CITY OF ALBUQUERQUE

PLANNING DEPARTMENT - Development Review Services



Richard J. Berry, Mayor

April 11, 2016

Mr. Philip W. Clark, P.E. Clark Consulting Engineers 19 Ryan Rd. Edgewood, NM 87015

RE:

PJ's Motorcycle Additions

12910 Central Ave SE Grading & Drainage Plan

Engineer's Stamp Date 3-16-16 (Revised 4-11-16) (File: L22D059)

Dear Mr. Clark:

Based upon the information provided in your submittal received 4-11-2016, the above referenced Grading and Drainage Plan is approved for Building Permit. Please attach a copy of this approved plan in the construction sets when submitting for a building permit.

PO Box 1293

Prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

Albuquerque

If you have any questions you can contact me at 924-3986.

New Mexico 87103

www.cabq.gov

Abiel Carrillo, P.E.

Sincerely,

Principal Engineer, Planning Dept. Development Review Services

Orig: Drainage file



City of Albuquerque

Planning Department

Development & Building Services Division

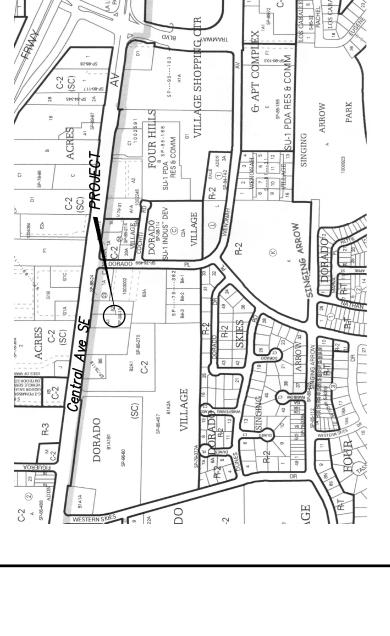
DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

Project Title: PJ's Motorcycle Additions	Building Permit #: City Drainage #: L22 / D
DRB#: EPC#:	Work Order#:
Legal Description: Tracts A-1 AND A-2 LANDS OF RICHARD RUST	
City Address: 12910 CENTRAL AVE SE	
Engineering Firm: Clark Consulting Engineers	Contact: Philip Clark
Address: 19 Ryan Road, Edgewood, NM 87015	
Phone#: 281-2444 Fax#: CELL#264-6	042 E-mail: CCEalbq@aol.com
Owner:	Contact:
Address:	
Phone#: Fax#:	E-mail:
A 11. A DIGITARD OFFICEN	Contact:
Address:	
Phone#: Fax#:	E-mail:
Other Contact: GEN CONTRACTOR, LATITUDE	Contact: ROB
Address:	
Phone#: Fax#:	E-mail:
DEPARTMENT: X HYDROLOGY/ DRAINAGE	CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT:
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B \geq GRADI

6000 SF BUILDING ADD'NS, PRIVATE ASPHALT DRIVEWAY AND PARKII LANDSCAPING IMPROVEMENTS. PROPOSED ELEVATIONS. SPOT ELEVATIONS AND EXISTING DRAINAGE PATTERNS AND EXISTING ADJACENT THE SITE IS NOT WITHIN OR ADJACENT BE PERMITTED TO DRAIN TO THE NEW H DEVELOPMENT. FREE DISCHARGE OF PRINDRAINAGE FACILITIES EXIST. A PORTION RAINFALL. ROOF DRAINAGE OF THE EXIST OULVERT. NO OFF—SITE FLOWS ENTER T PRESENTLY, THE SITE IS DEVELOPED , ON THE NORTH IS A 4—LANE PAVED, SIDEWALKS. THE SITE TERRAIN SLOPE 1. EXISTING CONTOURS, AND SI STRUCTURES
2. PROPOSED IMPROVEMENTS:
UPGRADES, NEW GRADE ELEVATIO
3. CONTINUITY BETWEEN EXISTI
4. QUANTIFICATION OF DEVELOR
FLOWS, including NEW PONDING R THE PROPOSED COMMERC ALBUQUERQUE. THE GRA FLOOD HAZARD ORDINAN ORDER TO FACILITATE TH

SITE IS NOT IMPACTED BY SPECIAL FLOOD HAZARD AREA



1,, = 20,

Scale:

ZONE MAP VCINITY

22

ALL WORK WITHIN THE RIGHT-OF-WAY SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECS. FOR PUBLIC WORKS CONSTRUCTION, 1986, W/9 UPDATES.

AN EXCAVATION/CONSTRUCTION PERMIT IS REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY R.O.W. AN APPROVED COPY OF THIS PLAN MUST BE SUBMITTED AT THE TIME OF APPLICATION.

560x63 Exist. St'd Curb/Gutter

NEM P. 2DMK

SE

AVE.

OENTRAL

12910

5598,45

9

359

**

PANEL

FIRM MAP

ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES, AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
 ALL LANDSCAPING AREA SHALL BE SOFT—LINED WITH NATIVE VEGETATION AND/OR GRAVEL.

ENTIRE ROOF VIA CHANNEL

MAXIMUM SITE GRADING WITHOUT EROSION PROTECTION: 3 HORIZONTAL TO 1 VERTICAL, 3:1. DIMENSIONS ARE TO FACE OF CURB UNLESS NOTED OTHERWISE. CONTRACTOR SHALL ENSURE THAT NO SITE SOILS/SEDIMENT ENTER THE RIGHT-OF-WAYS DURING CONSTRUCTION. REVECETATE ALL AREAS DISTURBED DUE TO CONSTRUCTION PER CITY OF ALBUQ. SPEC. 1012, NATIVE SEED MIX. 6.

8. NEW ASPHALT PAVEMENT SHALL CONSIST OF 2" ASPHALT CONCRETE OVER 6" CRUSHED/BLENDED ASPHALT BASE ON 8" COMPACTED SUBGRADE, 95% PROCTOR, ASTM D-1557

EXIST. SPOT ELEVATION
EXIST. CONTOUR
NEW SPOT ELEVATION
NEW CONTOUR
NEW SWALE **4** 24.0 **54** - 10 -424.0

FLOWLINE EXISTING POWER POLE FL O*PP*

 $\Gamma O \downarrow 1 - A$, CANADA VILLAGE

Exist. Be Rei

COV'RD AREA

NEW BUILDIN ADDITION

FIN. FLR. ELEV. 5600.3

NEW BUILDING ADDITION

FIN. FLR. ELEV. 5598.05

NATURAL GROUND, EXISTING TOP OF BERM

Щ

+0.<u>2° TA</u> A<u>LON</u>G E<u>DGE</u>

EXISTING REBAR AND CAP, E CHAIN LINK FENCE,

EXIST ASPHALT

M.,79,12.00S

LEGAL DESCRIPTION (EXISTING)

NORTHWEST PROPERTY CORNER, TOP REBAR/CAP SEE PLAN, ELEVATION = 5598.36, AS TIED FROM ACS CONTROL MONUMENT 4-L22 (ELEV. 5586.42) NAVD88

<u>TOPOGRAPHIC DESIGN SURVEY</u> PERFORMED AUGUST 2006 BY TYREE SURVEYING, AND SUPPLEMENTED FEBRUARY 2016.

LOT A-2 0.67 ACRES

LOT

,90[.]267 M<u>"97,75.</u>00S

2:1

POND PROV.5575 CF 92.5 IDESLOPE UNO WL RIPRAP IA., 8" DEPTH)

<u>E</u> 0.66[0.19] 0.92[0.36] 1.29[0.62] 2.36[1.50]

TREATM'T Q_{Peak} A 1.87[0.58]
B 2.60[1.19]
C 3.45[2.00]
D 5.02[3.39]

0.06 Ac.(0%) 0.06 Ac.(5%) 0.44 Ac.(37%) 0.7 Ac.(58%)

LANDSCAPING/POND GRAVEL & COMPACTED SOIL ROOF – PAVEMENT

UNDEVELOPED

9193 CF

VOLUME 100 =

1.2 Ac. 1.89 In.[1.16]

 $\it E$ Weighted

THEREFORE:

Q100

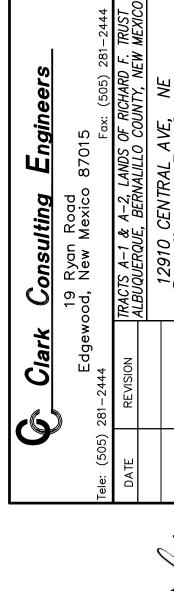
POND CALCS.
2900 SF, AREA OF TOP TO ELEV. 95
1560 SF, AREA OF BOTT, @ 92.5
THEN: 2900+1560/2 X 2.5 = 5575 CF PROVIDED
FIRST FLUSH, PER DPM, WATER QUALITY TABLE 2:
0.34"/12 X 0.7 AC. X 43560 = 864 CF, CONTAINED AT BOTTOM OF POND

OF NEW ASI

EDGE

EXIST. GRAVEL

LOT 0.67



Grading & Drainage
Plan

<u>Detonimila 98 ot</u>

TRACT B3A, UNIT DORADO VILLAGE 0.96

#10265

NMP

PHILIP W. CLARK

I, PHILIP W. CLARK, A PROFESSIONAL ENGINEER LICENSED IN ACCORDANCE WITH THE LAWS OF THE STATE OF NEW MEXICO, DO HEREBY CERTIFY THAT I HAVE VISITED THE SITE SHOWN HEREON, AND THAT THE CONTOURS SHOWN REPRESENT THE EXISTING GROUND CONDITIONS, AND DO FURTHER CERTIFY THAT NO SIGNIFICANT EARTHWORK NOR MAJOR DISTURBANCE OF THE EXISTING GROUND HAS OCCURRED ON THIS SITE SINCE THE CONTOURS WERE DETERMINED.

EXIST. ASPHALT

HYDROLOGIC METHODS PER SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL (DPM)
REVISED JANUARY 1993 FOR CITY OF ALBUQUERQUE, ADOPTED BY THE COUNTY OF BERNALILLO
DISCHARGE RATE: Q=QPEAK × AREA."Peak Discharge Rates For Small Watersheds"
VOLUMETRIC DISCHARGE: VOLUME = EWeighted × AREA
P100 = 2.96 Inches, Zone 3 Time of Concentration, TC = 10 Minutes
DESIGN STORM: 100-YEAR/6-HOUR, 10-YEAR/6-HOUR [] = 10 YEAR VALUES

A TIONS

CALCUL

DESIGN CRITERIA

LOT AREA = 1.2 ACRES, WHERE EXCESS PRECIP. 'W' = 1.8 In. [1.05]
PEAK DISCHARGE, Q100 = 5.0 CFS [3], WHERE UNIT PEAK DISCHARGE = 4.2 CFS/AC. [2.0]
THEREFORE: VOLUME 100 = 7841 CF [xx]

TREATMENTS, PEAK DISCHARGE AND VOLUMETRIC DISCHARGE

DEVELOPED CONDITIONS

DETERMINE LAND FOR STUDY AREA

LAND

1.3%

UNPLATTED TRACT

'8.8 @ .9;

5

12910 Centrally

NEW O.S. HEADER CURB

50,

EXIST. ASPHALT

EXISTING BUILDING

EXISTING CONC. SDWK. (FLUSH)

's MOTORCYCLES FIN. FLR. 5600.54

EXIST 10,0 WALL-FENCE

EXIST. ASPHALT

EGEND

DRAINAGE DIRECTION, EXISTING

NG OR G

NEW P.C.C., CONCRETE

TRACTS A—1 & A—2, LANDS OF RICHARD F. TRUST ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO PROJECT BENCHMARK

9

PJ's Motorcycles - 12910 Central Avenue

Reference: Development Process Manual, Section 22.2, Hydrology

Applicable Bernalillo County Precipitation Zone: 3

FOR REVIEW PURPOSES ONLY, NOT FOR DESIGN

Precipitation Depths_{100-Year} [inches]:

P ₆₀	P ₃₆₀	P ₁₄₄₀	P _{4days}	P _{10days}	
2.14	2.6	3.1	3.95	4.9	

Peak Discharge_{100Vear/6Hour} [cfs/acre]:

S 1001 cui/olloui 2									
A	В	С	D						
1.87	2.6	3.45	5.02						

for $t_c = .2$ hour Zone 3 Peak Intensity [in/hr]:

I = 5.38

Existing Conditions: Estimate from Google Earth Measurements

Land Treatment	Area [ft²]	Area [acre]	% of Total Area	Excess Prec. ₁₀₀₋₆ [in]	Weighted E [in]	Volume ₃₆₀ [acre-ft]	Volume ₁₄₄₀ [acre-ft]	Volume _{10day} [acre-ft]	Q _{100Year/6Hour} [cfs]	Rational Method, C	Rational Method, Q [cfs]
A		0.0000	0.00%	0.66		0.0000			0.0	0.39	0.0
В		0.0900	7.38%	0.92		0.0130			0.2	0.52	0.3
С		0.6000	49.18%	1.29		0.0864			2.1	0.66	2.1
D		0.5300	43.44%	2.36		0.0763			2.7	0.94	2.7
Totals	0.0000	1.2200	100.00%		1.7275	0.1756	0.1977	0.2772	5.0		5.1

Area D reduced because ex bldg drains north (0.12 acres)

check: 0.1756

 $Volume_{360} [CF] = 7651$

 $8613 = Volume_{1440} [CF]$

Proposed Conditions: From Proposed Plan Asumptions

Land Treatment	Area [ft²]	Area [acre]	% of Total Area	Excess Prec. ₁₀₀₋₆ [in]	Weighted E [in]	Volume ₃₆₀ [acre-ft]	Volume ₁₄₄₀ [acre-ft]	Volume _{10day} [acre-ft]	Q _{100Year/6Hour} [cfs]	Rational Method, C	Rational Method, Q [cfs]
A		0.0000	0.00%	0.66		0.0000		•	0.0	0.39	0.0
В		0.0600	4.92%	0.92		0.0094		-	0.2	0.52	0.2
С		0.4600	37.70%	1.29		0.0723			1.6	0.66	1.6
D		0.7000	57.38%	2.36		0.1100			3.5	0.94	3.5
Totals	0.0000	1.2200	100.00%		1.8857	0.1917	0.2209	0.3259	5.3		5.3

Area D reduced because ex bldg drains north (0.12 acres)

check: 0.1917

100-Yr, 6hr Storm Volume₃₆₀ [CF] =

8351

 $9622 = Volume_{1440} [CF]$

In a triangular hydrograph, the peak discharge occurs approximately when half of the volume is discharged. Since the proposed pond captures volume beyond half of the total 24-hour volume, then the pond will attenuate the peak discharge leaving the site. This will actually lower the historical peak discharge and as shown above will significantly lower the total 24-hr volume discharged. The 2-year and 10-year storms are expected to be completely retained.

First Flush is contained in the pond, .34"x0.7 ac=

864

CF