

CITY OF ALBUQUERQUE

Planning Department
Alan Varela, Director



Mayor Timothy M. Keller

July 1, 2024

David Thompson, PE
Thompson Engineering Consultants, Inc.
PO Box 65760
Albuquerque, NM 87193

**RE: Sombra del Oeste
Revised Grading and Drainage Plans
Engineer's Stamp Date: 06/20/24
Hydrology File: M09D032**

Dear Mr. Thompson:

Based upon the information provided in your submittal received 06/21/2024, the Revised Grading & Drainage Plans are approved for Grading Permit, Work Order and for action by Development Hearing Officer (DHO) on Preliminary Plat.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Dough Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov



City of Albuquerque

Planning Department
Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (DTIS)

Project Title: _____ Hydrology File # _____

Legal Description: _____

City Address, UPC, OR Parcel: _____

Applicant/Agent: _____ Contact: _____

Address: _____ Phone: _____

Email: _____

Applicant/Owner: _____ Contact: _____

Address: _____ Phone: _____

Email: _____

(Please note that a DFT SITE is one that needs Site Plan Approval & ADMIN SITE is one that does not need it.)

TYPE OF DEVELOPMENT: PLAT (#of lots) _____ RESIDENCE
DFT SITE ADMIN SITE

RE-SUBMITTAL: YES NO

DEPARTMENT: TRANSPORTATION HYDROLOGY/DRAINAGE

Check all that apply under Both the Type of Submittal and the Type of Approval Sought:

TYPE OF SUBMITTAL:

ENGINEER/ARCHITECT CERTIFICATION
PAD CERTIFICATION
CONCEPTUAL G&D PLAN
GRADING & DRAINAGE PLAN
DRAINAGE REPORT
DRAINAGE MASTER PLAN
CLOMR/LOMR
TRAFFIC CIRCULATION LAYOUT (TCL)
ADMINISTRATIVE
TRAFFIC CIRCULATION LAYOUT FOR DFT
APPROVAL
TRAFFIC IMPACT STUDY (TIS)
STREET LIGHT LAYOUT
OTHER (SPECIFY) _____

TYPE OF APPROVAL SOUGHT:

BUILDING PERMIT APPROVAL
CERTIFICATE OF OCCUPANCY
CONCEPTUAL TCL DFT APPROVAL
PRELIMINARY PLAT APPROVAL
FINAL PLAT APPROVAL
SITE PLAN FOR BLDG PERMIT DFT
APPROVAL
SIA/RELEASE OF FINANCIAL GUARANTEE
FOUNDATION PERMIT APPROVAL
GRADING PERMIT APPROVAL
SO-19 APPROVAL
PAVING PERMIT APPROVAL
GRADING PAD CERTIFICATION
WORK ORDER APPROVAL
CLOMR/LOMR
OTHER (SPECIFY) _____

DATE SUBMITTED: _____

THE PURPOSE OF THIS DRAINAGE PLAN IS TO PRESENT THE EXISTING AND PROPOSED DRAINAGE CONDITIONS FOR PROPOSED SOMBRA DEL OESTE SUBDIVISION, LOCATED IN THE 9001 BLOCK OF GIBSON BOULEVARD SW, IN ALBUQUERQUE. THE ZONE ATLAS PAGE FOR THE SITE IS M-09-Z.

THE PROJECT SITE IS LOCATED ON THE NORTH SIDE OF GIBSON BOULEVARD SE, BETWEEN UNSER BOULEVARD SW, AND SNOW VISTA BOULEVARD SW.

THE SITE IS CURRENTLY VACANT WITH DEVELOPED PROPERTIES SURROUNDING.

HYDROLOGIC ANALYSIS WAS PERFORMED UTILIZING THE DESIGN CRITERIA BASED ON CHAPTER 6, HYDROLOGY, OF THE DEVELOPMENT PROCESS MANUAL RELEASED 2020. TABLES WITHIN CHAPTER 6, WERE USED TO AID IN THE STUDY OF THE SITE HYDROLOGY.

THE STORM EVENT USED FOR THE FOLLOWING CALCULATIONS IS THE 100YR-6HR STORM. THE PROJECT SITE IS LOCATED IN ZONE 1 (WEST OF RIO GRANDE).

CURRENTLY THE SITE IS VACANT AND GENERALLY DRAINS FROM NORTH-WEST TO SOUTH-EAST. A PRIVATE STORM DRAINAGE SYSTEM WAS INSTALLED ALONG GIBSON TO CONVEY EXCESS RUNOFF FROM THE SITE. THE PRIVATE STORM DRAINAGE SYSTEM WILL BE ABANDONED AND WILL NOT BE UTILIZED FOR THIS DEVELOPMENT. THE PRIVATE STORM DRAINAGE SYSTEM ALLOWED FOR UP TO 25.9 CFS AND THEREFORE THIS SITE CAN HAVE FREE DISCHARGE UP TO THAT LIMIT.

TO THE WEST OF THE SITE IS THE AMOLE ARROYO THAT CONVEYS OFFSITE FLOWS PAST THE SITE. IT IS ALSO AN OVERHEAD POWER TRANSMISSION LINE FOR PNM ALONG THE WESTERN BOUNDARY OF THE SITE. NO OFFSITE FLOWS WILL ENTER THE SITE FROM THE WEST. THE NORTH SIDE OF THE SITE, CONTAINS A FULLY DEVELOPED SUBDIVISION THAT DIRECTS RUNOFF FROM ADJACENT PROPERTIES NORTH INTO THE ROADWAYS SYSTEM. NO OFFSITE FLOWS WILL ENTER THE SITE FROM THE NORTH. THE EAST SIDE IF THE SITE IS LOWER AND THEREFORE WILL NOT CREATE ANY OFFSITE FLOWS FROM THIS PROJECT. TO THE SOUTH IS GIBSON BOULEVARD SW. GIBSON DRAINS FROM WEST TO EAST.

THE PRE-DEVELOPED PEAK RUNOFF RATE FROM THE SITE IS 14.66 CFS (WELL BELOW THE ALLOWABLE DISCHARGE OF 25.59 CFS).

THE PROPOSED SUBDIVISION HAS BEEN DESIGNED TO ROUTE EXCESS RUNOFF THROUGH SHALLOW PONDING AREAS AND ON-SITE COLLECTION AND CONVEYANCE SYSTEMS TO REDUCE THE PEAK RUNOFF RATE BACK TO HISTORIC RATES, AND TO CONTAIN THE NECESSARY WATER QUALITY VOLUME AS REQUIRED BY THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL, CHAPTER 6.

FOR ANALYSIS OF EXCESS RUNOFF FLOW RATES AND WATER QUALITY VOLUME REQUIREMENTS THE SITE HAD BEEN DIVIDED INTO 5 PROPOSED BASINS.

BASIN PRO 1, IS ALONG THE WESTERN SIDE OF THE PROJECT AND ONLY CONTAINS THE BACKYARDS OF THE WESTERN LOTS ALONG SMOKETREE DRIVE SW. THIS BASIN PRO 1 CONTAINS 8,579 SF AND GENERATES A PEAK RUNOFF OF 0.41 CFS. RUNOFF FROM THIS BASIN IS DRAINED THROUGH THE WESTERN SIDE OF THE LOTS INTO THE OPEN AREA BELOW THE POWER LINES THAT HAS A DEPRESSION CREATED FROM BORROW, AND HAS AVAILABLE VOLUME OF 5,126 CUBIC FEET. THIS BASIN DOES NOT CONTAIN ANY IME=PERVIOUS SURFACE FOR WATER QUALITY.

BASIN PRO 2 IS LOCATED ALONG THE NORTH SIDE OF THE SITE. IT WILL DRAIN FROM THE WEST TO EAST AND INTO POND B, THAT WILL HAVE AN AVAILABLE VOLUME OF 1,962 CUBIC FEET. THE PEAK RUNOFF FROM THIS BASIN WILL BE 7.33 CFS AND AFTER ROUTING THROUGH POND B, WILL BE REDUCED TO 5.50 CFS THAT WILL BE CONVEYED VIA UNDERGROUND STORM PIPING TO POND C FOR FURTHER REDUCTION IN RUNOFF RATES. POND B WILL CONTAIN THE WATER QUALITY VOLUME OF 1,695 CF.

BASIN PRO 3 WAS CREATED TO BE ABLE TO SIZE THE CROSS LOT DRAINAGE AND TO DETERMINE RUNOFF RATES WITHIN VALLEY COTTONWOOD DRIVE SW. THE PEAK RUNOFF RATE FOR THE ENTIRE BASIN IS 1.25 CFS INCLUDING THE 0.42 CFS THAT IS CONVEYED VIA THE TWO BACKYARD CROSS LOT DRAINAGE SWALES. EACH SIDE OF THE RETAINING WALL WILL NEED TO CONVEY 0.21 CFS.

BASIN PRO 4 IS LOCATED ALONG THE SOUTHERN SIDE OF THE SITE AND GENERATES A PEAK RUNOFF RATE OF 6.66 CFS. BACKYARD CROSS LOT DRAINAGE WILL DRAIN 6 LOTS AND GENERATE A PEAK RUNOFF RATE OF 0.17 CFS. EXCESS RUNOFF FROM THIS BASIN WILL DRAIN TO A SERIES OF CATCH BASINS NEAR THE INTERSECTION OF MOUNTAIN ASH AVE SW AND VALLEY COTTONWOOD DR SW. EACH SIDE OF THE ROAD WILL NEED TO COLLECT A PEAK RUNOFF RATE OF 3.33 CFS. THIS CAN EASILY BE ACCOMPLISH VIA A TYPE "A" INLET PER DPM FIGURE 6.9.9. RUNOFF WILL THEN BE CONVEY UNDERGROUND TO POND C.

BASIN PRO 5 IS LOCATED ALONG THE EASTERN SIDE OF THE SITE. EXCESS RUNOFF FROM THIS BASIN IS 1.41 CFS. A SMALL PORTION WILL BE DRAINED VIA A CROSS LOT DRAINAGE SWALE INTO POND C. THE REMAINDER OF THE BASIN WILL DRAIN INTO VALLEY COTTONWOOD DR SW AND INTO POND C BY A TYPE "A" CATCH BASIN ON EACH SIDE OF THE ROAD IN A SUMP CONDITION.

POND C HAS BEEN SIZED TO CONTAIN THE WATER QUALITY VOLUME OF 3,032 CUBIC FEET. AS MENTIONED IN THE EXISTING CONDITIONS, THE PRIOR GRADING AND DRAINAGE PLANS INDICATED THIS SITE WAS DESIGNED TO RELEASE 25.59 CFS AND THEREFORE THE FULLY DEVELOPED PEAK FLOWRATE OF 16.60 CAN BE RELEASED WITHOUT ANY RESTRICTIONS.

THE UNDERGROUND STORM CONVEYANCE SYSTEM WILL BE SIZED TO CONVEY 12.25 CFS INTO GIBSON BOULEVARD SW AND CONNECTING TO THE EXISTING STORM DRAINAGE SYSTEM AT STAMPEDE DRIVE SW. BECAUSE THE PRIVATE STORM DRAINAGE FORMERLY CONVEYED THE 25.59 CFS INTO THE SAME SYSTEM, DOWNSTREAM CAPACITY WILL NOT BE AFFECTED BY CONNECTING INTO THE SYSTEM.

THE SITE DOES CONTAIN A TWO OFFSITE BASINS (LABELED OFF BASIN EX 1 AND EX 2) THAT ARE LOCATED UNDER THE POWERLINES AND THEREFORE NOT REALLY PART OF THE DEVELOPED PROPERTY. THE OFFSITE BASINS ARE PART OF A PRIOR APPROVED G&D PLAN

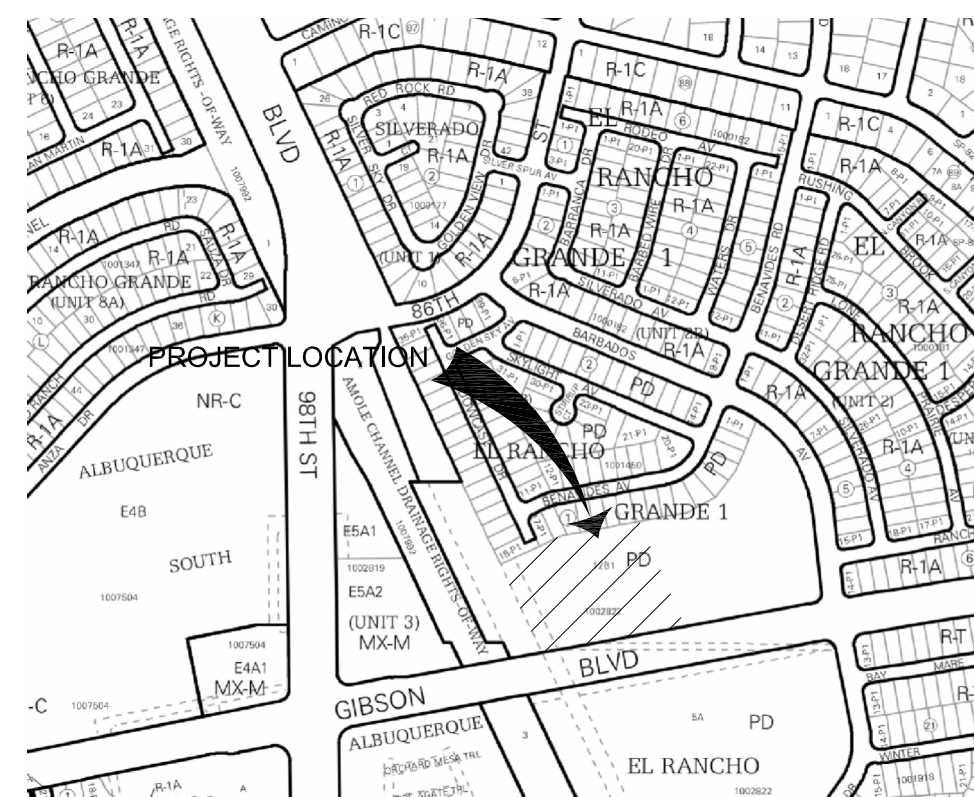
THE PROPOSED SUBDIVISION HAS BEEN DESIGNED TO DIVERT STREET AND ROOF RUNOFF INTO A SERIES OF WATER QUALITY PONDS PRIOR TO BEING CONVEYED VIA A NEW UNDERGROUND STORM PIPING SYSTEM CONNECTING TO THE EXISTING PUBLIC STORM DRAINAGE SYSTEM AT GIBSON BOULEVARD SW. AND STAMPEDE DRIVE SW. THE PROPOSED PEAK RATE (DEVELOPED FLOW 12.25 CFS) IS WELL BELOW THE ALLOWABLE OF 25.59 CFS, THERE SHOULD BE NOT AFFECT TO DOWNSTREAM FACILITIES.

Proposed summary						
Basin Name	Area (sf)	Pro Basin 1	Pro Basin 2	Pro Basin 3	Pro Basin 4	Pro Basin 5
Area (acres)		7907	8874.96	17679	77317.5	19633
%A Land treatment		1.02	2.037	0.406	1.775	0.45
%B Land treatment		100.0	26.8	53.5	80.0	50.9
%C Land treatment						
%D Land treatment			73.2	46.5	80.0	49.1
Soil Treatment (acres)						
Area "A"		0.00	0.00	0.00	0.00	0.00
Area "B"		0.18	0.55	0.22	0.35	0.23
Area "C"		0.00	0.00	0.00	0.00	0.00
Area "D"		0.00	1.49	0.19	1.42	0.22
Excess Runoff (acre-feet)						
0.10yr. 6hr.		0.0110	0.3117	0.0484	0.2867	0.0553
10yr. 6hr.		0.0039	0.1896	0.0272	0.1769	0.0313
2yr. 6hr.		0.0002	0.1149	0.0146	0.1092	0.0172
100yr. 24hr.		0.0110	0.3515	0.0535	0.3245	0.0612
Peak Discharge (cfs)						
100 yr.		0.39	7.33	1.25	6.62	1.41
10yr.		0.15	4.28	0.66	3.94	0.75
2yr.		0.00	2.34	0.30	2.22	0.35
Water Quality Ponding Volume (cf)		0.0	2274.9	287.7	2164.9	337.7
Water Quality Acre Feet		0.0000	0.0562	0.0066	0.0497	0.0077

AREA OF MITURAI FLOOD HAZARD

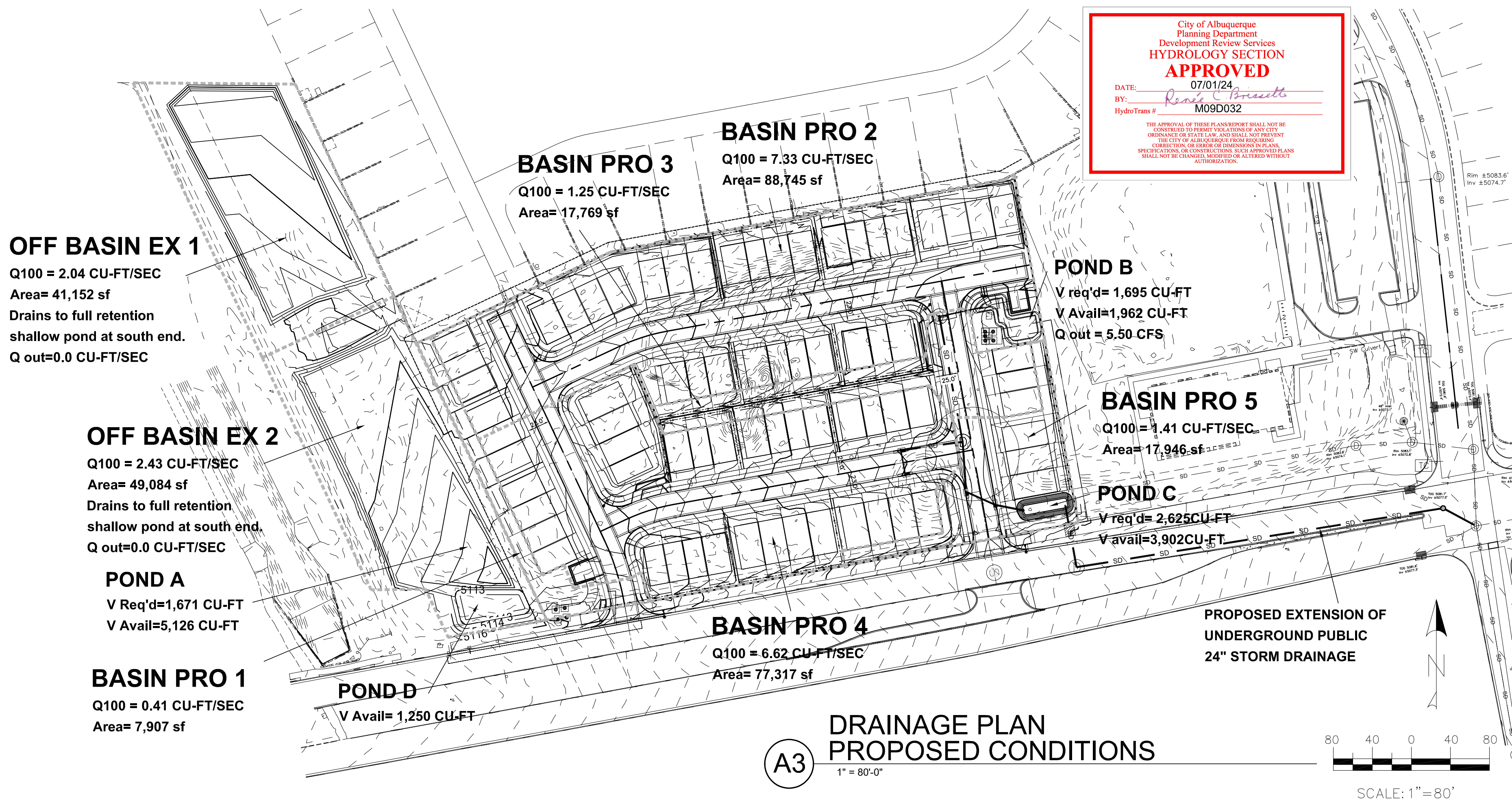
SHIMANEIWA

011-246-2631



ZONE ATLAS PAGE M-09
NOT TO SCALE

The surveyed area, as shown hereon, appears to lie within "ZONE X" (areas determined to be outside the 0.2% annual chance floodplain), shown on National Flood Insurance Program Flood Insurance Rate Map 35001C0336H REVISED 08/16/2012.

[illegible]

