

# City of Albuquerque

# Planning Department

# Development & Building Services Division

## DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

Project Title:	Building Permit #: City Drainage #:
DRB#: EPC#:	
Legal Description:	
City Address:	
Engineering Firm:	Contact:
Address:	
Phone#: Fax#:	E-mail:
Owner:	Contact:
Address:	
Phone#: Fax#:	
Architect:	Contact:
Address:	
Phone#: Fax#:	E-mail:
Other Contact:	Contact:
Address:	
Phone#: Fax#:	E-mail:
DEPARTMENT:  HYDROLOGY/ DRAINAGE TRAFFIC/ TRANSPORTATION MS4/ EROSION & SEDIMENT CONTROL	CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT:  BUILDING PERMIT APPROVAL  CERTIFICATE OF OCCUPANCY
TYPE OF SUBMITTAL:  ENGINEER/ ARCHITECT CERTIFICATION	PRELIMINARY PLAT APPROVAL
ENGINEER/ ARCHITECT CERTIFICATION	SITE PLAN FOR SUB'D APPROVAL
CONCEPTUAL G & D PLAN	SITE PLAN FOR BLDG. PERMIT APPROVAL
GRADING PLAN	FINAL PLAT APPROVAL SIA/ RELEASE OF FINANCIAL GUARANTEE
DRAINAGE MASTER PLAN	FOUNDATION PERMIT APPROVAL
DRAINAGE REPORT	GRADING PERMIT APPROVAL
CLOMR/LOMR	SO-19 APPROVAL
	PAVING PERMIT APPROVAL
TRAFFIC CIRCULATION LAYOUT (TCL)	GRADING/ PAD CERTIFICATION
TRAFFIC IMPACT STUDY (TIS)	WORK ORDER APPROVAL
EROSION & SEDIMENT CONTROL PLAN (ESC)	CLOMR/LOMR
OTHER (SPECIFY)	PRE-DESIGN MEETING
	OTHER (SPECIFY)
IS THIS A RESUBMITTAL?: Yes No	
DATE SUBMITTED:By:	
•	

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: \_\_\_\_

## DRAINAGE REPORT

For

# TUYET SON BUDDHIST CENTER LOT 2 BLOCK 5 HENDREN SUBDIVISION 12125 CENTRAL NE Albuquerque, New Mexico

Prepared by

Rio Grande Engineering PO Box 93924 Albuquerque, New Mexico 87199

AUGUST 2016



David Soule P.E. No. 14522

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Site Grading and Drainage Plan

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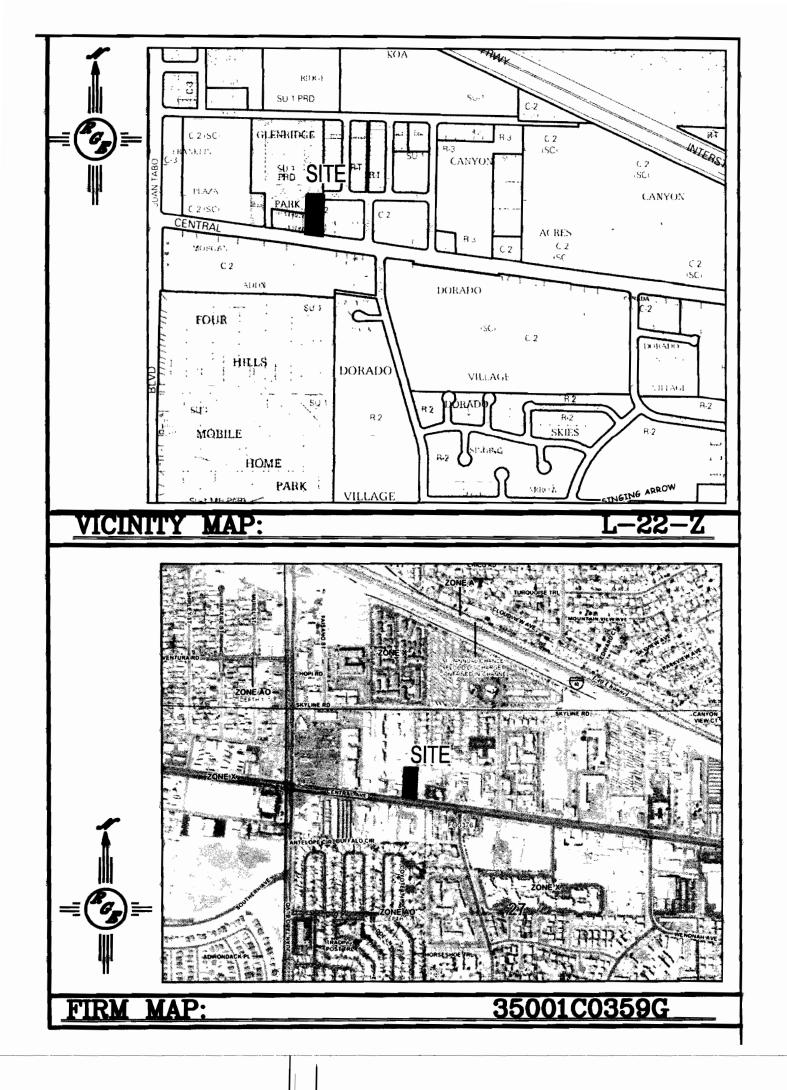
#### **PURPOSE**

The purpose of this report is to provide the Drainage Management Plan for the redevelopment of a 0.79 acre parcel of land located on Northeast Central. This plan will be utilized for the redevelopment of the subject property from a sales lot into a Buddhist retreat center with one building and several future buildings. This plan was prepared in accordance with the City of Albuquerque's Development Process Manual. This report will demonstrate that the proposed improvements do not adversely affect the surrounding properties, nor the upstream or downstream facilities.

#### INTRODUCTION

The subject of this report, as shown on the Exhibit A, is a0.79-acre parcel of land located on the north side of Central Avenue NE east between Burma and Glenridge road NE. The site is located in a fully developed are of the north east heights of Albuquerque. The legal description of this site is lot 2 block 5 Hendren subdivision. As shown on FIRM map 35001C0359, the site is located entirely within Flood Zone X. The site is currently developed as a completely compacted gravel and paved sales lot.

The site is located within the boundaries of the East Gateway sector plan. This area is completely developed. The proposed drainage solution must not increase existing flow rate and account for the 90<sup>th</sup> percentile storm. The site must retain the first .42" of rainfall, resulting in .34" of run off. The site shall maintain existing drainage patterns



#### **EXISTING CONDITIONS**

The site is currently developed as a completely semi impervious sales lot. It appears to have been a lot for either car of mobile homes. The site has been completely paved, with asphalt base course with no landscaping. The site currently discharges 2.98 cfs to the adjacent property. The flows pass over private property and enter the public roadway they are conveyed to the Glen ridge roadway. The existing upland flows are diverted around the site to either central or Linn roadway due to solid fence on east property line. The site allows the upland flow to pass thru the site via an asphalt rundown at the rear of the site that appears to be previous half street

#### PROPOSED CONDITIONS

The proposed improvements consist of a single new building with a dumpster and storage building with future building sites. The onsite storm water will be conveyed within the parking areas and within water quality ponds located in all landscape area. The site will drain from east to west and be conveyed via landscape swales and water harvest pond to the north west corner. The site will generate 3.22 cfs, which exceeds the existing condition. To reduce the flow to existing, a 9" tall and 1.5' wide weir and berm will be constructed to throttle the flow to less than historical. The water storage area will not effect any structures.

As shown in Appendix A, the site was modeled using AHYMO. The stage storage table for the onsite detention is also enclosed. The site will discharge 2.58 cfs during the 100-year 6- hour event. The site will harvest 1526 cf of storm water, which exceeds the 489 cf required. In the event of clogging, the pond will overtop the berm and discharge at the historic locations.

### **SUMMARY AND RECOMMENDATIONS**

This site is a redevelopment of a completely developed parcel of land located in a fully developed watershed located within the north east heights. The developed condition will produce

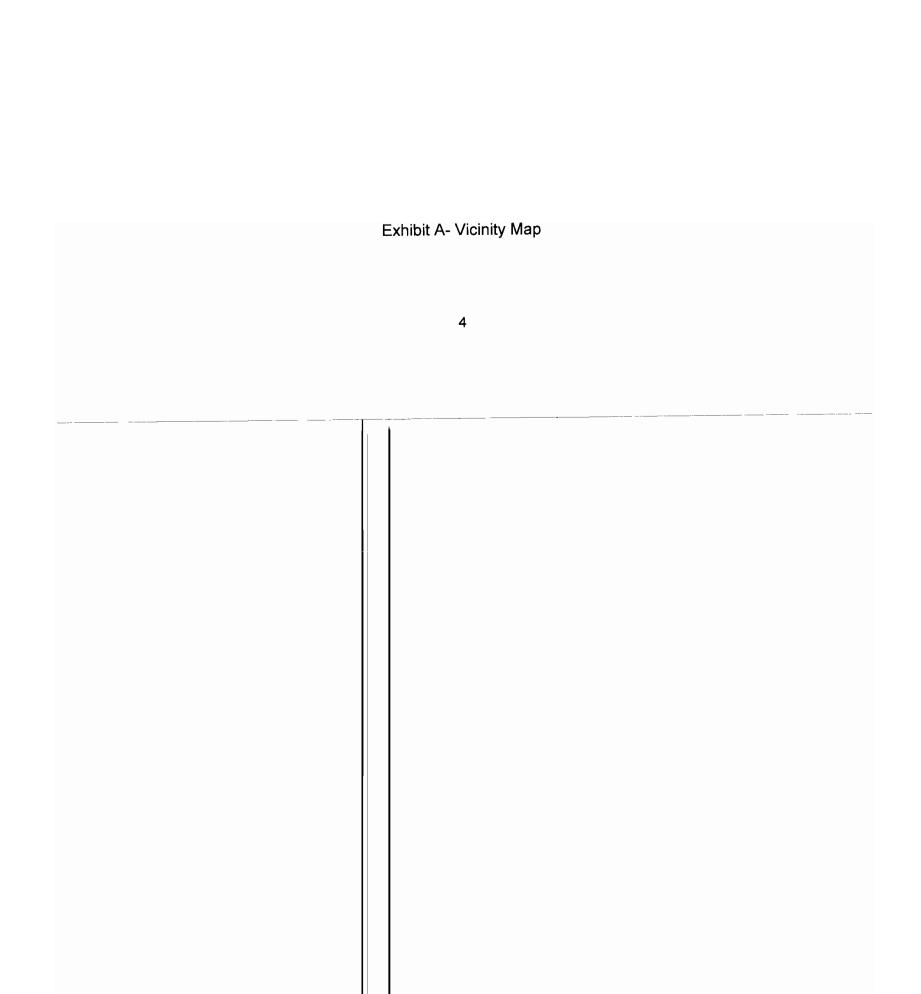
a peak discharge rate less than existing. The site has been designed to retain the 90<sup>th</sup> percentile storm event. The site will maintain the existing drainage patterns

The proposed site development does not adversely affect the upstream or downstream facilities. The site was designed in conformance to City of Albuquerque Drainage Policy.

Therefore, we request approval of the site-grading plan. Since this site encompasses less than 1 acre, an Erosion Control Plan, NPDES permit and SWPPP may not be required prior to any construction activity.

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SITE HYDROLOGY
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# Weighted E Method TUYET CENTER

Existing Developed Basins

							100-Year, 6-l	nr.					
Basin	Агеа	Агеа	Treatment A		Treatme	nt B	Treatmer	nt C	Treatme	nt D	Weighted E	Volume	Flow
	(sf)	(acres)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	(ac-ft)	(ac-ft)	cfs
EXISTING	34514.00	0.792	0%	0	0.0%	0.000	80.0%	0.63387	20%	0.158	1.504	0.099	2.98
PROPOSED	34514	0.792	0%	0	20.0%	0.158	30.0%	0.2377	50%	0.396	1.751	0.116	3.22

#### Equations:

Weighted E = Ea\*Aa + Eb\*Ab + Ec\*Ac + Ed\*Ad / (Total Area)

Volume = Weighted D \* Total Area

Flow = Qa \* Aa + Qb \* Ab + Qc \* Ac + Qd \* Ad

Where for 100-year, 6-hour storm (zone 4)

Qa= 1.87 Qb= 2.6 Qc= 3.45 Eb= 0.92 Ec= 1.29 Ed = 2.36Qd = 5.02

WATER QUALITY REQUIRED 489 cf 1526 cf WATER QUALITY PROVIDED EXISTING

THIS SITE EXCEEDS THE EXISTING DISCHARGE RATE THERERFOR THE DISCHARGE SHALL BE CONTROLLED BY A WIER THE PROPOSED POND OUTFALL WAS MODELLED UTILIZING AHYMO, THE DISHARGE LEAVING THE SITE WILL BE 2.56 CFS THE SITE EXCEEDS THE WATER QUALITY REQUIR

# STAGE STORAGE VOLUME CALCULATIONS

OUTFALL POND

# POND OUTLET

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVI	VOLUME AC-FT	Q (CFS)
,			i			
51.50	0.00 5					,
51.75	0.00	202.0000		1526	0.035	0.00
52.00	0.25	460.0000	82.7500	1608.75	0.037	0.55
52.25	0.50	2156.0000	327.0000	1935.75	0.044	1.56
52.50	0.75	4102.0000	782.2500	2718	0.062	2.87
(				l .		l

wier equation Q=2.95XWXH^1.5 W=1.5'

#### pondrout090516.txt

\*S \*S AHYMO - BHUDDIST POND ROUTING

TIME=0.0 PUNCH CODE=0

RAINFALL

START

TYPE=2

QUARTER=0.0 ONE= 2.20 IN SIX= 2.66 IN DAY= 3.12 IN DT = 0.05 HR

COMPUTE NM HYD

ID=1 HYD NO=101 DA= .0012375 SQ MI PER A=0 PER B=20 PER C=30 PER D=50 TP=-.133 MASSRAIN=-1

PRINT HYD

ID=1 CODE=3

\* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
ROUTE RESERVOIR

ID=2 HYD NO=102 INFLOW=1 CODE=3
OUTFLOW(CFS) STORAGE(AC-FT) ELEV
0.00 0.035 51.75
0.55 0.037 52.00 ELEV(FT) 51.75 52.00 52.25 52.50 1.56 2.87 0.044 0.062

PRINT HYD

ID=2 CODE=3

FINISH

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#### AHYMO.OUT

```
AHYMO PROGRAM (AHYMO-S4)

RUN DATE (MON/DAY/YR) = 09/06/2016
                                                         - Version: S4.01a - Rel: 01a
              START TIME (HR:MIN:SEC) = 17:02:16
                                                         USER NO.=
RioGrandeSingleA41963517
              INPUT FILE = ents and Settings\Owner\Desktop\2016 jobs\16106-central
budda\pondrout090516.txt
```

\*S \*S AHYMO - BHUDDIST POND ROUTING

**START** 

TIME=0.0 PUNCH CODE=0

**RAINFALL** 

TYPE=2

QUARTER=0.0 ONE= 2.20 INSIX= 2.66 INDAY= 3.12 IN DT = 0.05 HR

24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE AREAS (NM & AZ) - D1 = 0.050000 HOURS END TIME = 0.0000 0.0031 0.0062 0.0096 0.0133 DT = 24.000002 HOURS 0.0171 0.0214 0.0575 0.0690 0.0807 0.0274 0.0368 0.0470 0.0927 0.1052 0.1178 0.1467 0.1319 0.1893 0.1627 0.2211 0.2637 0.5408 2.0808 2.3781 2.4603 0.6955 2.1531 0.3120 0.3717 0.4513 0.9358 1.8656 2.3311 2.4404 1.6372 1.3477 1.9803 2.2106 2.2607 2.4190 2.2974 2.4300 2.3588 2.4505 2.4903 2.3931 2.4683 2.4862 2.4983 2.5022 2.4775 2.4820 2.4943 2.4862 2.5132 2.5363 2.5565 2.5750 2.5920 2.5096 2.5331 2.5537 2.5059 2.5167 2.5393 2.5593 2.5202 2.5268 2.5481 2.5235 2.5423 2.5452 2.5620 2.5800 2.5967 2.5509 2.5646 2.5673 2.5699 2.5873 2.5725 2.5897 2.5775 2.5944 2.5825 2.5990 2.5849 2.6012 2.6078 2.6226 2.6364 2.6100 2.6034 2.6057 2.6122 2.6143 2.6164 2.6205 2.6345 2.6476 2.6600 2.6721 2.6840 2.6246 2.6383 2.6266 2.6402 2.6185 2.6286 2.6421 2.6306 2.6326 2.6494 2.6617 2.6738 2.6512 2.6548 2.6565 2.6458 2.6530 2.6652 2.6772 2.6891 2.6635 2.6755 2.6874 2.6583 2.6703 2.6669 2.6789 2.6686 2.6806 2.6857 2.6975 2.7092 2.6823 2.6908 2.6925 2.6840 2.6958 2.7075 2.7191 2.7306 2.7419 2.7531 2.7642 2.7752 2.7860 2.7967 2.8073 2.8178 2.8282 2.7009 2.7125 2.7240 2.6992 2.7109 2.7025 2.7142 2.7042 2.7158 2.6941 2.7059 2.7208 2.7322 2.7435 2.7175 2.7224 2.7257 2.7273 2.7354 2.7467 2.7579 2.7371 2.7483 2.7595 2.7338 2.7451 2.7387 2.7499 2.7289 2.7403 2.7515 2.7547 2.7563 2.7611 2.7689 2.7798 2.7906 2.8013 2.8118 2.7626 2.7736 2.7658 2.7767 2.7674 2.7783 2.7705 2.7814 2.7721 2.7829 2.7891 2.7998 2.8103 2.7845 2.7876 2.7922 2.8043 2.8148 2.8028 2.8133 2.7952 2.7983 2.8088 2.8193 2.8058 2.8163 2.8208 2.8223 2.8237 2.8252 2.8311 2.8413 2.8355 2.8456 2.8282 2.8384 2.8267 2.8296 2.8326 2.8340 2.8369 2.8398 2.8427 2.8442 2.8499 2.8599 2.8514 2.8613 2.8485 2.8471 2.8542 2.8528 2.8556 2.8570 2.8585 2.8627 2.8641 2.8655 2.8683 2.8697 2.8711 2.8725 2.8739

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2.8753

2.8669

AHYMO.OUT 2.8767 2.8780 2.8794 2.8808 2.8822 2.8836 2.8849 2.8890 2.8985 2.8904 2.8998 2.8863 2.8877 2.8917 2.8944 2.8931 2.8958 2.8971 2.9012 2.9025 2.9038 2.9091 2.9052 2.9065 2.9078 2.9105 2.9118 2.9131 2.9170 2.9261 2.9183 2.9274 2.9209 2.9300 2.9144 2.9157 2.9196 2.9222 2.9248 2.9235 2.9287 2.9351 2.9439 2.9526 2.9338 2.9363 2.9389 2.9325 2.9376 2.9401 2.9427 2.9514 2.9452 2.9539 2.9414 2.9464 2.9477 2.9501 2.9551 2.9563 2.9575 2.9600 2.9649 2.9588 2.9612 2.9624 2.9636 2.9661 2.9697 2.9780 2.9709 2.9673 2.9685 2.9721 2.9733 2.9745 2.9756 2.9768 2.9792 2.9804 2.9815 2.9851 2.9839 2.9862 2.9874 2.9885 2.9909 2.9897 2.9932 3.0011 2.9955 2.9920 2.9943 2.9966 2.9977 2.9989 3.0000 3.0045 3.0023 3.0034 3.0056 3.0068 3.0123 3.0200 3.0079 3.0090 3.0101 3.0112 3.0145 3.0134 3.0167 3.0178 3.0254 3.0156 3.0189 3.0211 3.0222 3.0243 3.0318 3.0232 3.0265 3.0275 3.0286 3.0297 3.0329 3.0402 3.0339 3.0412 3.0350 3.0423 3.0307 3.0360 3.0371 3.0392 3.0381 3.0433 3.0443 3.0464 3.0535 3.0454 3.0474 3.0494 3.0484 3.0504 3.0515 3.0555 3.0624 3.0565 3.0634 3.0525 3.0545 3.0575 3.0585 3.0614 3.0644 3.0595 3.0604 3.0654 3.0702 3.0673 3.0663 3.0683 3.0692 3.0712 3.0721 3.0778 3.0787 3.0731 3.0740 3.0750 3.0759 3.0769 3.0806 3.0871 3.0843 3.0815 3.0825 3.0834 3.0797 3.0852 3.0862 3.0889 3.0880 3.0898 3.0907 3.0916 3.0925 3.0934 3.0943 3.0952 3.0961 3.0970 3.0979 3.1005 3.1023 3.0988 3.0996 3.1014 3.1031 3.1040 3.1049 3.1057 3.1066 3.1075 3.1083 3.1092 3.1100 3.1109 3.1117 3.1125 3.1134 3.1142 3.1151 3.1159 3.1175 3.1184 3.1192 3.1200 3.1167

COMPUTE NM HYD

ID=1 HYD NO=101 DA= .0012375 SQ MI PER A=0 PER B=20 PER C=30 PER D=50 TP=-.133 MASSRAIN=-1

K = 0.072485HR TP = 0.133000HR K/TP RATIO = 0.545000 SHAPE CONSTANT, N = 7.106428

UNIT PEAK = 2.4484 CFS UNIT VOLUME = 0.9952 B = 526.28 P60 = 2.2000

AREA = 0.000619 SQ MI IA = 0.10000 INCHES INF = 0.04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = 0.050000

PRINT HYD

ID=1 CODE=3

PARTIAL HYDROGRAPH 101.00

TIME FLOW

TIME FLOW Page 2

TIME

FLOW

			AHYMO	OUT		
TIME	FLOW HRS	TIME CFS	FLOW HRS	CFS	HRS	CES
HRS	CFS 0.000	HRS 0.0	CFS			CFS
14.850	0.0	19.800	4.950 0.0	0.0	9.900	0.0
15.000	0.150 0.0	0.0 19.950	5.100 0.0	0.0	10.050	0.0
15.150	0.300 0.0	0.0 20.100	5.250 0.0	0.0	10.200	0.0
15.300	0.450	0.0 20.250	5.400	0.0	10.350	0.0
15.450	0.600	0.0	5.550	0.0	10.500	0.0
	0.750	0.0	5.700	0.0	10.650	0.0
15.600	0.0	20.550	0.0 5.850	0.0	10.800	0.0
15.750	0.0 $1.050$	0.2	0.0 6.000	0.0	10.950	0.0
15.900	0.0 1.200	20.850 0.3	0.0 6.150	0.0	11.100	0.0
16.050	$\begin{matrix} 0.0 \\ 1.350 \end{matrix}$	21.000 1.1	0.0 6.300	0.0	11.250	0.0
16.200	0.0 1.500	21.150 3.5	0.0 6.450	0.0	11.400	0.0
16.350	0.0 1.650	21.300 2.1	0.0 6.600	0.0		
16.500	0.0 1.800	21.450	0.0		11.550	0.0
16.650	0.0	1.0 21.600	6.750 0.0	0.0	11.700	0.0
16.800	1.950 0.0	0.5 21.750	6.900 0.0	0.0	11.850	0.0
16.950	2.100 0.0	0.3 21.900	7.050 0.0	0.0	12.000	0.0
17.100	2.250 0.0	0.2 22.050	7.200 0.0	0.0	12.150	0.0
17.250	2.400	0.1 22.200	7.350	0.0	12.300	0.0
17.400	2.550	0.1	7.500	0.0	12.450	0.0
	2.700	0.0	7.650	0.0	12.600	0.0
17.550	2.850	0.0	7.800	0.0	12.750	0.0
17.700	0.0 3.000	0.0	0.0 7.950	0.0	12.900	0.0
17.850	0.0 3.150	22.800 0.0	0.0 8.100	0.0	13.050	0.0
18.000	0.0 3.300	22.950 0.0	0.0 8.250	0.0	13.200	0.0
18.150	0.0 3.450	23.100 0.0	0.0 8.400	0.0	13.350	0.0
18.300	0.0 3.600	23.250	0.0 8.550	0.0	13.500	0.0
18.450	0.0	23.400	0.0			
18.600	3.750 0.0	0.0 23.550	8.700 0.0	0.0	13.650	0.0
18.750	3.900	0.0 23.700	8.850 0.0	0.0	13.800	0.0
18.900	4.050 0.0	0.0 23.850	9.000	0.0	13.950	0.0
19.050	4.200 0.0	0.0 24.000	9.150 0.0	0.0	14.100	0.0
19.200	4.350	0.0 24.150	9.300	0.0	14.250	0.0
			Page	e 3		

			AHYMO	.OUT		
19.350	4.500 0.0	0.0	9.450	0.0	14.400	0.0
19.500	4.650	0.0	9.600	0.0	14.550	0.0
19.650	4.800	0.0	9.750	0.0	14.700	0.0

RUNOFF VOLUME = 2.07286 INCHES = 0.1368 ACRE-FEET PEAK DISCHARGE RATE = 3.49 CFS AT 1.500 HOURS BASIN AREA = 0.0012 SQ. MI.

52.50

OUTFLOW TIME **INFLOW ELEV** VOLUME (FEET) (HRS) (CFS) (AC-FT) (CFS) 51.75 51.75 51.75 51.75 51.75 51.75 0.035 0.00 0.00 0.00 0.15 0.00 0.035 0.00 0.30 0.00 0.035 0.00 0.00 0.00 0.035 0.45 0.035 Ŏ.60 0.00 0.00 0.75 0.01 0.035 0.06 0.13 0.29 0.70 2.04 51.78 51.81 0.035 0.035 0.90 0.08 1.05 0.160.036 1.20 0.3551.88 1.10 3.49 2.15 52.04 52.34 1.35 0.038 0.051 52.44 0.058 2.55 1.65 52.31 52.09 1.80 1.95 1.87 0.93 0.048 0.97 0.53 0.040 51.94 0.036 0.41 2.10 0.30 0.22 0.14 0.08 2.25 0.19 0.12 51.85 0.036 0.036 0.035 51.81 51.79 51.77 2.55 0.062.70 0.05 0.03 0.04 0.035 0.02 0.035 51.76 51.76 51.76 51.76 51.75 51.75 51.75 51.75 0.02 0.02 3.00 3.15 3.30 0.01 0.01 0.035 0.035 0.01 0.035 3.45 0.01 0.01 3.60 3.75 0.01 0.01 0.035 0.01 0.035 0.035 3.90 0.01 0.014.05 0.010.035 0.014.20 0.01 0.035 0.01

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			ALIVAIO OUT	
4.35 4.65 4.895 5.240 5.25 5.35 5.35 6.345 6.345 6.35 7.35 7.35 7.35 7.35 8.25	0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01	51.75 51.75 51.75 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76	AHYMO.OUT 0.035	0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
8.40 8.55 8.70 8.85 9.00 9.15 9.30 9.45 9.60 9.75 10.20 10.35 10.50 10.65 11.40 11.55 11.70 11.85 12.15 12.30 12.15 12.30 12.30 12.30 12.30 12.30	0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01	51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76 51.76	0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035	0.01 0.01

12.20			AHYMO.OUT	
13.20 13.35 13.50 13.65 13.80 13.95 14.10 14.25 14.40 14.55 14.70 14.85 15.00 15.15 15.30 15.45 15.60 15.75 15.60 16.05 16.05 16.05 16.50 16.50 16.65	0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01	51.76 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75	0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035	0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
16.80 16.95 17.10 17.25 17.40 17.55 17.70 17.85 18.00 18.15 18.30 18.45 18.60 18.75 19.05 19.05 19.65 19.65 19.65 19.80 19.95 20.10 20.25 20.40 20.55 20.70 20.85 21.00 21.30 21.30 21.75 21.90	0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01	51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75 51.75	0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035 0.035	0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01

			AHYMO.OUT			
22.05	0.01	51.75	0.035	0.01		
22.20	0.01	51.75	0.035	0.01		
22.35	0.01	51.75	0.035	0.01		
22.50	0.01	51. <i>7</i> 5	0.035	0.01		
22.65	0.01	51.75	0.035	0.01		
22.80	0.01	51.75	0.035	0.01		
22.95	0.01	51.75	0.035	0.01		
23.10	0.01	51.75	0.035	0.01		
23.25	0.01	51.75	0.035	0.01		
23.40	0.01	51.75	0.035	0.01		
23.55	0.01	51.75	0.035	0.01		
23.70	0.01	51.75	0.035	0.01		
23.85	0.01	51.75	0.035	0.01		
24.00	0.01	51.75	0.035	0.01		
24.15	0.00	51.75	0.035	0.00		
PEAK DISCHAR		2.576 CF		OCCURS AT HOUR	1.60	
MAXIMUM WATER				2.444		
MAXIMUM STORA	AGE =	0.0580	AC-FT	INCREMENTAL T	IME=	0.050000HRS

PRINT HYD ID=2 CODE=3

DARTIAL	10/000000	102 00
PARITAL	HYDROGRAPH	102.00

	TIME	FLOW		TIME	FLOW	TIME	FLOW		
TIME	FLOW HRS	CFS	TIME	FLOW HRS	CFS	unc	CEC		
HRS	CFS		HRS	CFS	CFS	HRS	CFS		
	0.000	0.0		4.950	0.0	9.900	0.0		
14.850	0.0		19.800	0.0					
15.000	$\substack{0.150\\0.0}$	0.0	19.950	5.100 0.0	0.0	10.050	0.0		
13.000	0.300	0.0	19.930	5.250	0.0	10.200	0.0		
15.150	0.0		20.100	0.0	0.0	10.200	0.0		
	0.450	0.0		5.400	0.0	10.350	0.0		
15.300	0.0 0.600	0.0	20.250	0.0 5.550	0.0	10 500	0.0		
15.450	0.00		20.400	0.0	0.0	10.500	0.0		
	0.750	0.0		5.700	0.0	10.650	0.0		
15.600	0.0		20.550	0.0					
15.750	0.900 0.0	0.1	20.700	5.850 0.0	0.0	10.800	0.0		
13.730	1.050	0.1	20.700	6.000	0.0	10.950	0.0		
15.900	0.0		20.850	0.0		10.550			
15.050	1.200	0.3	24 222	6.150	0.0	11.100	0.0		
16.050	0.0 1.350	0.7	21.000	0.0 6.300	0.0	11.250	0.0		
16.200	0.0		21.150	0.300	0.0	11.230	0.0		
	1.500	2.0		6.450	0.0	11.400	0.0		
16.350	0.0		21.300	0.0					
16.500	1.650 0.0	2.5	21.450	6.600 0.0	0.0	11.550	0.0		
10.300	1.800	1.9	21.430	6.750	0.0	11.700	0.0		
16.650	0.0		21.600	0.0		221700	0.0		
15 000	1.950	0.9	24 750	6.900	0.0	11.850	0.0		
16.800	0.0 2.100	0.4	21.750	0.0 7.050	0.0	12.000	0.0		
16.950	0.0		21.900	0.0	0.0	12.000	0.0		
	2.250	0.2		7.200	0.0	12.150	0.0		
17.100	0.0		22.050	0.0	0.0	12 200	0.0		
17.250	2.400 0.0	0.1	22.200	7.350 0.0	0.0	12.300	0.0		
17.230	2.550	0.1		7.500	0.0	12.450	0.0		
17.400	0.0		22.350	0.0					
	Page 7								

aye 7

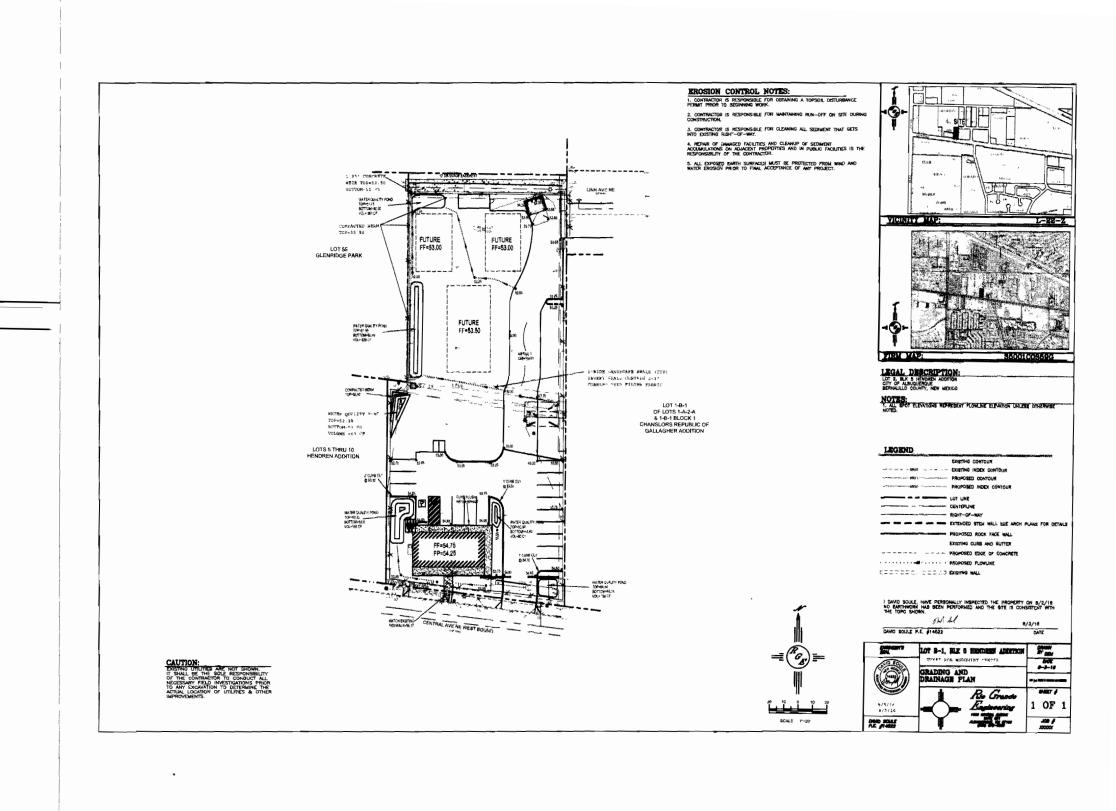
	AHYMO.OUT									
17.550	2.700 0.0	0.0 22.500	7.650	0.0	12.600	0.0				
17.700	2.850	0.0	7.800 0.0	0.0	12.750	0.0				
	3.000	0.0	7.950	0.0	12.900	0.0				
17.850	0.0 3.150	0.0	$0.0 \\ 8.100$	0.0	13.050	0.0				
18.000	0.0 3.300	0.0	0.0 8.250	0.0	13.200	0.0				
18.150	0.0 3.450	0.0	0.0 8.400	0.0	13.350	0.0				
18.300	0.0 3.600	23.250 0.0	0.0 8.550	0.0	13.500	0.0				
18.450	0.0 3.750	23.400 0.0	0.0 8.700	0.0	13.650	0.0				
18.600	0.0 3.900	23.550 0.0	0.0 8.850	0.0	13.800	0.0				
18.750	0.0 4.050	23.700 0.0	0.0 $9.000$	0.0	13.950	0.0				
18.900	0.0 4.200	23.850 0.0	0.0 9.150	0.0	14.100	0.0				
19.050	0.0 4.350	24.000 0.0	0.0 9.300	0.0	14.250	0.0				
19.200	0.0 4.500	24.150 0.0	0.0 9.450	0.0	14.400	0.0				
19.350	0.0 4.650	0.0	9.600	0.0	14.550	0.0				
19.500	0.0 4.800	0.0	9.750	0.0	14.700	0.0				
19.650	0.0	0.0	3.730	0.0	17.700	0.0				

RUNOFF VOLUME = 2.07264 INCHES = 0.1368 ACRE-FEET
PEAK DISCHARGE RATE = 2.58 CFS AT 1.600 HOURS BASIN AREA = 0.0012 SQ. MI.

FINISH

NORMAL PROGRAM FINISH END TIME (HR:MIN:SEC) = 17:02:16

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To: <u>"David Soule (david@riograndeengineering.com)"</u>

Cc: <u>Carrillo, Abiel X.</u>

**Subject:** Tuyet Son Buddhist Center

Date: Tuesday, September 20, 2016 2:33:00 PM

Mr. Soule:

This email is being sent in lieu of an attached comment letter in order to expedite our response to previous comments.

Response to comments should continue to be included in the resubmittal. A reply to these comments via email will not be considered a resubmittal.

Based upon the information provided in your resubmittal received 4/29/16, the above referenced Grading and Drainage Report and plan cannot be approved for Preliminary Plat, Grading Permit or Building Permit until the following comments are addressed:

- Provide a detail for the compacted Berm. Will it be protected from erosion?
- At the northern end of the property there is a box with a door, is this a future building or a pond?
- Are the ponds sized for the future buildings?
- Are the dark lines between the existing pavement and new pavement curbing?
- What is the double box at elevation point 53.00?
- Provide a water block at the entrance of this site.
- Increase the font on the flow points in Central Ave.
- At the front of the new building, is the dotted line a swale as called out in the center of the property?

If so, how will flows pass the sidewalk from east to west. Provide a detail or a note.

If you should have any questions feel free to contact me or Abiel Carrillo at 924-3986.

Rudy E. Rael, CE, CFM

Engineer Associate, Hydrology Planning Department 600 2nd St. NW Suite 201 Albuquerque NM 87102 (505) 924-3977