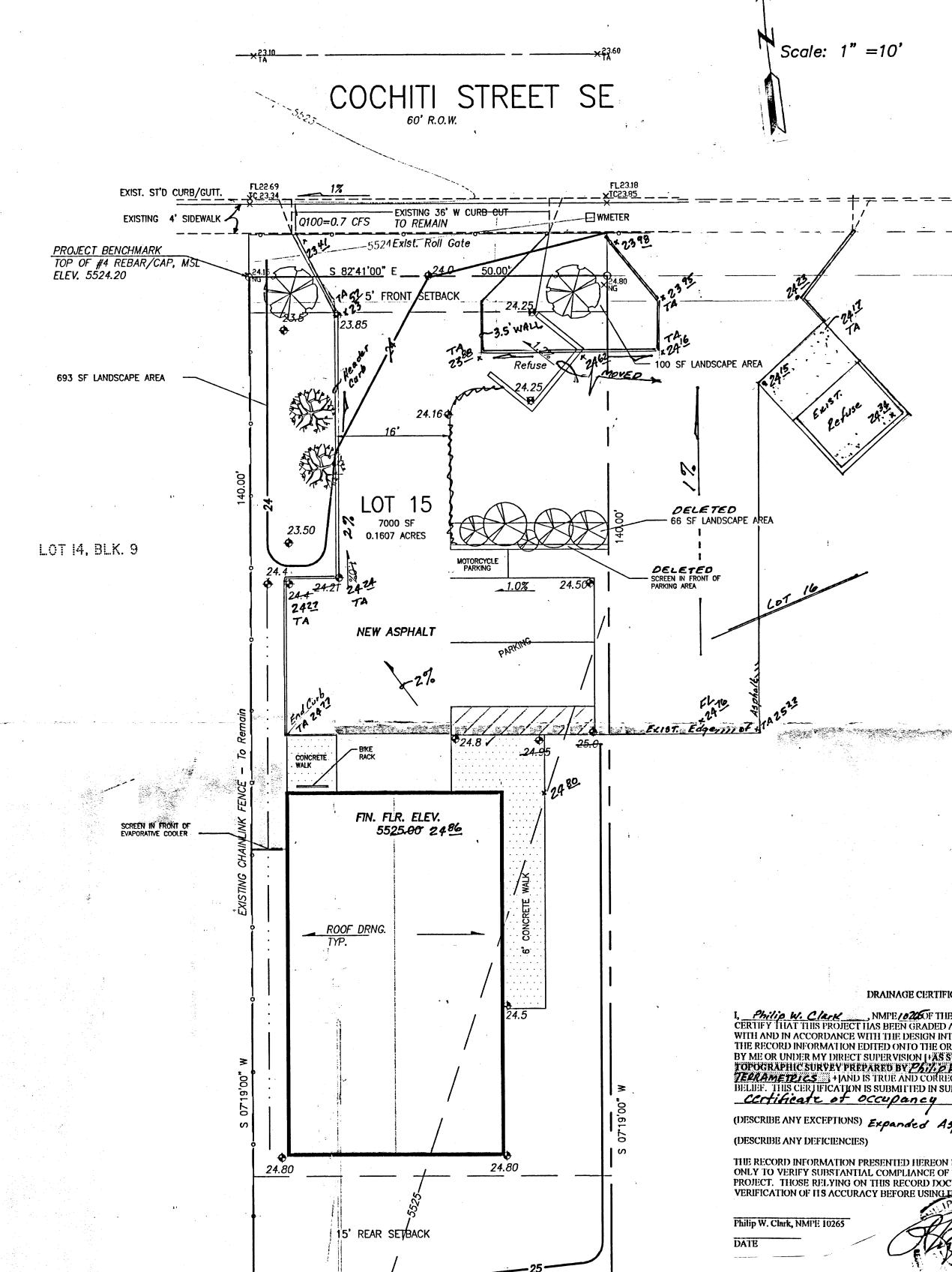
Q10 = 0.45 CFS

DOWNSTREAM ANALYSIS THE SITE HISTORICALLY, AND AS DEVELOPED WILL OUTFALL RUN-OFF TO

THE NORTH TO COCHITI ROAD

FREE DISCHARGE IS ACCEPTABLE SINCE THE DOWNSTREAM STORM DRAINAGE FACILITIES HAVE CAPACITY. THE PROJECT TIME TO PEAK IS MUCH LESS THAN OVERALL BASIN TIME TO PEAK & INCREASE DUE TO DEVELOPMENT IS NEGLIGIBLE. (INCREASE FROM THE EXISTING. (.10±CFS)

A PORTION OF DEVELOPED RUNOFF WILL BE ROUTED TO AND/OR THROUGH REQUIRED LANDSCAPING.



I, PHILIP W. CLARK, A PROFESSIONAL ENGINEER LICENSED IN ACCORDANCE WITH THE LAWS OF THE STATE OF NEW MEXICO, DO HEREBY CERTIFY THAT I HAVE VISITED THE SITE SHOWN HEREON, AND THAT THE CONTOURS SHOWN REPRESENT THE EXISTING GROUND CONDITIONS, AND DO FURTHER CERTIFY THAT NO EARTHWORK OF ANY KIND, NOR ANY DISTURBANCE OF THE EXISTING GROUND HAS OCCURRED ON THIS SITE SINCE THE CONTOURS WERE

N 82'41'00'''W

ZONE L-21

VICINITY MAP

- 1. ANY WORK WITHIN THE RIGHT-OF-WAY SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECS. FOR PUBLIC WORKS CONSTRUCTION, 7TH EDITION W/ AMEND.
- 3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES, AND
- 4. CONTRACTOR SHALL ENSURE THAT NO SITE SOILS/SEDIMENT OR SILT ENTER THE RIGHT-OF-WAYS DURING CONSTRUCTION.
- 5. MAXIMUM SITE GRADING WITHOUT EROSION PROTECTION: 3 HORIZONTAL TO 1 VERTICAL, 3:1. ALL DIMENSIONS TO FACE OF CURB, UNLESS NOTED OTHERWISE.

LEGEND

	257	~ / Ac R
EXIST. SPOT ELEVATION	+24.0 25227	(As.Buil.1
EXIST. CONTOUR	10	
NEW SPOT ELEVATION	4 24.0	
NEW CONTOUR	12	
NEW SWALE		
DRAINAGE DIRECTION, EXISTING		
NEW P.C.C., CONCRETE		
TOP OF CURB, EXISTING	TC	
FLOWLINE	FL	
FACE OF CURB/FACE OF CURB	F-F	
Top of Asphalt	TA	

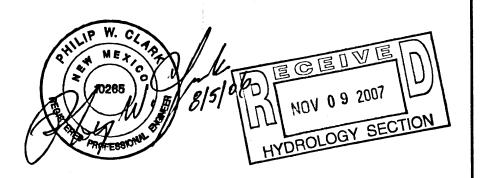
DRAINAGE CERTIFICATION

I, Philip W. Clark NMPE/0260F THE FIRM Clark Consulfing English, HEREBY CERTIFY THAT THIS PROJECT HAS BEEN GRADED AND WILL DRAIN IN SUBSTANTIAL COMPLIANCE WITH AND IN ACCORDANCE WITH THE DESIGN INTENT OF THE APPROVED PLAN DATED 8/5/06
THE RECORD INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY ME OR UNDER MY DIRECT SUPERVISION 1+AS SUPPLEMENTAL DATA TO THE ORIGINAL TOPOGRAPHIC SURVEY PREPARED BY PANA PARE TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR

(DESCRIBE ANY EXCEPTIONS) Expanded Asphalt & Refuse to Dwners

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE ASPECTS OF THIS PROJECT. THOSE RELYING ON THIS RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE.





PROJECT DATA

LEGAL DESCRIPTION (Existing Platting)

LOT 15. BLOCK 9, EAST CENTRAL BUSINESS ADDITION ALBUQUERQUE, NEW MEXICO

PROJECT BENCHMARK

TOP OF NUMBER 4 REBAR/CAP AT THE PROJECT MARKED NORTHWEST PROPERTY CORNER, MSL ELEVATION = 5524.20 (TIE FROM ACS 1-L21, LOCATED IN THE MEDIAN OF CENTRAL AVE. 81' WEST OF THE JUAN TABO INTERSECTION)

TOPOGRAPHIC DESIGN SURVEY

COMPILED BY CLARK CONSULTING ENGINEERS FROM DESIGN SURVEY PERFORMED BY TERRAMETRICS OF NEW MEXICO, 8/2006

Clark Consulting Engineers Edgewood, New Mexico 87015 Fax: (505) 281-2444 LOT15, BLOCK 9, EAST CENTRAL BUS. ADDITION 11/6/07 AS.BUILT LEVAN WAREHOUSE 11508 COCHITI ROAD, SE Grading & Drainage

DESIGNED BY: PWC | DRAWN BY: CCE | JOB #: Levan_GD

CHECKED BY: PWC DATE: 7/31/06 FILE #: G/D

1. All work shall conform to the 2003 International Building Code.

Live loads:

Roof load 20 psf
Floor load 40 psf
Seismic zone 2b requirements
Wind loading 75 mph, exposure C

- 2. Cast in place concrete:
- a. Compressive strength of cast in place concrete 3000 psi at 28 days
- b. Reinforcing steel shall be ASTM A-615 Grade 60 #5 and larger, Grade 40 #4 and smaller.
 - 3. Wood
- a. Unless otherwise noted on drawings lumber shall be No. 2 Ponderosa Pine with allowable repetitive use fiber bending stress of 975 psi, single use fiber bending stress of 850 psi, and elastic modulus of 1500000 psi.
- b. Where Hem-Fir is specified on plans ot shall be No. 1 with allowable repetitive use fiber bending stress of 1200 psi, single use fiber bending stress of 1200 psi, and elastic modulus of 1500000 psi.
- c. Micro-Lam lumber shall satisfy the following design values:

 Bending (Fb) = 2600 psi

 Horizontal shear (Fy) = 285 psi

 Modulus of elasticity (E) = 1800000 psi

 Compression perpendicular to grain = 650 psi

 Compression parallel to grain (Fc) = 2460 psi

Drilling or notching of Micro-Lam lumber is not allowed.

4. Design Soil bearing pressure 1500 psf with footings placed on natural ground and slab placed on engineered compacted fill.

CONSTRUCTION CRITERIA

- 1. Lap reinforcing bars 32 diameters unless otherwise noted.
- 2. Construction joints location and type shall have prior approval by Engineer.
- 3. Fill material shall consist of soils that conform to the following characteristics:

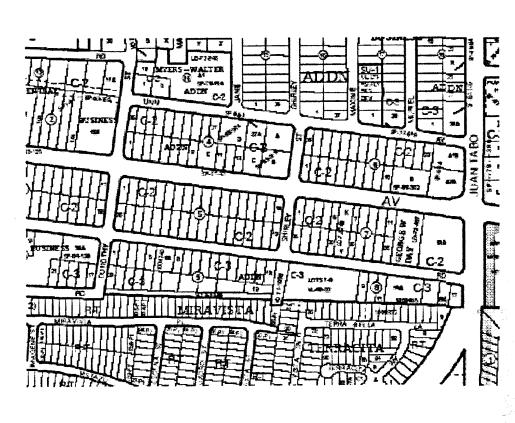
Sieve Size Percent Passing
(Square openings) by weight

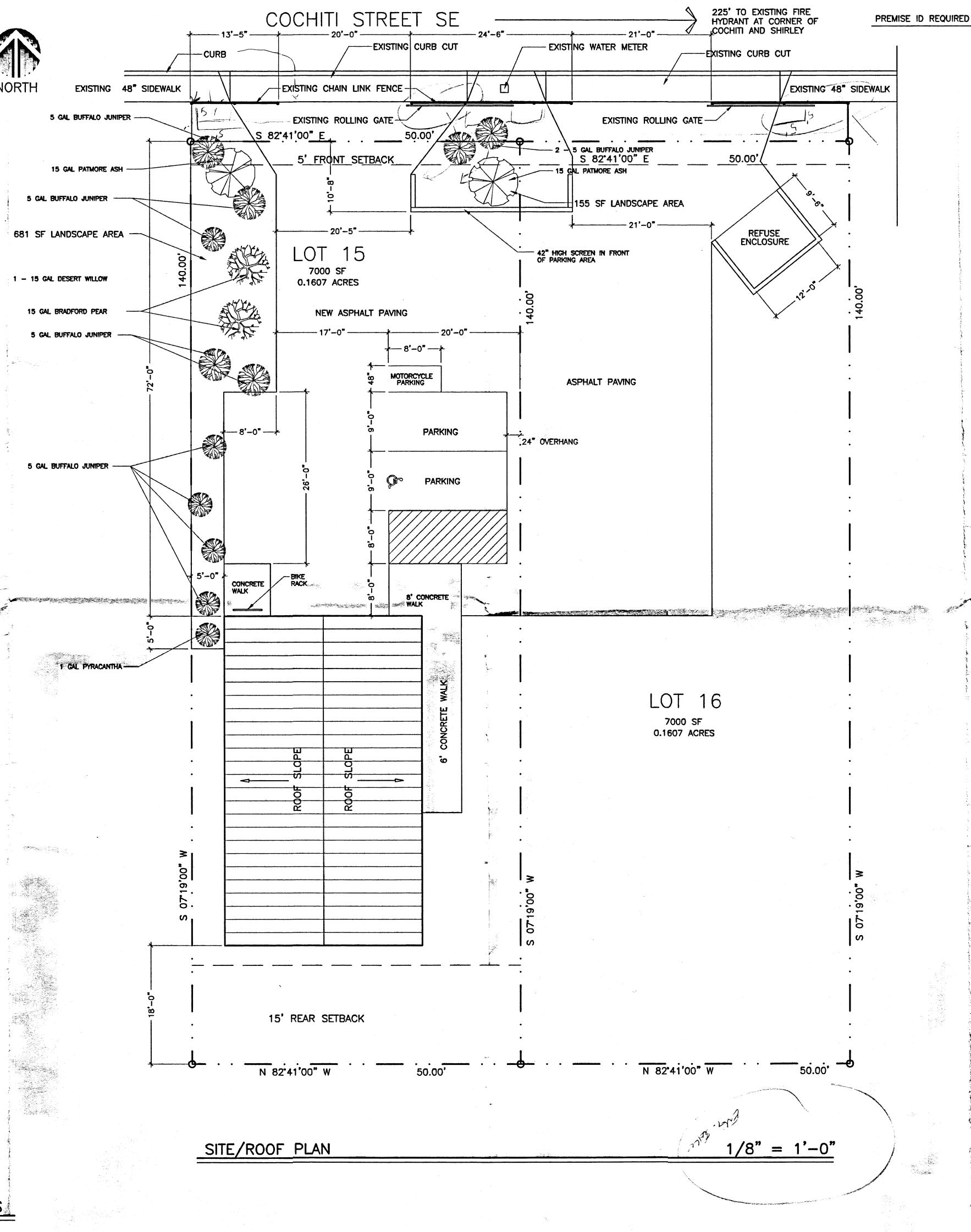
3 inch
No. 4 50-100
No. 200 10-40

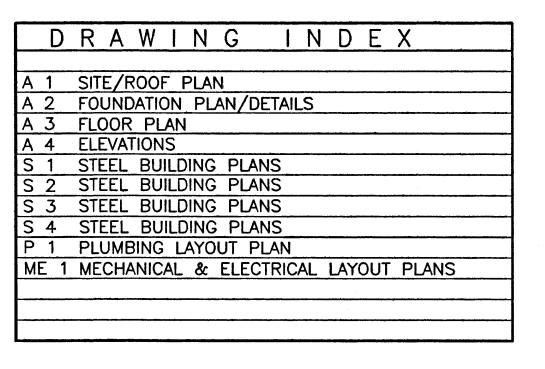
The plasticity index of the material shall not exceed 10. Testing shall be in conformance with ASTM D 423 and 424 for P.I. and D-1557 for density.

- 4. Where slabs are placed on fill the native soil shall be scarified to a minimum depth of 12 inches, watered as necessary to bring the moisture content as close as possible to optimum moisture content, and compacted to 95% of maximum density.

 Fill shall be spread in loose depth layers not exceeding 8 in. watered and compacted. Moisture content at the time of compaction shall be 2% below optimum moisture or higher. A minimum density of 95% of maximum density shall be obtained. Optimum moisture content and maximum density for each soil type shall be determined in accordance with ASTM D 1557.
- 5. Contractor is responsible for any temporary bracing required to hold structural elements in place until work is complete.
- 6. Contractor shall coordinate slab openings with Mechanical and Electrical drawings. (Mechanical and Electrical openings are not shown on Structural drawings.)
- 7. All conditions shown on the plan shall be field verified by the contractor. If discrepancies exist they shall be brought to the attention of the Architect and Engineer before work proceeds.







A D D R E S S

11508 COCHITI STREET SE
ALBUQUERQUE, NEW MEXICO 87123

O W N E R

LEVANCO, LLC

808 KIVA DRIVE SE

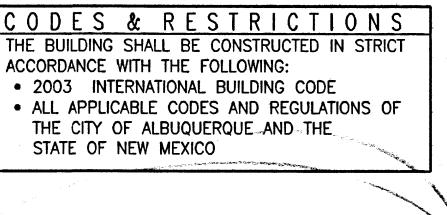
ALBUQUERQUE, NM 87123

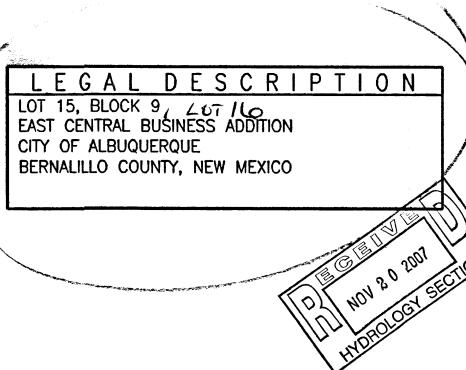
BUILDING	DATA
DUIL DING	TOTAL 1500 S.F.
BUILDING	101AL 1300 3.F.
OCCUPANT LOAD	3
OCCUPANCY GROUP	S-2
OCCOLANCE GROOT	3-2
TYPE OF CONSTRUCTION	VB

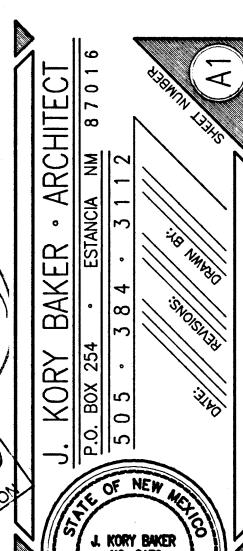
DATA

	3112 0717						
	ZONING: C-3 ZONE ATLAS PAGE: L-21-Z						
	PARKING REQUIRED: 1 SPACE PER 2000 SF						
	OF LEASABLE AREA						
	1500 SF = 1 SPACE						
	PARKING PROVIDED:						
	2 SPACES INCLUDING 1 VAN ACCESIBLE						
	HANDICAPPED SPACE						
	LANDSCAPE AREA REQUIRED:						
	GROSS SITE AREA = 7000 SF						
	BUILDING AREA = 1500 SF						
	NET SITE AREA $= 5500 \text{ SF}$						
No.	15% OF NET AREA = 825 SF REQUIRED						
	LANDSCAPE AREA PROVIDED: 836 SF						

SITE







BUILDING

LEVANCO

VICINITY MAP

NITC

DESIGN CRITERIA

1. All work shall conform to the 2003 International Building Code.

Live loads:

Roof load 20 psf
Floor load 40 psf
Seismic zone 2b requirements
Wind loading 75 mph, exposure C

- 2. Cast in place concrete:
- a. Compressive strength of cast in place concrete 3000 psi at 28 days
- b. Reinforcing steel shall be ASTM A-615 Grade 60 #5 and larger, Grade 40 #4 and smaller.
 - 3. Wood
- a. Unless otherwise noted on drawings lumber shall be No. 2 Ponderosa Pine with allowable repetitive use fiber bending stress of 975 psi, single use fiber bending stress of 850 psi, and elastic modulus of 1500000 psi.
- b. Where Hem-Fir is specified on plans ot shall be No. 1 with allowable repetitive use fiber bending stress of 1200 psi, single use fiber bending stress of 1200 psi, and elastic modulus of 1500000 psi.
- c. Micro-Lam lumber shall satisfy the following design values:

 Bending (Fb) = 2600 psi

 Horizontal shear (Fy) = 285 psi

 Modulus of elasticity (E) = 1800000 psi

 Compression perpendicular to grain = 650 psi

 Compression parallel to grain (Fc) = 2460 psi

Drilling or notching of Micro-Lam lumber is not allowed.

4. Design Soil bearing pressure 1500 psf with footings placed on natural ground and slab placed on engineered compacted fill.

CONSTRUCTION CRITERIA

- 1. Lap reinforcing bars 32 diameters unless otherwise noted.
- 2. Construction joints location and type shall have prior approval by Engineer.
- 3. Fill material shall consist of soils that conform to the following characteristics:

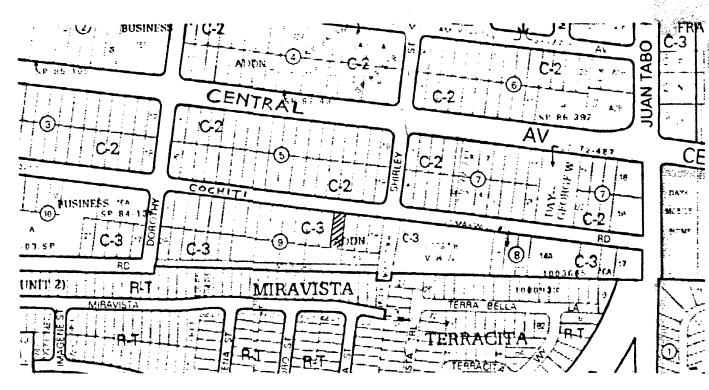
Sieve Size Percent Possing (Square openings) by weight 3 inch 100

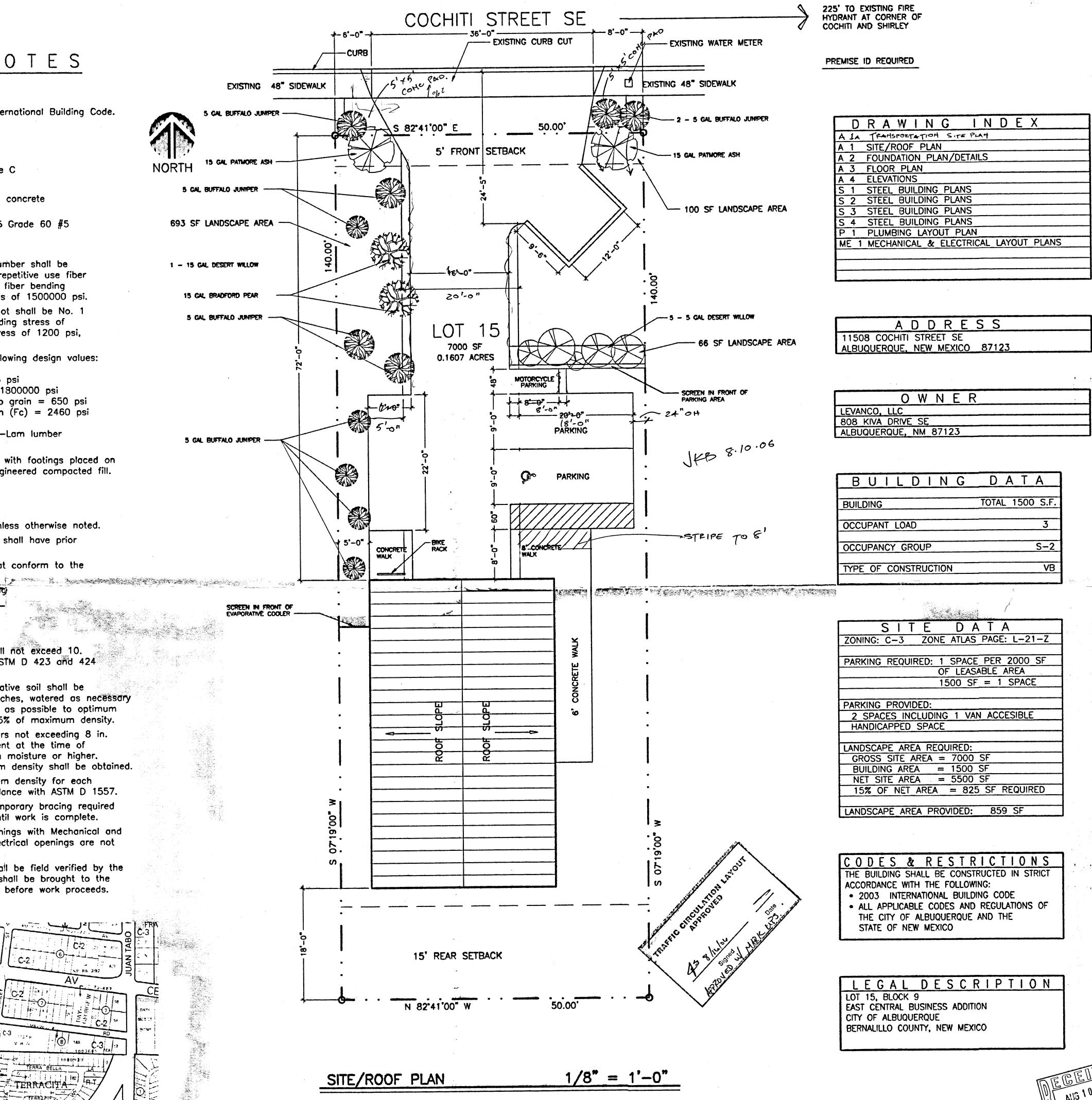
No. 4 50-100
No. 200 10-40
The plasticity index of the material

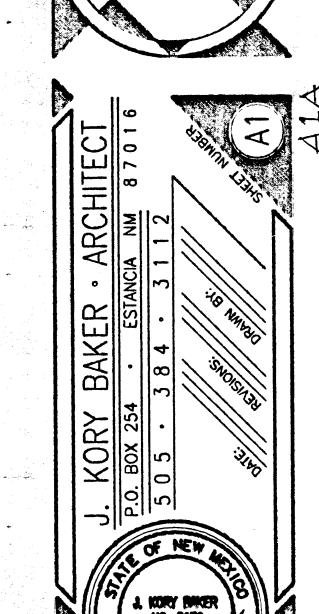
The plasticity index of the material shall not exceed 10. Testing shall be in conformance with ASTM D 423 and 424 for P.I. and D-1557 for density.

- 4. Where slabs are placed on fill the native soil shall be scarified to a minimum depth of 12 inches, watered as necessary to bring the moisture content as close as possible to optimum moisture content, and compacted to 95% of maximum density.

 Fill shall be spread in loose depth layers not exceeding 8 in. watered and compacted. Moisture content at the time of compaction shall be 2% below optimum moisture or higher. A minimum density of 95% of maximum density shall be obtained. Optimum moisture content and maximum density for each soil type shall be determined in accordance with ASTM D 1557.
- 5. Contractor is responsible for any temporary bracing required to hold structural elements in place until work is complete.
- 6. Contractor shall coordinate slab openings with Mechanical and Electrical drawings. (Mechanical and Electrical openings are not shown on Structural drawings.)
- 7. All conditions shown on the plan shall be field verified by the contractor. If discrepancies exist they shall be brought to the attention of the Architect and Engineer before work proceeds.







LEVANCO.

GENERAL NOTES

DESIGN CRITERIA

1. All work shall conform to the 2003 International Building Code.

Live loads:

Roof load 20 psf
Floor load 40 psf
Seismic zone 2b requirements
Wind loading 75 mph, exposure C

- 2. Cast in place concrete:
- a. Compressive strength of cast in place concrete 3000 psi at 28 days
- b. Reinforcing steel shall be ASTM A-615 Grade 60 #5 and larger, Grade 40 #4 and smaller.
 - 3. Wood
- a. Unless otherwise noted on drawings lumber shall be No. 2 Ponderosa Pine with allowable repetitive use fiber bending stress of 975 psi, single use fiber bending stress of 850 psi, and elastic modulus of 1500000 psi.
- b. Where Hem-Fir is specified on plans ot shall be No. 1 with allowable repetitive use fiber bending stress of 1200 psi, single use fiber bending stress of 1200 psi, and elastic modulus of 1500000 psi.
- c. Micro-Lam lumber shall satisfy the following design values:

 Bending (Fb) = 2600 psi

 Horizontal shear (Fy) = 285 psi

 Modulus of elasticity (E) = 1800000 psi

 Compression perpendicular to grain = 650 psi

 Compression parallel to grain (Fc) = 2460 psi

Drilling or notching of Micro-Lam lumber is not allowed.

4. Design Soil bearing pressure 1500 psf with footings placed on natural ground and slab placed on engineered compacted fill.

CONSTRUCTION CRITERIA

- 1. Lap reinforcing bars 32 diameters unless otherwise noted.
- 2. Construction joints location and type, shall have prior approval by Engineer.
- 3. Fill material shall consist of soils that conform to the

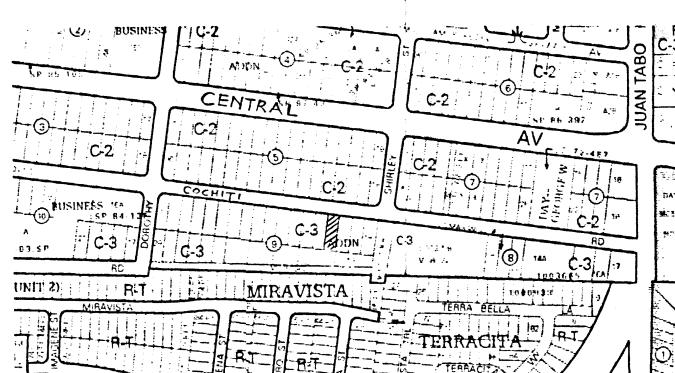
Sieve Size Percent Passing
(Square openings) by weight

3 inch
No. 4 50-100
No. 200 10-40

The plasticity index of the material shall not exceed 10. Testing shall be in conformance with ASTM D 423 and 424 for P.I. and D-1557 for density.

- 4. Where slabs are placed on fill the native soil shall be scarified to a minimum depth of 12 inches, watered as necessary to bring the moisture content as close as possible to optimum moisture content, and compacted to 95% of maximum density. Fill shall be spread in loose depth layers not exceeding 8 in. watered and compacted. Moisture content at the time of compaction shall be 2% below optimum moisture or higher. A minimum density of 95% of maximum density shall be obtained. Optimum moisture content and maximum density for each
- soil type shall be determined in accordance with ASTM D 1557.

 5. Contractor is responsible for any temporary bracing required to hold structural elements in place until work is complete.
- 6. Contractor shall coordinate slab openings with Mechanical and Electrical drawings. (Mechanical and Electrical openings are not shown on Structural drawings.)
- 7. All conditions shown on the plan shall be field verified by the contractor. If discrepancies exist they shall be brought to the attention of the Architect and Engineer before work proceeds.



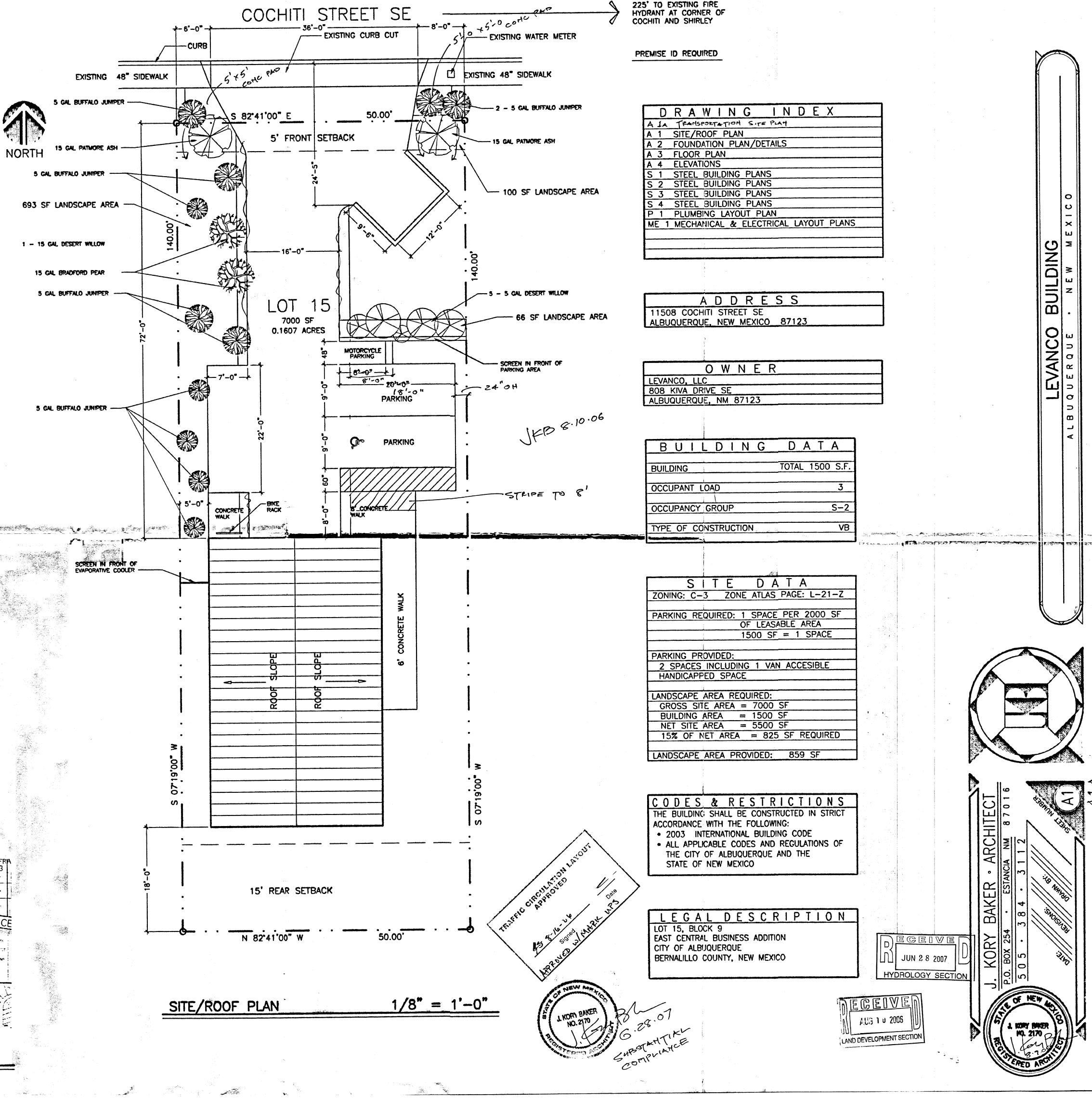
ecessary
mum
isity.
in.
bbtained.

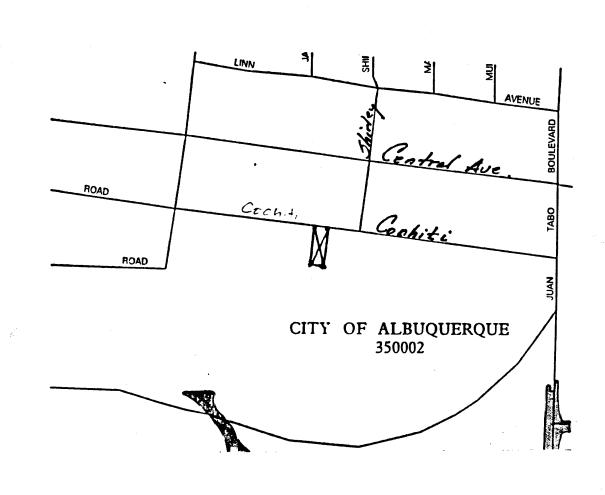
1557.
uired

all and not

by the the
ds.

NTS





FIRM MAP

GRADING & DRAINAGE PLAN

THE PROPOSED WAREHOUSE PROJECT IS LOCATED IN A DEVELOPED BUSINESS PARK IN THE SOUTHEAST AREA OF ALBUQUERQUE OFF JUAN TABO BLVD. IMMEDIATELY SOUTH OF CENTRAL AVENUE. THE GRADING AND DRAINAGE SCHEME HEREON IS IN COMPLIANCE WITH THE BERNALILLO COUNTY FLOOD HAZARD ORDINANCE, NO.88-46, AND THE CITY STORM DRAINAGE ORDINANCE. THE PLAN IS REQUIRED IN ORDER TO FACILITATE THE OWNER'S REQUEST FOR BUILDING PERMIT. THE

PANEL # 359 F

1. EXISTING CONTOURS, AND SPOT ELEVATIONS AND EXISTING DRAINAGE PATTERNS. AND IMPROVEMENTS. 2. PROPOSED IMPROVEMENTS: 1500 SF WAREHOUSE BUILDING, CONCRETE FLAT WORK. NEW

3. CONTINUITY BETWEEN EXISTING AND PROPOSED ELEVATIONS. 4. QUANTIFICATION OF DEVELOPED FLOWS GENERATED BY THE IMPROVEMENTS WHICH CONTRIBUTE TO THE EXISTING FLOWS.

PRESENTLY, THE SITE A DIRT, "HARD-PAN" SURFACE PRIMARILY WITHOUT VEGETATION. THE SITE IS BOUNDED BY DEVELOPED THOUGH VACANT COMMERCIALLY ZONED PROPERTY. THE SITE FALLS APPROXIMATELY 1% FROM SOUTHEAST TO NORTHWEST.

PRIMARY ACCESS TO THE SITE IS FROM COCHITI ROAD AND WILL REMAIN SO. SITE RUNOFF WILL BE ALLOWED TO DRAIN THROUGH THE SITE, OR PONDED IN DEPRESSED LANDSCAPE AREAS. THE SITE HAS HISTORICALLY DRAINED TO THE NORTHWEST.

HISTORICAL DOWNSTREAM OUTFALL LOCATIONS WILL REMAIN UNCHANGED WITH DEVELOPMENT. FREE DISCHARGE OF SITE RUNOFF IS ACCEPTABLE SINCE DOWNSTREAM CAPACITY EXISTS WITH THE MINIMAL INCREASE DUE TO DEVELOPMENT. A PORTION OF SITE RUNOFF IS ROUTED THROUGH PROPOSED LANDSCAPING.

CALCULATIONS

THE NORTH TO COCHITI ROAD

REQUIRED LANDSCAPING.

GRADE ELEVATIONS, AND LANDSCAPING.

DESIGN_CRITERIA HYDROLOGIC METHODS PER SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL (DPM)
REVISED JANUARY 1993 FOR CITY OF ALBUQUERQUE, ADOPTED BY THE COUNTY OF BERNALILLO
DISCHARGE RATE: Q=QPEAK x AREA..."Peak Discharge Rates For Small Watersheds"
VOLUMETRIC DISCHARGE: VOLUME = EWeighted x AREA
P100 = 2.90 Inches, Zone 4 Time of Concentration, TC = 10 Minutes
DESIGN STORM: 100-YEAR/6-HOUR, 10-YEAR/6-HOUR [] = 10 YEAR VALUES EXISTING CONDITIONS

LOT AREA = 0.16 ACRES, WHERE EXCESS PRECIP. 'Composite' =1.46 In. [0.73].

PEAK DISCHARGE, Q100 = 0.6 CFD [0.4], WHERE UNIT PEAK DISCHARGE 'C' = 3.73 CFS/AC. [2.26]

THEREFORE: VOLUME 100 = 848 CFD [424] DEVELOPED CONDITIONS NO .0028 ACRE & = 170 F3 DETERMINE LAND TREATMENTS, PEAK DISCHARGE AND VOLUMETRIC DISCHARGE FOR STUDY AREA Q Peak
2.20[0.87]
2.92[1.45]
3.73[2.26]
5.25[3.57] AREA LAND_TREATM'T UNDEVELOPED LANDSCAPING -0.035 Ac. COMPACTED SOIL & Slopes > 0.035 Ac. ROOF - PAVEMENT 0.09 Ac. THEREFORE: E_{Weighted} = 2.04 In.[1.21] & Q100 = 0.71 CFS VOLUME 100 = 1185 CF VOLUME 10 = 703 CF DOWNSTREAM ANALYSIS THE SITE HISTORICALLY, AND AS DEVELOPED WILL OUTFALL RUN-OFF TO

FREE DISCHARGE IS ACCEPTABLE SINCE THE DOWNSTREAM STORM DRAINAGE FACILITIES HAVE CAPACITY. THE PROJECT TIME TO PEAK IS MUCH LESS

THAN OVERALL BASIN TIME TO PEAK & INCREASE DUE TO DEVELOPMENT IS

A PORTION OF DEVELOPED RUNOFF WILL BE ROUTED TO AND/OR THROUGH

NEGLIGIBLE. (INCREASE FROM THE EXISTING. (.10±CFS)

Scale: 1" =10' COCHITI STREET SE - EXISTING 36' W CURB-CUT Q100=0.7 CFS TO REMAIN PROJECT BENCHMARK

TOP OF #4 REBAR/CAP, MSL ELEV. 5524.20 - 100 SF LANDSCAPE AREA 693 SF LANDSCAPE AREA 7000 SF 0.1607 ACRES LOT 14, BLK. 9 1.0% THE RESERVE OF THE PARTY OF THE FIN. FLR. ELEV. SCREEN IN FRONT OF EVAPORATIVE COOLER *5525.00* ROOF DRNG.

LOT 15, BLOCK 9, EAST CENTRAL BUSINESS ADDITION ALBUQUERQUE, NEW MEXICO I, PHILIP W. CLARK, A PROFESSIONAL ENGINEER LICENSED IN ACCORDANCE WITH THE LAWS OF THE STATE OF NEW MEXICO, DO HEREBY CERTIFY THAT I HAVE VISITED THE SITE SHOWN HEREON, AND THAT THE CONTOURS

15' REAR SET/BACK

SHOWN REPRESENT THE EXISTING GROUND CONDITIONS, AND DO FURTHER CERTIFY THAT NO EARTHWORK OF ANY

KIND, NOR ANY DISTURBANCE OF THE EXISTING GROUND HAS OCCURRED ON THIS SITE SINCE THE CONTOURS WERE

PROJECT BENCHMARK TOP OF NUMBER 4 REBAR/CAP AT THE PROJECT MARKED NORTHWEST PROPERTY CORNER, MSL ELEVATION = 5524.20 (TIE FROM ACS 1-L21, LOCATED IN THE MEDIAN OF CENTRAL AVE. 81' WEST OF THE JUAN TABO INTERSECTION)

TOPOGRAPHIC DESIGN SURVEY

PROJECT DATA

LEGAL DESCRIPTION (Existing Platting)

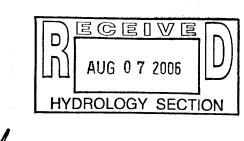
COMPILED BY CLARK CONSULTING ENGINEERS FROM DESIGN SURVEY PERFORMED BY TERRAMETRICS OF NEW MEXICO, 8/2006

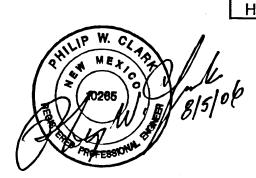
VICINITY MAP ZONE L-21 Scale: 1" = 750'

- 1. ANY WORK WITHIN THE RIGHT-OF-WAY SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECS. FOR PUBLIC WORKS CONSTRUCTION, 7TH EDITION W/ AMEND. 1
- 2. AN EXCAVATION/CONSTRUCTION PERMIT IS REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY R.O.W. AN APPROVED COPY OF THIS PLAN MUST BE SUBMITTED AT THE TIME OF APPLICATION.
- 3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES, AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
- 4. CONTRACTOR SHALL ENSURE THAT NO SITE SOILS/SEDIMENT OR SILT ENTER THE RIGHT-OF-WAYS DURING CONSTRUCTION.
- 5. MAXIMUM SITE GRADING WITHOUT EROSION PROTECTION: 3 HORIZONTAL TO 1 VERTICAL, 3:1. ALL DIMENSIONS TO FACE OF CURB, UNLESS NOTED OTHERWISE.

EXIST. SPOT ELEVATION _--10-__ EXIST. CONTOUR NEW SPOT ELEVATION **�** 24.0 NEW CONTOUR NEW SWALE DRAINAGE DIRECTION, EXISTING NEW P.C.C., CONCRETE TOP OF CURB, EXISTING FLOWLINE

FACE OF CURB/FACE OF CURB





	6	C	<i>ark</i> Edgev	Cons	Sulting Road Mexico d	E ng	<u>inee</u>	ers_
***	Tele: (505) 281-2444 Fax: (505) 281-244					281-2444		
	DATE REVISION			LOT15, BLO	CK 9, EAST			
	LEVAN WAREHOUSE 11508 COCHITI ROAD, SE							
	Grading & Drainage							
					Plai	7		
100			BY: CCE	JOB #: Lev	an_GD			
449			DATE:	7/31/06	FILE #: G/I		1 (DF 1