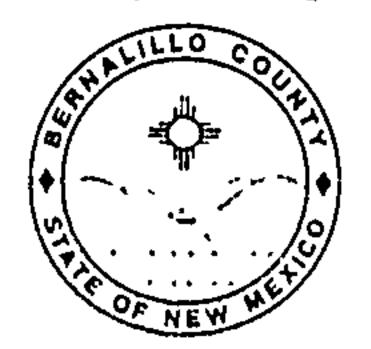
County of Bernalillo

State of Nesu Mexico

STEVE D. GALLEGOS, CHAIRMAN
DISTRICT 2
KEN SANCHEZ, VICE CHAIRMAN
DISTRICT 1
TOM RUTHERFORD, MEMBER
DISTRICT 3
BARBARA J. SEWARD, MEMBER
DISTRICT 4
LES HOUSTON, MEMBER
DISTRICT 5

JUAN R. VIGIL, COUNTY MANAGER



2400 BROADWAY, S.E. ALBUQUERQUE, NEW MEXICO 87102 PUBLIC WORKS (505) 848-1500 DAVID K. ANDERSON, ASSESSOR
JUDY D. WOODWARD, CLERK
THOMAS J. MESCALL, PROBATE JUDGE
JOE BOWDICH, SHERIFF
ORLANDO VIGIL, TREASURER

February 4, 1999

Frank D. Lovelady, P.E. Lovelady & Associates 300 Alamosa Road NW Albuquerque, New Mexico 87107

RE: Engineer's for Bowlin's Convenience Store, Rio Puerco (M3/D1) (PWD-97-207) (PWDN-990013) Engineer's Certification Dated 1/27/99.

Dear Mr. Lovelady:

Prior to release of the Certificate of Occupancy for the above referenced site, the following comments from County Public Works and my office must be addressed:

- 1. You state that a portion of the eastern parking lot is not complete. Do you propose to recertify after this is complete? Also, it appears that the drainage patterns in this area do not match the approved plan. Please certify how this area drains.
- 2. A site inspection showed that the swale leading to the sink hole needed more definition in order to provide the drainage from this site.
- 3. The inspection also showed an opening in the concrete west of the canopy. This must be addressed.
- 4. Does the drop inlet that is currently under construction in the State right-of-way between this site and the frontage road impact your site?

If you have any questions, please call me at 924-3982, or contact Brad Catanach at the County.

Sincerely,

Susan M. Calongne, P.E.

City/County Floodplain Administrator

c: Brad Catanach, P.E., Bernalillo County Public Works Division

Author: bradc@mercury.bernco.gov (Brad Catanach) at INTERNET

Date: 2/8/99 9:13 AM

Priority: Normal Receipt Requested

BCC: Susan Calongne at CABQ-DOWNTOWN TO: scalongne@cabq.gov at INTERNET Subject: Bowlins Site, Comments

Susan,

Below in italics and bold are comments from G. Cashwell for Frank Lovelady's Bowlins. DO NOT SEND THE LETTER UNTIL I TALK TO ROGER ABOUT THIS SITE, THANKS.

Thursday, February 04, 1999

Performed "Pre-Final" inspection for Bowlin Convenience Store (PWD 97-207) this date with the following items noted:

- 1) The final surfacing of the Eastern Parking Area is not complete.
- The drainage flows of the Eastern Parking Area do not appear to be the same as those shown on drainage plan.
- A drop inlet is in the process of construction in the State R/W between the store and the frontage road.
- 4) There is presently an open rectangular opening in the concrete just west of the canopy that will need to be covered and filled in. (Open now for gas piping testing).
- The swale leading into the sink hole needs more definition in order to allow drainage into hole.
- 6) Unresolved issue of North/South road easement (Roger Paul).

Grady Cashwell

Sr. Construction Inspector, Bernalillo County Public Works Development Review Section

BERNALILLO COUNTY NEW SUBMITTAL	PWD SUBMITTAL Use for all PWD applications EXCEPT Street Excavation
RESUBMITTAL AS-BUILT PLOWS AS	PWDN 990013
FINAL SIGNOFF TODAYS DATE: January 27, 1999	CASE NO: 97-207
owner Bowlin Travel Centers PHONE 266-598/	
MAILING ADDRESS 150 LOUISLAHA CITY ALBO ZIP 8	7/08
AGENT/ CONTRACTOR FYDYK D LOVelady PHONE -345-	2267 ·:
MAILING ADDRESS 300 ALAMOSA NW CITY ALB ZIP &	7710-7
STATE EXP LICENSE NO. DATE VOLUME CLASS	
ARCHITECT/ENGINEER FRANK D. LOVELADY LICENSE NO. NIMPE	6572 PHONE 345-2257
SITE ADDRESS/DIRECTIONS Take I-40 - West to Rio Pue	ZONE ATLAS NO.: N-4.
Take First exit Site is north of the fi	rontage-Road-
LEGAL DESCRIPTION Tract B-1 Land of Bow_Lin	
	LOT SIZE: 5.0 AC
EXISTING PROPOSED BUILDING(S) AND USE: BUILDING(S):	
UPC 1 -004-054-262-265-10:1	1-143
TYPE OF SUBMITTAL REPLAT TRAFFIC IM	PACT ANALYSIS / TRAFFIC STUDY
	CTURE LIST / DESIGN REVIEW •
☐ MAJOR SUBDIVISION ☐ SPECIAL US	E PERMIT
CONSTRUCTION DRAWINGS	NG PERMIT
GRADING & DRAINAGE PLAN	ERMIT
AS-CONSTRUCTED GRADING & DRAINAGE PLAN	
☐ VARIANCE REQUEST \ ☐ OTHER (Spec	cify):
LAND DIVISION	: ▶
The issuance of a permit or a review or approval of plan specifications, computations, and shop drawings, so fany variance or violation of any of the provisions of any COUNTY or STATE codes, ordinances, standar approval of plans, specifications, computations, and shop drawings prevent any authorized COUNTY representations are correction of errors in said plans, specifications, computations, or shop drawings or from stop thereunder when in violation of any COUNTY or STATE codes, ordinances, standards, or policies.	rds, or policies. Nor shall such issuance of a permit or sentative or COUNTY inspector form thereafter
Owner Pagent Contractor Signature Land Woodlad	Date 1-27-99
BERNALILLO COUNTY USE ONLY	
C/R's:	TOTAL FEE:
	Receipt No.:
	Received By:

27%

DRAINAGE INFORMATION SHEET

APPLICANT'S NAME: Bowlin Travel Center	S ZONE ATLAS/DRNG. FILE #: M-3 /D-1
DRB #: EPC #:	• •==
LEGAL DESCRIPTION: TRACT B-1 LAND	
CITY ADDRESS:	
ENGINEERING FIRM: LOVELAD & ASSOC.	CONTACT: FRANK D. LOVELADY
ADDRESS: 300 ALAMOLA NW 87	107 PHONE:345-2267
OWNER: Bowlin Travel Centers	CONTACT: CINDY BIGGERS
ADDRESS: 150 Louis 14NA HE	PHONE: 244-5986
ARCHITECT: GENCON	
ADDRESS: P.O. BOX 448 MESILLA 14M	
SURVEYOR: HARRIS BURVEYING	CONTACT: TONY HARIZIS.
4 D D D D D D D D D D D D D D D D D D D	
CONTRACTOR: GEN CON	CONTACT: LARRY BROWN
ADDRESS: P.D. BOX 448 MESILLA NM	PHONE: 505 523-4556
TYPE OF SUBMITTAL: DRAINAGE REPORT DRAINAGE PLAN CONCEPTUAL GRADING & DRAINAGE PLAN GRADING PLAN EROSION CONTROL PLAN ENGINEER'S CERTIFICATION OTHER PRE-DESIGN MEETING: YES NO COPY PROVIDED	CHECK TYPE OF APPROVAL SOUGHT: SKETCH PLAT APPROVAL PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D APPROVAL S. DEV PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL S.A.D. DRAINAGE REPORT DRAINAGE REQUIREMENTS SUBDIVISION CERTIFICATION OTHER (SPECIFY)
BY: FEB 18, 1999 Revised 02/98	国の国で国の国际 FEB 18 1999 D HYDROLOGY SECTION

County Public will works has usues

Telephone (505)345-2267

February 11,1999

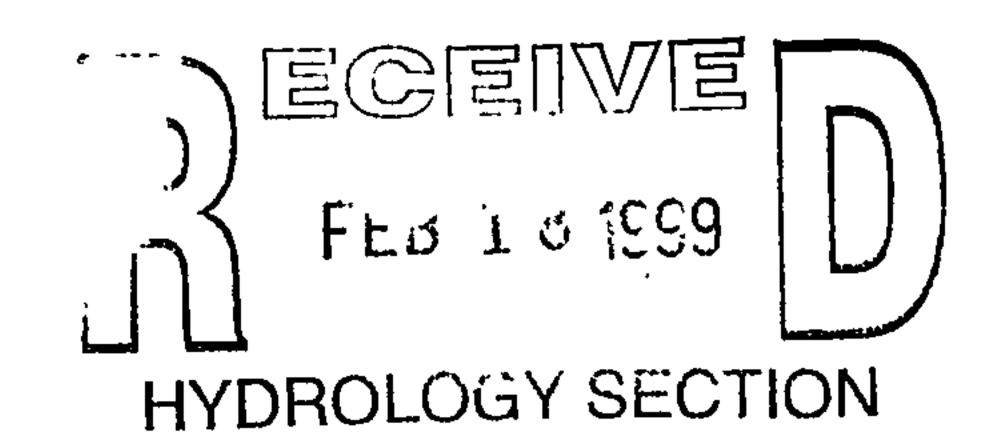
Suṣạn M. Calongne, P.E.

City/County Floodplain Administrator

c/o Bernalillo County Public Works Division

2400 Broadway S.E.

Albuquerque, NM 87102



RE: ENGINEER'S CERTIFICATION FOR BOWLIN'S CONVENIENCE STORE, RIO PUERCO (M3/D1) (PWD-97-207)(PWDN-990013) ENGINEER'S CERTIFICATION DATED 1-27-99.

Dear Ms. Calongne:

This letter is in response to your letter of February 9, 1999. Your letter had four (4) comments with regard to my submittal dated 1-27-99. My response follows:

- The eastern parking lot is paved with 3" of asphalt over existing grades. The flow arrows have been revised to reflect this. No new spot elevations are shown. To find the as-constructed spot elevations in this area, add 0.25' to the existing spot elevations shown on the original topo. Paving was finished in this area on February 10, 1999.
- The swale leading to the sink hole has been bladed and lined with gravel to provide more definition as you requested.
- The opening in the concrete west of the canopy was an inspection hole for a splice in a fuel line. This hole has now been filled with concrete.
- The drop inlet was originally part of the old station's drainage system. It is not in the highway right-of-way but it is in the strip that Bowlin has agreed to give the State. The original topo did not pick up this inlet but it was discovered during grading. It was too low to function with the new grades so the contractor raised the grate elevation so that it will drain the area adjacent to the right-of-way

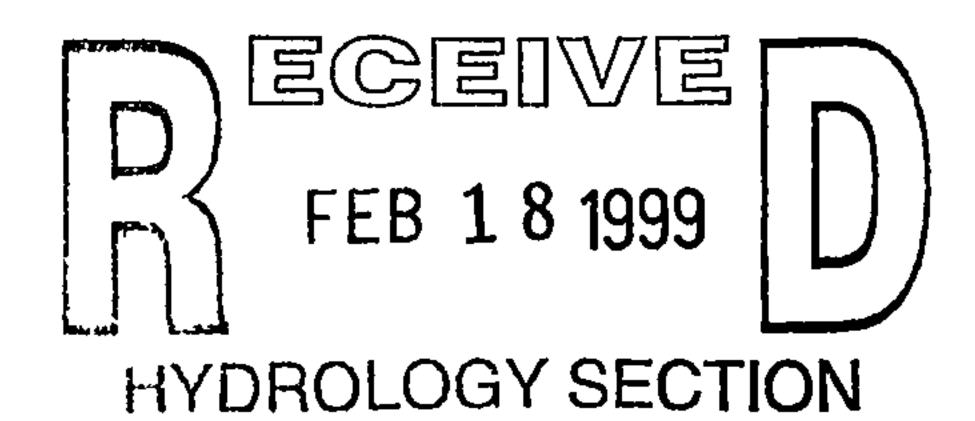
outside of the new pavement. The inlet is connected by pipe to the other inlet which is near the SE corner of the canopy. This inlet has been shown on the plan.

If you have any questions or require additional information please do not hesitate to contact me.

Sincerely,

Frank D. Lovelady, P.E.

c. Brad Catanach, P.E., Bernalillo County Public Works Division Larry Brown, Gen Con



Live to Ausan Calongne

,

County of Bernalillo

State of New Mexico



2400 BROADWAY, S.E.
ALBUQUERQUE, NEW MEXICO 87102
PUBLIC WORKS (505) 848-1500

DAVID K. ANDERSON, ASSESSOR
JUDY D. WOODWARD, CLERK
THOMAS J. MESCALL, PROBATE JUDGE
JOE BOWDICH, SHERIFF
ORLANDO VIGIL, TREASURER

March 2, 1998

Frank D. Lovelady, P.E. Lovelady & Associates 300 Alamosa Road NW Albuquerque, New Mexico 87107

RE: Drainage Report and Grading and Drainage Plan for Bowlin's Convenience Store, Rio Puerco (M3/D1) (PWD-97-207) Engineer's Stamp Dated 2/5/98.

Dear Mr. Lovelady:

BOARD OF COUNTY COMMISSIONERS

TOM RUTHERFORD, CHAIR

BARBARA J. SEWARD, VICE CHAIR

STEVE D. GALLEGOS, MEMBER

JUAN R. VIGIL, COUNTY MANAGER

KEN SANCHEZ, MEMBER

LES HOUSTON, MEMBER

DISTRICT 3

DISTRICT 4

DISTRICT 1

DISTRICT 2

DISTRICT 5

Based on the information provided in the submittal of February 10, 1998, the above referenced plan is approved for Building Permit release.

Please be advised that the Engineer's Certification must be submitted to and approved by County Public Works and my office prior to Certificate of Occupancy release.

If you have any questions, please call me at 924-3982, or contact Brad Catanach at the County.

Sincerely,

Susan M. Calongne, P.E.

City/County Floodplain Administrator

c: Andrew Garcia, City Hydrology
Brad Catanach, Bernalillo County Public Works Division

=File=

Rio Pigerco, vanimous min	COMPANION DILLIAM BITTE 11. M-2 / D-1
OJECT TITLE: Bowlin's Convenience Store	erante annun II.
:GAL DESCRIPTION: Tract B-1 Land of Bo	SPORK URDER 1. STATE OF GOD T. HILL
	M
ITY ADDRESS: No: Puerco. M./	
IGINEERING FIRM: Lovelady & Associates	CONTACT: Frank D. Lovelady
ADDRESS: 300. A LAMOSA NW	PHONE: 345-2267:
MER: BOWLING, INC.	CONTACT: FRANK LOVELADY
: ADDRESS: 150 LOUISIANA N	E PHONE: 266-5985
RCHITECT: <u>S. Duane Dorsey</u>	CONTACT: S. Duane Dorsey
ADDRESS: Las Cruces NM	PHONE:
URVEYOR: Harris Surveying Co	CONTACT: Tony Harris
ADDRESS: 2412 - D MONROE NE	B7110 PHONE: 889-8056
GEN CON	CONTACT:
ADDRESS: P.O. Box 448 Mesilla	NM PHONE: 523-4556.
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NO	S.A.D. DRAINAGE REPORT.
COPY PROVIDED -	DRAINAGE REQUIREMENTS
	OTHER (SPECIFY)
ATE SUBMITTED: FEB 60,1998 BY: Frank O. Loveling	Review By City/county Flood Plain Administrator DEGETVE DFEB 1 U 1998 HYDROLOGY SECTION

Case Routing Slip

06.	-Feb-9	8 pwd	-97	- 207	Category:	Resubmi	tt
Applicant Na Applicant Ca		Frank D Lo Drainage En			Application E	Date:	06-Feb-98 27-Feb-98
		Owner Info		Ag	ent info	Egr/s	Svy Info
Name	Bowlin	s Inc		FRANK D	LOVELADY	ļ	
Address	150 Lo	uisiana Blvd N	1E	300 Alamo	sa NW		
City	Albuqu	erque		Albuquerqu	1e		
State	NM			NM			
ZIP	87108			87107	_		
Ph	266-59	85		345-2267			
Legal Description: TR B LAND OF BOWLIN & LANDS OF GEORGE T HILL UPC: Zone Map M-3 Street Address: I-40 No Frontage Rd Submittal Type: Design Review							
DRAN Z	ments R	equired From		INSP	TRAF	UTIL	OTHE

Brad

Original

2/9/98

DIFEBIO1998

HYDROLOGY SECTION

	MEW SUBMITTAL	Use for all PWD applications EXCEPT Street	EXCAVADO
(D' RESUBMITTAL	720	>
	☐ FINAL SIGNOFF TODAY'S DATE:	5B.6,1998 . CASENO: PWD 97	207
NER	OWNER BOWLIN'S, INC.	PHONE 266-5985	
8	MULING ADDRESS 150 LOUISIANA BLVD.	NE cm ALBUQ ZIP 87108	
	AGENT! CONTRACTOR FRANK D. LOVELADY	PHONE :- 345-2:267	
	MULING ADDRESS 300 ALAMOSA HU	CITY ALBUQ ZIP 87107	• •
O	STATE	OLUME CLASS	
	ARCHITECTIENGINEER FRANK D. LOVELA	DY LICENSENO. HMDE.6512 PHONE 345-226	7
7	SITE ADDRESS / DIRECTIONS TAKE I-4-D WE	ST TO RIO PUERCO. ZONE ATLAS NO .: CHATA. M	1-3_
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đ	GRADING & DRAINAGE PLAN	BUILDING PERMIT	
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_] VARIANCE REQUEST	OTHER (Specify):	
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ol ap	any variance or violation of any of the provisions of any COUNTY	s, computations, and shop drawings, shall not be interpreted to be a permit for, or an or STATE codes, ordinances, standards, or policies. Nor shall such issuance of a percent any authorized COUNTY representative or COUNTY inspector form thereafter ations, or shop drawings or from stopping construction operations which are being constructed, standards, or policies.	e s
	Owner Agent Contractor Signature France	el D. Foreldy Date FEBGIL	998
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DRAINAGE REPORT

FOR

BOWLIN'S CONVENIENCE STORE AND TRAVEL STORE RIO PUERCO

Bernalillo County, New Mexico

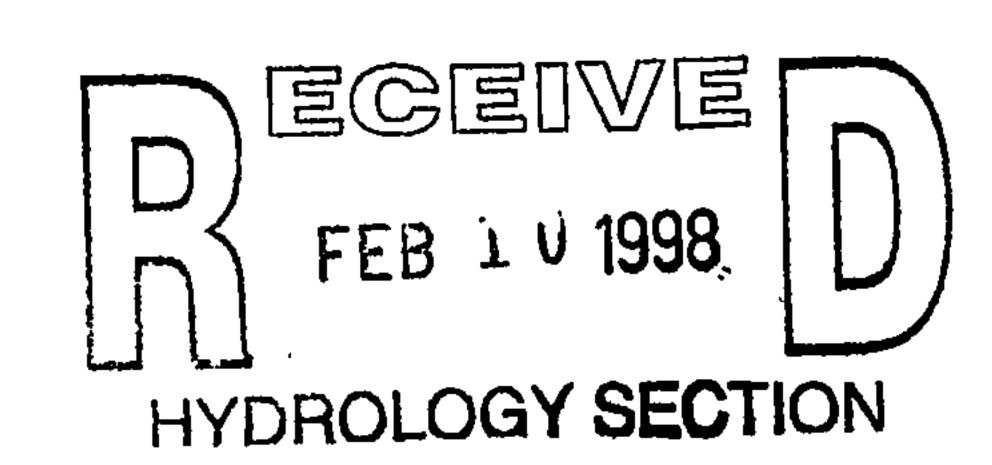
December 2, 1997 (Revised February 5, 1998)

Prepared by:

Frank D. Lovelady, P.E.

300 Alamosa NW

Albuquerque, New Mexico 87107



DRAINAGE REPORT

· FOR

BOWLIN'S CONVENIENCE STORE AND TRAVEL STORE

RIO PUERCO

Bernalillo County, New Mexico



ENGINEER'S STAMP

SCOPE:

This report has been prepared for the purpose of obtaining a building permit for the construction of a convenience store/travel store on property abutting the north frontage road of Interstate Highway 40 east of the existing bridge crossing the Rio Puerco in Bernalillo County, New Mexico. The on-site runoff generated by the 100-year storm has been addressed and off-site flow generated by the unnamed arroyo west of the site is also addressed including the capacity of the arroyo and box culvert and it impact on the site. Initial steps have been taken in the process of initiating a Traffic Impact Analysis.

EXISTING CONDITIONS:

The site includes Tract B-1, Lands of Bowlin (0.8975 ac) and 1.25 acres west of Tract B-1 and 0.11 acre north of Tract B-1, both of which are a part of "Northerly Portion of Lands of George T. Hill, a Tract containing 32.8782 acres. The site is located adjacent to the North Frontage Road of Interstate Highway 40 approximately 1,000 feet east of the existing east bridge abutment of the North Frontage Road bridge over the Rio Puerco. Directly east of the site is an unnamed arroyo which has a concrete box culvert under the North Frontage Road and the westbound and eastbound lanes of I-40. The box culvert is approximately 225' east of the east boundary of the site.

The site is geographically located on a relatively flat bench of land lying between the Rio Puerco and the unnamed arroyo. At the widest part, adjacent to the I-40 North Frontage Road, the bench is approximately 1,400 feet between the west bank of the unnamed arroyo and the east bank of the Rio Puerco. At the narrowest point, 400 to 500 feet north of the frontage road, the width is approximately 900 feet. A berm has been constructed at some time in the past from the unnamed arroyo to the Rio Puerco at approximately a 30 degree angle with Interstate Highway 40 alignment. The berm

apparently was constructed to protect property improvements in the bench area from flooding resulting from overflow of the unnamed arroyo and to convey any such overflow into the Rio Puerco. The property improvements consist of a residence, a mobile home and several garage and service station buildings. The garage and service station buildings have recently been razed. The residence and mobile home are still present.

Adjacent to the site, the distance between the frontage road pavement and the right-of-way line is approximately 50 feet. The distance between the north edge of the westbound lane and the south edge of pavement on the frontage road is approximately 85 feet. The exit ramp from I-40 westbound lanes to the frontage road is east of the unnamed arroyo.

PROPOSED CONDITIONS:

It is proposed to construct a convenience store and fuel pump canopy as shown (See Grading and Drainage Plan at the back of this report). The Highway and Transportation Department District Engineer for District 3 has indicated a need to move the frontage road further north to give a wider separation at the off-ramp location. However, this situation has been resolved in meetings that representatives of Bowlin's have had with the Highway and Transportation Department.

FLOOD INSURANCE RATE MAPS (FIRM):

The site is covered by Flood Insurance Rate Maps (FIRM) 35001C0287 D and 35001C0291 D. The site is, for the most part, in Zone AO (depth 1). The extreme easterly end of the site in Zone AE which encompasses the unnamed arroyo and a reasonable distance on either side of the arroyo. The definitions for the two zones are as follows:

Zone AO Flood depths of one to three feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities are also determined.

Zone AE - Base flood elevations determined.

Portions of the two FIRM maps which show the site have been spliced together and are presented in Figure 2.

FLOOD INSURANCE STUDY:

The Federal Emergency Management Agency has published a two-volume study entitled "Flood Insurance Study, Bernalillo County, New Mexico and Incorporated Areas, Volumes 1 and 2 of 2", Revised September 1996. Pages from this study have been included in this report as Figures 4 through 8. These pages are pertinent to the site in that they include information relative to the unnamed arroyo adjacent to the site.

OFF-SITE DRAINAGE CALCULATIONS:

There is really no off-site flow, as such, effecting the site other than overflow from the unnamed arroyo east of the site. The site is adjacent to the unnamed arroyo and is, therefore, subject to overflow from the arroyo that may occur because the box culvert under I-40 is not sufficiently large to accommodate the 100-year peak discharge. This can be seen from the FEMA Flood Profile which is shown in the back of this report. Figure 5 shows Table 2, a FEMA listing of peak discharge rates for various analysis points, including the unnamed Tributary No. 1, which is believed to be the unnamed arroyo adjacent to the site. The 100-year peak discharge rate for the unnamed arroyo is 4,450 cubic feet per second. The sections of the unnamed arroyo shown in Figure 4 show that the first section above the box culvert, Section D, has a water surface elevation of 5282.5' (NGVD). The second section, Section E, which is 419 feet upstream from Section D, has

a water surface elevation of 5282.5' (NGVD). The third section, Section F, which is 881 feet upstream from Section E, has a water surface elevation of 5282.6' (NGVD).

It should be noted that this arroyo has an extremely flat channel slope. The elevation shown for the culvert under the frontage road is approximately 66.5 and the quad map shows the 80 contour crossing the arroyo approximately 3300 feet upstream from the culvert. This is a slope of 0.0041 ft. per ft. Another thing that should be noted is that the culvert is only a short distance downstream from the site and it acts as a grade-control structure.

The cross sectional areas and mean velocities at Sections C and D shown in Figure 4, Floodway Data, are shown again in the following table along with the calculated peak discharge:

Section	Area (sf)	Mean Velocity(fps)	Peak Discharge(cfs)
C	167	8.4	1,403 *
D	1,935	2.3	4,450 *

^{*}Calculated as follows: (Peak Discharge = Area x Mean Velocity)

The peak discharge rate at Section C, the first section south of I-40, is 1403 cfs. The Peak discharge rate at Section D, the first section north of I-40, is 4,450 cfs. This is also the figure for peak discharge shown in Figure 6 for the Unnamed Tributary No. 1. These figures indicate that a quantity of 4,450 - 1,430 = 3,047 cfs, almost 70 percent of the flow in the arroyo, overflows the arroyo banks and flows into the Rio Puerco as indicated by the AO flood zone. It is believed that the highest velocity would occur in the arroyo as the culvert reaches capacity, i.e., when the flow in the arroyo is 1,403 cfs. This is slightly larger than the ten year peak discharge, which is shown in Figure 6 as 1,290 cfs.

The flow is analyzed by means of Manning's Equation to determine the velocity and Froude number. The section analyzed begins near the building and ends where it intersects the 5273 contour. The contours only show the steep, high bank on the west and a gradually sloping area to the east . For the purposes of calculating the cross-sectional area, the east bank is assumed to begin to rise where the cross section intersects the 5273 contour. In the event the bank is further east, this assumption will be conservative.

By trial and error, a water surface elevation of 74.7 was found to result in a flow of 1,425 cf as shown below: Total Area = 220 sf. P = 78.5 ft. R = 220 / 78.5 = 2.80 N = 0.035.

$$V = (1.486 / N)(R)^{2/3} (S)^{1/2} = 1.486 / 0.035)(2.80)^{2/3}(0.0059)^{1/2} = 6.48 \text{ fps}$$

$$Q = AV = 220 \times 6.48 = 1,425 \text{ cfs}$$

FROUDE NUMBER:

The maximum depth is 74.7 - 69.0 = 5.7 feet. This is Y_1 .

$$F_R = (V / gd)1/2$$
 $d = Y_1 + V^2 / 2g = 5.7 + 6.48^2 / (2 x 32.2) = 6.35 ft.$

$$F_R = (6.48 / (32.2 \times 6.35)^{1/2} = 0.45$$

SCOUR DEPTH CALCULATIONS:

$$Y_S = Y_1(4 F_R^{0.33}) = 5.7 (4 \times 0.45^{0.33}) = 17.52 \text{ ft.}$$

RIPRAP TYPE SELECTION:

Refer to "Urban Storm Drainage Criteria Manual, Volume 2", Denver Regional Council of Governments:

Use Table 5-5. The riprap selection factor, R, is given by the following equation:

$$R = VS^{0.17} / (S_S - 1)^{0.66}$$
 $S_S = 2.5$ $S = 0.0059 \text{ ft. / ft. } V = 6.48 \text{ fps}$

$$R = 6.48 \times 0.0059^{0.17} / (2.65 - 1)^{0.66} = 1.95$$
 1.4<1.95<3.2 Use Type VL riprap.

RIPRAP BLANKET THICKNESS:

- A.

Refer to "Urban Storm Drainage Criteria Manual, Volume 2", Denver Regional Council of Governments, Section 5.4.2 Rock Size and Lining Dimensions.

The riprap blanket thickness should be at least 1.75 times d_{50} . (See Grading and Drainage Plan. d_{50} is the same as K_m shown in the riprap specification table.) Therefore, the blanket thickness should be 1.75 x K_m or 1.75 x 6'' = 10.5'' thick. Use 12"

GRANULAR BEDDING UNDER RIPRAP BLANKET:

Refer to "Urban Storm Drainage Criteria Manual, Volume 2", Denver Regional Council of Governments, Table 5-4. Also refer to "Soil Survey of Bernalillo County and Parts of Sandoval and Valencia Counties, New Mexico", Soil Conservation Service. The type of soil is Ha, Hantz, which is a silty clay, qualifying as a fine grained soil. For Type M riprap and fine-grained soils, 4" of Type I bedding and 4" of Type II bedding is required. One 12" layer of Type II bedding may be substituted.

RIPRAP LIMITS:

The slope of the channel is very flat so only one section was used to evaluate flow in the channel. The erosion protection determined above is designed only for 1,403 cfs. As the culvert reaches capacity, the depth will increase, and the velocity will decrease.

The lower limits of the riprap would normally be determined by the scour analysis. However, in this case, the scour equation yields a scour depth of 17 feet, which, considering the downstream constraint of the box culvert acting as a grade control structure, is somewhat excessive. Another method of calculating the depth of riprap is to calculate the critical slope. The elevation of the toe of the riprap blanket would be equal to the elevation of the culvert inlet or grade-control structure plus the critical slope multiplied by the distance between the riprap protection and the culvert.

Critical slope: $S_c = (14.56 \text{ n}^2) / D_m^{1/3}$ (Brater/King 8-83)

Where, D_m is the area divided by the width at mean depth, or 220 / 23 = 8.46, and n is the Manning's coefficient.

 $S_c = [14.56 (0.035)^2] / 8.46^{1/3} = 0.0088 \text{ ft./ft.}$

Elev. of riprap toe = $5265.5^* + 400 \times 0.0088 = 5269.0$ Use elevation 5268.0 for an additional margin of protetion. (*5265.5 is elev. of culvert, or grade control structure as shown on flood profiles, 78P.) It should be noted that the toe of the riprap slope, as measured 24' from the top of the bank, falls around the 5271 contour. So it will be buried approximately 3'.

The upper limits of riprap would normally be placed at the depth previously calculated as d, or 6.35', above the lowest level of the channel cross section (5269.0), plus a reasonable freeboard. This would be 6269.0 + 6.35 + 1.0 = 76.35. However, due to the backwater condition that will occur during the 100-year storm, the flow in the arroyo will gradually

rise with a decreasing velocity all the way to the top of the bank (approximately contour 80). Since this is on a curve, it is prudent to carry the riprap the additional 3.65 vertical feet to the top of bank.

RIPRAP THICKNESS:

ON-SITE DRAINAGE CALCULATIONS:

The site is almost entirely paved with a small area of landscaping on either side of the proposed convenience store.

Tract size - 2.2575 Acres.

Precipitation Zone 1, (DPM Part A.1)

Excess Precipitation, E (DPM Part A.5)

$$A = 0.44$$
" $B = 0.67$ " $C = 0.99$ " $D = 1.97$ " (100-year, 6-hour)

Assumptions:

Treatment A, Areas remaining natural	0 sf	0.0000 ac.
Treatment B, Landscaping	836 sf	0.0096 ac
Treatment C, Gravel Areas	0 sf	0.0000 ac
Treatment D, Building and Paved Areas	97,501 sf	2.2479 ac.
Totals	98,337 sf	2.2575 ac.

Volume, 100-year, 6-hour:

$$V = (E / 12)(A)$$

Treat B $V = (0.67/12)836 = 47 \text{ cf}$
 $V = (1.97 / 12)(97,501) = 16,006 \text{ cf}$

Total $V = (1.97 / 12)(97,501) = 16,053 \text{ cf}$

Peak Discharge, 100-year:

$$Q = (q)(A)$$

Peak Discharge per Acre (DPM, Table A-9);

q:
$$A = 1.29 B = 2.03 C = 2.87 D = 4.37$$

Treat B $Q = 1.29 \times 0.0096 = 0.01 \text{ cfs}$

Treat D $Q = 4.37 \times 2.1033 = 9.19 \text{ cfs}$

Total 9.20

FIGURE 1 - VICINITY MAP

SHOWN ON A PORTION OF USGS QUADRANGLE MAP, LA MESITA NEGRA,

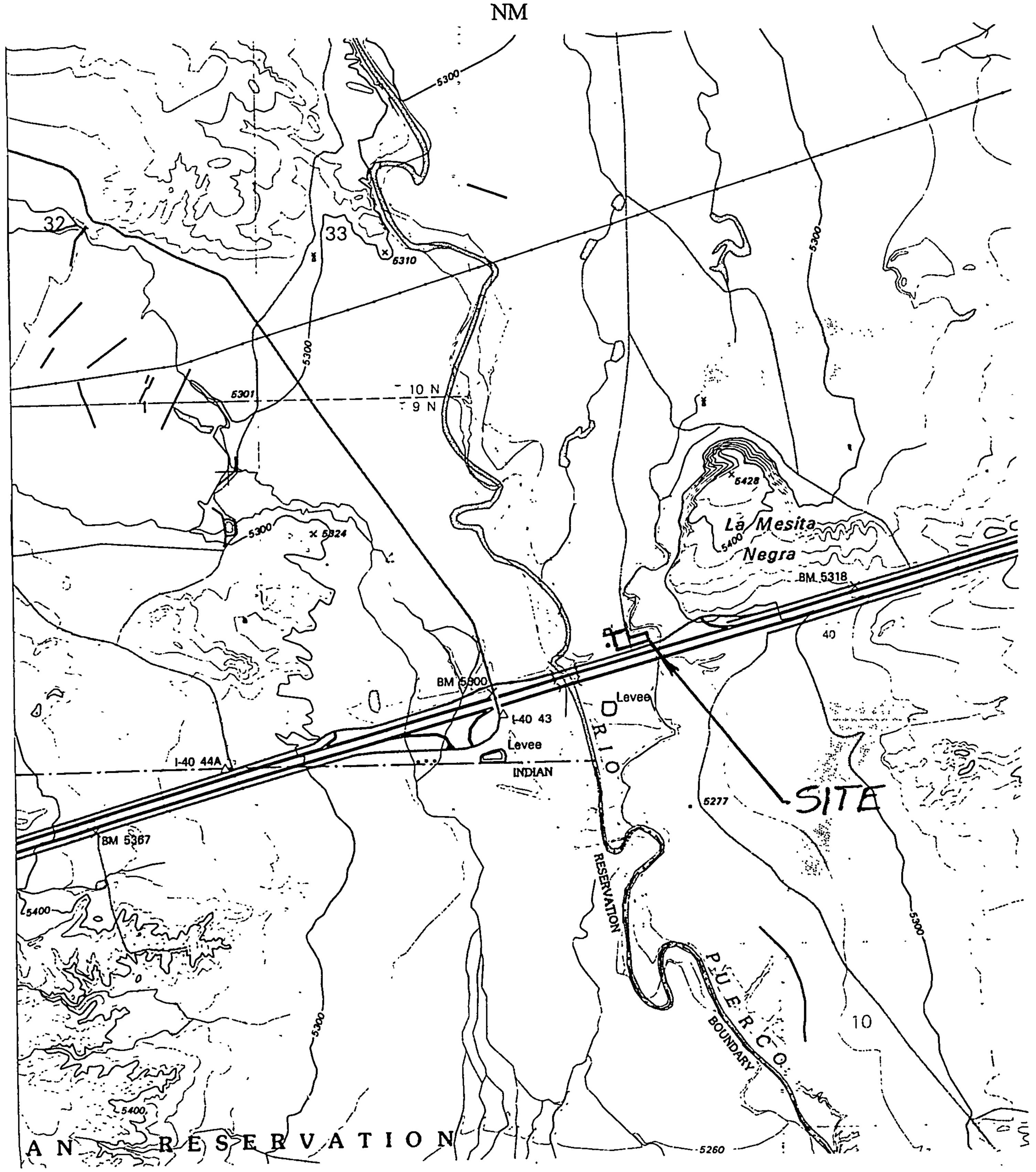


FIGURE 2 - FLOOD INSURANCE RATE MAP (FIRM)

PORTION OF MAPS 35001C0287 D AND 35001C0291 D

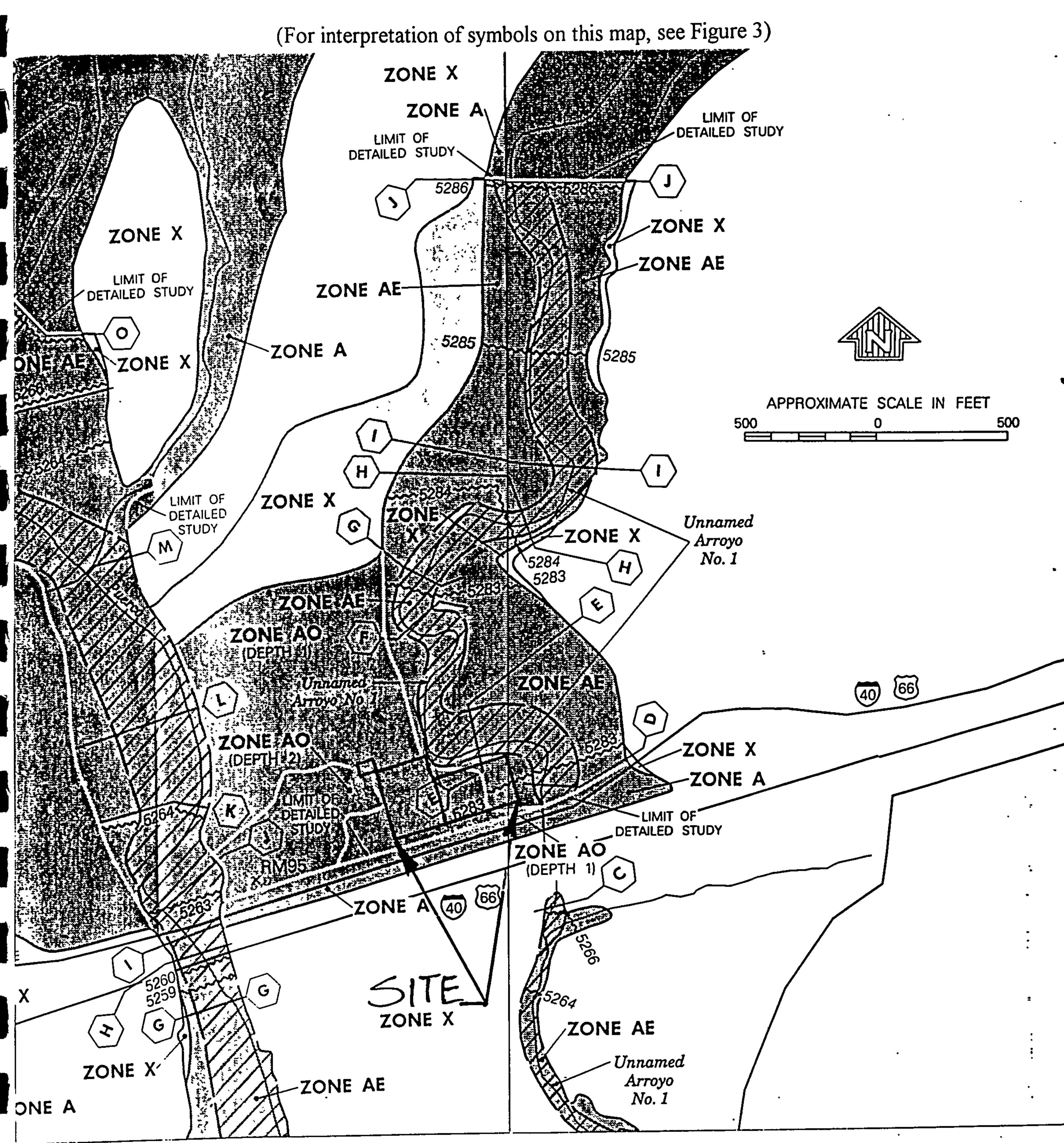


FIGURE 3 - LEGEND AND NOTES FOR FIRM MAP.

LEGEND SPECIAL FLOOD HAZARD AREAS INUNDATED BY 100-YEAR FLOOD No base flood elevations determined. ZONE A ZONE AE Base flood elevations determined. Flood depths of 1 to 3 feet (usually areas ZONE AH elevations determined. Flood depths of 1 to 3 feet (usually sheet ZONE AO flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined. To be protected from 100-year flood by ZONE A99 Federal flood protection system under construction; no base elevations determined. Coastal flood with velocity hazard (wave ZONE V action); no base flood elevations determined. Coastal flood with velocity hazard (wave ZONE VE action); base flood elevations determined. FLOODWAY AREAS IN ZONE AE OTHER FLOOD AREAS Areas of 500-year flood; areas of 100-year ZONE X flood with average depths of less than 1 foot or with drainage areas less than square mile; and areas protected by levees from 100-year flood. OTHER AREAS Areas determined to be outside 500-year ZONE X floodplain. Areas in which flood hazards are ZONE D undetermined. UNDEVELOPED COASTAL BARRIERS Otherwise Identified Identified Protected Areas 1990 1983 Coastal barrier areas are normally located within or adjacent to Special Flood Hazard Areas. Flood Boundary Floodway Boundary Zone D Boundary Dividing Special Flood Boundary Boundary Zones. Hazard and Different Dividing Areas Elevations Base Flood Coastai Hazard Flood Within Special Zones. Line; Flood Elevation Base Elevation in Feet. See Map Index for Elevation Datum.

Cross Section Line

Where

(EL 987)

RM7

Flood Elevation

See Map Index for Elevation Datum.

Within

Uniform

Elevation Reference Mark

in Feet

Zone.

• M2

River Mile

97°07′30″. 32°22′30″

Horizontal Coordinates Based on North American Datum of 1927 (NAD 27) Projection.

NOTES

This map is for use in administering the National Flood Insurance Program; it does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size or all planimetric features outside. Special Flood Hazard Areas

Coastal base flood elevations apply only landward of 0.0 NGVD, and include the effects of wave action, these elevations may also differ significantly from those developed by the National Weather Service for hurricane evacuation planning.

Areas of Special Flood Hazard (100-year flood) include Zones A, AE, AH, AO, A99, V, and VE.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the Federal Emergency Management Agency.

Floodway widths in some areas may be too narrow to show to scale. Floodway widths are provided in the Flood Insurance Study Report.

This map may incorporate approximate boundaries of Coastal Barrier Resource System Units and for Otherwise Protected Areas established under the Coastal Barrier Improvement Act of 1990 (PL 101-591).

Corporate limits shown are current as of the date of this map. The user should contact appropriate community officials to determine if corporate limits have changed subsequent to the issuance of this map.

For community map revision history prior to countywide mapping, see Section 6.0 of the Flood Insurance Study Report.

For adjoining map panels and base map source see separately printed Map Index.

MAP REPOSITORY
Refer to Repository Listing on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP:

SEPTEMBER 20 1996

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL:

Refer to the FLOCD INSURANCE RATE MAP EFFECTIVE DATE shown on this map to determine when actuarial rates apply to structures in zones where elevations or depths have been established.

To determine if flood insurance is available, contact an insurance agent or call the National Flood Insurance Program at (800) 638-6620.



APPROXIMATE SCALE IN FEET

500 0 500

FIGURE 4 - FLOODWAY DATA

(From Flood Insurance Study, Bernalillo County, New Mexico and Incorporated Areas - FEMA, Revised September 20, 1996)

FLOODING SC	FLOODING SOURCE FLOODWAY		OURCE FLOODWAY BASE FLOOD WATER SURFACE ELEV						
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE	MEAN VELOCITY (FEET PER	REGULATORY	FLOODWAY	FLOODWAY	HICREASE	
Frost Arroyo (Cont'd) AA AB AC AD Unnamed Arroyo No. 1 A B C D E F G H I J	12,660 ¹ 12,310 ¹ 12,880 ¹ 13,365 ¹ 1,300 ² 2,030 ² 3,100 ² 3,550 ² 3,974 ² 4,855 ² 5,105 ² 5,625 ² 5,924 ² 7,155 ²	150 100 120 120 120 177 180 140 150 93 173 193	358 483 320 323 140 266 167 1,935 1,660 892 593 643 1,059 936	8.5 6.3 9.5 9.4 10.0 5.3 8.4 2.3 2.7 5.0 7.5 6.9 4.2 4.8	6,710.8 6,711.6 6,714.8 6,723.1 5,259.5 5,266.0 5,282.5 5,282.6 5,282.6 5,282.6 5,284.2 5,284.6 5,285.8	6,710.8 6,711.6 6,714.8 6,723.1 5,255.4 ³ 5,259.5 5,266.0 5,282.5 5,282.6 5,282.6 5,284.2 5,284.6 5,285.8	6,711.1 6,711.8 6,714.8 6,723.6 5,255.4 5,259.5 5,266.0 5,282.5 5,282.6 5,282.6 5,282.6 5,284.2 4,285.1 5,286.1	0.3 0.2 0.0 0.0 0.0 0.0 0.0 0.0 0.5 0.3	

¹Feet Above Confluence With San Pedro Creek
2Feet Above Confluence With Rio Puerco

³Elevations Computed Without Consideration of Backwater Effects

FIGURE 5 - SUMMARY OF PEAK DISCHARGES - RIO PUERCO

(From Flood Insurance Study, Bernalillo County, New Mexico and Incorporated Areas - FEMA, Revised September 20, 1996)

Table 2. Summary of Discharges

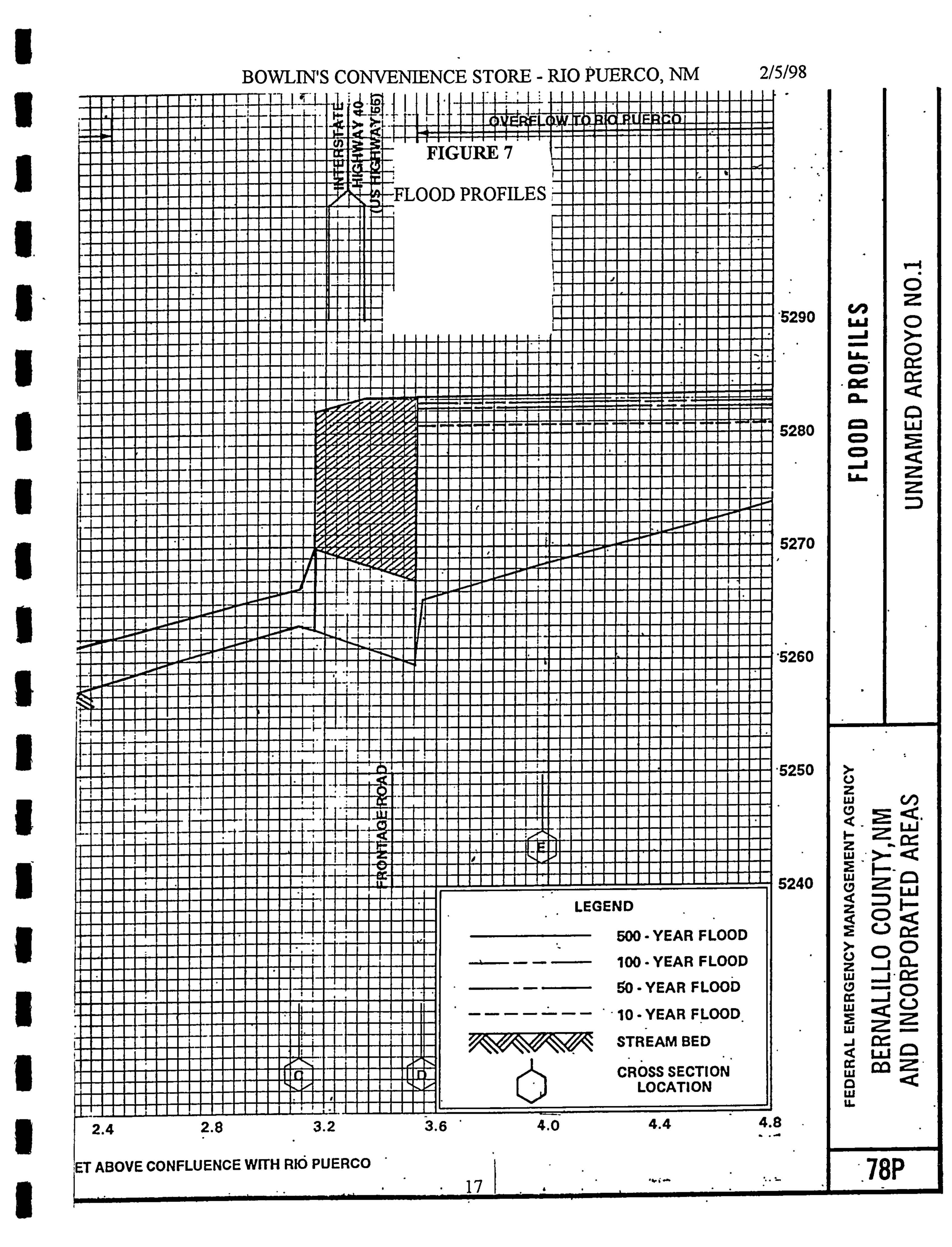
	Drainage Area	Peak D	eak Discharges (cubic feet per second)			
Flooding Source and Location	(square miles)	10-Year	50-Year	100-Year	500-Year	
Arroyo de las Calabacillas						
At confluence with Rio Grande Upstream of confluence with Black's	98.0	2,700	8,500	12,700	28,000	
Arroyo	85.1	2,460	7,800	11,900	26,200	
At upstream study limit	71.9	2,300	7,200	11,000	24,500	
Canada del Ojo			0 070	10.000	15,800	
At Interstate Highway 40	76.5	5,100	8,870	10,800	13,800	
Cedro Canyon Arroyo	•				10.040	
At confluence with Tijeras Arroyo	18.9	1,830	. 3,730	5,420	- 10,840	
Frost Arroyo					500	
At downstream study limit	24.8	3,520	6,480	8,740	17,500	
Upstream of unnamed tributary	10.5	1,710	2,910	3,790	7,580	
Rio Grande			10.000	15 000	24 200	
At downstream study limit	14,650	3,300	10,200	15,200	34,200	
At upstream study limit	14,100	4,000	11,850	17,860	37,900	
Rio Puerco	0 400	15 400	26,900	32,600	47,900	
At Interstate Highway 40	2,490	15,400	20,700	32,000		
San Pedro Creek	7 0/	1 040	2 140	2,990	5,980	
Within detailed study limit	7.24	1,040	2,140	2,770	2,000	

FIGURE 6 - SUMMARY OF PEAK DISCHARGES - UNNAMED ARROYO

(From Flood Insurance Study, Bernalillo County, New Mexico and Incorporated Areas - FEMA, Revised September 20, 1996)

Table 2. Summary of Discharges (Cont'd)

•	Drainage Area	Peak D	ischarges (cul	bic feet per	second)
Flooding Source and Location	(square miles)	10-Year	50-Year	100-Year	500-Year
Tijeras Arroyo					
At downstream study limit Upstream of confluence with Cedro	67.1	5,080	9,970	14,060	28,100
Canyon Arroyo	37.5	4,120	7,890	11,100	22,200
Upstream of confluence with Arroyo San Antonio Upstream of confluence with South	. 20.6	2,670	5,210.	7,320	14,640
Diversion Channel	114	4,340	9,150	14,700	29,400
At USGS gage, 1 mile upstream of study limit	75.3	5,120	10,000	14,300	28,600
Unnamed Tributary No. 1 At Interstate Highway 40	8.78	1,290	3,220	4,450	8,550
Arroyo A-B					
At Sage Road	0.1	81	149	179	1,767
Arroyo A-C At the Arenal Main Canal	0.2	122	247	263	372
Arroyo B-A At Unser Boulevard	0.1	29	67	• 90	126
Arroyo B-B At Unser Boulevard	0.5	71	225	338	544
Arroyo B-Ç At Unser Boulevard	0.7 -	138	342	494	757



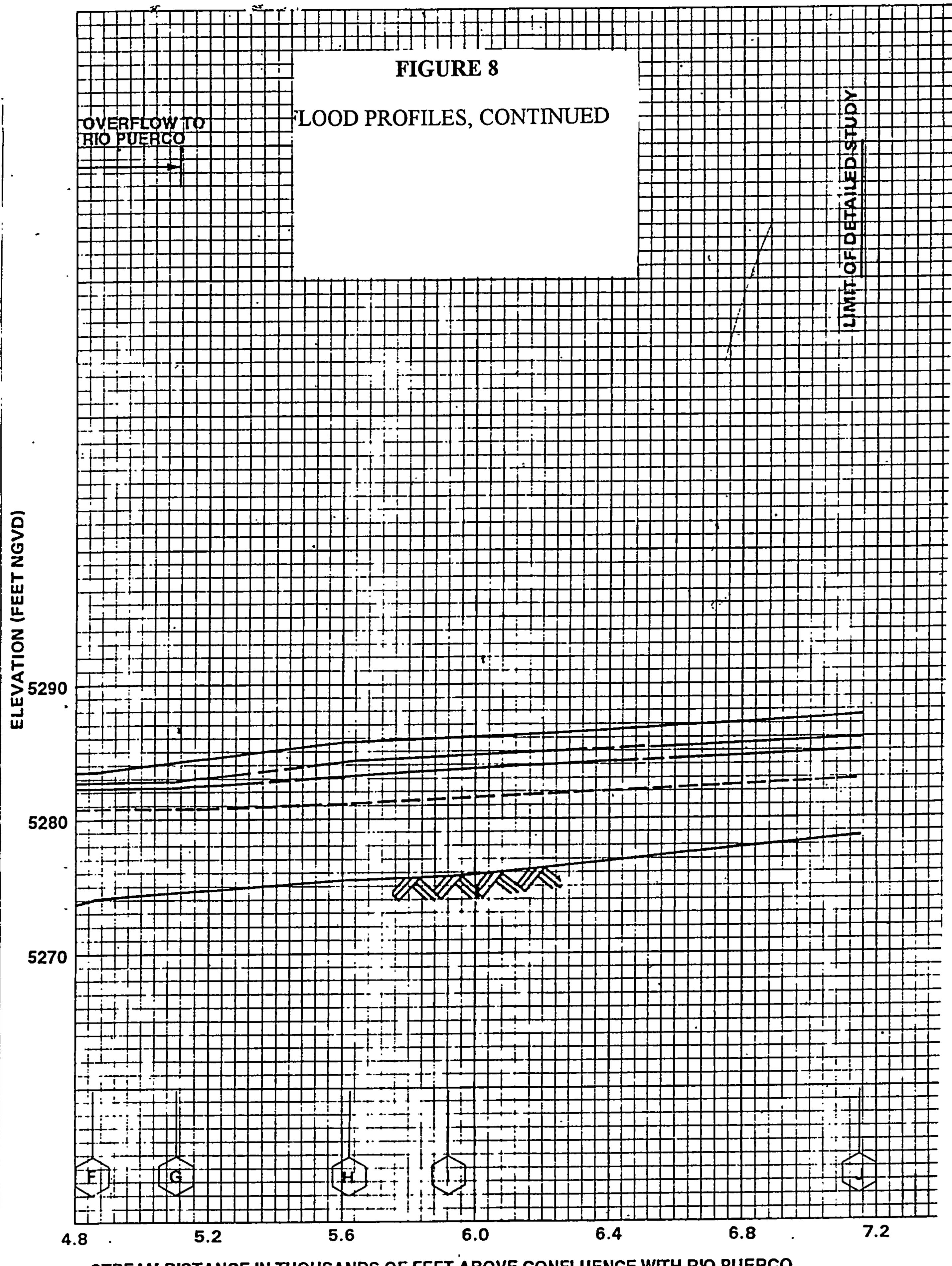


FIGURE 9

BERNALILLO COUNTY SOIL MAP

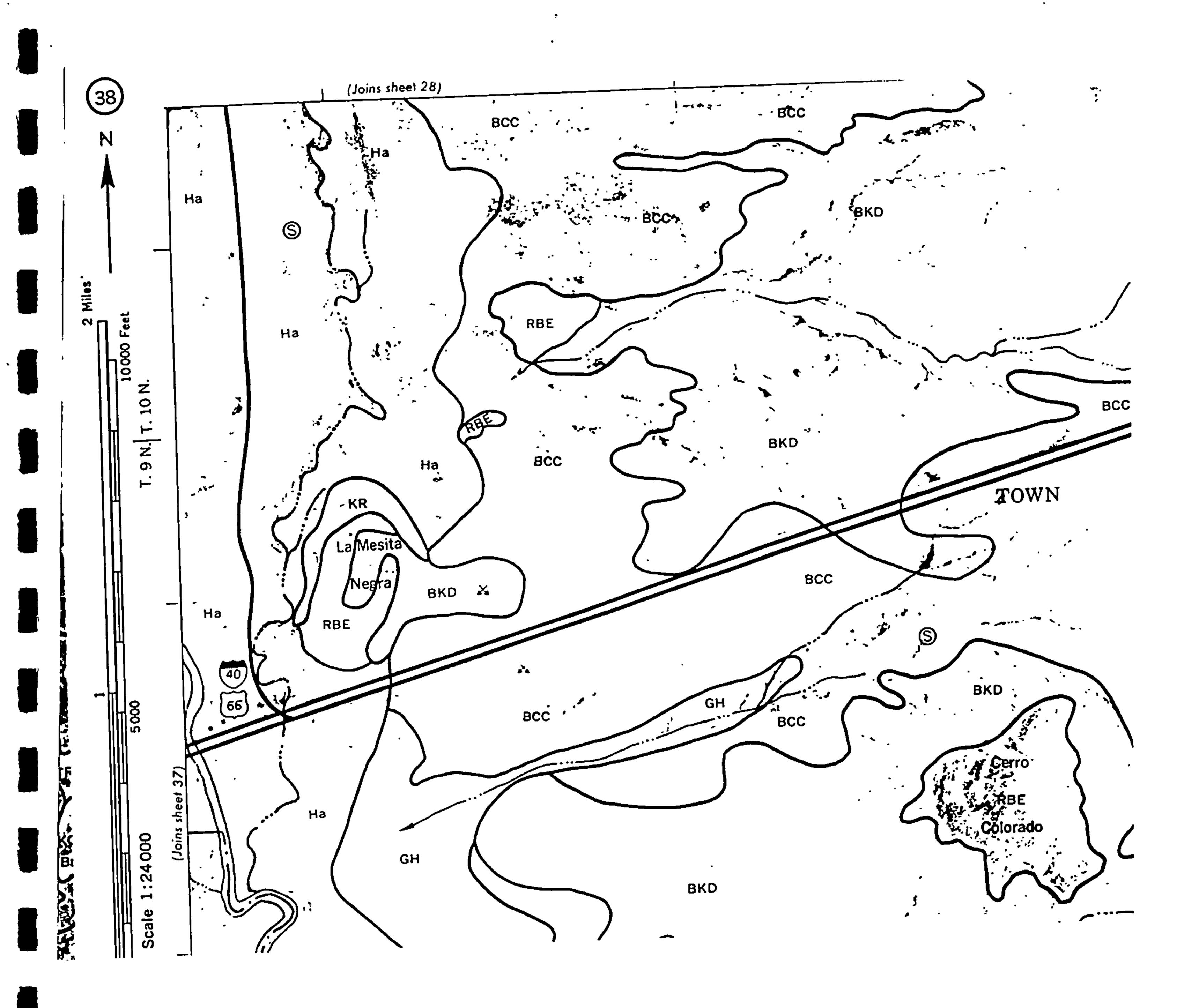
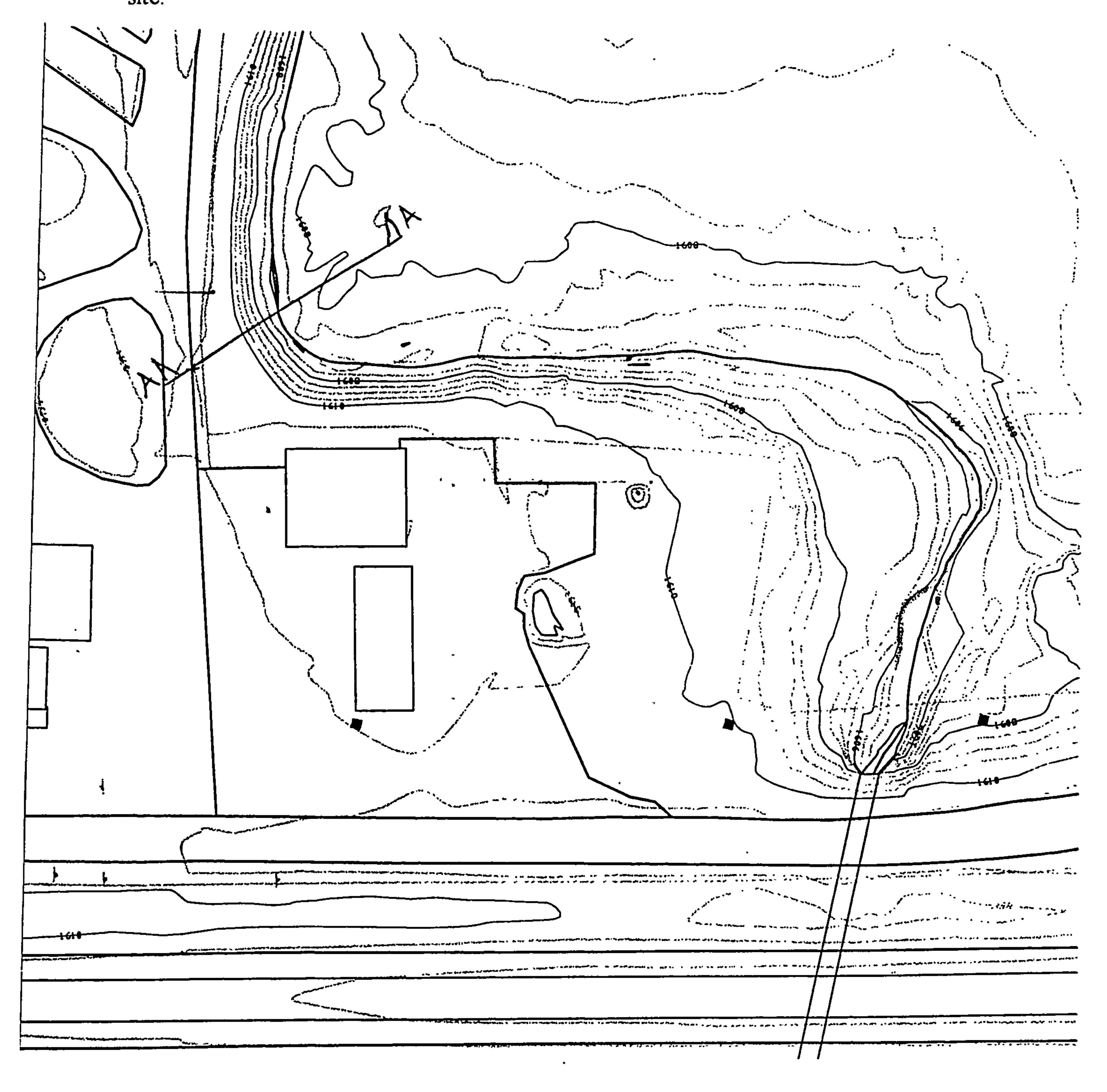


FIGURE 10

HIGHWAY DEPARTMENT TOPO MAP - SHOWING SHAPE OF UNNAMED ARROYO

This map is show for the purpose of showing the shape of the east bank of the unnamed arroyo which is not fully shown on the topographic survey that was performed for this site.



CONCLUSIONS:

- 1. The finish floor of the proposed convenience store should be at a minimum elevation of
- 5283.00, the elevation of the 100-year AO Floodplain.
- 3. The issue of moving the alignment of the existing frontage road (Old U.S. 66) to the
- north has been resolved with the New Mexico State Highway and Transportation Department.

County of Vernalillo

State of New Mexico

BOARD OF COUNTY COMMISSIONERS
TOM RUTHERFORD, CHAIR
DISTRICT 3
BARBARA J. SEWARD, VICE CHAIR
DISTRICT 4
KEN SANCHEZ, MEMBER
DISTRICT 1
STEVE D. GALLEGOS, MEMBER
DISTRICT 2
LES HOUSTON, MEMBER
DISTRICT 5
JUAN R. VIGIL, COUNTY MANAGER



2400 BROADWAY, S.E. ALBUQUERQUE, NEW MEXICO 87102 PUBLIC WORKS (505) 848-1500 DAVID K. ANDERSON, ASSESSOR
JUDY D. WOODWARD, CLERK
THOMAS J. MESCALL, PROBATE JUDGE
JOE BOWDICH, SHERIFF
ORLANDO VIGIL, TREASURER

December 23, 1997

Frank D. Lovelady, P.E. Lovelady & Associates 300 Alamosa Road NW Albuquerque, New Mexico 87107

RE: Drainage Report and Grading and Drainage Plan for Bowlin's Convenience Store, Rio Puerco (M3/D1) (PWD-97-207) Engineer's Stamp Dated 12/2/97.

Dear Mr. Lovelady:

This letter is a compilation of comments from myself as well as from Bernalillo County Public Works Division. Prior to approval for Building Permit release, the following comments must be addressed:

- 1. It appears that this site must be protected against the arroyo on the northeast corner. Has bank protection been provided? Or is it proposed? A HEC-RAS or other analysis based on the downstream culvert capacity will be required. Consider a scour analysis based on the culvert capacity.
- How will the realignment of the frontage road effect this site? Is information from the Highway Department available? Or will this information be provided in the future?
- The Flood Insurance Study gives the elevation of the water surface adjacent to this site as 5283. Please provide justification for the floor elevation used.

If you should have any questions regarding these comments, please call me at 924-3982.

Sincerely,

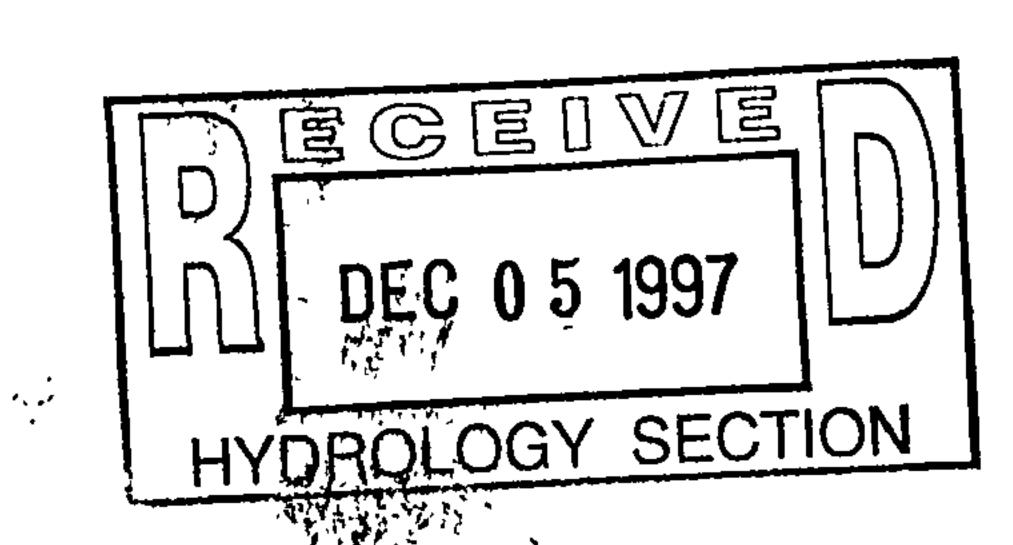
Susan M. Calongne, P.E.

City/County Floodplain Administrator

c: Brad Catanach, Bernalillo County Public Works Division
File

Rio Pilerco, unamata ma	OURITATION DITTON
20 JECT TITLE: Bowlin's Convenience Store	ZONE ATLAS/DRNG. FILE #: M-5/
TOTAL	WORK ORDER #:
EGAL DESCRIPTION: Tract B-1 Land of Bo	wlins & Por. Lands of Geo. T. Hill
ITY ADDRESS: Rio. Puerco. N.1	η,
NGINEERING FIRM: Lovelady & Associates	CONTACT: Frank D. Lovelady
ADDRESS: 300 A LAMOSA NW	
MIER: BOWLIN'S INC.	CONTACT: FRANK LOVELAOY
: ADDRESS: 150 LOUISIANA N	E PHONE: 266-5985
	CONTACT: S. Duane Dorsey
ADDRESS: Las Cruces NM	
SURVEYOR: Harris Surveying Co	contact: Tony Harris
ADDRESS: 2412 - D MONROE NE	B7110 PHONE: 889-8056
CONTRACTOR: - GEN CON	CONTACT:
ADDRESS: P.O. Box 448 Mesilla	NM PHONE: 523-4556.
EROSION CONTROL PLAN ENGINEER'S CERTIFICATION OTHER PRE-DESIGN MEETING: YES NO COPY PROVIDED DEC 0 5 1997 HYDROL SECTION	SKETCH PLAT APPROVAL PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D. APPROVAL S. DEV. PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL PAVING PERMIT APPROVAL S.A.D. DRAINAGE REPORT DRAINAGE REQUIREMENTS OTHER (SPECIFY) Review By City/County Flood Digin Administrator
DATE SUBMITTED: Dec 3,1997	Flood Plain Administrator
BY: Frank D'Toweliebe,	

If encionates are not as much, bindy notify us at crist.

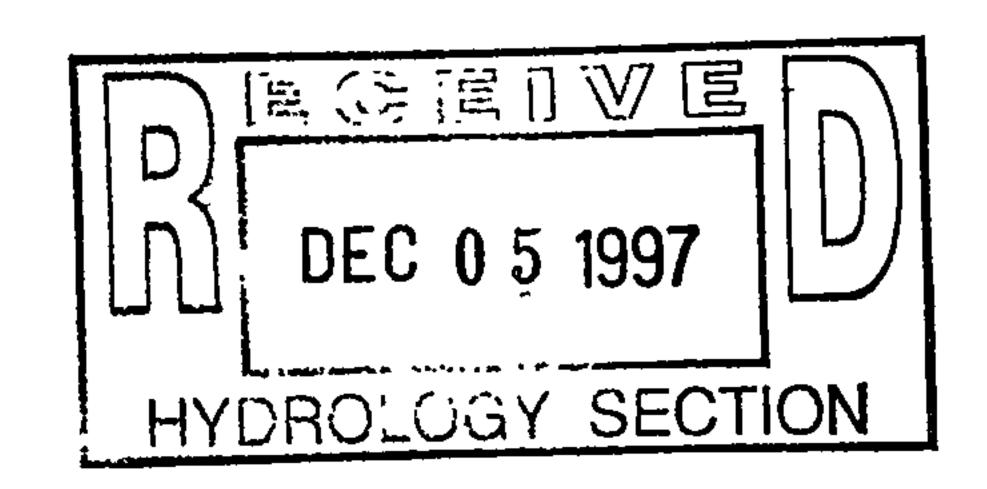


COPY TO

Case Routing Slip

03-	Dec-9	7 PWD - 97	- 207	Category:	Submittal		
Applicant Na	ame:	BOWLINS INC		Application D)ate: 03-De	: 03-Dec-97	
Applicant Ca	ategory:	Owner	ļ	Due Date:	24-De	ec-97	
		Owner Info	Agen	t Info	Egr/Svy Inf	0	
Name	BOWL	INS INC	FRANK D LC	OVELADY		-	
Address	150 LC	UISIANA BLVD N	300 ALAMOS	SANW	•		
City	Albuqu	erque	ALBUQUERO	QUE			
State	NM		NM		ł (
ZIP	87108		87107		•		
Ph	266-59	85	345-2267		 		
Legal Descr	iption:	TR B LAND OF BC	WLIN & N PO	RT LANDS OF (GEORGE HILL		
UPC:					Zone Map	M-3	
Street Addre	ess:	WEST OF RIO PUE	ERCO				
Submittal Type:		Grading & Drainage	Plan			<u></u>	
Comments Required From:							
DRAN Z	DRE	ENGA	INSP	TRAF	UTIL [] OTHE		
				-		•	
	•						

Brud



Organal 12-4-97

BERNALILLO COUNTY PWD SUBMITTAL Use for all PWD applications EXCEPT Street Excavation MEW SUBMITTAL RESURVITI. IL CASE NO: 1001-97-207 FINAL SIGNOFF TODAY'S DATE: WNER PHONE 266-5985 BOWLIN'S INCi OWNER 150 LOUISIANA BLV.D. NE CON ALBUQ ZIP 87108 MAILING **ADDRESS** AGENT! FRAKK. D. LOVELADY PHONE :- 345-2267 CONTRACTOR -MAILING CITY ALBUQ ZIP 87107. 300 ALAMOSA HU ADDRESS EXP STATE CLASS VOLUME LICENSE NO. FRANK D. LOVELADY LICENSENO HMDE 6512 PHONE 345-2267 ARCHITECT/ENGINEER ATLAS NO.: W/A . M-3 TAKE I-40 WEST TO RIO PUERCO. SITE ADDRESS / DIRECTIONS MOIL TAKE EXIT BEFORE CROSSING BRIDGE SITE IS ON THE RIGHT INFORMA TRACT B LAND OF BOWLING AND A **LEGAL DESCRIPTION** PORTION OF LANDS OF GEDRGE T. HILL PROPOSED EXISTING NO EXISTING BUILDINGS CONVENIENCE STORE BUILDING(S): BUILDING(S) AND USE: UPC TYPE OF SUBMITTAL TRAFFIC IMPACT ANALYSIS / TRAFFIC STUDY REPLAT INFRASTRUCTURE LIST / DESIGN REVIEW MINOR-SUBDIVISION SPECIAL USE PERMIT MAJOR SUBDIVISION [三〇][三 BARRICADING PERMIT CONSTRUCTION DRAWINGS **BUILDING PERMIT** GRADING & DRAINAGE PLAN DEC. Q 5, 1997 INSPECTION AS-CONSTRUCTED GRADING & DRAINAGE PLAN OTHER (Specify): HYDROLOGY SECTION VARIANCE REQUEST LAND DIVISION The issuance of a permit or a review or approval of plan specifications, computations, and shop drawings, shall not be interpreted to be a permit for, or an approval of any variance or violation of any of the provisions of any COUNTY or STATE codes, ordinances, standards, or policies. Nor shall such issuance of a permit or approval of plans, specifications, computations, and shop drawings prevent any authorized COUNTY representative or COUNTY inspector form thereafter requiring the correction of errors in said plans, specifications, computations, or shop drawings or from stopping construction operations which are being carried on thereunder when in violation of any COUNTY or STATE codes, ordinances, standards, or policies. Owner

BERNALILLO COUNTY USE ONLY

Received By:

CL_PWD App-Rev: November 29, 1995

TOTAL FEE:

Receipt No.:

DRAINAGE REPORT

FOR

BOWLIN'S CONVENIENCE STORE AND TRAVEL STORE RIO PUERCO

Bernalillo County, New Mexico

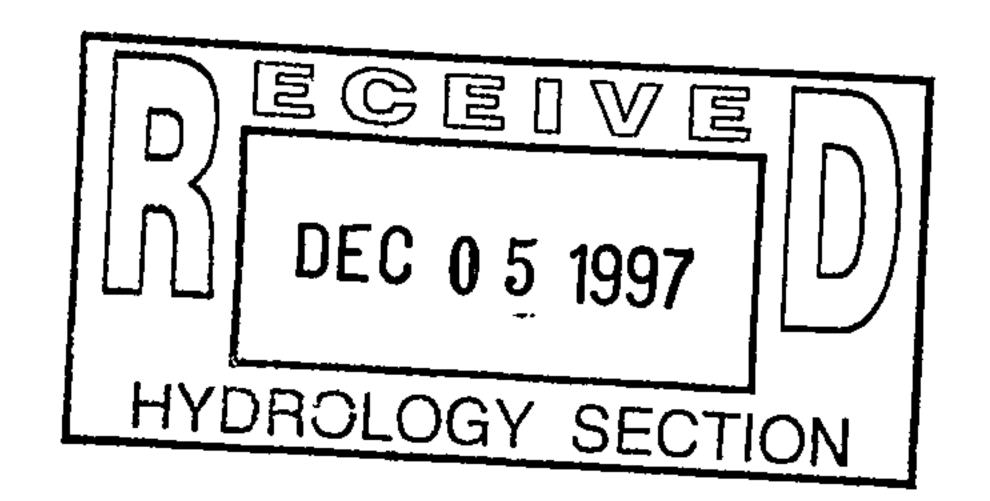
December 2, 1997

Prepared by:

Frank D. Lovelady, P.E.

300 Alamosa NW

Albuqueque, New Mexico 87107



DRAINAGE REPORT

FOR

BOWLIN'S CONVENIENCE STORE AND TRAVEL STORE

RIO PUERCO

Bernalillo County, New Mexico



ENGINEER'S STAMP

SCOPE:

This report has been prepared for the purpose of obtaining a building permit for the construction of a convenience store/travel store on property abutting the north frontage road of Interstate Highway 40 east of the existing bridge crossing the Rio Puerco in Bernalillo County, New Mexico. The on-site runoff generated by the 100-year storm has been addressed and off-site flow generated by the unnamed arroyo west of the site is also addressed including the capacity of the arroyo and box culvert and it impact on the site. Initial steps have been taken in the process of initiating a Traffic Impact Analysis.

EXISTING CONDITIONS:

The site includes Tract B-1, Lands of Bowlin (0.8975 ac) and 1.25 acres west of Treact B-1 and 0.11 acre north of Tract B-1, both of which are a part of "Northerly Portion of Lands of George T. Hill, a Tract containing 32.8782 acres. The site is located adjacent to the North Frontage Road of Interstate Highway 40 approximately 1,000 feet east of the existing east bridge abutment of the North Frontage Road bridge over the Rio Puerco. Directly east of the site is an unnamed arroyo which has a concrete box culvert under the North Frontage Road and the westbound and eastbound lanes of I-40. The box culvert is approximately 225' east of the east boundary of the site.

The site is geographically located on a relatively flat bench of land lying between the Rio Puerco and the unnamed arroyo. At the widest part, adjacent to the I-40 North Frontage Road, the bench is approximately 1,400 feet between the west bank of the unnamed arroyo and the east bank of the Rio Puerco. At the narrowest point, 400 to 500 feet north of the frontage road, the width is approximately 900 feet. A berm has been

constructed at some time in the past from the unnamed arroyo to the Rio Puerco at approximately a 30 degree angle with Interstate Highway 40 alignment. The berm apparently was constructed to protect property improvements in the bench area from flooding resulting from overflow of the unnamed arroyo and to convey any such overflow into the Rio Puerco. The property improvements consist of a residence, a mobile home and several garage and service station buildings. The garage and service station buildings have recently been razed. The residence and mobile home are still present.

Adjacent to the site, the distance between the frontage road pavement and the right-of-way line is approximately 50 feet. The distance between the north edge of the westbound lane and the south edge of pavement on the frontage road is approximately 85 feet. The exit ramp from I-40 westbound lanes to the frontage road is east of the unnamed arroyo.

PROPOSED CONDITIONS:

It is proposed to construct a convenience store and fuel pump canopy as shown (See Grading and Drainage Plan at the back of this report). The Highway and Transportation Department District Engineer for District 3 has indicated a need to move the frontage road / further north to give a wider separation at the off-ramp location. The frontage road would then transition back to its present location between the convenience store and the Rio Puerco Bridge. Of course, the Highway Department has design standards that must be adhered to but the question that arises is this: Why should the transition to the present alignment take place between the convenience store and the Bridge? It would seem that a transition with reverse curves would not be appropriate for the approach to a bridge. It would seem that the approach to the bridge should be as straight as possible for as long a distance as possible. Again, it would seem that the transition from a wider separated frontage road alignment to the existing frontage road alignment would be more logically made east of the unnamed arroyo. Offsetting the frontage road at the location of the convenience store will have a very negative impact on the proposed development. Other development currently taking place in the area may be influencing the constraints being placed on this site. Therefore, until a Traffic Impact Analysis has been approved, this

report is based on the assumption that the owner will be allowed to develop the site as shown.

FLOOD INSURANCE RATE MAPS (FIRM):

The site is covered by Flood Insurance Rate Maps (FIRM) 35001C0287 D and 35001C0291 D. The site is, for the most part, in Zone AO (depth 1). The extreme easterly end of the site in Zone AE which encompasses the unnamed arroyo and a reasonable distance on either side of the arroyo. The definitions for the two zones are as follows:

Zone AO Flood depths of one to three feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities are also determined.

Zone AE - Base flood elevations determined.

Portions of the two FIRM maps which show the site have been spliced together and are presented in Figure 2.

FLOOD INSURANCE STUDY:

The Federal Emergency Management Agency has published a two-volume study entitled "Flood Insurance Study, Bernalillo County, New Mexico and Incorporated Areas, Volumes 1 and 2 of 2", Revised September 1996. Pages from this study have been included in this report as Figures 4 through 8. These pages are pertinent to the site in that they include information relative to the unnamed arroyo adjacent to the site.

OFF-SITE DRAINAGE CALCULATIONS:

There is really no off-site flow, as such, effecting the site. The site is adjacent to the unnamed arroyo and is, therefore, subject to overflow from the arroyo that may occur because the box culvert under I-40 is not sufficiently large to accomodate the 100-year peak discharge. This can be seen from the FEMA Flood Profile which is shown in the back of this report. Figure 5 shows Table 2, a FEMA listing of peak discharge rates for various analysis points, including the unnamed Tributary No. 1, which is believed to be the unnamed arroyo adjacent to the site. The 100-year peak discharge rate for the unnamed arroyo 4,450 cubic feet per second. The sections of the unnamed arroyo shown in Figure 4 show that the first section above the box culvert, Section D, has a water surface elevation of 5282.5' (NGVD). The second section, Section E, which is 419 feet upstream from Section D, has a water surface elevation of 5282.5' (NGVD). The third section, Section F, which is 881 feet upstream from Section E, has a water surface elevation of 5282.6' (NGVD).

The mean velocity at Section E, adjacent to the site, is 2.7 feet per second. This allows the Froude Number, F_R , to be calculated as follows:

 $F_R = (V^2/gd)^{1/2}$ $d = Y_1 + V^2/2g$ $Y_1 = Flow depth, or water surface elevation minus bottom elevation (<math>Y_1 = 5282.5 - 5270.0 = 12.5$)

$$d = 12.5 + (2.7^2 / 2 * 32.2) = 12.5 + 0.11 = 12.61$$

$$F_R = [2.7^2 / (32.2 \times 12.61)]^{1/2} = (0.134)$$

ON-SITE DRAINAGE CALCULATIONS:

The site is almost entirely paved with a small area of landscaping on either side of the proposed convenience store.

Tract size - 2.2575 Acres.

Precipitation Zone 1, (DPM Part A.1)

Excess Precipitation, E (DPM Part A.5)

$$A = 0.44$$
" $B = 0.67$ " $C = 0.99$ " $D = 1.97$ " (100-year, 6-hour)

Assumptions:

Treatment A, Areas remaining natural	0 sf	0.0000 ac.
Treatment B, Landscaping	836 sf	0.0096 ac
Treatment C, Gravel Areas	0 sf	0.0000 ac
Treatment D, Building and Paved Areas	97,501 sf	2.2479 ac.
Totals	98,337 sf	2.2575 ac.

Volume, 100-year, 6-hour:

$$V = (E / 12)(A)$$

Treat B
$$V = (0.67/12)836 = 47 cf$$

Treat D
$$V = (1.97/12)(97.501) = 16,006 cf$$

Total
$$= 16,053 \text{ cf}$$

Peak Discharge, 100-year:

$$Q = (q)(A)$$

Peak Discharge per Acre (DPM, Table A-9);

q:
$$A = 1.29 B = 2.03 C = 2.87 D = 4.37$$

Treat B $Q = 1.29 \times 0.0096 = 0.01 \text{ cfs}$

Treat D $Q = 4.37 \times 2.1033 = 9.82 \text{ cfs}$

Total 9.93

FIGURE 1 - VICINITY MAP

SHOWN ON A PORTION OF USGS QUADRANGLE MAP LA MESITA NEGRA, NM

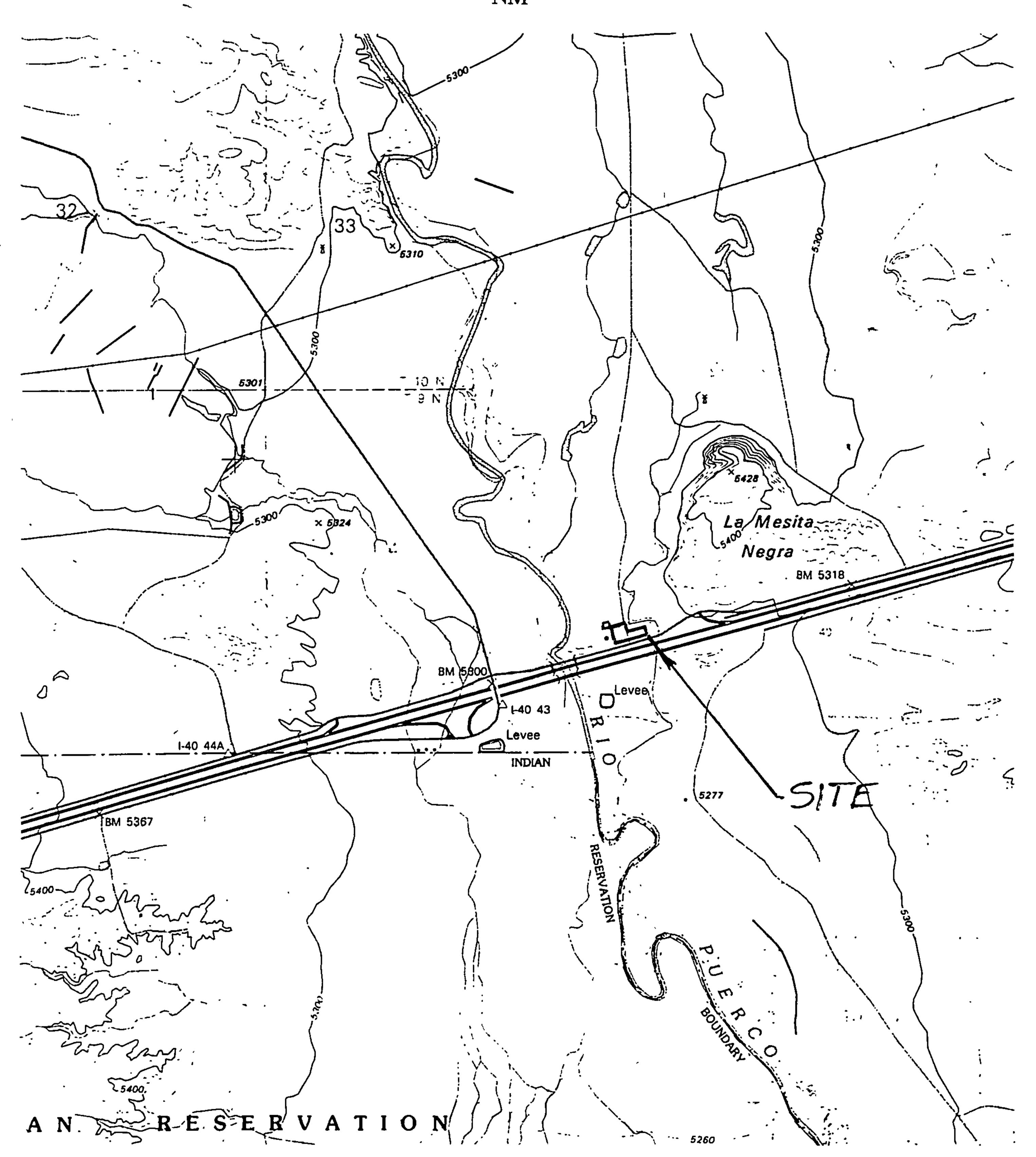


FIGURE 2 - FLOOD INSURANCE RATE MAP (FIRM)

PORTION OF MAPS 35001C0287 D AND 35001C0291 D

(For interpretation of symbols on this map, see Figure 3)

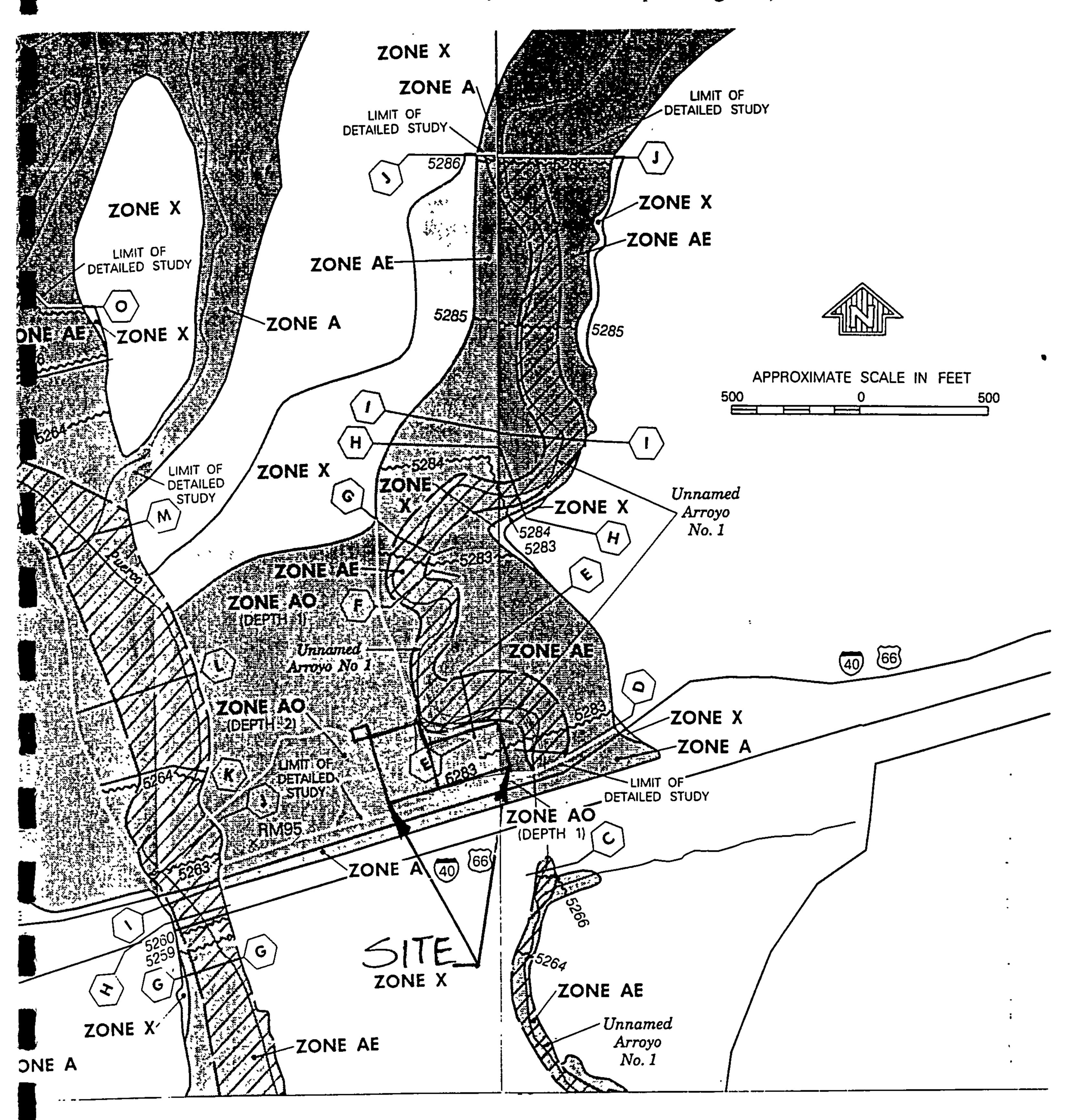
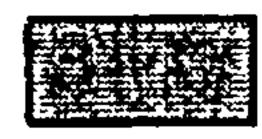


FIGURE 3 - LEGEND AND NOTES FOR FIRM MAP.

LEGEND



SPECIAL FLOOD HAZARD AREAS INUNDATED BY 100-YEAR FLOOD

ZONE A

No base flood elevations determined.

ZONE AE

Base flood elevations determined.

ZONE AH

Flood depths of 1 to 3 feet (usually areas of ponding); base flood elevations

determined.

ZONE AO

Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.

ZONE A99

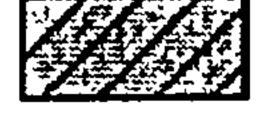
To be protected from 100-year flood by Federal flood protection system under construction; no base elevations determined.

ZONE V

Coastal flood with velocity hazard (wave action); no base flood elevations determined.

ZONE VE

Coastal flood with velocity hazard (wave action); base flood elevations determined.



FLOODWAY AREAS IN ZONE AE

OTHER FLOOD AREAS

ZONE X

Areas of 500-year flood; areas of 100-year flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 100-year flood.



OTHER AREAS

ZONE X

Areas determined to be outside 500-year floodplain.

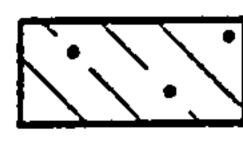
ZONE D

Areas in which flood hazards are undetermined.

UNDEVELOPED COASTAL BARRIERS







Identified 1983

Identified 1990

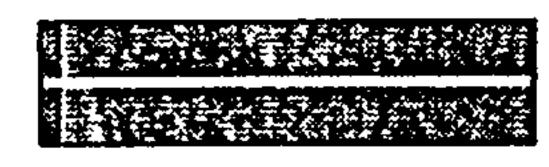
Otherwise Protected Areas

Coastal barrier areas are normally located within or adjacent to Special Flood Hazard Areas.

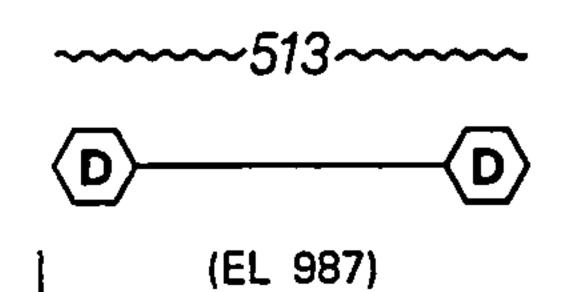
Flood Boundary

Floodway Boundary

Zone D Boundary



Dividing Special Flood Boundary Boundary Hazard Zones, and Different Dividing Areas **Elevations** Flood Base Coastal Special Flood Hazard Within Zones.



RM7 ×

Base Flood Elevation Line; Elevation in Feet. See Map Index for Elevation Datum.

Cross Section Line

Base Flood Elevation in Feet Where Uniform Within Zone. See Map Index for Elevation Datum. Elevation Reference Mark

• M2

River Mile

97°07'30", 32°22'30"

Horizontal Coordinates Based on North American Datum of 1927 (NAD 27) Projection.

NOTES

This map is for use in administering the National Flood Insurance Program: - it does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size or all planimetric features outside. Special Flood Hazard Areas

Coastal base flood elevations apply only landward of 0.0 NGVD, and include the effects of wave action, these elevations may also differ significantly from those developed by the National Weather Service for humane evacuation planning.

Areas of Special Flood Hazard (100-year flood) include Zones A. AE, AH, AO, A99, V, and VE.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the Federal Emergency Management Agency.

Floodway widths in some areas may be too narrow to show to scale. Floodway widths are provided in the Flood Insurance Study Report.

This map may incorporate approximate boundaries of Coastal Barrier Resource System Units and or Otherwise Protected Areas established under the Coastal Barrier Improvement Act of 1990 (PL 101-591).

Corporate limits shown are current as of the date of this map. The user should contact appropriate community officials to determine if corporate limits have changed subsequent to the issuance of this map.

For community map revision history prior to countywide mapping, see Section 6.0 of the Flood Insurance Study Report.

For adjoining map panels and base map source see separately printed Map Index

MAP REPOSITORY

Refer to Repository Listing on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP:

SEPTEMBER 20 1996

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL:

Refer to the FLOUD INSURANCE RATE MAP EFFECTIVE DATE shown on this map to determine when actuarial rates apply to structures in zones where elevations or depths have been established.

To determine if flood insurance is available, contact an insurance agent or call the National Flood Insurance Program at (800) 638-6620.



APPROXIMATE SCALE IN FEET

500 0 500

FIGURE 4 - FLOODWAY DATA

(From Flood Insurance Study, Bernalillo County, New Mexico and Incorporated Areas - FEMA, Revised September 20, 1996)

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECONO)	REGULATORY	FLOODWAY	FLOODWAY NGVD)	INCREASE
Frost Arroyo (Cont'd) AA AB AC AD Unnamed Arroyo No. 1 A B C D E F G H I J	12,660 ¹ 12,310 ¹ 12,880 ¹ 13,365 ¹ 1,300 ² 2,030 ² 3,100 ² 3,550 ² 3,974 ² 4,855 ² 5,105 ² 5,625 ² 5,924 ² 7,155 ²	150 100 120 120 140 180 140 150 93 173 193	358 483 320 323 140 266 167 1,935 1,660 892 593 643 1,059 936	8.5 6.3 9.5 9.4 10.0 5.3 8.4 2.3 2.7 5.0 7.5 6.9 4.2 4.8	6,710.8 6,711.6 6,714.8 6,723.1 5,259.5 5,266.0 5,282.5 5,282.6 5,282.6 5,282.6 5,284.2 5,284.6 5,285.8	6,710.8 6,711.6 6,714.8 6,723.1 5,255.4 5,259.5 5,266.0 5,282.5 5,282.6 5,282.6 5,284.2 5,284.6 5,284.8	6,711.1 6,711.8 6,714.8 6,723.6 5,255.4 5,259.5 5,266.0 5,282.5 5,282.6 5,282.6 5,282.6 5,284.2 4,285.1 5,286.1	0.3 0.0 0.0 0.0 0.0 0.0 0.0 0.5 0.3

lFeet Above Confluence With San Pedro Creek

²Feet Above Confluence With Rio Puerco

³Elevations Computed Without Consideration of Backwater Effects

FIGURE 5 - SUMMARY OF PEAK DISCHARGES - RIO PUERCO

(From Flood Insurance Study, Bernalillo County, New Mexico and Incorporated Areas - FEMA, Revised September 20, 1996)

Table 2.	Summary	of	Discharges
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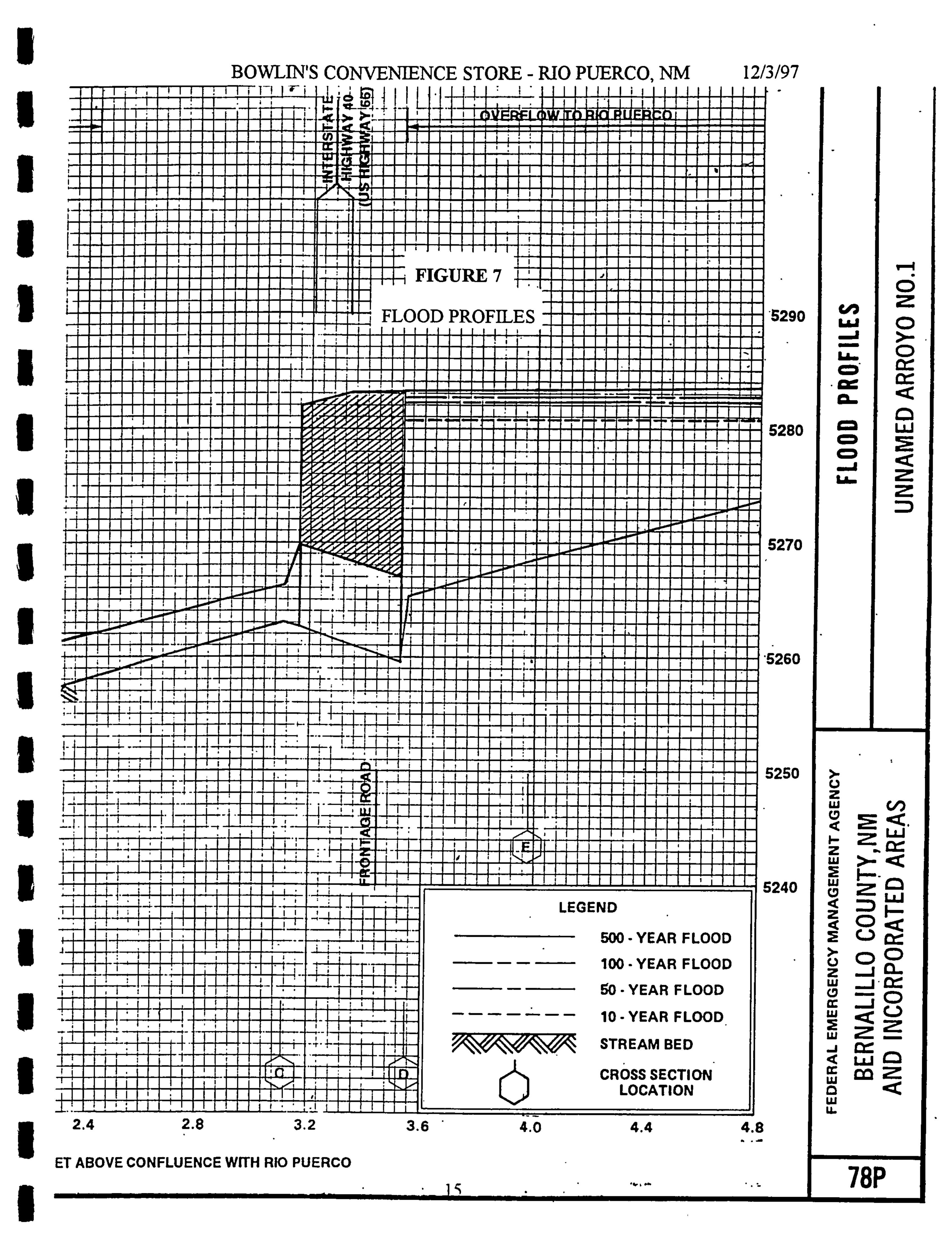
			•	•
Drainage Area	Peak D	ischarges (cul	oic feet per	second)
(square miles)	10-Year	50-Year	100-Year	500-Year
98.0	2,700	8.500	12.700	28,000
	•	- •		
85.1	2,460	7,800	11.900	26,200
71.9	2,300	7,200	11,000	24,500
76.5	5,100	8,870	10,800	15,800
18.9	. 1,830	3,730	5,420	- 10,840
24.8	3,520	6.480	8.740	17,500
10.5	1,710	2,910	3,790	7,580
			•	
14,650	3.300	10.200	15,200	34,200
14,100	4,000	11,850	17,860	37,900
······································				
2,490	15,400	26,900	32,600	47,900
7.24	1,040	2,140	2,990	5,980
	98.0 85.1 71.9 76.5 18.9 24.8 10.5 14,650 14,100	98.0 2,700 85.1 2,460 71.9 2,300 76.5 5,100 18.9 1,830 24.8 3,520 10.5 1,710 14,650 3,300 14,100 4,000 2,490 15,400	(square miles) 10-Year 50-Year 98.0 2,700 8,500 85.1 2,460 7,800 71.9 2,300 7,200 76.5 5,100 8,870 18.9 1,830 3,730 24.8 3,520 6,480 10.5 1,710 2,910 14,650 3,300 10,200 14,100 4,000 11,850 2,490 15,400 26,900	(square miles) 10-Year 50-Year 100-Year 98.0 2,700 8,500 12,700 85.1 2,460 7,800 11,900 71.9 2,300 7,200 11,000 76.5 5,100 8,870 10,800 18.9 1,830 3,730 5,420 24.8 3,520 6,480 8,740 10.5 1,710 2,910 3,790 14,650 3,300 10,200 15,200 14,100 4,000 11,850 17,860 2,490 15,400 26,900 32,600

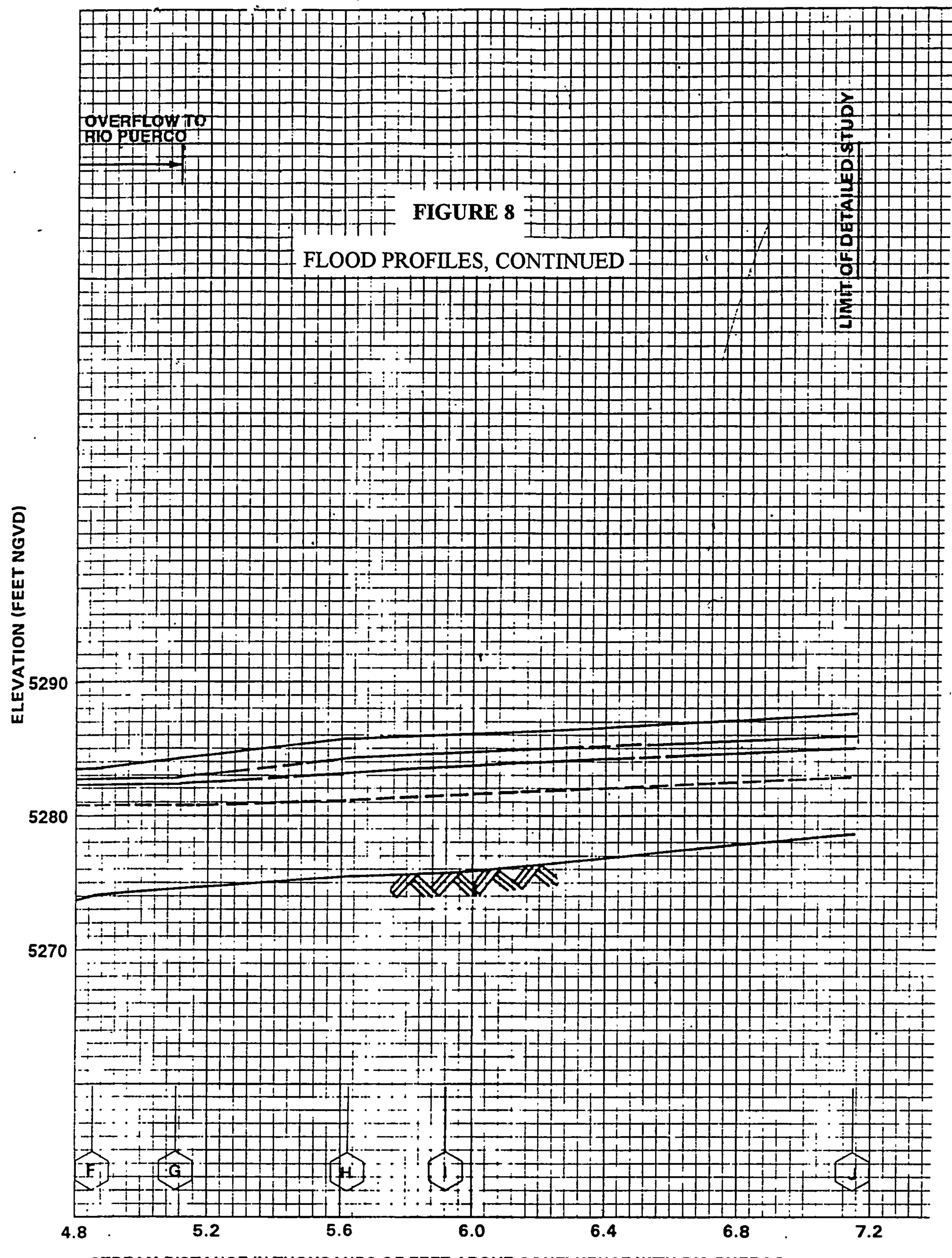
FIGURE 6 - SUMMARY OF PEAK DISCHARGES - UNNAMED ARROYO

(From Flood Insurance Study, Bernalillo County, New Mexico and Incorporated Areas - FEMA, Revised September 20, 1996)

Table 2. Summary of Discharges (Cont'd)

•	Drainage Area	Peak Discharges (cubic feet per second)			
Flooding Source and Location	(square miles)	10-Year	50-Year	100-Year	500-Year
Tijeras Arroyo					
At downstream study limit Upstream of confluence with Cedro	67.1	5,080	9,970	14,060	28,100
Canyon Arroyo	37.5	4,120	7,890	11,100	22,200
Upstream of confluence with Arroyo San Antonio Upstream of confluence with South	. 20.6	2,670	5,210.	7,320	14,640
Diversion Channel	114	4,340	9,150	14,700	29,400
At USGS gage, 1 mile upstream of study limit	75.3	5,120	10,000	14,300	28,600
Unnamed Tributary No. 1 At Interstate Highway 40	8.78	1,290	3,220	4,450	8,550
Arroyo A-B At Sage Road	0.1	81	149	179	1,767
Arroyo A-C At the Arenal Main Canal	0.2	122	247	263	372
Arroyo B-A At Unser Boulevard	0.1	29	67	90	126
Arroyo B-B At Unser Boulevard	0.5	71	225	338	544
Arroyo B-C At Unser Boulevard	0.7 .	138	342	494	757





STREAM DISTANCE IN THOUSANDS OF FEET ABOVE CONFLUENCE WITH RIO PUERCO

CONCLUSIONS:

- 1. The finish floor of the proposed convenience store should be at a minimum elevation of 5282.50, the elevation of the 100-year water surface at Section E of the unnamed arroyo. The elevation of the existing frontage road in front of the convenience store is 81.70 so the proposed finish floor elevation is somewhat conservative.
- 2. Attempts to obtain information from the Highway Department or from their consultant with regard to hydrologic or hydraulic studies of the unnamed arroyo were unsuccessful. Design of scour protection, if required, and rundown and other details cannot be successfully completed until this information becomes available.
- Moving the alignment of the existing frontage road (Old U.S. 66) to the north will leave little possibility for developing the site in any reasonable manner. It will also result in a less desireable approach to the bridge over the Rio Puerco.