CITY OF ALBUQUERQUE

Planning Department
Alan Varela, Interim Director

Mayor Timothy M. Keller

February 14, 2025

Phillip W. Clark, P.E.
Clark Consulting Engineers
19 Ryan Rd.
Edgewood, NM 87015

Curtis J. Derichs, P.E.
Civil Design Professionals (CDP)
7050 Territory Pass
Lakeville, MN 55044

RE: Home 2 Suites Hotel
3021 University Blvd. SE
Grading and Drainage Plan
Engineer's Stamp Date: 02/11/25
Hydrology File: M15D047

Dear Mr. Clark and Mr. Derichs,

Based upon the information provided in your submittal received 02/14/2025, the submitted Grading & Drainage Plan is Approved for a Grading Permit, Foundation Permit and Building Permit. Please attach a copy of this approved plan in the construction sets for Building Permit processing together with a copy of this letter.

In addition to the stamped Approved Grading & Drainage Plan, attached to this letter for reference are the files: 24-0987 Response Memo and 24-0987 Final Plans, for repairing the retaining walls at the southwest area of the project.

Also, if the project total area of disturbance (including the staging area and any work within the adjacent Rights-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 505-924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 505-924-3362 or richardmartinez@cabq.gov.

Sincerely,

Richard Martinez, P.E.
Senior Engineer, Hydrology
Planning Department

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

GRADING & DRAINAGE PLAN

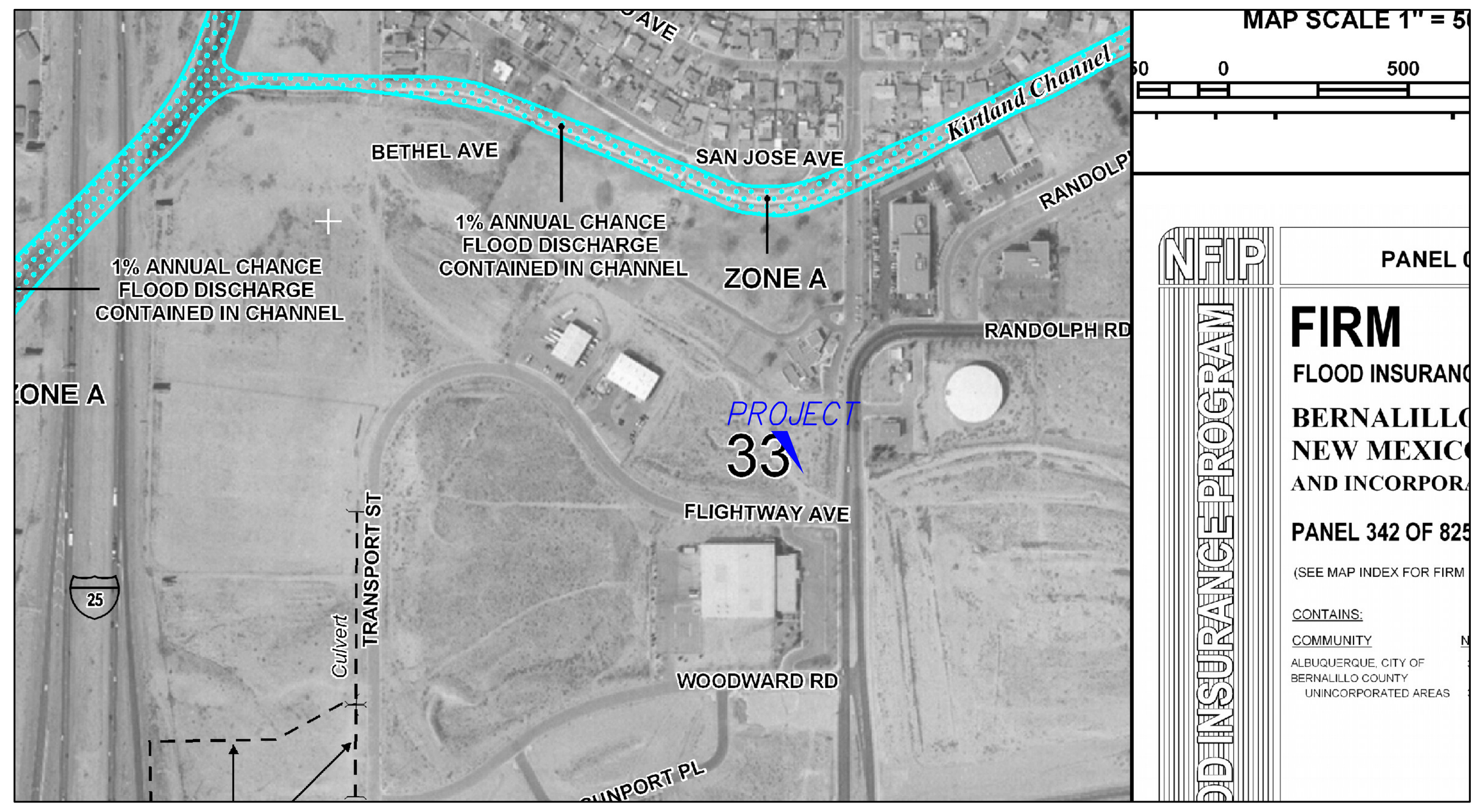
THE BUSINESS PARK ZONED PROJECT IS LOCATED IN THE DEVELOPED SUNPORT PARK SUBDIVISION (AIRPORT AREA) APPROX. 2 MILES SOUTH OF THE DOWNTOWN CORE OF ALBUQUERQUE, NM. THE GRADING & DRAINAGE SCHEME HEREON IS IN COMPLIANCE WITH THE CITY OF ALBUQUERQUE FLOOD HAZARD ORDINANCE, AND THE CITY STORM DRAINAGE ORDINANCE. THE PLAN IS REQUIRED IN ORDER TO FACILITATE THE OWNER'S REQUEST FOR EXCAVATION AND GRADING & SUBSEQUENT BUILDING PERMIT. THE PLAN SHOWS:

1. EXISTING CONTOURS, AND SPOT ELEVATIONS AND EXISTING DRAINAGE PATTERNS AND EXISTING IMPROVEMENTS: INCLUDING SURFACE UTILITIES/EASEMENTS.
2. PROPOSED IMPROVEMENTS: 85% IMPERVIOUS HOTEL SITE DEVELOPMENT, NEW GRADE ELEVATIONS, RETAINING WALLS, 2– DRIVE ENTRANCES, AND EROSION CONTROL.
3. CONTINUITY BETWEEN EXISTING AND PROPOSED ELEVATIONS.
4. QUANTIFICATION AND ANALYSIS OF UPSTREAM OFF–SITE FLOWS (IF APPLICABLE) PLUS THE DEVELOPED FLOWS GENERATED BY THE IMPROVEMENTS, INCLUDING PONDING REQUIREMENTS.

THE PURPOSE OF THE PLAN IS TO ESTABLISH CRITERIA FOR CONTROLLING STORM RUNOFF AND EROSION, AND ESSENTIALLY ALLOWING HISTORIC FLOWS TO CONTINUE TO DRAIN THROUGH THE PROPERTY. PRESENTLY, THE SITE IS BOUNDED ON ALL SIDES BY DEVELOPED PROPERTY. FLIGHTWAY AVENUE ON THE SOUTH IS AN IMPROVED ASPHALT MAJOR LOCAL ROADWAY MAINTAINED BY THE CITY OF ALBUQUERQUE. THE SITE CURRENTLY DRAINS AT APPROXIMATELY 7% TO TO THE WEST.

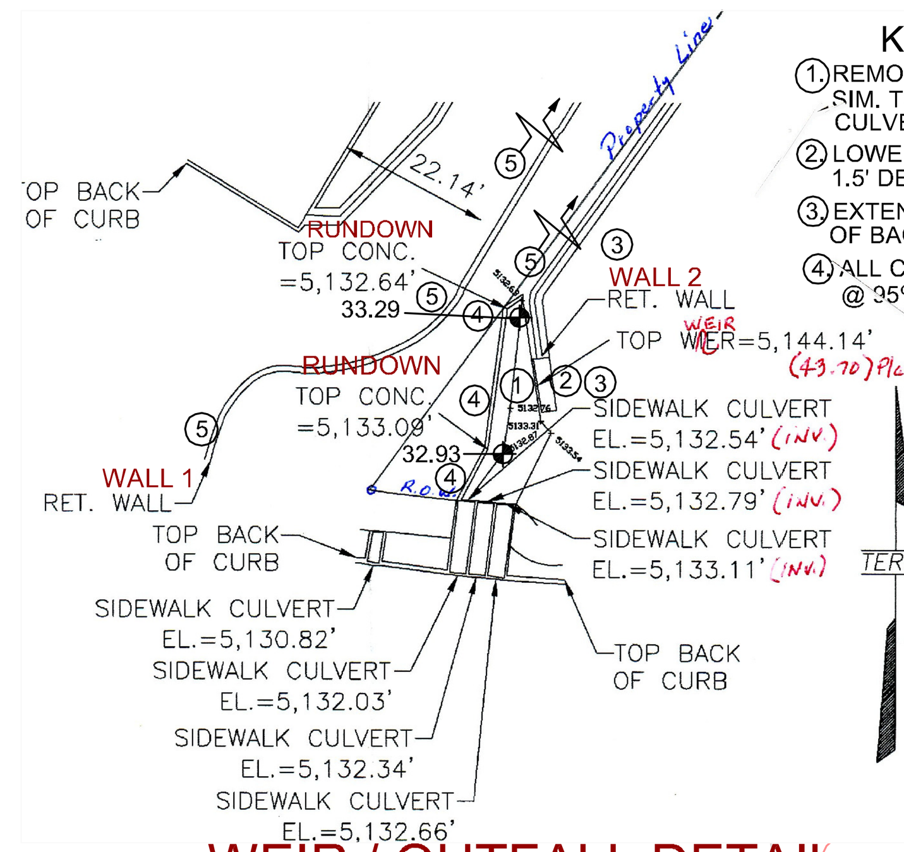
HISTORICAL SITE RUNOFF OUTFALL LOCATIONS WILL REMAIN UNCHANGED. THE SITE IS NOT IMPACTED ADVERSELY BY ANY OFF–SITE DRAINAGE FLOWS.

FREE DISCHARGE OF DEVELOPED FLOW IS ACCEPTABLE SINCE DOWNSTREAM CAPACITY EXISTS WITHIN THE OVERALL MASTERPLAN FOR THE SUNPORT PARK REGIONAL AREA. RUNOFF WILL NOT BE ALLOWED TO CONCENTRATE AT THE WEST PROPERTY BOUNDARY. THE OVERALL BASIN SHALL DRAIN TO THE SW TO THE EXPANDED SIDEWALK CULVERT(S) LOCATED AT THE SW CORNER, WITH THE ROUTING OF SITE RUNOFF THRU DEPRESSED LANDSCAPING AREAS.



FIRM MAP

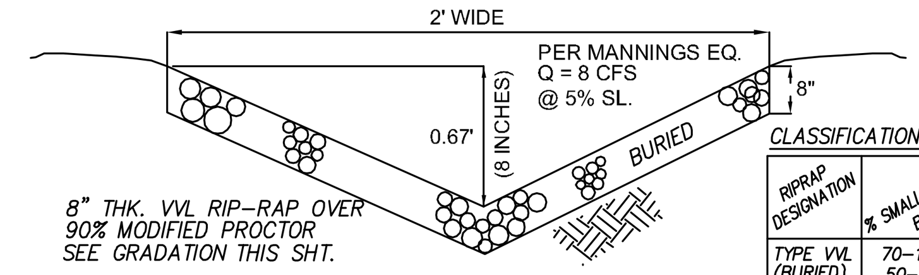
PANEL 35001C0342 H



WEIR / OUTFALL DETAIL

KEYED NOTES

1. REMOVE / REPLACE CONCRETE RUNDOWN. SIM. TO COA 2260. ENSURE -2% MIN. SLOPE TO CULVERT INVERT.
2. LOWER WEIR 5-INCHES (0.42'), ENSURE 1.5' DEPTH OF POND
3. EXTEND CLAY LINER (POND) TO TOP OF BACK OF RET. WALL 2.
4. ALL COMPACTION PER ASTM D1557, @ 95% MOD. PROCTOR
5. PERFORM FINAL GRADING AT THE BASE OF THE ENTIRE LENGTH OF BOTH RET. WALLS 1 & 2. ENSURE 3:1 GRADING AWAY FROM WALL, AND RE-SEED PER COA. 1012

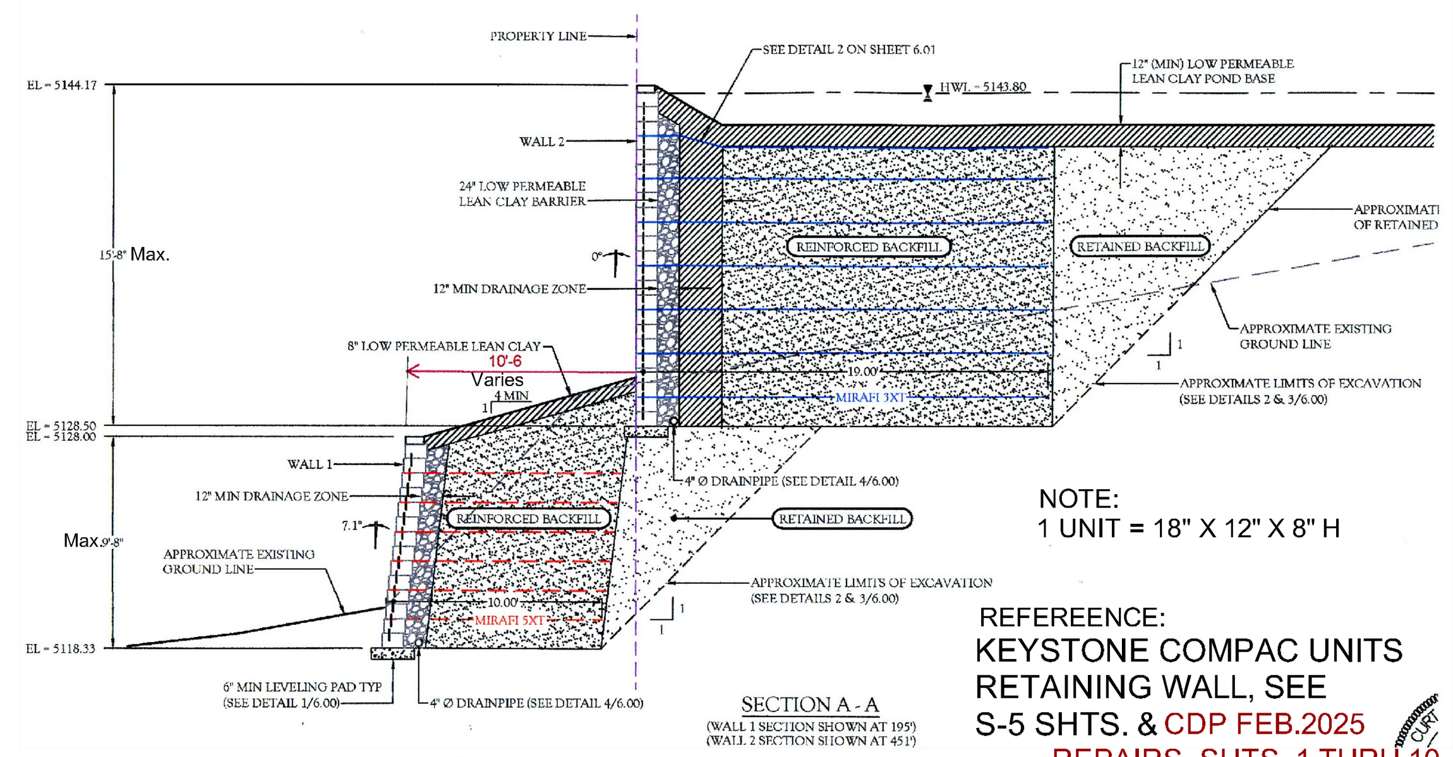


CLASSIFICATION AND GRADATION OF RIPRAP

RIPRAP DESIGNATION	% SMALLER THAN BY WEIGHT	INTERMEDIATE DIMENSION (IN.)	MEAN PARTICAL SIZE (IN.)
TYPE VVL (BURIED)	70–100 50–70 35–50 2–10	9 6 4 2	4

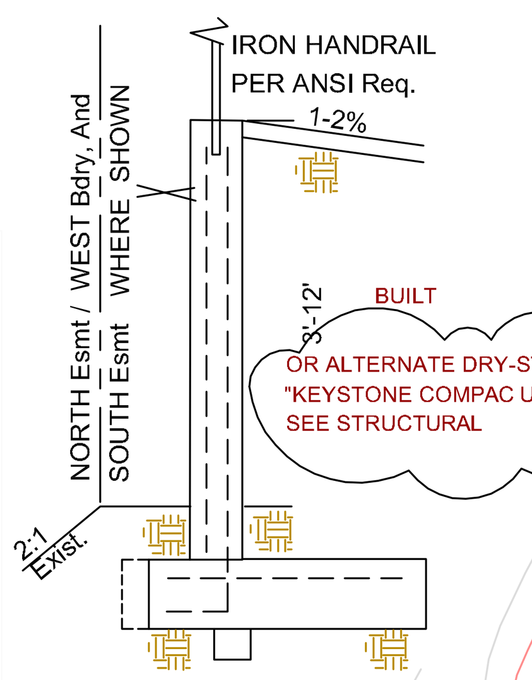
SWALE SECTION

NO SCALE



NOTE:
1 UNIT = 18" X 12" X 8" H

REFEREENCE:
KEYSTONE COMPAC UNITS RETAINING WALL, SEE S-5 SHTS. & CDP FEB.2025 REPAIRS, SHTS. 1 THRU 10.



RPCC RETAINING WALL SECTION

NO SCALE

CALCULATIONS

DESIGN CRITERIA

NOTE: THIS SITE IS PART OF THE SUNPORT PHASE 1 MASTER DRAINAGE PLAN BY ANDREWS, ASBURY & ROBERT, INC DATED 1996 AND HAS FREE DISCHARGE INTO FLIGHTWAY AVE.

HYDROLOGIC METHODS PER CHAPTER 6, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL (DPM) 2020 EDITION FOR CITY OF ALBUQUERQUE.
DISCHARGE RATE: Q=QPEAK x AREA.."Peak Discharge Rates For Small Watersheds"
VOLUMETRIC DISCHARGE: VOLUME = EWeighted x AREA
P100–6 Hr= 2.29 In., Zone 2, TC = 12 Min.
DESIGN STORM: 100–YEAR/6–HOUR, 10–YEAR/6–HOUR [ ] = 10 YEAR VALUES

EXISTING CONDITIONS — A&B

TOTAL AREA = 1.88 ACRES, WHERE EXCESS PRECIP. (AVG.) =0.71 In. [0.23]
PEAK DISCHARGE, Q100 = 4.5 CFS [1.8], WHERE UNIT PEAK DISCHARGE 'B' = 2.4 CFS/AC. [0.95]
THEREFORE: VOLUME 100 = 4845 CF [1570]

DEVELOPED CONDITIONS – HOTEL

DETERMINE LAND TREATMENTS, PEAK DISCHARGE AND VOLUMETRIC DISCHARGE FOR STUDY AREA

	AREA	LAND TREATM'T	Q_{Peak}	E
UNDEVELOPED	––– Ac.	A	1.71[0.41]	0.62[0.15]
LANDSCAPING / Harvesting	0.1 Ac.(6%)	B	2.36[0.95]	0.80[0.30]
GRAVEL & COMPACTED SOIL	0.2 Ac.(11%)	C	3.05[1.59]	1.03[0.48]
ROOF – PAVEMENT	1.5 Ac.(83%)	D	4.34[2.71]	2.33[1.51]
	1.80 Ac.			

THEREFORE: $E_{Weighted}$ = 2.0 In. &
Q100 = 7.3 CFS VOLUME 100 = 13068 CF±

WATER QUALITY STORM EVENT

FIRST FLUSH - TOTAL SITE, 0.42"/12 X 1.5 X43560 = 2287 CF

RECOMMENDATION:
1. ROUTE BASIN THRU DEPRESSED LANDSCAPE AREAS.
2. SEE NOTE 4.

WEIR CALC.

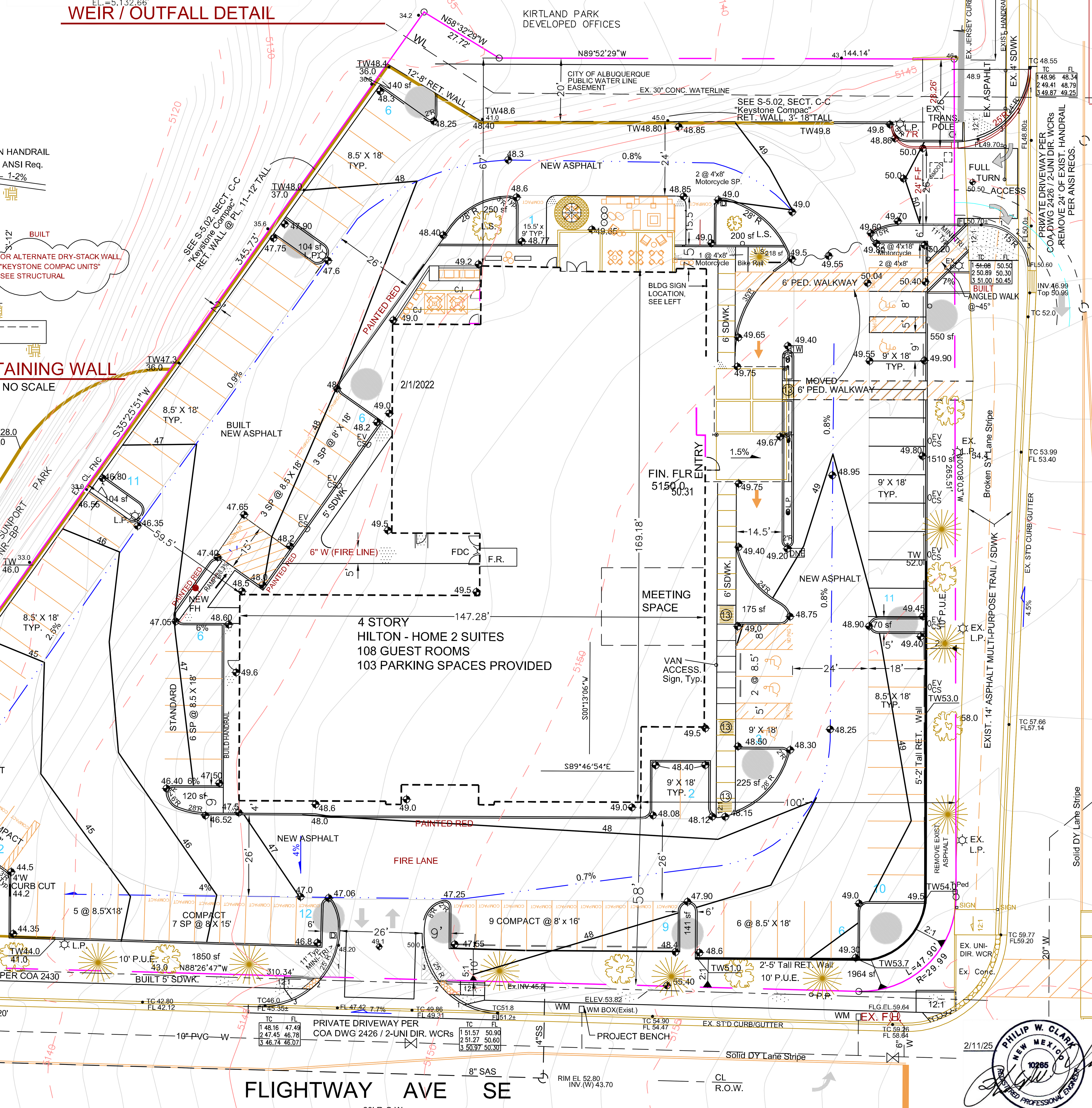
$Q = CLH^{3/2}$ WHERE: C=2.7
THEN: L = 8' H= 6" Q = 7.3 CFS
or Provide 4 SF NOTCH EQUIVALENT

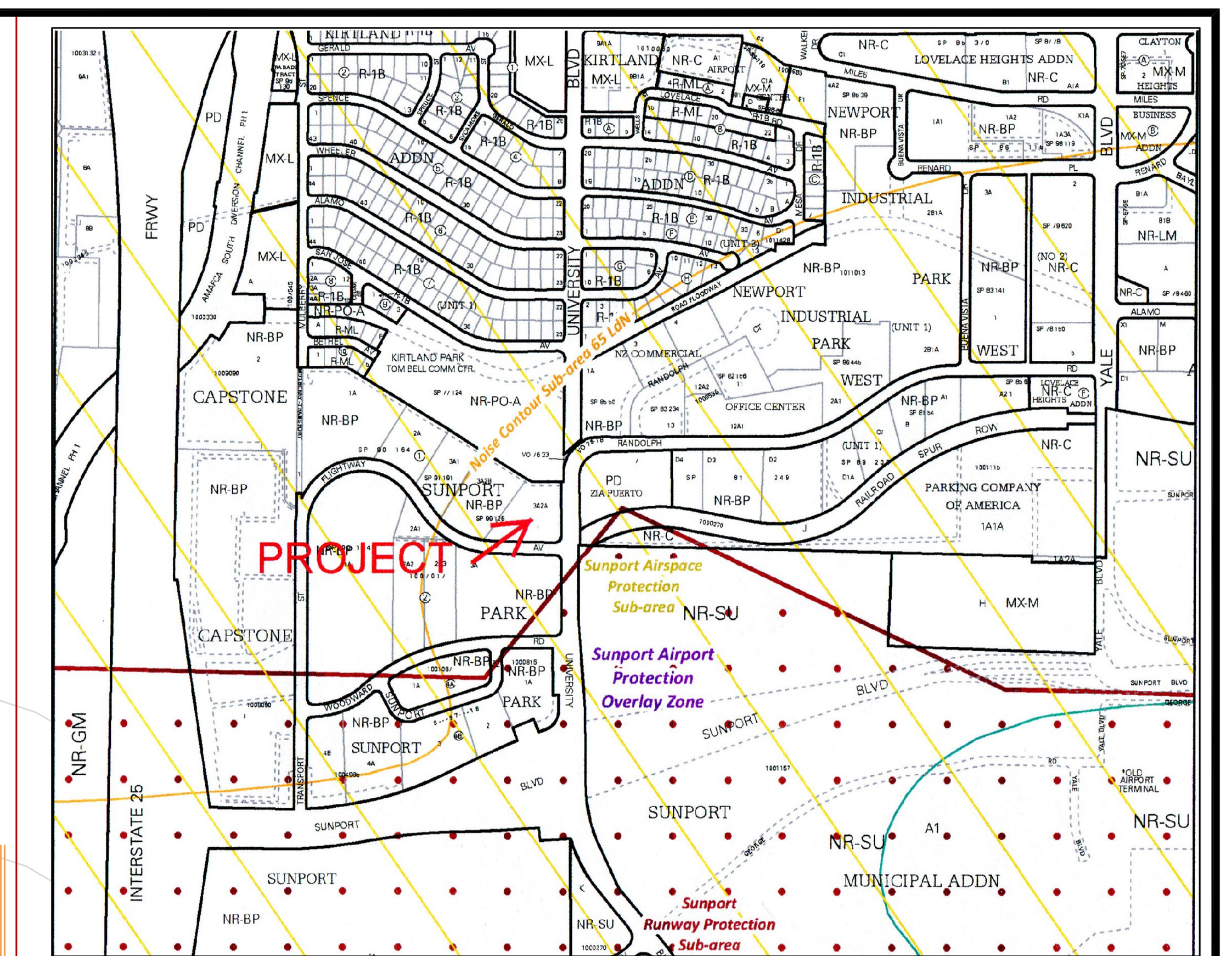
City of Albuquerque
Planning Department
Development Review Services
HYDROLOGY SECTION
APPROVED
DATE: 02/14/2025
BY: [signature]
HydroTrans # M15D047

THE APPROVAL OF THESE PLANS/REPORT SHALL NOT BE CONSTRUED TO PERMIT VIOLATIONS OF ANY CITY ORDINANCE OR STATE LAW, AND SHALL NOT PREVENT THE CITY OF ALBUQUERQUE FROM REQUIRING CORRECTION OR ERROR OR DIMENSIONS IN PLANS, SPECIFICATIONS, OR CONSTRUCTIONS. SUCH APPROVED PLANS SHALL NOT BE CHANGED, MODIFIED OR ALTERED WITHOUT AUTHORIZATION.

APPROVAL OF GRADING & DRAINAGE PLAN(S) SHALL EXPIRE TWO (2) YEARS AFTER THE APPROAL DATE BY THE CITY IF NO BUILDING PERMIT HAS BEEN PULLED ON THE DEVELOPMENT.

Scale: 1" = 20'





ZONE M-15

VICINITY MAP

NOTES

1. ALL WORK WITHIN THE RIGHT-OF-WAY SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECS. FOR PUBLIC WORKS CONSTRUCTION, 2020 EDITION.
2. AN EXCAVATION/CONSTRUCTION PERMIT IS REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY R.O.W. AN APPROVED COPY OF THIS PLAN MUST BE SUBMITTED AT THE TIME OF APPLICATION.
3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES, AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
4. CONTRACTOR SHALL ENSURE THAT NO SITE SOILS/SEDIMENT OR SILT ENTER THE RIGHT-OF-WAYS DURING CONSTRUCTION, AND ALL NPDES, MS-4 PROCEDURES AND BMP (BEST MGMT PRACTICE) GUIDELINES ARE FOLLOWED. SEE REQUIRED SWPP PLAN.
5. REVEGETATE ALL AREAS DISTURBED DUE TO CONSTRUCTION PER CITY OF ALBUQ. SPEC. 1012, NATIVE SEED MIX USING HAY MULCH, FOR EAST SIDE APPLICATION, HATCHITA BLUE GRAMA DOMINANT MIX PER PARAGRAPH4.1.2.
6. MAXIMUM SITE GRADING WITHOUT EROSION PROTECTION: 2 HORIZONTAL TO 1 VERTICAL, 2:1, SLOPES < 4' TALL
7. ALL EARTHWORK PREPARATION AND SUBGRADE PREPARATION FOR BUILDING PAD AND PAVEMENT AREAS SHALL CONFORM TO THE RECOMMENDATIONS CONTAINED IN THE GEO-TECH REPORT.
   PAVEMENT SECTION SHALL CONSIST OF 6" RPCC (REINFORCED PORTLAND CEMENT CONCRETE) OVER 8" SUBGRADE PREPARATION, 95% Modified Proctor. OR, 4" ASPHALT OVER 6" AGGREGATE BASE COURSE, NMDOT SPEC. SEE / REF: 250105.E IN SPECS. FOR PAVING UPGRADE @ PORT COSHERE.

LEGEND

EXIST. SPOT ELEVATION	•24.0
EXIST. CONTOUR	5110
NEW SPOT ELEVATION	12.0 (ADD 5100 FOR MSL)
NEW CONTOUR	12
NEW SWALE	
DRAINAGE DIRECTION, EXISTING	
NEW P.C.C., CONCRETE	
FLOWLINE	FL
WATER BLOCK	
RIPRAP STONE TYP. VVL, 4" Avg. Dia.	8" BURY UNO (UNLESS NOTED OTHERWISE)

PROJECT DATA

ZONED NR-BP GSF 67,500 BLDG FOOTPRINT 18,214, F.A.R.=0.22
TYPE V-A CONSTRUCTION - SPRINKLERED

LEGAL DESCRIPTION

LOT 3-A-2-A, SUNPORT PARK
ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO

PROJECT BENCHMARK - On Flightway, SEE PLAN

TOP OF IRON WATER METER COVER (NE CORNER) MSL ELEVATION = 5153.82, AS TIED FROM ACS, I-25-30, NAVD 88, 5041.30 FEET.

TOPOGRAPHIC DESIGN SURVEY

PROVIDED BY ADVANTAGE SURVEYING, LLC, DATED APRIL 2019, AND SUPPLEMENTED SEPTEMBER, 2019 BY CCE.

AS-BUILT GRADES PROVIDED BY ANDREW MEDINA, P.S., SEPTEMBER 2023. & DEC. 2024

2/11/25
PHILIP W. CLARK NEW MEXICO 10265 REGISTERED PROFESSIONAL ENGINEER

Clark Consulting Engineers
Edgewood, New Mexico 87015
Tele: (505) 281–2444 Cell/Txt: (505) 264–6042

	DATE	REVISION
1	2/11/25	Remove/Replace Rundown, Regrade Walls1/2, Add Riprap Lower Weir (Pond) REF. CDP Repairs

LOT 3–A–2–A, BLOCK 1, SUNPORT PARK ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO

ABQ AIRPORT LODGING, LLC

Grading & Drainage Plan

DESIGNED BY: PWC	DRAWN BY: CCE	JOB #: PATEL_H2SH	C 102
CHECKED BY: PWC	START DATE: 9/12/19	FILE #: G/D	

CIVIL DESIGN PROFESSIONALS
INNOVATIVE ENGINEERING SOLUTIONS

Date: February 10, 2025

Client: ABQ Airport Lodging
6004 Riverside Dr
Irving, TX 75039

Project: ABQ Home2Suites Retaining Walls
Albuquerque, New Mexico
Project #: 24-0987

Ritesh Patel:

CDP has received the comments from the City of Albuquerque (attached) in response to our retaining wall repair plans (CDP Project# 24-0987). I will address each of there comments related to CDP below:

1. The specification was added (comment number 7)
2. Grading was revised beneath the base of wall to align closer to the base of wall while still maintaining a 12" embedment while not exceeding a maximum 2.5H:1V toe slope (comment number 8).
3. CDP has incorporated the updated site and grading plan from Clark Consulting Engineers who are the Civil Engineers on the project who are responsible for this plan (comment number 9).
4. An additional section was was provided at the existing building showing the revised grading and rip-rapped swale at the base of wall, directing storm water run-off away from the building (comment number 10).

If you have any questions, please feel free to give me a call.

Sincerely,

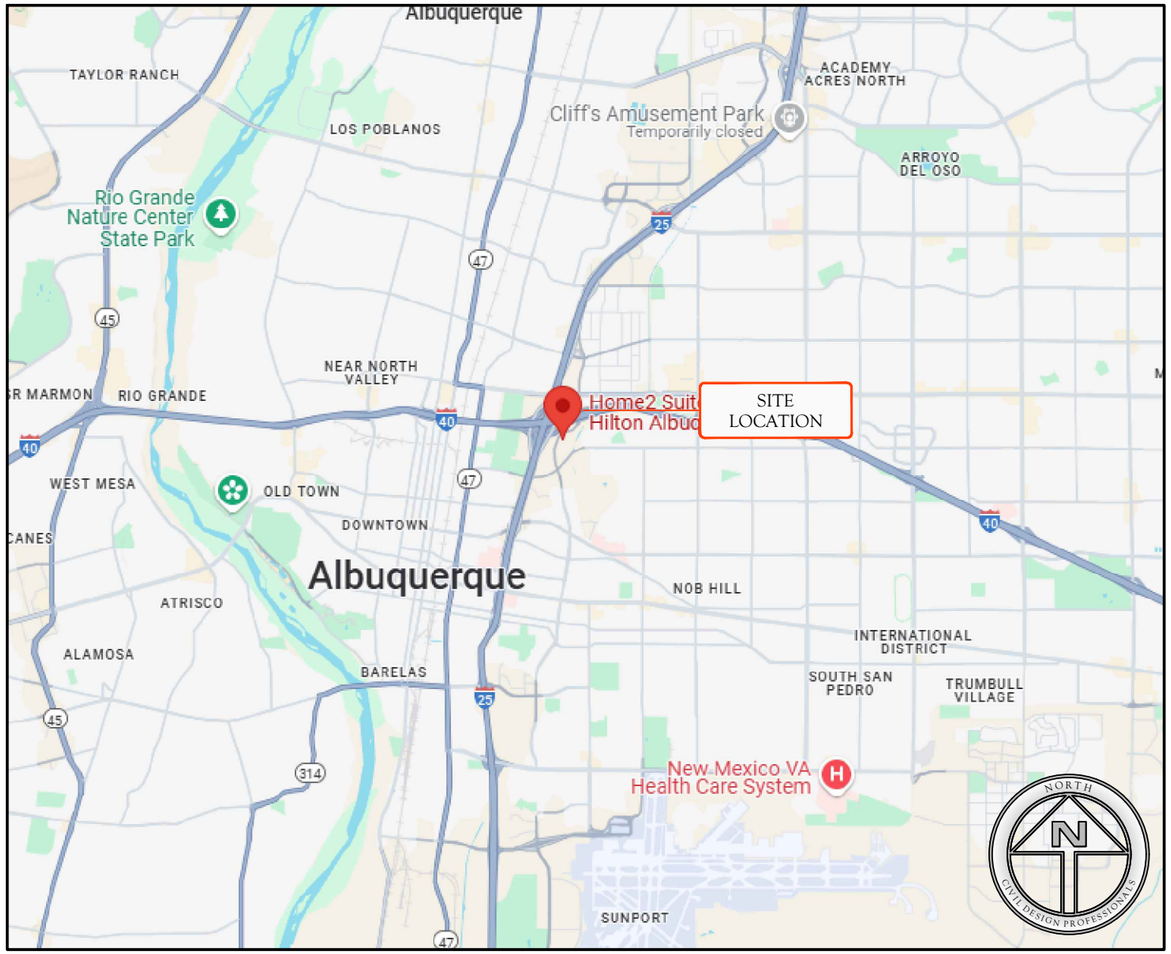
Michael Johnson
Civil Design Professionals

Civil Design Professionals • 7050 Territory Pass Lakeville, MN 55044 • Office (952) 303-5312 • www.cdp-us.com

ABQ HOME2SUITES
ALBUQUERQUE, NEW MEXICO

PROJECT INFORMATION:

1. SITE CIVIL ENGINEER:
   CLARK CONSULTING ENGINEERS
2. GEOTECHNICAL INFORMATION:
   ECS SOUTHWEST, LLP (SEE SPECIFICATION 1.02 AND 1.07 ON SHEET 2.00).
3. GENERAL CONTRACTOR:
   ABQ AIRPORT LODGING
4. WALL CONTRACTOR:
   STRUCTURES, INC.
5. GEOTECHNICAL ENGINEER (CONSTRUCTION):
   TO BE DETERMINED
6. UNIT TYPE:
   KEYSTONE COMPAC
7. REINFORCEMENT:
   MIRAFI 3XT



PROJECT VICINITY MAP

NOT TO SCALE

SHEET INDEX

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDPUS.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

Designed By: MRJ	Project: ABQ HOME2SUITES ALBUQUERQUE, NEW MEXICO	Registration No: 22936
Scale: N.T.S.		Project No: 24-0987
Date: NOV 25, 2024	Title: TITLE SHEET	Sheet No: 1.00

CURTIS J. DERICHS
NEW MEXICO
22936
PROFESSIONAL ENGINEER

DERICHS, CURTIS, P.E. Date: 2/13/2025

SECTION 1: GENERAL INFORMATION

1.01 SCOPE OF WORK:

A. THE SCOPE OF WORK FOR THE PROJECT INCLUDES FURNISHING AND INSTALLING A SEGMENTAL BLOCK RETAINING WALL SYSTEM IN ACCORDANCE WITH CIVIL DESIGN PROFESSIONALS' (CDP) DESIGN PLANS AND SPECIFICATIONS. WORK INCLUDES PREPARING THE FOUNDATION SOIL, FURNISHING AND INSTALLING THE LEVELING PAD, CONCRETE RETAINING WALL BLOCKS, DRAINAGE AGGREGATE, AND BACKFILL IN CONFORMANCE WITH THE LINES, GRADES, AND DIMENSIONS SHOW.

B. MULTIPLE CONTRACTORS (FENCE, WALL, GRADING, ETC.) MAY BE INVOLVED IN THE COMPLETION OF THE OVERALL PROJECT. CDP'S DESIGN PLANS DO NOT DEFINE SCOPE OF WORK FOR INDIVIDUAL ENTITIES. SEE CONTRACT DOCUMENTS FOR SPECIFIC DETAILS ON THE SCOPE OF WORK THAT WILL BE PROVIDED BY ALL PARTIES.

1.02 GENERAL NOTES:

A. THE OWNER IS RESPONSIBLE FOR OBTAINING A GEOTECHNICAL INVESTIGATION WITH BORINGS DRILLED ALONG THE RETAINING WALL FOLLOWING INDUSTRY STANDARDS. THE MINIMUM REQUIRED SOIL BORINGS WILL BE PER THE GEOTECHNICAL ENGINEER'S RECOMMENDATION AND SATISFACTION.
   A.1. NCMA SECTION 12.1.1 SUBSURFACE EXPLORATION.
   A.2. AASHTO SECTION 10.4.2 SUBSURFACE EXPLORATION.

B. THE OWNER OR OWNER'S REPRESENTATIVE HAS PROVIDED TESTED SOIL PARAMETERS FOR THE PROPOSED EARTH STRUCTURE. CONSTRUCTION VERIFICATION IS IMPERATIVE TO ENSURE THAT THE SOIL AT THE WALL LOCATIONS IS CONSISTENT WITH THE TESTED SOIL. FAILURE TO VALIDATE CONSISTENCY WITH TESTED PARAMETERS SHALL RENDER THESE PLANS VOID.

C. OWNER SHALL ENSURE THAT RETAINING WALL CONSTRUCTION PLANS ARE DISTRIBUTED TO GENERAL CONTRACTOR, RETAINING WALL CONTRACTOR, SITE CIVIL ENGINEER, GEOTECHNICAL ENGINEERS, INSPECTORS, AND ANY OTHER PERTINENT PARTIES.

D. THE SITE CIVIL ENGINEER SHALL REVIEW THE RETAINING WALL CONSTRUCTION PLAN ELEVATIONS, GRADES, AND DRAINAGE PATTERNS FOR COMPLIANCE WITH THE SITE CIVIL DESIGN PLANS.

1.03 CONSTRUCTION NOTES:

A. THE CONTRACTOR SHALL CALL 811 TO HAVE UTILITIES LOCATED AND ANY OTHER APPLICABLE ENTITY BEFORE BEGINNING WORK. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY ALL LOCATIONS AND DEPTHS OF EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION AND AVOID IMPACTING THEM.

B. THE CONTRACTOR SHALL COORDINATE RELOCATION OF ALL EXISTING CONDUITS AND SERVICES WITH THE UTILITY PROVIDER. IF CONFLICTS EXIST, THE SITE CIVIL ENGINEER SHALL BE CONTACTED IMMEDIATELY.

C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL FEDERAL, STATE, AND LOCAL REQUIREMENTS FOR EXECUTION OF WORK, INCLUDING LOCAL BUILDING INSPECTION AND CURRENT OSHA STANDARDS.

D. THE WORK SHALL BE PERFORMED IN A GENERAL SEQUENCE DEVELOPED BY THE CONTRACTOR IN ACCORDANCE WITH THE REQUIREMENTS OF THE CONTRACT. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND FOR THE SEQUENCES AND PROCEDURES TO BE USED.

E. EXCAVATION SUPPORT, IF REQUIRED, IS THE RESPONSIBILITY OF THE CONTRACTOR, INCLUDING THE STABILITY OF THE EXCAVATION AND ITS INFLUENCE ON ADJACENT PROPERTIES AND STRUCTURES.

F. IF THE CONTRACTOR FINDS A CONFLICT, ERROR, OR DISCREPANCY WITHIN OR BETWEEN THE CONTRACT DOCUMENTS AND DESIGN PLANS, THE CONTRACTOR SHALL IMMEDIATELY REPORT THE ISSUE TO THE RESPECTIVE ENGINEER IN WRITING. THE CONTRACTOR SHALL OBTAIN A WRITTEN INTERPRETATION OR CLARIFICATION FROM THE ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION. WORK DONE BEFORE THE ENGINEER RENDERS A DECISION IS AT THE SOLE RISK OF THE CONTRACTOR.

G. UTILITIES SHALL BE PLACED DURING THE CONSTRUCTION OF THE RETAINING WALL. GEOGRIDS SHALL NOT BE CUT TO INSTALL FUTURE UTILITIES.

1.04 TECHNICAL REFERENCES

A. NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA) DESIGN MANUAL FOR SEGMENTAL RETAINING WALLS - 3RD EDITION (5TH PRINTING - 2012).

B. OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) TECHNICAL MANUAL.

C. UNIFIED SOIL CLASSIFICATION SYSTEM (USCS).

1.05 TESTING REFERENCES

A. AMERICAN SOCIETY OF TESTING MATERIALS (ASTM) INTERNATIONAL.

B. AMERICAN CONCRETE INSTITUTE (ACI).

C. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO).

D. NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA).

1.06 DESIGN INFORMATION

A. *DESIGN METHOD:*	NCMA
B. *RETAINING WALL INFORMATION:*	
UNIT TYPE:	KEYSTONE COMPAC
REINFORCEMENT:	MIRAFI 3XT
PERCENT COVERAGE OF GEOGRID	100%
C. *LOADING CONDITIONS:*	
LIVE LOAD SURCHARGE	0 PSF
DEAD LOAD SURCHARGE (BUILDING)	0 PSF
HYDROSTATIC LOADING	N/A (DRAINAGE PROVIDED)
D. *WALL GEOMETRY:*	
TOE SLOPE:	VARIES (SEE GRADING PLAN)
BACK SLOPE:	VARIES (SEE GRADING PLAN)
BATTER:	7.1°
E. *INTERNAL STABILITY:*	**(MINIMUM FOS)**
GEOGRID STRENGTH	1.5
GEOGRID PULLOUT	1.5
GEOGRID CONNECTION	1.5
BLOCK TO BLOCK SLIDING	1.5
F. *EXTERNAL STABILITY:*	
BASE SLIDING	1.5
BEARING	2.0
OVERTURNING (GRAVITY)	1.5
OVERTURNING (REINFORCED)	2.0
GLOBAL STABILITY	1.3
G. *SEISMIC:*	
SEISMIC CONDITIONS	75% OF STATIC FOS
PEAK GROUND ACCELERATION (A)	0.2g

SEISMIC ANALYSES AND DESIGN HAVE BEEN PERFORMED IN ORDER TO MITIGATE/REDUCE THE RISK OF CATASTROPHIC FAILURE AND RISK TO LIFE AND LIMB. IN THE EVENT OF A SEISMIC EVENT, AESTHETICS AND/OR MINOR DAMAGE MAY OCCUR.

1.07 PROVIDED SOIL PARAMETERS

	Φ	c	γ	SOIL TYPE
REINFORCED BACKFILL	0°	2000 PSF	140 PCF	CONCRETE
RETAINED BACKFILL	40°	0 PSF	120 PCF	SILTY SAND (SM)
FOUNDATION SOIL	40°	0 PSF	120 PCF	SILTY SAND (SM)
LEVELING PAD	40°	0 PSF	135 PCF	AGGREGATE

2.00 GENERAL INFORMATION:

A. GENERAL INFORMATION: DOCUMENTATION OR REFERENCE TO DOCUMENTATION SHALL BE PROVIDED BY THE PROPERTY OWNER TO THE NORTHWEST OF THIS THIS PROJECT SPECIFICALLY ALLOWING CONSTRUCTION OR REPAIRS TO BE INITIATED AND COMPLETED ON THE PROPERTY TO THE NORTHWEST OF THIS PROJECT (REFERENCE SECOND AMENDMENT TO THE CONSTRUCTION AND MAINTENANCE EASEMENT AGREEMENT BETWEEN LESARDE ENTERPRISES, LLC AND ABQ AIRPORT LODGING, LLC).

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

Designed By: MRJ
Scale: N.T.S.
Date: NOV 25, 2024

Project: ABQ HOME2SUITES
ALBUQUERQUE, NEW MEXICO

Title: SPECIFICATIONS: GENERAL INFORMATION

Registration No: 22936
Project No: 24-0987
Sheet No: 2.00

CURTIS J. DERICHS
NEW MEXICO
22936
PROFESSIONAL ENGINEER

DERICHS, CURTIS J., P.E. Date: 2/13/2025

SECTION 2: MATERIALS:

2.01 DEFINITIONS

A. *ADHESIVE*: HIGH STRENGTH CONSTRUCTION ADHESIVE FOR BONDING CONCRETE TO CONCRETE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS.
B. *COLLECTION DRAIN SYSTEM*: A SYSTEM FOR COLLECTING AND REMOVING WATER FROM BEHIND THE RETAINING WALL.
C. *DENSE GRADED AGGREGATE*: LOW PERMEABLE MATERIAL USED TO HELP FACILITATE DRAINAGE THROUGH THE FACE OF THE RETAINING WALL.
D. *DRAINAGE AGGREGATE*: CLEAN CRUSHED ANGULAR STONE LOCATED WITHIN AND DIRECTLY BEHIND THE RETAINING WALL UNITS TO THE DEPTH SPECIFIED ON THE CROSS SECTION, INCLUDING UNIT CORE FILL (IF APPLICABLE).
E. *EXPANSION MATERIAL*: 0.5-INCH FELT EXPANSION BOARD OR POLYSTYRENE FOAM BOARD.
F. *FOUNDATION SOIL*: SOIL IMMEDIATELY BENEATH THE RETAINING WALL LEVELING PAD AND REINFORCED SOIL.
G. *GEOGRID*: A GEOSYNTHETIC MATERIAL MANUFACTURED OF HIGH TENSILE MATERIALS SPECIFICALLY FOR THE PURPOSE OF REINFORCING AND CREATING A STRUCTURAL SOIL MASS.
H. *GEOTEXTILE FABRIC*: A GEOSYNTHETIC MATERIAL MANUFACTURED FOR THE PURPOSE OF SOIL SEPARATION AND DRAINAGE.
I. *LEVELING PAD*: A CONCRETE OR COMPACTED SOIL PAD WHICH SERVES AS A FLAT SURFACE FOR PLACING THE INITIAL COURSE OF UNITS (SEE SPECIFICATION 2.05).
J. *LOW PERMEABLE SOIL CAP*: LOW PERMEABLE SOIL, CONTAINING A MINIMUM OF 40% PASSING THE NO. 200 SIEVE AND A LIQUID LIMIT (LL) AND PLASTICITY INDEX (PI) OF LESS THAN 30 AND 15 RESPECTIVELY, PLACED OVER THE REINFORCED AND/OR RETAINED BACKFILL.
K. *NO FINES CONCRETE*: CONCRETE MATERIAL POURED BEHIND AND WITHIN THE RETAINING WALL UNITS TO CREATE A DEEPER COMPOSITE MASS.
L. *REINFORCED BACKFILL*: SOIL EXTENDING FROM THE BACK OF THE DRAINAGE AGGREGATE TO THE ENDS OF THE EMBEDDED GEOGRID.
M. *RETAINED BACKFILL*: SOIL DIRECTLY BEHIND THE RETAINING WALL REINFORCED BACKFILL. THE RETAINED ZONE IS DEFINED AS A LINE THAT EXTENDS UPWARD AT A 1H:1V FROM THE BACK BASE OF THE REINFORCED ZONE TO THE TOP OF WALL ELEVATION.
N. *RETAINING WALL UNIT (SRW)*: A DRY CAST CONCRETE SEGMENTAL RETAINING WALL FACING UNIT SAWN, CUT, SPLIT, OR OTHERWISE FINISHED OR SHAPED.

2.02 KEYSTONE RETAINING WALL UNITS

A. RETAINING WALL SHALL BE COMPRISED OF KEYSTONE UNITS MANUFACTURED BY A LICENSED PRODUCER.
B. KEYSTONE WALL UNITS SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI. WEIGHT OF CONCRETE SHALL BE 125 PCF. MAXIMUM ABSORPTION OF 13 PCF.
C. BLOCKS SHALL BE FREE OF CRACKS OR OTHER DEFECTS THAT WOULD INTERFERE WITH THE PROPER PLACING OF THE UNIT OR SIGNIFICANTLY IMPAIR THE STRENGTH OR PERFORMANCE OF THE CONSTRUCTION. MINOR CRACKS INCIDENTAL TO THE STANDARD METHOD OF MANUFACTURING OR MINOR CHIPPING RESULTING FROM CUSTOMARY METHODS OF HANDLING IN SHIPMENT AND DELIVERY, ARE NOT GROUNDS FOR REJECTION.
D. FIVE PERCENT OF A SHIPMENT CONTAINING CHIPS NOT LARGER THAN 1-INCH IN ANY DIMENSION, OR CRACKS NOT WIDER THAN 0.02-INCHES AND NOT LONGER THAN 25% OF THE NOMINAL HEIGHT OF THE UNITS IS PERMITTED.
E. THE FACE OF UNITS EXPOSED IN WALL CONSTRUCTION SHALL NOT SHOW CHIPS OR CRACKS, NOT OTHERWISE PERMITTED, OR OTHER IMPERFECTIONS WHEN VIEWED FROM A DISTANCE OF NOT LESS THAN 20-FEET UNDER DIFFUSED LIGHTING.
F. IF PINS OR CLIPS ARE USED BY THE RETAINING WALL SUPPLIER TO INTERCONNECT SRW UNITS, THEY SHALL CONSIST OF NON-DEGRADING POLYMER OR GALVANIZED STEEL AND BE MADE FOR THE EXPRESS USE WITH THE SRW UNITS SUPPLIED.

2.03 GEOTEXTILE FABRIC

A. GEOTEXTILE FABRIC, IF REQUIRED, SHALL BE MIRAFI 180N OR APPROVED EQUAL (8OZ MINIMUM) AND SHALL MEET THE PROPERTIES SPECIFIED BY THE MANUFACTURER.
B. ALL GEOTEXTILE SEAMS SHALL BE OVERLAPPED BY A MINIMUM OF 12-INCHES.

2.04 GEOGRID

A. GEOGRID SHALL BE THE TYPE AND STRENGTH SPECIFIED IN THE CONSTRUCTION DRAWINGS AND SHALL MEET THE PROPERTIES SPECIFIED BY THE MANUFACTURER. ANY SUBSTITUTION REQUIRES WRITTEN APPROVAL BY CDP PRIOR TO WALL CONSTRUCTION.
B. GEOGRID SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

2.05 LEVELING PAD

A. SOIL LEVELING PAD SHALL CONSIST OF COMPACTED SAND, GRAVEL, CRUSHED STONE, OR ANY COMBINATION THEREOF (USE SOIL TYPE GP, GW, SP, OR SW PER USCS).
B. CONCRETE LEVELING PAD MAY BE UNREINFORCED AND SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI. CONCRETE MUST CURE A MINIMUM OF 12-HOURS PRIOR TO PLACING ANY KEYSTONE UNITS.

2.06 COLLECTION DRAIN SYSTEM

A. SUBSURFACE DRAINAGE SYSTEM CONSTRUCTED OF 4" PERFORATED HDPE OR PVC PIPE MANUFACTURED IN ACCORDANCE WITH AASHTO M252 OR ASTM D3034 RESPECTIVELY. IF THE PIPE IS NOT PLACED WITHIN CLEAN CRUSHED STONE IT SHALL BE WRAPPED IN A GEOTEXTILE FABRIC.
B. USE NON-PERFORATED PIPE TO OUTLET THROUGH FACE OF WALL, BENEATH WALL, AND WHEN CONNECTING TO DRAINAGE STRUCTURES.
C. PIPE FITTINGS SHALL BE PER MANUFACTURER'S RECOMMENDATION.

2.07 DRAINAGE AGGREGATE

A. MATERIAL SHALL BE DURABLE CLEAN CRUSHED STONE HAVING AT LEAST TWO FRACTURED FACES AND COMPLY WITH THE FOLLOWING GRADATION:

SIEVE SIZE	% PASSING
1-INCH	100
$\frac{3}{4}$ - INCH	75 - 10
NO. 4	0 - 10
NO. 50	0 - 5

B. ROUNDED AGGREGATE (E.G. RIVER ROCK AND PEA GRAVEL) IS NOT ACCEPTABLE DRAINAGE AGGREGATE MATERIAL.

2.08 FOUNDATION SOIL

A. THE FOUNDATION SOIL IS ASSUMED TO BE SILTY SAND (SM) EXHIBITING A MINIMUM EFFECTIVE INTERNAL FRICTION ANGLE OF 40° AND A COHESION OF 0 PSF.
B. THE FOUNDATION SOIL SHALL BE FREE OF DEBRIS, HIGH PLASTIC CLAY, FROST, ICE, ORGANIC MATTER (<1%), AND OTHER DELETERIOUS MATERIALS.

2.09 REINFORCED BACKFILL

A. MATERIAL SHALL BE CONCRETE EXHIBITING A MINIMUM EFFECTIVE INTERNAL FRICTION ANGLE OF 0° AND A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,000PSI AND A MINIMUM UNIT WEIGHT OF 140 PCF.

2.10 RETAINED BACKFILL

A. MATERIAL SHALL BE SILTY SAND (SM) EXHIBITING A MINIMUM EFFECTIVE INTERNAL FRICTION ANGLE OF 40°, A COHESION OF 0 PSF, AND COMPLY WITH THE FOLLOWING GRADATION:

SIEVE SIZE	% PASSING
2-INCH	100
3/4-INCH	100 - 75
NO. 4	20 - 100
NO. 40	0 - 60
NO. 200	0 - 25

PLASTICITY INDEX (PI)<10
4.5 < PH < 9

B. MATERIAL SHALL BE FREE OF DEBRIS, HIGH PLASTIC CLAY, ICE, ORGANIC MATTER (<1%), AND OTHER DELETERIOUS MATERIALS.
C. MATERIAL MAY BE SITE EXCAVATED WHEN THE ABOVE REQUIREMENTS ARE MET.

2.11 BACK SLOPE FILL MATERIAL

A. MATERIAL SHALL BE APPROVED STRUCTURAL FILL PER THE GEOTECHNICAL ENGINEER AND SHALL EXHIBIT THE MINIMUM EFFECTIVE STRESS PARAMETERS REQUIRED TO PERMANENTLY MAINTAIN SLOPE STABILITY.
B. MATERIAL SHALL BE FREE OF DEBRIS, HIGH PLASTIC CLAY, ICE, ORGANIC MATTER (<1%), AND OTHER DELETERIOUS MATERIALS.

2.12 DENSE GRADED AGGREGATE

A. MATERIAL SHALL EXHIBIT A MINIMUM EFFECTIVE INTERNAL FRICTION ANGLE OF 40° AND COMPLY WITH THE FOLLOWING GRADATION:

SIEVE SIZE	% PASSING
1-INCH	100
$\frac{3}{4}$ - INCH	95 - 100
NO.4	35 - 70
NO. 40	10- 35
NO. 200	5 - 15

PLASTICITY INDEX (PI) < 10
LIQUID LIMIT (LL) < 20

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

Designed By: MRJ
Scale: N.T.S.
Date: NOV 25, 2024

Project: ABQ HOME2SUITES
ALBUQUERQUE, NEW MEXICO

Title: SPECIFICATIONS: MATERIALS

Registration No: 22936
Project No: 24-0987
Sheet No: 2.01

CURTIS J. DERICHS
NEW MEXICO
22936
PROFESSIONAL ENGINEER

DERICHS, CURTIS, P.E. Date: 2/13/2025

SECTION 3: EXECUTION

3.01 CONSTRUCTION STAKING

A. STAKING SHALL BE PERFORMED BY A LICENSED SURVEYOR.
B. WALL STATIONING SHOWN IS RELATIVE TO EACH WALL AND NOT TO ANY OTHER STATIONING SHOWN ON THE CONTRACT PLANS (UNLESS OTHERWISE NOTED). STATION 0.00 IS ON THE LEFT END OF WALL AS VIEWED FROM THE FRONT OF THE WALL (UNLESS OTHERWISE NOTED ON WALL ELEVATION).

3.02 EXCAVATION

A. CONTRACTOR SHALL EXCAVATE TO THE LINES AND GRADES SHOWN ON THE CONSTRUCTION DRAWINGS. CONTRACTOR SHALL BE CAREFUL NOT TO DISTURB EMBANKMENT AND FOUNDATION MATERIALS BEYOND LINES SHOWN. EXCAVATION FOR CONSTRUCTION OF THE RETAINING WALL SHALL CONFORM TO OSHA REQUIREMENTS FOR SAFE EXCAVATION.

3.03 SOIL COMPACTION

A. *COARSE GRAINED SOILS*: ALL COARSE GRAINED SOIL SHALL BE PLACED IN MAXIMUM 9-INCH LOOSE LIFTS AND COMPACTED TO A MINIMUM OF 95% OF ITS STANDARD PROCTOR DENSITY AS DETERMINED BY ASTM D698. THE MOISTURE CONTENT OF THE BACKFILL MATERIAL, PRIOR TO AND DURING CONSTRUCTION, SHALL BE UNIFORMLY DISTRIBUTED THROUGHOUT EACH LAYER AND SHALL BE WITHIN A RANGE OF ±2° OF OPTIMUM MOISTURE CONTENT.
B. *FINE GRAINED SOILS*: ALL FINE GRAINED SOIL SHALL BE PLACED IN MAXIMUM 8-INCH LOOSE LIFTS AND COMPACTED TO A MINIMUM OF 95% OF ITS STANDARD PROCTOR DENSITY AS DETERMINED BY ASTM D698. THE MOISTURE CONTENT OF THE BACKFILL MATERIAL, PRIOR TO AND DURING CONSTRUCTION, SHALL BE UNIFORMLY DISTRIBUTED THROUGHOUT EACH LAYER AND SHALL BE WITHIN A RANGE OF ±2° OF OPTIMUM MOISTURE CONTENT.
C. *CRUSHED STONE*: CRUSHED STONE SHALL BE PLACED IN MAXIMUM 12-INCH LOOSE LIFTS AND COMPACTED WITH A MINIMUM OF THREE (3) PASSES OF A VIBRATORY COMPACTOR (OR OTHER SUITABLE EQUIPMENT) CAPABLE OF EXERTING A MINIMUM OF 3,000 LBS OF CENTRIFUGAL FORCE AND TO THE SATISFACTION OF THE GEOTECHNICAL ENGINEER.
D. D. ONLY LIGHTWEIGHT HAND-OPERATED COMPACTION EQUIPMENT SHALL BE USED WITHIN 3-FEET OF THE BACK OF WALL.

3.04 FOUNDATION SOIL PREPARATION

A. FOUNDATION SOIL SHALL BE EXCAVATED FOR PLACEMENT OF THE LEVELING PAD AND BACKFILL MATERIAL AS SHOWN ON THE CONSTRUCTION DRAWINGS OR AS DIRECTED BY THE GEOTECHNICAL ENGINEER (WHICHEVER IS MORE STRINGENT).
B. FOUNDATION SHALL BE EXAMINED BY THE GEOTECHNICAL ENGINEER TO CONFIRM THAT THE ACTUAL FOUNDATION CONDITIONS MEET OR EXCEED THE DESIGN ASSUMPTIONS. AT A MINIMUM, FOUNDATION SOIL SHALL BE PROOF-ROLLED BEFORE CONSTRUCTION PROCEEDS. SUBGRADE MATERIAL NOT MEETING THE REQUIRED STRENGTH SHALL BE REMOVED AND REPLACED WITH SUITABLE STRUCTURAL FILL PER THE GEOTECHNICAL ENGINEER.
C. OVER-EXCAVATED AREAS SHALL BE BACKFILLED WITH APPROVED STRUCTURAL FILL AND COMPACTED AS PER S*PECIFICATION 3.03*.

3.05 BASE LEVELING PAD

A. LEVELING PAD MATERIALS SHALL BE PLACED UPON AN APPROVED FOUNDATION AS SHOWN ON THE CONSTRUCTION DRAWINGS TO A MINIMUM THICKNESS OF --- INCHES, OR AS SHOWN ON WALL PROFILE SHEETS, AND COMPACTED AS PER S*PECIFICATION 3.03 (SOIL LEVELING PAD).*
B. LEVELING PAD SHALL BE PREPARED TO ENSURE COMPLETE CONTACT WITH THE BASE RETAINING WALL UNITS.

3.06 MODULAR WALL UNIT INSTALLATION

A. THE FIRST COURSE OF KEYSTONE UNITS SHALL BE CAREFULLY PLACED ON TOP OF AND IN FULL CONTACT WITH THE LEVELING PAD. THE BASE UNITS SHALL BE CLOSELY ABUTTED TOGETHER AT THE LOCATIONS AND ELEVATIONS SHOWN ON THE APPROVED CONSTRUCTION DRAWINGS. THE HORIZONTAL GAP BETWEEN UNITS SHALL NOT EXCEED 1/4-INCH. EACH UNIT SHALL BE CHECKED FOR PROPER ELEVATION, ALIGNMENT, AND THAT IT IS LEVEL.
B. UNITS ARE PLACED SIDE BY SIDE FOR FULL LENGTH OF WALL ALIGNMENT. ALIGNMENT MAY BE ACHIEVED WITH THE AID OF A STRING LINE OR OFFSET FROM A BASE LINE.
C. DRAINAGE AGGREGATE SHALL BE PLACED BETWEEN THE UNITS, WITHIN THE UNITS (UNIT CORE FILL, IF APPLICABLE), AND IMMEDIATELY BEHIND THE UNITS TO THE DRAINAGE ZONE DEPTH SPECIFIED ON THE CROSS SECTION. THE DRAINAGE AGGREGATE SHALL BE PLACED AND COMPACTED AS PER S*PECIFICATION 3.03*.
D. PRIOR TO PROCEEDING TO THE NEXT COURSE, SWEEP EXCESS MATERIAL FROM TOP OF UNITS.
E. UNITS MAY BE SHIMMED WITH GEOGRID REINFORCEMENT, ASPHALT ROOFING SHINGLES, OR ROLLED ROOFING TO MAINTAIN FACE BATTER AND UNIFORM BLOCK ELEVATIONS. SHIMMING MATERIAL SHALL MAINTAIN A MAXIMUM NOMINAL THICKNESS OF 1/8-INCH AND SHALL ONLY OCCUR AT COURSES OF BLOCK WHERE PRIMARY LAYERS OF REINFORCEMENT ARE NOT PRESENT. CONTRACTOR SHALL TAKE PRECAUTIONS TO CONTINUOUSLY SUPPORT A SHIMMED UNIT TO PREVENT POINT LOADING THAT MAY INDUCE UNIT CRACKING. THE WALL CONTRACTOR MAY ALSO GRIND UNITS AS NECESSARY TO MAINTAIN FACE BATTER AND UNIFORM BLOCK ELEVATIONS.
F. SEE MANUFACTURER'S INFORMATION FOR ADDITIONAL INSTALLATION REQUIREMENTS.

3.07 COLLECTION DRAIN PLACEMENT

A. INSTALL COLLECTION DRAIN SYSTEM PER THE APPROVED CONSTRUCTION SHOP DRAWINGS.
B. THE COLLECTION DRAIN SYSTEM SHALL DRAIN SURFACE WATER INFILTRATION AND GROUNDWATER AWAY FROM THE REINFORCED AND RETAINED BACKFILL ZONES. THE COLLECTION DRAIN SYSTEM SHALL OUTLET INDEPENDENT OF THE STORM DRAIN SYSTEMS AT LOCATIONS THAT MOVE THE WATER AWAY FROM THE WALL WHEREVER POSSIBLE.
C. THE DRAIN PIPE SHALL BE PLACED TO MAINTAIN GRAVITY FLOW (1% MINIMUM).
D. DO NOT CONNECT ANY OTHER DRAINS INTO THE COLLECTION DRAIN SYSTEM.
E. DO NOT OUTLET UPPER TIERED WALL COLLECTION DRAIN SYSTEM ONTO LOWER TIERED WALL.
F. TIERED WALL DRAINS SHALL REMAIN INDEPENDENT OF EACH OTHER.

3.08 CONCRETE INSTALLATION

A. POUR CONCRETE IN MAXIMUM 8" LIFTS TO THE DEPTHS AND ELEVATION SPECIFIED ON WALL ELEVATION PLAN SHEETS. PLACE GEOTEXTILE FILTER FABRIC BETWEEN NO FINES CONCRETE AND BOTH THE FOUNDATION AND RETAINED MATERIAL.

3.09 GEOGRID INSTALLATION

A. GEOGRID SHALL BE ORIENTED WITH THE HIGHEST STRENGTH AXIS PERPENDICULAR TO THE WALL ALIGNMENT. CORRECT ORIENTATION (ROLL DIRECTION) SHALL BE VERIFIED BY CONTRACTOR PRIOR TO WALL INSTALLATION.
B. GEOGRID SHALL BE PLACED AT THE ELEVATIONS AND TO THE EXTENTS SHOWN ON THE CONSTRUCTION DRAWINGS OR AS DIRECTED BY THE ENGINEER. GEOGRID SHALL BE PLACED IN A MANNER TO ENSURE 100% COVERAGE PARALLEL TO THE WALL FACE.
C. THE GEOGRID SHALL BE LAID HORIZONTALLY ON COMPACTED BACKFILL. THE GEOGRID SHALL BE PULLED TAUT (50LBS/FT) TO ELIMINATE LOOSE FOLDS AND PRETENSION THE REINFORCEMENT. STAKE OR SECURE THE BACK EDGE OF THE GEOGRID PRIOR TO BACKFILLING AND COMPACTION.
D. GEOGRID LAYERS SHALL BE ONE CONTINUOUS PIECE FOR THEIR ENTIRE EMBEDMENT LENGTH. OVERLAP OR SPLICING OF THE GEOGRID IN THE DESIGN STRENGTH DIRECTION (PERPENDICULAR TO THE WALL FACE) SHALL NOT BE PERMITTED.
E. TRACKED OR RUBBER TIRE EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEOGRID REINFORCEMENT. A MINIMUM FILL THICKNESS OF 6-INCHES IS REQUIRED PRIOR TO OPERATION OF EQUIPMENT OVER THE GEOGRID REINFORCEMENT. TURNING OF EQUIPMENT SHOULD BE KEPT TO A MINIMUM TO PREVENT DISPLACING THE FILL AND DAMAGING OR MOVING THE GEOGRID REINFORCEMENT.
F. NO CHANGES TO GEOGRID WITHOUT WRITTEN APPROVAL OF CDP: INCLUDING, BUT NOT LIMITED TO, LAYOUT, LENGTH, TYPE, OR ELEVATION SHALL BE PERMITTED.
G. SEE MANUFACTURER'S INFORMATION FOR ADDITIONAL REQUIREMENTS REGARDING THE GEOGRID INSTALLATION.

3.10 REINFORCED BACKFILL PLACEMENT

A. REINFORCED BACKFILL MATERIAL SHALL BE PLACED, SPREAD, AND COMPACTED IN SUCH A MANNER THAT MINIMIZES THE DEVELOPMENT OF SLACK IN THE GEOGRID.
B. REINFORCED BACKFILL MATERIAL SHALL BE PLACED AND COMPACTED AS PER S*PECIFICATION 3.03* OR PER THE GEOTECHNICAL ENGINEERS RECOMMENDATION (WHICHEVER IS MORE STRINGENT).

3.11 RETAINED BACKFILL PLACEMENT

A. RETAINED BACKFILL MATERIAL SHALL BE PLACED AND COMPACTED AS PER *SPECIFICATION 3.03* OR PER THE GEOTECHNICAL ENGINEERS RECOMMENDATION (WHICHEVER IS MORE STRINGENT).

3.12 BACK SLOPE PLACEMENT

A. ALL BACKFILL PLACED ABOVE THE REINFORCED AND RETAINED BACKFILL SHALL BE PLACED AND COMPACTED AS PER SPECIFICATION 3.03 OR PER THE GEOTECHNICAL ENGINEERS RECOMMENDATION (WHICHEVER IS MORE STRINGENT).

3.13 CAPSTONE AND FREESTANDING UNIT INSTALLATION

A. PRIOR TO PLACING CAPSTONES AND FREESTANDING UNITS, CLEAN BLOCK AND APPLY ADHESIVE PER THE MANUFACTURER'S RECOMMENDATIONS.
B. TRIM SIDES OF INTERIOR CAPSTONES TO ENSURE PROPER FIT OF WALL CAPSTONE. DO NOT LEAVE CUT SURFACES EXPOSED TO VIEW IN THE FINISHED WALL.
C. ADHESIVE SHALL BE DESIGNED TO WITHSTAND MOISTURE AND TEMPERATURE EXTREMES, REMAIN FLEXIBLE, AND SPECIFICALLY FORMULATED FOR BONDING MASONRY TO MASONRY.

3.14 SITE DRAINAGE

A. POOR PERFORMANCE AND/OR FAILURE OF RETAINING WALLS DURING AND AFTER CONSTRUCTION CAN OCCUR IF UNANTICIPATED STORM WATER IMPACTS THE WALL. THEREFORE IT IS CRITICAL THAT ANY POTENTIAL DRAINAGE ISSUES THAT BECOME APPARENT DURING OR AFTER CONSTRUCTION BE ADDRESSED IMMEDIATELY TO AVOID RETAINING WALL PERFORMANCE ISSUES.
B. CONTRACTOR SHALL PROTECT RETAINING WALLS AGAINST SURFACE WATER RUNOFF FROM ADJACENT AREAS AT ALL TIMES THROUGH THE USE OF BERMS, DIVERSION DITCHES, TEMPORARY DRAINS, OR ANY OTHER MEANS NECESSARY.
C. AT THE END OF EACH DAYS OPERATION, THE CONTRACTOR SHALL SLOPE THE LAST LIFT AWAY FROM THE WALL FACING TO DIRECT RUNOFF AWAY FROM THE WALL.
D. AT COMPLETION OF WALL CONSTRUCTION, BACKFILL SHALL BE PLACED LEVEL WITH FINAL TOP OF WALL ELEVATION. IF FINAL GRADING, PAVING, LANDSCAPING, OR STORM DRAINAGE INSTALLATIONS ADJACENT TO THE WALL ARE NOT PLACED IMMEDIATELY AFTER THE WALL CONSTRUCTION IS COMPLETED, TEMPORARY GRADING AND DRAINAGE SHALL BE PROVIDED TO ENSURE WATER RUNOFF IS NOT DIRECTED TOWARDS THE WALL NOR ALLOWED TO COLLECT OR POND BEHIND THE WALL UNTIL FINAL CONSTRUCTION ADJACENT TO THE WALL IS COMPLETED.
E. ALL SLOPES ABOVE OR BELOW THE RETAINING WALL SHALL BE IMMEDIATELY VEGETATED AND PROTECTED FROM EROSION. SLOPES ABOVE THE RETAINING WALL STEEPER THAN AN 8H:1V SHALL HAVE A SILT FENCE INSTALLED AND MAINTAINED UNTIL ADEQUATE VEGETATION CAN BE ESTABLISHED.
F. THE RETAINING WALLS ARE NOT DESIGNED TO RESIST CONCENTRATED FLOWS: INCLUDING, BUT NOT LIMITED TO, DOWNSPOUTS, SUMP PUMPS, AND SWALES. ALL CONCENTRATED FLOWS SHALL BE COLLECTED IN A SUB-DRAIN SYSTEM, DIRECTED AWAY FROM AND/OR AROUND THE ENDS OF RETAINING WALLS. ANY CHANGE IN SURFACE WATER DIRECTION OR CONNECTION INTO SITE STORM SEWER SYSTEM SHALL BE SUBMITTED TO THE SITE CIVIL ENGINEER FOR REVIEW AND APPROVAL.

3.15 AS-BUILT CONSTRUCTION TOLERANCES:

A. *HORIZONTAL ALIGNMENT*: ±0.75-INCHES OVER ANY 10-FOOT DISTANCE; 3-INCHES MAXIMUM.
B. *WALL BATTER*: WITHIN ±1° (NOT LESS THAN 0°) OF DESIGN BATTER.
C. *CORNERS, BENDS, AND CURVES*: ±2-FEET FROM THEORETICAL POSITION.
D. *MAXIMUM DIFFERENTIAL SETTLEMENT*: L/200 (0.5% OF REFERENCES LENGTH)).
E. *TOTAL SETTLEMENT*: 2-INCHES MAXIMUM.

3.16 CONSTRUCTION ADJACENT TO WALL:

A. THE OWNER OR OWNER'S REPRESENTATIVE IS RESPONSIBLE FOR ENSURING THAT CONSTRUCTION ADJACENT TO THE WALL DURING AND POST CONSTRUCTION WILL NOT DISTURB THE WALL OR PLACE TEMPORARY OR PERMANENT LOADS ON THE WALL EXCEEDING THE DESIGN LOADS, INCLUDING BUT NOT LIMITED TO WATER PRESSURE, TEMPORARY GRADES, EQUIPMENT LOADING, AND FUTURE STRUCTURES.

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

Designed By: MRJ
Scale: N.T.S.
Date: NOV 25, 2024

Project: ABQ HOME2SUITES ALBUQUERQUE, NEW MEXICO

Title: SPECIFICATIONS: EXECUTION

Registration No: 22936
Project No: 24-0987
Sheet No: 2.02

DERICHS, CURT J, P.E. Date: 2/13/2025

SECTION 4: QUALITY ASSURANCE

4.01 OWNER/GENERAL CONTRACTOR:

A. THE OWNER OR GENERAL CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND CONTRACTING FOR PROFESSIONAL GEOTECHNICAL ENGINEERING AND RETAINING WALL CONSTRUCTION INSPECTION SERVICES PER S*PECIFICATION 4.02 AND 4.03* RESPECTIVELY.

B. THE OWNER SHALL BE RESPONSIBLE FOR LONG TERM MAINTENANCE (SEE SPECIFICATION 4.05)

4.02 PROFESSIONAL GEOTECHNICAL ENGINEER:

A. THE GEOTECHNICAL ENGINEER SHALL OBSERVE SITE SOIL CONDITIONS FOR COMPLIANCE WITH RETAINING WALL DESIGN PLANS PRIOR TO WALL CONSTRUCTION. IF THE GEOTECHNICAL ENGINEER DETERMINES THE ONSITE SOIL WILL NOT EXHIBIT THE EFFECTIVE STRESS PARAMETERS ASSUMED IN THE DESIGN PLANS, THE WALL CONSTRUCTION SHALL NOT COMMENCE UNTIL AN APPROPRIATE SOLUTION IS DETERMINED.

B. THE GEOTECHNICAL ENGINEER SHALL INSPECT AND EVALUATE THE FOUNDATION SOILS AT THE RETAINING WALL LOCATIONS, PRIOR TO CONSTRUCTION, TO ENSURE THEY WILL SAFELY SUPPORT THE MAXIMUM APPLIED LOADS PROVIDED ON THE WALL PROFILES WITHOUT FAILURE OR EXCESSIVE DIFFERENTIAL SETTLEMENT PER SPECIFICATION 3.14D. ANY UNSUITABLE SOIL OR IMPROPERLY COMPACTED EMBANKMENT MATERIAL SHALL BE REMOVED AND REPLACED AS DIRECTED BY THE GEOTECHNICAL ENGINEER TO ACHIEVE ADEQUATE BEARING CAPACITY AND ACCEPTABLE SETTLEMENT LIMITS.

C. THE GEOTECHNICAL ENGINEER SHALL INSPECT WALL EXCAVATION AND RETAINED SOILS FOR GROUNDWATER AND SEEPAGE. IF EITHER CONDITION IS OBSERVED, THE GEOTECHNICAL ENGINEER SHALL IMMEDIATELY HALT THE RETAINING WALL CONSTRUCTION AND NOTIFY CDP.

D. WALL BACKFILL MATERIAL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER FOR COMPLIANCE WITH THE MINIMUM STRENGTH ASSUMPTIONS AND GRADATION LIMITS PER SECTION 2 OF THESE SPECIFICATIONS.

E. WALL BACKFILL SOIL SHALL BE TESTED BY THE GEOTECHNICAL ENGINEER FOR MOISTURE, DENSITY, AND COMPACTION EVERY 2-FEET VERTICALLY, 100-FEET TO 200-FEET C/C, OR PER THE PROJECT SPECIFICATIONS, IF MORE STRINGENT, TO ENSURE COMPLIANCE WITH THE MINIMUM COMPACTION REQUIREMENTS IN *SPECIFICATION 3.03*.

4.03 RETAINING WALL CONSTRUCTION INSPECTOR:

A. THE RETAINING WALL CONSTRUCTION SHALL BE INSPECTED BY A LICENSED PROFESSIONAL ENGINEER OR QUALIFIED TECHNICIAN (IF NOT THE GEOTECHNICAL ENGINEER). THE INSPECTOR SHALL HAVE ADEQUATE KNOWLEDGE OF THE PROJECT AND BE FAMILIAR WITH THE MEANS AND METHODS OF RETAINING WALL CONSTRUCTION. IF THE RETAINING WALL INSPECTOR IS NOT EMPLOYED BY THE GEOTECHNICAL ENGINEER, THE GEOTECHNICAL ENGINEER SHALL BE CONSULTED IN THOSE MATTERS PERTAINING TO SOIL CONDITIONS AND WALL PERFORMANCE.

B. THE INSPECTOR IS RESPONSIBLE FOR READING AND UNDERSTANDING THE RETAINING WALL DESIGN AND CONSTRUCTION PLANS AND SPECIFICATIONS. THE INSPECTOR SHALL BE IN POSSESSION OF A COMPLETE SET OF THESE DOCUMENTS WHEN PERFORMING ON-SITE INSPECTIONS.

C. THE INSPECTOR SHALL INSPECT THE RETAINING WALL UNITS, GEOGRID (TYPE, PLACEMENT, ORIENTATION, AND DEPTH), WALL ELEVATIONS, GRADES, BACK SLOPE, AND TOE SLOPE CONDITIONS FOR CONFORMANCE WITH THE APPROVED SHOP DRAWINGS.

D. THE INSPECTOR SHALL IMMEDIATELY NOTIFY THE WALL CONTRACTOR OF ANY DEFICIENCIES DISCOVERED IN THE RETAINING WALL INSTALLATION AND PROVIDE THE CONTRACTOR A REASONABLE OPPORTUNITY TO CORRECT THE DEFICIENCY.

E. THE INSPECTOR SHALL NOTIFY THE GENERAL CONTRACTOR, OWNER, AND CDP OF ANY CONSTRUCTION DEFICIENCIES THAT HAVE NOT BEEN CORRECTED IN A TIMELY MANNER.

F. THE INSPECTOR SHALL DOCUMENT AND MAINTAIN RECORDS OF ALL INSPECTION RESULTS.

4.04 RETAINING WALL CONTRACTOR:

A. PRIOR TO PLACING THE BLOCKS, THE CONTRACTOR SHALL INSPECT THE RETAINING WALL UNITS TO ENSURE THEY DO NOT CONTAIN ANY VISIBLE DEFECTS PER SPECIFICATION 2.02C.

B. THE CONTRACTOR SHALL ESTABLISH AND MAINTAIN QUALITY CONTROL FOR THE CONSTRUCTION OF THE RETAINING WALL TO ENSURE COMPLIANCE WITH THE CONTRACT REQUIREMENTS. THIS INCLUDES, BUT IS NOT LIMITED TO, TAKING PHOTOGRAPHS THROUGHOUT THE WALL CONSTRUCTION AND MAINTAINING ALL QUALITY CONTROL RECORDS.

4.05 MAINTENANCE:

A. THE RETAINING WALL(S) SHOULD BE INSPECTED EVERY SIX MONTHS FOR MOVEMENT, SOIL TENSION CRACKS, EROSION ADJACENT TO THE RETAINING WALL STRUCTURES, AND FOR SURFICIAL SLOPE STABILITY WHEN A SLOPE EXISTS ABOVE OR BELOW THE RETAINING WALL(S).

B. SURFICIAL SLOPE INSTABILITY TYPICALLY IMPACTS THE UPPER 3 TO 5 FEET OF THE SUBSURFACE PROFILE. REGULAR MAINTENANCE SHOULD BE ANTICIPATED TO IDENTIFY AND ADDRESS POTENTIAL SOIL CREEP OR EROSION. THIS INCLUDES REPLACING OR REPLANTING TREES AND GRASSES, AS NECESSARY, AND GRADING THE SLOPE TO REDUCE SOIL CREEP AND EROSION. IF FUTURE SURFICIAL SLOPE EROSION OCCURS, CDP RECOMMENDS THE SLOPE FACE BE RESTORED AS SOON AS PRACTICAL. CDP ALSO RECOMMENDS IRRIGATED LANDSCAPING BE SETBACK A MINIMUM OF 20-FEET FROM THE CREST OF THE SLOPES.

C. FILL SLOPES SHOULD BE RE-VEGETATED AS SOON AS POSSIBLE AFTER GRADING AND PROTECTED FROM EROSION UNTIL VEGETATION IS ESTABLISHED. SLOPE PLANTING SHOULD CONSIST OF GROUND COVER, SHRUBS, AND TREES POSSESSING DEEP, DENSE ROOT STRUCTURES THAT REQUIRE MINIMUM IRRIGATION.

4.06 CONFORMANCE LETTER:

A. IF A CONFORMANCE LETTER IS REQUIRED, IT MAY BE PROVIDED UNDER A SEPARATE PROPOSAL.

B. IN ORDER TO PROVIDE A CONSTRUCTION CONFORMANCE LETTER, THE FOLLOWING ITEMS WILL BE REQUIRED:

B.1. FOUNDATION INSPECTION/TESTING
B.2. SOIL PROPERTY TESTING (SIEVE, SHEAR, MOISTURE, ETC.)
B.3. SOIL COMPACTION TESTING
B.4. INSPECTION LOGS
B.5. CONSTRUCTION PHOTOGRAPHS, INCLUDING: EXCAVATION, LEVELING PAD, DRAINPIPE AND OUTLETS, BLOCK PLACEMENT AND STEPS, FILTER FABRIC AND/OR GEOGRID REINFORCEMENT (IF APPLICABLE), FINISHED GEOMETRY (BACK SLOPE, TOE SLOPE, BATTER, ETC.)

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

Designed By: MRJ
Scale: N.T.S.
Date: NOV 25, 2024

Project: ABQ HOME2SUITES
ALBUQUERQUE, NEW MEXICO

Title: SPECIFICATIONS:
QUALITY ASSURANCE

Registration No: 22936
Project No: 24-0987
Sheet No: 2.03

CURTIS J. DERICHS
NEW MEXICO
22936
PROFESSIONAL ENGINEER

DERICHS, CURTIS, P.E. Date: 2/13/2025

NOTES:

1. THE SITE PLAN SHOWN WAS REPRODUCED FROM CLARK CONSULTING ENGINEERS PLAN DATED 10/26/2021 (PROJECT NO: PATER_H2SH). BASE OF EXISTING WALLS 1 AND 2 AND PROPOSED WALL 4 SHALL BE FINISHED GRADED BELOW BASE OF WALLS AS SHOWN WITH A 3H:1V (MAX) SLOPE. THE BASE OF WALLS 1 & 2 SHALL HAVE A MINIMUM OF 16" EMBEDMENT AFTER FINISH GRADING.

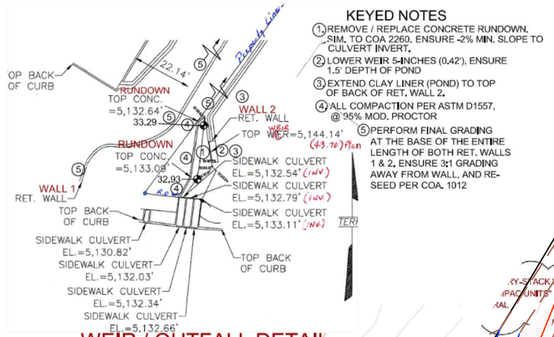
2. HORIZONTAL CONTROL FOR THE RETAINING WALL(S) SHALL BE AS INDICATED ON THE CIVIL PLANS.

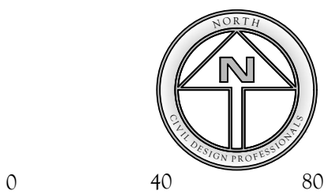
3. THE APPROXIMATE LOCATION OF UTILITIES KNOWN TO EXIST AS SHOWN ON THE PLANS ARE BASED ON THE BEST INFORMATION AVAILABLE AT THE TIME OF PLAN PREPARATION.

Know what's **below.**
Call before you dig.

LEGEND:

- PROPOSED WALL 4
- EXISTING WALLS
- PROPERTY LINE
- PROPOSED CONTOURS
- GEOGRID LIMITS





0 40 80 120

GRAPHIC SCALE IN FEET

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

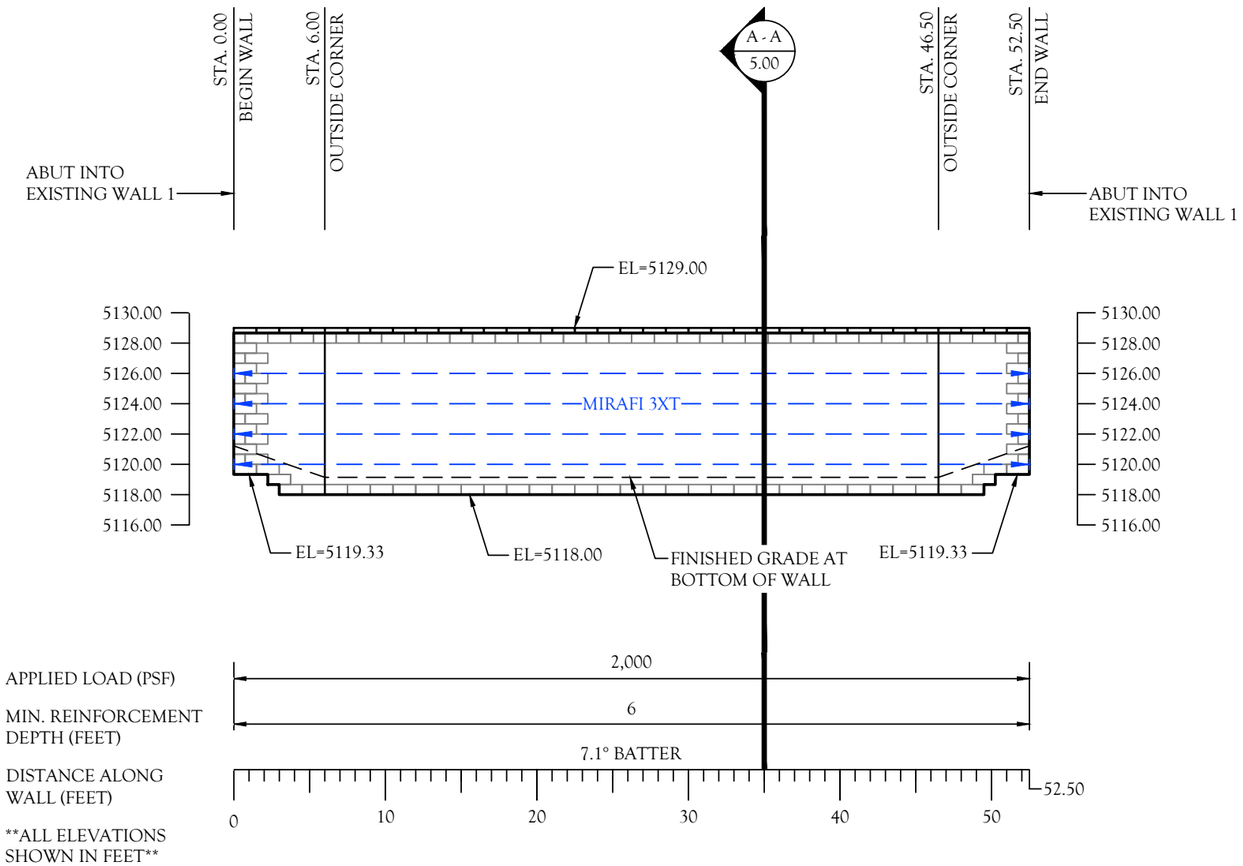
Designed By: MRJ	Project: ABQ HOME2SUITES ALBUQUERQUE, NEW MEXICO	Registration No: 22936
Scale: 1" = 40'		Project No: 24-0987
Date: NOV 25, 2024	Title: SITE PLAN	Sheet No: 3.00

CURTIS J. DERICHS
NEW MEXICO
22936
PROFESSIONAL ENGINEER

DERICHS, CURTIS J, P.E. Date: 2/13/2025

GENERAL NOTES:

1. ABUT WALL 4 INTO EXISTING WALL 1.
2. REMOVE CAP AND TOP UNIT OF EXISTING WALL 1 AND GRADE OVER TOP OF WALL 1.
3. POUR CONCRETE TO WITHIN 18" OF THE TOP OF THE WALL.



KEYSTONE WALL 4 - FRONT FACE ELEVATION

CIVIL DESIGN
PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

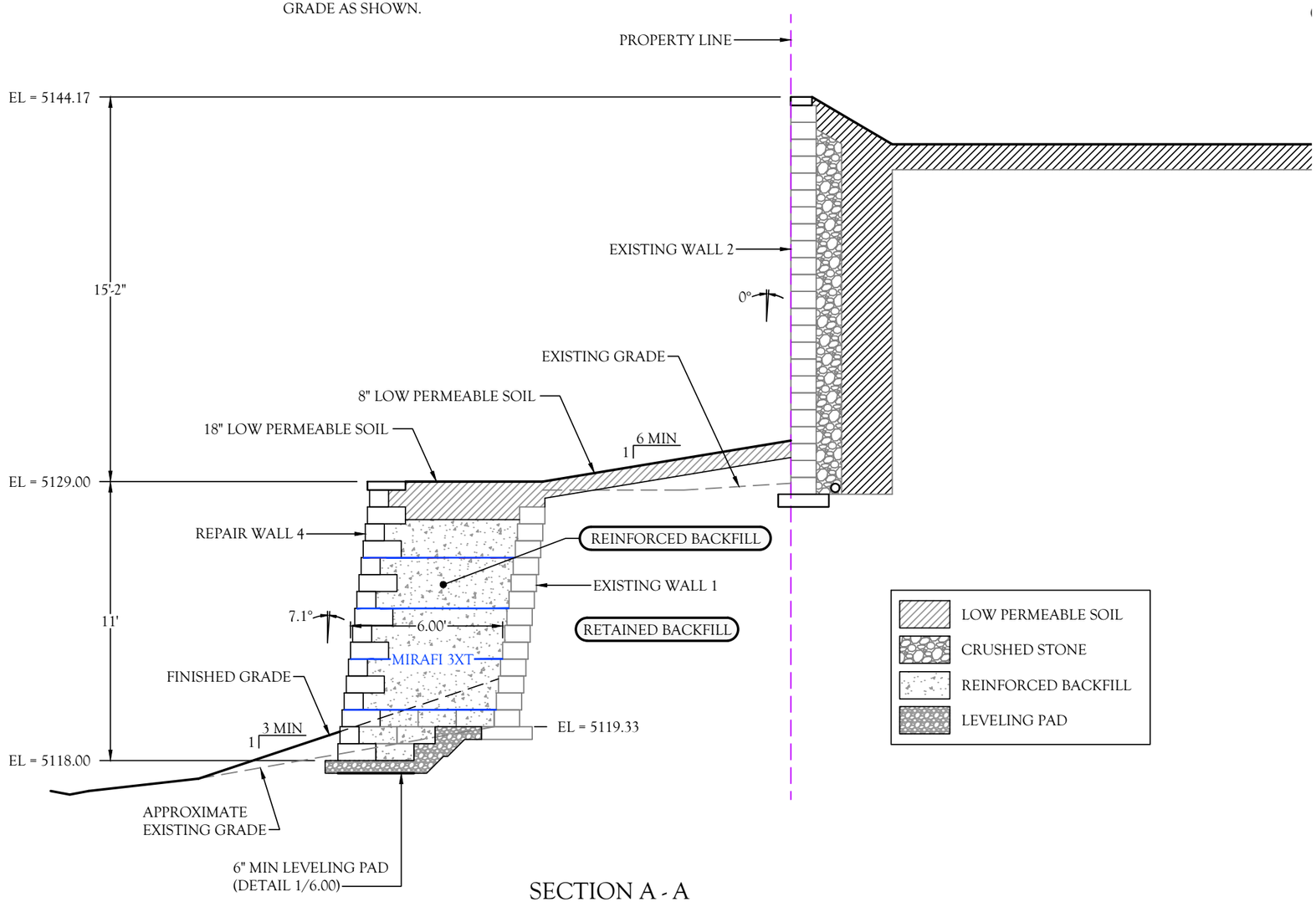
Designed By: MRJ	Project: ABQ HOME2SUITES ALBUQUERQUE, NEW MEXICO	Registration No: 22936
Scale: 1" = 10'		Project No: 24-0987
Date: NOV 25, 2024	Title: WALL 4 ELEVATION	Sheet No: 4.00

CURTIS J. DERICHS
NEW MEXICO
22936
PROFESSIONAL ENGINEER

DERICHS, CURTIS J, P.E. Date: 2/13/2025

GENERAL NOTES:

1. THE SECTION SHOWN IS A REPRESENTATIVE WALL SECTION. THE WALL HEIGHTS, ELEVATIONS, TOE SLOPES, BACK SLOPES, AND EMBEDMENT DEPTHS VARY ACCORDING TO THE ELEVATION PLAN AND SITE PLAN RESPECTIVELY. SECTIONS AND DETAILS APPLY TO SAME AND SIMILAR CONDITIONS UNLESS SPECIFICALLY NOTED OTHERWISE.
2. THE WALL SHALL BE BACKFILLED WITH CONCRETE (MAXIMUM 8" LIFTS).
3. REMOVE THE TOP UNIT AND CAP OF EXISTING WALL 1 BEHIND WALL 4 AND FINISHED GRADE AS SHOWN.



SECTION A - A
(WALL 4 SHOWN AT 35')

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

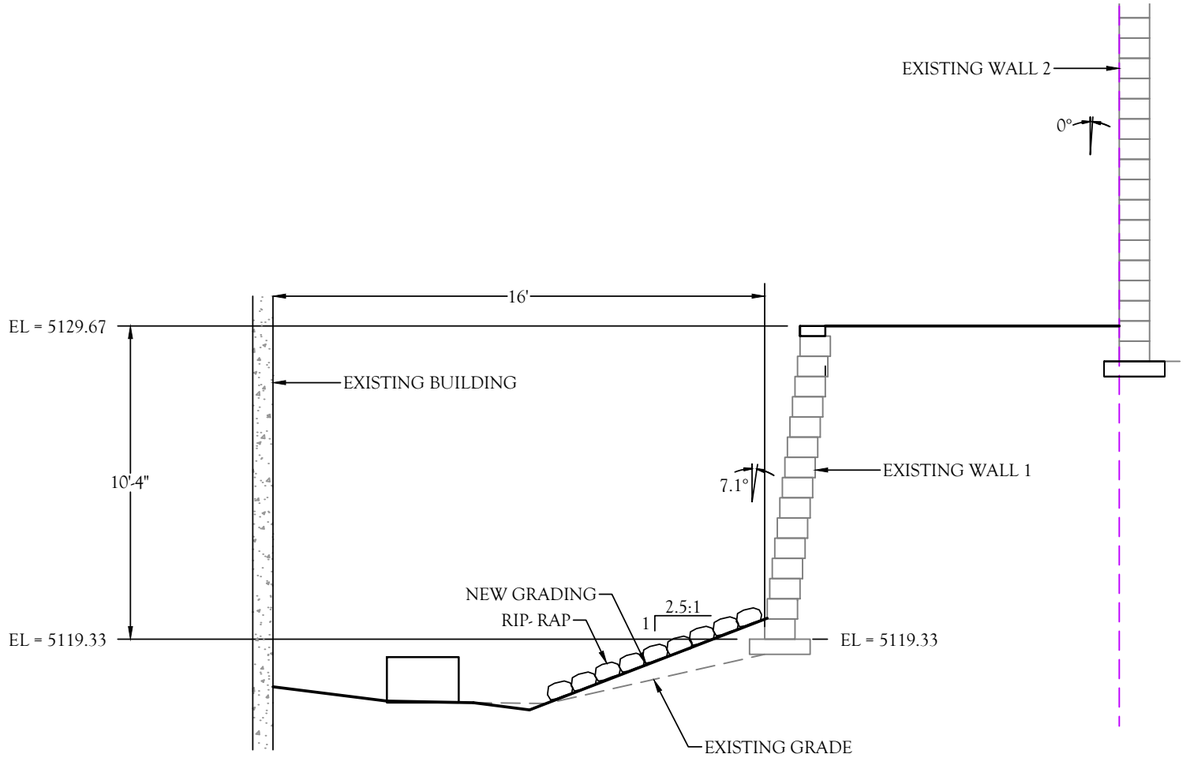
Designed By: MRJ	Project: ABQ HOME2SUITES ALBUQUERQUE, NEW MEXICO	Registration No: 22936
Scale: 1" = 5'		Project No: 24-0987
Date: NOV 25, 2024	Title: SECTION A - A	Sheet No: 5.00

CURTIS J. DERICHS NEW MEXICO 22936 PROFESSIONAL ENGINEER

DERICHS, CURTIS J, P.E. Date: 2/13/2025

GENERAL NOTES:

1. FINISHED GRADE BELOW THE WALL AS PER THE GRADING PLAN TO A MAX SLOPE OF 2..5H:IV. GRADE TO ESTABLISH A MINIMUM EMBEDMENT OF 12" AT THE BASE OF WALL.
2. FINISH GRADE TO CREATE SWALE THAT DIRECTS DRAINAGE AWAY FROM BUILDING AND INTO CONCRETE CHANNEL NEAR PARKING LOT AREA.
3. RIP-RAP BASE OF SLOPE BENEATH THE WALL AS SHOWN.



SECTION B - B
(WALL 1 SHOWN AT 128')

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

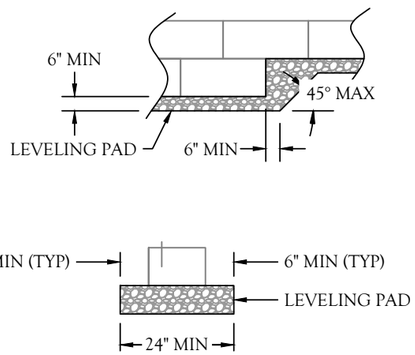
Designed By: MRJ	Project: ABQ HOME2SUITES ALBUQUERQUE, NEW MEXICO
Scale: 1" = 5'	Title: SECTION B - B
Date: NOV 25, 2024	Registration No: 22936
	Project No: 24-0987
	Sheet No: 5.00

CURTIS J. DERICHS NEW MEXICO 22936 PROFESSIONAL ENGINEER

DERICHS, CURTIS, P.E. Date: 2/13/2025

NOTES:

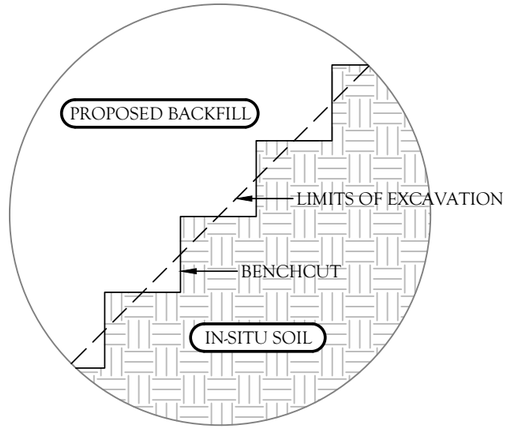
1. THE LEVELING PAD SHALL BE PER SPECIFICATION 2.05.
2. THE BASE FOUNDATION SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF THE LEVELING PAD.
3. IF THE LEVELING PAD IS AT AN ELEVATION LOWER THAN THE 100-YEAR HIGH WATER LEVEL IN FRONT OF THE WALL, THE LEVELING PAD, UNLESS CONCRETE, SHALL BE WRAPPED WITH AN 8OZ. MINIMUM FILTER FABRIC.



1 6.00 LEVELING PAD
SCALE: N.T.S.

NOTES:

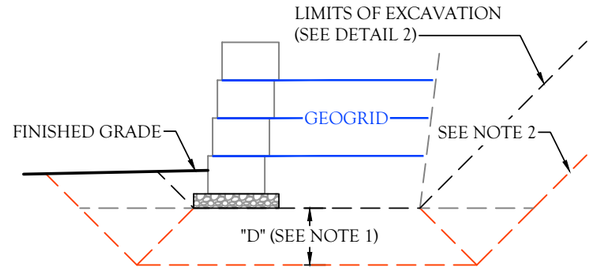
1. DURING WALL EXCAVATION, BENCHCUT LIMITS OF EXCAVATION, WHEN ONSITE SOILS ALLOW, TO INCREASE BOND BETWEEN THE IN-SITU SOILS AND NEW BACKFILL.
2. BENCHCUT USING MINIMUM 24-INCH HORIZONTAL BENCHES (TYPICAL).



2 6.00 TYPICAL BENCHCUT DETAIL
SCALE: N.T.S.

NOTES:

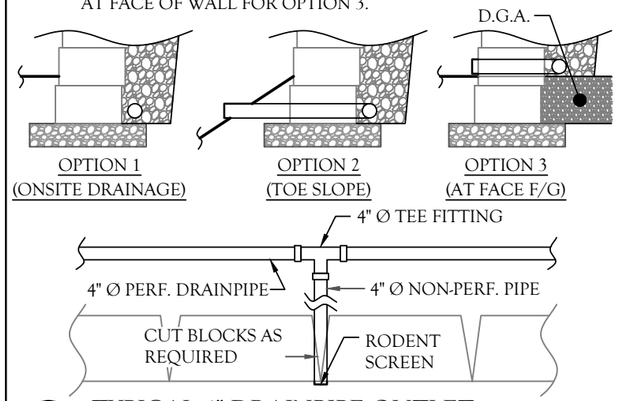
1. UPON EXCAVATION, WHERE UNSUITABLE SOILS ARE FOUND, SUBCUT TO DEPTH "D" AS REQUIRED BY THE ONSITE GEOTECHNICAL ENGINEER AND REPLACE WITH SUITABLE COMPACTED STRUCTURAL FILL TO ACHIEVE THE REQUIRED BEARING CAPACITY.
2. APPROXIMATE LIMITS OF EXCAVATION VARIES WHERE SUBCUT IS REQUIRED. ACTUAL LIMITS AND SIDE SLOPES SHALL BE DETERMINED BY OSHA REGULATIONS OR THE ONSITE GEOTECHNICAL ENGINEER.



3 6.00 TYPICAL REINF. WALL SUBCUT DETAIL
SCALE: N.T.S.

NOTES:

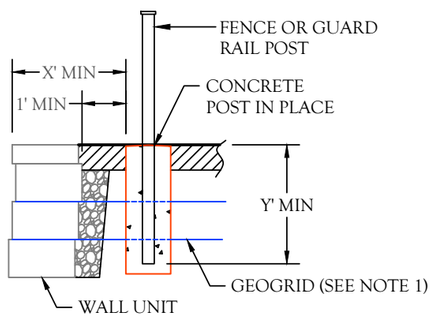
1. 4" DIAMETER CORRUGATED PERFORATED PLASTIC DRAINPIPE WRAPPED WITH A GEOTEXTILE FABRIC.
2. OUTLET INTO ONSITE DRAINAGE SYSTEM (OPTION 1).
3. OUTLET AT 40' O.C. MAXIMUM AND LOW POINTS ALONG WALL. PROVIDE RODENT SCREEN AND NON-PERFORATED PLASTIC PIPE FOR OUTLET THROUGH THE WALL (OPTIONS 2 AND 3).
4. A DENSE GRADED AGGREGATE SHALL FOLLOW THE F/G AT FACE OF WALL FOR OPTION 3.



4 6.00 TYPICAL 4" DRAINPIPE OUTLET
SCALE: N.T.S.

NOTES:

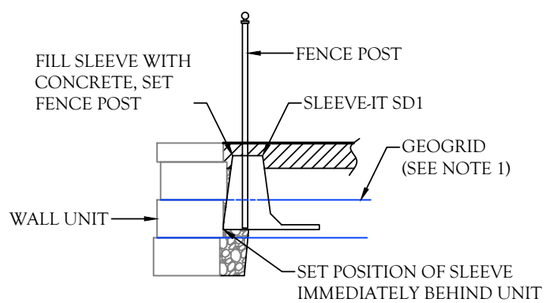
1. PLACE SONOTUBES AT POST LOCATIONS DURING WALL CONSTRUCTION, AND HAND CUT GEOGRID AROUND SONOTUBE (IF APPLICABLE).
2. X=3.0' MIN. AND Y=3.0' MIN. FOR STANDARD FENCING; X=3.0' MIN. AND Y=5.0' MIN. FOR FOR GUARDRAILS. EDGE OF SONOTUBE SHALL NOT BE CLOSER THAN 1-FOOT FROM THE BACK OF THE CLOSEST BLOCK.
3. SEE NCMA FOR ADDITIONAL REQUIREMENTS.
4. FENCE AND RAIL DESIGNED BY OTHERS.



5 6.00 TYP. FENCE & GUARDRAIL INSTALL
SCALE: N.T.S.

NOTES:

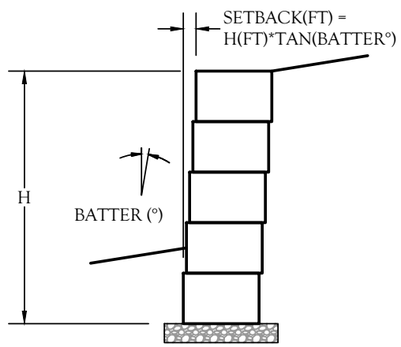
1. PLACE SLEEVE-IT SD1 AT POST LOCATIONS DURING WALL CONSTRUCTION, AND HAND CUT GEOGRID AROUND SLEEVE-IT (IF APPLICABLE).
2. FENCING SYSTEMS APPROVED FOR USE WITH THE SLEEVE-IT SD1 ARE LIMITED TO THE FOLLOWING HEIGHTS: CHAIN LINK UP TO 8 FT, PRIVACY UP TO 6 FT (WOODEN, PVC, METAL). MAXIMUM POST SIZE 4" x 4".
3. SEE STRATA SYSTEMS INC. SLEEVE-IT FENCE POST INSTALLATION DETAILS FOR ADDITIONAL CONSTRUCTION REQUIREMENTS.
4. FENCE DESIGNED BY OTHERS.



6 6.00 OPTIONAL FENCE INSTALLATION
SCALE: N.T.S.

NOTES:

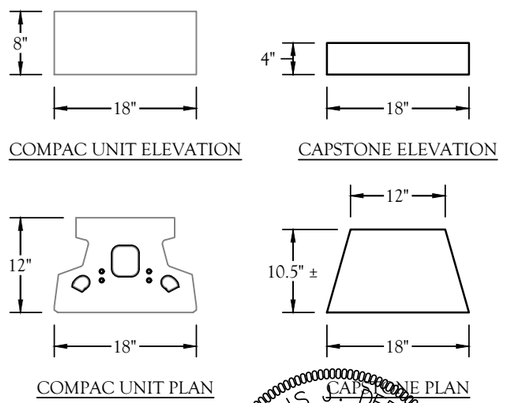
1. THE SETBACK, H*TANGENT(BATTER), DISTANCE VARIES BASED ON WALL HEIGHT AND BATTER AS SHOWN BELOW.
2. THE WALL BATTER IS LISTED UNDER THE PROFILE.
3. THE SURVEYOR AND SITE CIVIL ENGINEER SHALL CONFIRM ALIGNMENT AND SUFFICIENT SPACE AT TOP AND BOTTOM OF WALL FOR THE PROJECT REQUIREMENTS.



7 6.00 SETBACK DETAIL
SCALE: N.T.S.

COMPAC UNIT	
WIDTH:	18"
HEIGHT:	8"
DEPTH:	12.0"
WEIGHT:	85 LBS.

CAPSTONE	
WIDTH:	18"
HEIGHT:	4"
DEPTH:	10.5"
WEIGHT:	45 LBS.



COMPAC UNIT ELEVATION
CAPSTONE ELEVATION
COMPAC UNIT PLAN
CAPSTONE PLAN

8 6.00 COMPAC UNIT
SCALE: N.T.S.

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

Designed By: MRJ
Scale: AS NOTED
Date: NOV 25, 2024

Project: ABQ HOME2SUITES ALBUQUERQUE, NEW MEXICO

Title: DETAILS

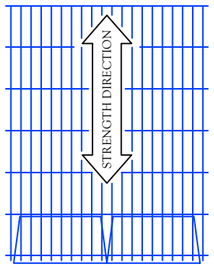
Registration No: 22936
Project No: 24-0987
Sheet No: 6.00

CURTIS J. DERICHS NEW MEXICO 22936 PROFESSIONAL ENGINEER

DERICHS, CURTIS J, P.E. Date: 2/13/2025

NOTES:

1. COMPACTED BACKFILL SHALL BE LEVEL WITH THE TOP OF THE WALL PRIOR TO GEOGRID PLACEMENT.
2. TYPICALLY GEOGRID IS PLACED WITHIN 1" OF THE FRONT FACE OF WALL UNIT. (GEOGRID CONNECTION DEVICES SHALL BE INSTALLED AS DIRECTED BY THE BLOCK MANUFACTURER.)
3. GEOGRID SHALL LAY FLAT ON THE WALL UNIT AND COMPACTED BACKFILL SOILS BEHIND WALL UNIT.
4. PLACE THE NEXT COURSE OF WALL UNITS. PULL GEOGRID TAUT TO REMOVE SLACK AND WRINKLES.
5. STAKE AS REQUIRED TO KEEP GEOGRID TAUT DURING BACKFILL PLACEMENT.

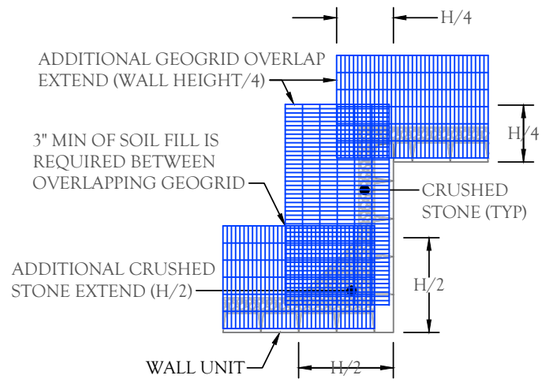


1 6.01 GEOGRID ORIENTATION

SCALE: N.T.S.

NOTES:

1. INSTALL GEOGRID IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATION AND CONSTRUCTION DRAWINGS.
2. 'H' IS WALL HEIGHT.



2 6.01 GEOGRID INSTALLATION ON CORNERS

SCALE: N.T.S.

CIVIL DESIGN PROFESSIONALS
7050 TERRITORY PASS LAKEVILLE, MN 55044
PHONE: (952) 303-5312 | WEBSITE: WWW.CDP-US.COM
SITE SOLUTION PROFESSIONALS, INC. D.B.A. CIVIL DESIGN PROFESSIONALS

No.	Date	Revision	By
1	02/10/2025	REVISED AS PER CITY OF ALBUQUERQUE COMMENTS	MRJ
2			
3			
4			
5			
6			

Designed By: MRJ
Scale: AS NOTED
Date: NOV 25, 2024

Project: ABQ HOME2SUITES
ALBUQUERQUE, NEW MEXICO

Title: DETAILS

Registration No: 22936
Project No: 24-0987
Sheet No: 6.01

CURTIS J. DERICHS
NEW MEXICO
22936
PROFESSIONAL ENGINEER

DERICHS, CURTIS, P.E. Date: 2/13/2025