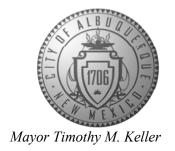
# CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



March 14, 2022

David Thompson, PE Thompson Engineering Consultants, Inc. PO Box 65760 Albuquerque, NM 87193

RE: Sombra del Oeste

> **Grading and Drainage Plans** Engineer's Stamp Date: 02/08/22

**Hydrology File: M09D032** 

Dear Mr. Thompson:

Based upon the information provided in your submittal received 02/08/2022, the Grading & Drainage Plans are approved for Grading Permit, Work Order and for action by the DRB on

Platting.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control

(ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the

Stormwater Quality Engineer (Dough Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior

to any earth disturbance. NM 87103

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

www.cabq.gov

PO Box 1293

Albuquerque

Sincerely,

Renée C. Brissette, P.E. CFM Senior Engineer, Hydrology

Renée C. Brissette

Planning Department



## City of Albuquerque

### Planning Department

#### Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 11/2018)

Project Title:	Building F	Permit #:	: Hydrology File #:			
DRB#:	EPC#:		Work Order#:			
Legal Description:						
City Address:						
Applicant:			Contact:			
Address:						
			E-mail:			
Owner:			Contact:			
Address:						
			E-mail:			
TYPE OF SUBMITTAL: PLAT	(75 # OF LOTS)	RESIDENCE	DRB SITE ADMIN SITE			
IS THIS A RESUBMITTAL?:						
<b>DEPARTMENT:</b> TRAFFIC/ TI	RANSPORTATION _	HYDROLOG	Y/ DRAINAGE			
TYPE OF SUBMITTAL:  ENGINEER/ARCHITECT CERT PAD CERTIFICATION CONCEPTUAL G & D PLAN GRADING PLAN DRAINAGE MASTER PLAN DRAINAGE REPORT FLOODPLAIN DEVELOPMENT ELEVATION CERTIFICATE CLOMR/LOMR TRAFFIC CIRCULATION LAYO TRAFFIC IMPACT STUDY (TIS OTHER (SPECIFY) PRE-DESIGN MEETING?	PERMIT APPLIC DUT (TCL)	BUILICERTPRELISITE ISITE IFINAISIA/ FFOUNGRAISO-19PAVIIGRAIWORF				
DATE SUBMITTED:	By:	OTHE	DPLAIN DEVELOPMENT PERMIT  ER (SPECIFY)			

FEE PAID:\_\_\_

### I. PURPOSE AND SCOPE

THE PURPOSE OF THIS DRAINAGE PLAN IS TO PRESENT THE EXISTING AND PROPOSED DRAINAGE CONDITIONS FOR PROPOSED SOMBRA DEL OESTE SUBDIVISION, LOCATED IN THE 9001 BLOCK OF GIBSON BOULEVARD SW, IN ALBUQUERQUE. THE ZONE ATLAS PAGE FOR THE SITE IS M-09-7

#### II. SITE DESCRIPTION AND HISTORY

THE PROJECT SITE IS LOCATED ON THE NORTH SIDE OF GIBSON BOULEVARD SE, BETWEEN UNSER BOULEVARD SW, AND SNOW VISTA BOULEVARD SW.

THE SITE IS CURRENTLY VACANT WITH DEVELOPED PROPERTIES SURROUNDING.

#### III. COMPUTATIONAL PROCEDURES

HYDROLOGIC ANALYSIS WAS PERFORMED UTILIZING THE DESIGN CRITERIA BASED ON CHAPTER 6, HYDROLOGY, OF THE DEVELOPMENT PROCESS MANUAL RELEASED 2020. TABLES WITHIN CHAPTER 6, WERE USED TO AID IN THE STUDY OF THE SITE HYDROLOGY.

#### IV. PRECIPITATION

THE STORM EVENT USED FOR THE FOLLOWING CALCULATIONS IS THE 100YR-6HR STORM. THE PROJECT SITE IS LOCATED IN ZONE 1 (WEST OF RIO GRANDE).

#### V. EXISTING DRAINAGE CONDITIONS

CURRENTLY THE SITE IS VACANT AND GENERALLY DRAINS FROM NORTH-WEST TO SOUTH-EAST. A PRIVATE STORM DRAINAGE SYSTEM WAS INSTALLED ALONG GIBSON TO CONVEY EXCESS RUNOFF FROM THE SITE. THE PRIVATE STORM DRAINAGE SYSTEM WILL BE ABANDONED AND WILL NOT BE UTILIZED FOR THIS DEVELOPMENT. THE PRIVATE STORM DRAINAGE SYSTEM ALLOWED FOR UP TO 25.9 CFS AND THEREFORE THIS SITE CAN HAVE FREE DISCHARGE UP TO THAT LIMIT.

TO THE WEST OF THE SITE IS THE AMOLE ARROYO THAT CONVEYS OFFSITE FLOWS PAST THE SITE. THE IS ALSO AN OVERHEAD POWER TRANSMISSION LINE FOR PNM ALONG THE WESTERN BOUNDARY OF THE SITE. NO OFFSITE FLOWS WILL ENTER THE SITE FROM THE WEST. THE NORTH SIDE OF THE SITE, CONTAINS A FULLY DEVELOPED SUBDIVISION THAT DIRECTS RUNOFF FROM ADJACENT PROPERTIES NORTH INTO THE ROADWAYS SYSTEM. NO OFFSITE FLOWS WILL ENTER THE SITE FROM THE NORTH. THE EAST SIDE IF THE SITE IS LOWER AND THEREFORE WILL NOT CREATE ANY OFFSITE FLOWS FRO THIS PROJECT. TO THE SOUTH IS GIBSON BOULEVARD SW. GIBSON DRAINS FROM WEST TO EAST.

THE PRE-DEVELOPED PEAK RUNOFF RATE FROM THE SITE IS 10.19 CFS (WELL BELOW THE ALLOWABLE DISCHARGE OF 25.59 CFS).

#### VI. PROPOSED DRAINAGE CONDITIONS

THE PROPOSED SUBDIVISION HAS BEEN DESIGNED TO ROUTE EXCESS RUNOFF THROUGH SHALLOW PONDING AREAS AND ON-SITE COLLECTION AND CONVEYANCE SYSTEMS TO REDUCE THE PEAK RUNOFF RATE BACK TO HISTORIC RATES, AND TO CONTAIN THE NECESSARY WATER QUALITY VOLUME AS REQUIRED BY THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL, CHAPTER 6.

FOR ANALYSIS OF EXCESS RUNOFF FLOW RATES AND WATER QUALITY VOLUME REQUIREMENTS THE SITE HAD BEEN DIVIDED INTO 5 PROPOSED BASINS.

BASIN PRO 1, IS ALONG THE WESTERN SIDE OF THE PROJECT AND ONLY CONTAINS THE BACKYARDS OF THE WESTERN LOTS ALONG SMOKETREE DRIVE SW. THIS BASIN PRO 1 CONTAINS 8,852 SF AND GENERATES A PEAK RUNOFF OF 0.48 CFS. RUNOFF FROM THIS BASIN IS DRAINED INTO POND A THAT HAS AN AVAILABLE VOLUME OF 1, 704 CUBIC FEET. THERE IS NO WATER QUALITY VOLUME REQUIRED FROM THIS BASIN AS THERE IS NO IMPERVIOUS SURFACE. DRAINAGE FROM THE HOUSES WILL BE REQUIRED TO DRAIN TOWARD THE STREET TO REDUCE THE CROSS LOT DRAINAGE.

BASIN PRO 2 IS LOCATED ALONG THE NORTH SIDE OF THE SITE. IT WILL DRAIN FROM THE WEST TO EAST AND INTO POND B, THAT WILL HAVE AN AVAILABLE VOLUME OF 2,428 CUBIC FEET. THE PEAK RUNOFF FROM THIS BASIN WILL BE 7.44 CFS AND AFTER ROUTING THROUGH POND B, WILL BE REDUCED TO 4.71 CFS THAT WILL BE CONVEYED VIA UNDERGROUND STORM PIPING TO POND C FOR FURTHER REDUCTION IN RUNOFF RATES.

BASIN PRO 3 WAS CREATED TO BE ABLE TO SIDE THE CROSS LOT DRAINAGE AND TO DETERMINE RUNOFF RATES WITHIN VALLEY COTTONWOOD DRIVE SW. THE PEAK RUNOFF RATE FOR THE ENTIRE BASIN IS 1.24 CFS INCLUDING THE 0.42 CFS THAT IS CONVEYED VIA THE TWO BACKYARD CROSS LOT DRAINAGE SWALES. EACH SIDE OF THE RETAINING WALL WILL NEED TO CONVEY 0.21 CFS.

BASIN PRO 4 IS LOCATED ALONG THE SOUTHERN SIDE OF THE SITE AND GENERATES A PEAK RUNOFF RATE OF 6.73 CFS. BACKYARD CROSS LOT DRAINAGE WILL DRAIN 6 LOTS AND GENERATE A PEAK RUNOFF RATE OF 0.17 CFS. EXCESS RUNOFF FROM THIS BASIN WILL DRAIN TO A SERIES OF CATCH BASINS NEAR THE INTERSECTION OF MOUNTAIN ASH AVE SW AND VALLEY COTTONWOOD DR SW. EACH SIDE OF THE ROAD WILL NEED TO COLLECT A PEAK RUNOFF RATE OF 3.37 CFS. THIS CAN EASILY BE ACCOMPLISH VIA A TYPE A INLET PER DPM FIGURE 6.9.9. RUNOFF WILL THEN BE CONVEY TO POND C.

BASIN PRO 5 IS LOCATED ALONG THE EASTERN SIDE OF THE SITE. EXCESS RUNOFF FROM THIS BASIN IS 1.55 CFS. A SMALL PORTION WILL BE DRAINED VIA A CROSS LOT DRAINAGE SWALE INTO POND C. THE REMAINDER OF THE BASIN WILL DRAIN INTO VALLEY COTTONWOOD DR SW AND INTO POND C BY A TYPE A CATCH BASIN ON EACH SIDE OF THE ROAD IN A SUMP CONDITION.

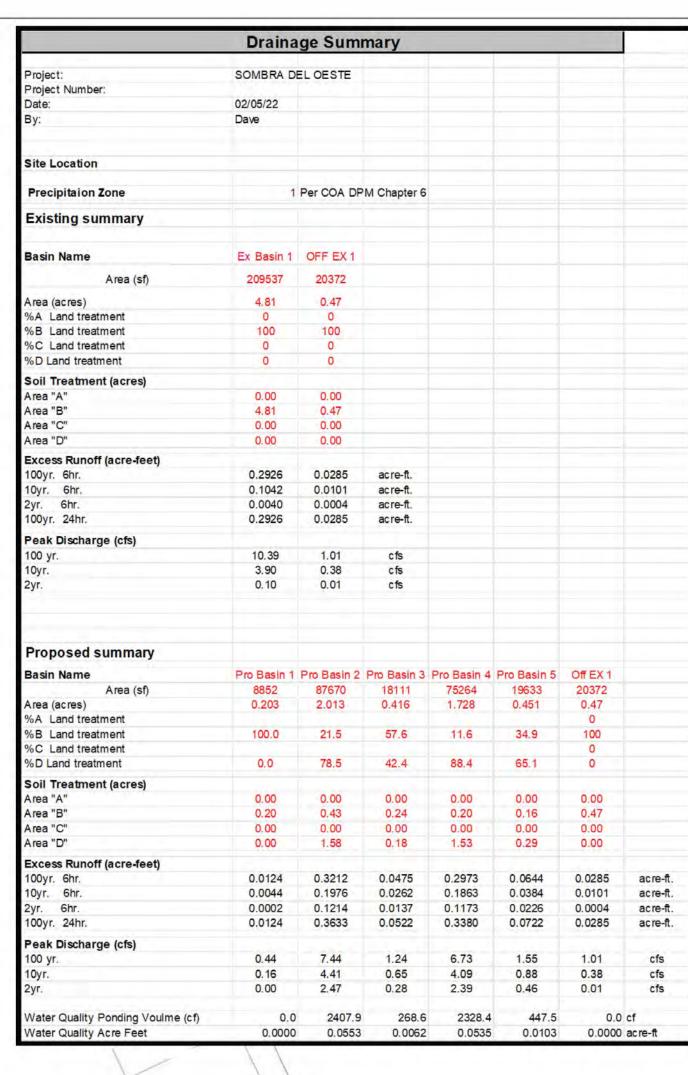
POND C HAS BEEN SIZED TO CONTAIN THE WATER QUALITY VOLUME OF 3,157 CUBIC FEET. AS MENTIONED IN THE EXISTING CONDITIONS, THE PRIOR GRADING AND DRAINAGE PLANS INDICATED THIS SITE WAS DESIGNED TO RELEASE 25.59 CFS AND THEREFORE THE FULLY DEVELOPED PEAK FLOWRATE OF 17.44 CAN BE RELEASED WITHOUT ANY RESTRICTIONS.

THE UNDERGROUND STORM CONVEYANCE SYSTEM WILL BE SIZED TO CONVEY 10.19 CFS INTO GIBSON BOULEVARD SW AND OVER TO AN EXISTING STORM DRAINAGE SYSTEM AT STAMPEDE DRIVE SW. BECAUSE THE PRIVATE STORM DRAINAGE FORMERLY CONVEYED THE 10.21 CFS INTO THE SAME SYSTEM, DOWNSTREAM CAPACITY WILL NOT BE AFFECTED BY CONNECTING INTO THE SYSTEM.

THE SITE DOES CONTAIN A SINGLE BASIN (LABELED OFF BASIN EX 1) THAT IS LOCATED UNDER THE POWERLINES AND THEREFORE NOT REALLY PART OF THE DEVELOPED PROPERTY. THIS BASIN HAS BEEN DESIGNED TO HAVE FULL 100 YEAR EVENT RETENTION. SHOULD THIS POND BE EXCEEDED THE EXCESS RUNOFF WOULD ENTER GIBSON BOULEVARD SW.

## VII. CONCLUSIONS

THE PROPOSED SUBDIVISION HAS BEEN DESIGNED TO DIVERT STREET AND ROOF RUNOFF INTO A SERIES OF WATER QUALITY PONDS PRIOR TO BEING CONVEYED VIA A NEW UNDERGROUND STORM PIPING SYSTEM CONNECTING TO THE EXISTING PUBLIC STORM DRAINAGE SYSTEM AT GIBSON BOULEVARD SW, AND STAMPEDE DRIVE SW. THE PROPOSED PEAK RATE (DEVELOPED FLOW 10.21 CFS) IS WELL BELOW THE ALLOWABLE OF 25.59 CFS. THERE SHOULD BE NOT AFFECT TO DOWNSTREAM FACILITIES.



Pond Routing and Volumes		Pond A	Pond B	Pond C			Pond D	
		Basin 1	Basin 2	Basin 3	Basin 4	Basin 5	Off EX 1	
Incoming Flow Rate	Qin	0.44	7.44	1.24	6.73	1.55	1.01	cfs
Allowable Discharge Rate	Qout	0.00	4.71	0.50	4.50	0.50	0.00	10.21 Total discharge
Hyrdology Zone		1	- 1	- 4	1	1	1	per Figure A-1
Area Total	At	0.203	2.013	0.416	1.728	0.451	0.468	acres
Area Type A	Aa	0	Ó	0	0	0	Ó	%
Area Type B	Ab	35	35 20	35	35	35	100	%
Area Type C	Ac	20	20	20	20	20	0	%
Area Type D Impervious	Ad	55	55	55	55	55	0	%
Excess runoff rates	A	0.44	0.44	0.44	0.44	0.44	0.44	
	В	0.67	0.67	0.67	0.67	0.67	0.67	
	C	0.99	0.99	0.99	0.99	0.99	0.99	
	D	1.97	1.97	1,97	1.97	1.97	1,97	
Weighted E (Exces Runoff)		1.52	1.52	1.52	1.52	1.52	0.67	
Time of Concentration		0.2	1.2	2.2	3.2	4.2	5.2	hours
Time to Peak		0.228	0.928	1.628	2 328	3.028	3.773	hours
=0.7*Tc + ((1.6-(Ad/At)/12)								
Time of Base		1.341	0.726	0.931	0.683	0.792	0.654	hours
=2.107"E"At/Qp-(.25"Ad/At)								
Duration of Peak		0.138						hours
Time for end of peak		0.365	1.065	1.765	2.465			hours
Time when storage begins		0.000	0.587	0.654				hours
Time incoming is less that discharge		1.341	0.941	1.266	1.875	1.558	0.654	hours
Volume Required during storm	acre-inch	0.325		0.278				acre inch
Volume Required during storm	cf	1178	2436	1011	1840	1367	1198	cubic feet
Volume Stored in Basin during storm	cf	1178	2428			3157	1250	Total Stored
Top Area		1412	1996			1324	2160	
Bottom Area		860	1241			480	1265	
Top Elev		5012.23	5099.25			5098.73	5113.50	
Bot Elev		5010.73	5097.75			5095.23	5112.75	
Volume Available by ponds	cf	1704	2428			3157	1250	cubic feet
Volume Available total by basin	cf	1178	2428			3157	1250	









The surveyed area, as shown hereon, appears to lie within "ZONE X" (areas determined to be outside the 0.2% annual chance floodplain), shown on National Flood Insurance Program Flood Insurance Rate Map 35001C0336H REVISED 08/16/2012.





SCALE: 1"=80"

SHEET No.

CD1 of 2

