

DRAINAGE STUDY FOR SUN GATE SUBDIVISION (FORMERLY KNOWN AS EL RANCHO GRANDE UNIT 12)

JANUARY 8, 2004

Prepared for:

Curb Inc.

6301 Indian School NE. Suite 208

Albuquerque, NM 87109

*Rec
1/9/04*

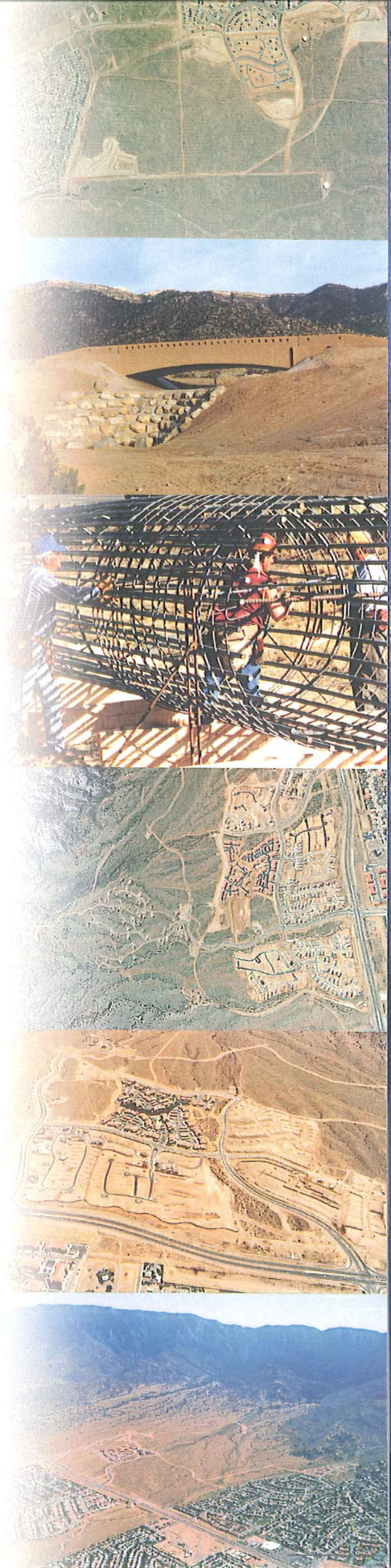
Bohannon ▲ **Huston** INC.

ENGINEERING ▲

SPATIAL DATA ▲

ADVANCED TECHNOLOGIES ▲

N-9/07



**DRAINAGE STUDY
FOR
SUN GATE SUBDIVISION
(FORMERLY KNOWN AS EL RANCHO GRANDE UNIT 12)**

January 8, 2004

Prepared for:

**CURB INC.
6301 INDIAN SCHOOL NE - SUITE 208
ALBUQUERQUE, NM 87110**

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PREPARED BY:


Rick L. Beltramo, P.E.


Date

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- EXHIBIT 1 - PRELIMINARY PLAT
- EXHIBIT 2 - GRADING PLAN
- EXHIBIT 3 - DEVELOPED CONDITIONS BASIN MAP
- EXHIBIT 4 - MASTER STORM DRAIN BASIN MAP

I. INTRODUCTION

This drainage study establishes a drainage management plan for the proposed development of Sun Gate Subdivision (formerly known as El Rancho Grande Unit 12). The Sun Gate Subdivision is approximately 21 acres of residential (R-2) land to be subdivided into 130 single family residential lots. The property is in the Rio Bravo Sector Plan and is located on Albuquerque's southwest mesa, south of the future Gibson Boulevard and west of Blake Road.

Sun Gate is in the Amole Arroyo Watershed and encompassed by the Amole-Hubbell Drainage Management Plan. In addition, a draft Drainage Management Plan (DMP) is being developed for the Gibson Boulevard corridor between 118th Street and the Amole Arroyo. The drainage area covered by the DMP is approximately 300 acres of residential, commercial, and special use zoned property that bound the north and south side of the future Gibson Boulevard between 118th Street to the west and the Amole Arroyo to the east. Sun Gate is in the Drainage Management Plan (DMP) area. Approval of the Gibson Boulevard DMP by the City of Albuquerque and AMAFCA is not required for approval of this Drainage Study for Sun Gate.

This study provides hydrologic and hydraulic analysis and provides a drainage management plan as necessary to support the planned 130-unit development. More specifically, this report is submitted in conjunction with the preliminary plat application. Preliminary plat approval and grading plan approval is requested. Prior to final plat and building permit approvals of this project, the City of Albuquerque (COA) must approve final grading plans and work order construction plans.

II. METHODOLOGY

Existing and proposed site hydrological conditions were analyzed for the 100-year, 6-hour storm in accordance with the revised Section 22.2, Hydrology, of the Development Process Manual (DPM) for the City of Albuquerque, dated January 1993. The Arid-lands Hydrologic Model (AHYMO) was utilized to determine peak flow rates for design of the storm drainage improvements within the project. The 100-year, 6-hour storm is used as the design event. The results are included in **Appendix A**. Street capacities were analyzed using Manning's equation, consistent with the revised DPM Section 22.2. The storm sewer system is analyzed

using current DPM methods for gravity flow conditions. All data and calculations supporting this study are located in **Appendix B**.

The hydrologic analysis is also based on the approved drainage report: Amole-Hubbell Drainage Management Plan, Volume I, Final Facilities Plan Report dated July 22, 1999, prepared by Leedshill-Herkenhoff, Inc.

III. EXISTING CONDITIONS

A. Topography

Sun Gate is currently undeveloped land with grades ranging from approximately 1% to 6%. The area generally slopes from northwest to southeast. Soils in the area have an SCS soil classification of BCC (Bluepoint loamy fine sand). BCC soils consist of deep, somewhat excessively drained soils formed in sandy alluvial soils, with rapid permeability, slow runoff characteristics, and severe hazard for wind erosion. Vegetation is light consisting mostly of native grasses.

B. Existing Drainage Patterns

Sun Gate is located in the Amole Arroyo Drainage Basin. The site generally drains from northwest to southeast. The only development in the area that has altered the natural drainage pattern of the area is El Rancho Grande Unit 11. This development diverts the offsite flow at the northwest corner of Unit 14 and prevents it from entering the site. The majority of the remaining offsite flow is conveyed south by a natural arroyo that is parallel to the western boundary of Units 14 and 15. Units 14 and 15 currently drain to offsite ponds.

IV. PROPOSED DEVELOPED CONDITIONS

Sun Gate subdivision is a proposed single-family residential development with 130 lots on 21 acres. Proposed street configurations are shown on the *Preliminary Plat*, **Exhibit 1**. The Amole-Hubbell DMP allows for full discharge of developed flows from the Amole Arroyo Basin to the Amole and Hubbell Lake storage facilities. The drainage concepts for Sun Gate is consistent with

those presented in the "Draft Master Drainage Study for the Gibson Boulevard Corridor between 118th Street and the Amole Arroyo". The Sun Gate drainage area corresponds to Basin DB8 in the Gibson Boulevard DMP.

The percent impervious land treatment for the proposed conditions is determined from Table A-5 of the DPM, Section 22.2. The land treatment values used in the AHYMO analysis are the same as the Gibson Boulevard DMP.

A. Offsite Flows

No offsite flows reach the site from the south or east because the natural ground slopes away from Sun Gate on these sides. The offsite flow from the north is intercepted by El Rancho Grande Unit 10 and Gibson Blvd..

The property to the west of Sun Gate is being developed as El Rancho Grande Units 14 and 15. The majority of the offsite flow to the west is conveyed south by a natural arroyo that is parallel to the western boundary of Units 14 and 15. A drainage swale will be graded west of Messina Drive that will convey this flow into the natural arroyo parallel to the western boundary. The temporary ponds created with Units 14 and 15 will be removed with the construction of Sun Gate and a master planned storm drain to be constructed in Open Range Avenue.

B. Onsite Flows

Developed runoff from Sun Gate will be conveyed by the internal street system to Garden Gate Lane, where it will be collected by a public storm drain system. Inlets are located along Garden Gate Lane at the east stub terminus and the intersection of Stone Gate Way. These inlets collect the runoff from the residential streets into a storm drain that discharges to the east at Open Range Avenue. See **Appendix B** for street capacity and inlet capacity calculations. This drainage plan proposes discharging 77.4 cfs to the storm drain in Open Range Avenue and Blake Road.

The storm drain outfall alignment for Sun Gate to the Amole Arroyo has not been identified at this time. The outfall alignment will be determined in the final Gibson

Boulevard DMP and constructed with future phases of the El Rancho Grande development. Therefore, interim facilities will be constructed to accept developed flows from Sun Gate. A retention pond sized to retain the 100-year, 24-hour storm volume will be constructed on Tract 34D-1-A, Lands of Curb Inc. just east of Blake Road. The pond will remain in place until all downstream storm drain improvements identified in the Gibson Boulevard DMP are in place and/or Tract 34-D-1-A is developed. See **Exhibit 2, Grading Plan**, for the location of the pond.

C. FEMA Floodplain

As designated on Panel 336 of 825 (Map number 35001C0336D) of the National Flood Insurance Program, Flood Insurance Rate Maps published by FEMA for Bernalillo County, New Mexico, effective date September 20, 1996, there is no existing flood hazard zone (zone AO) within the proposed development. See the FEMA Floodplain exhibit provided at the end of the report text.

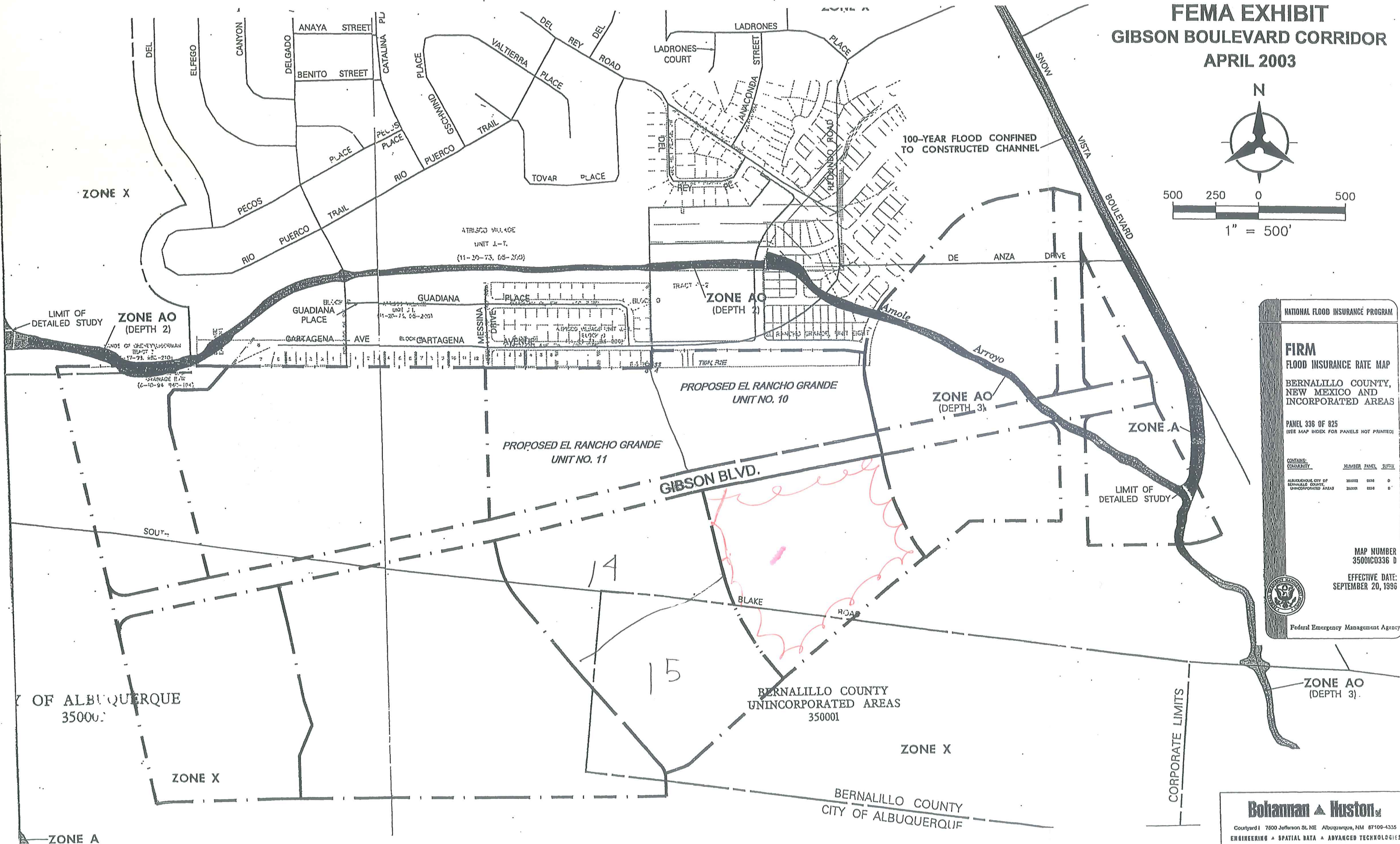
V. CONCLUSION

This report provides a detailed study of the developed runoff and street capacities for the proposed Sun Gate Subdivision. Included is the preliminary plat, proposed conditions basin map, grading plan, infrastructure list, and all necessary hydrologic and hydraulic analyses. This drainage plan maintains the overall drainage pattern of the area and allows for the safe management of storm runoff in permanent as well as interim conditions.

FEMA EXHIBIT GIBSON BOULEVARD CORRIDOR APRIL 2003



500 250 0 500
1" = 500'



NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP

BERNALILLO COUNTY,
NEW MEXICO AND
INCORPORATED AREAS

PANEL 336 OF 825
(SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS:	COMMUNITY	NUMBER	PANEL	SUFFIX
ALBUQUERQUE CITY OF	350002	0336	0	
BERNALILLO COUNTY, UNINCORPORATED AREAS	350001	0336	0	

MAP NUMBER
35001C0336 D

EFFECTIVE DATE:
SEPTEMBER 20, 1996



Federal Emergency Management Agency

ZONE AO (DEPTH 3)

Bohannon & Huston

Courtyard I 7600 Jefferson St. NE Albuquerque, NM 87109-4335
ENGINEERING • SPATIAL DATA • ADVANCED TECHNOLOGIES

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APPENDICES

- APPENDIX A - AHYMO INPUT AND SUMMARY FILES FOR
DEVELOPED CONDITIONS
- APPENDIX B - STREET CAPACITY AND STORM DRAIN
INLET ANALYSIS
- APPENDIX C - INFRASTRUCTURE LIST

APPENDIX A

AHYMO INPUT AND SUMMARY FILES
DEVELOPED CONDITIONS


```

*S*          PROJECT NAME:  SUNGATE SUBDIVISION (ERG 12)
*S*          DATE:    JANUARY 7, 2004
*S*          INPUT FILE NAME:  SUNGATE.HYM
*S*          OUTUPUT FILE NAME:  SUNGATE.OUT
*S*          PROJECT NUMBER:  040154
*S*          COMMENTS:    100 YEAR-6 HOUR STORM
*S* ///////////////////////////////////////////////////////////////////
START        TIME=0.0 HR PUNCH CODE=0
RAINFALL     TYPE=1      RAIN QUARTER=0.0
              RAIN ONE=1.90 IN    RAIN SIX=2.20 IN
              RAIN DAY=2.60 IN    DT=0.05 HRS

*****
*S*****
COMPUTE NM HYD      ID=1      HYD NO=BASIN.A      DA=0.0084 SQ MI
                    PER A=0.0   PER B=22.5   PER C=22.5   PER D=55.0
                    TP=-0.1333 HR    MASSRAIN=-1
PRINT HYD           ID=1      CODE=1
*S*****
COMPUTE NM HYD      ID=2      HYD NO=BASIN.B      DA=0.0037 SQ MI
                    PER A=0.0   PER B=22.5   PER C=22.5   PER D=55.0
                    TP=-0.1333 HR    MASSRAIN=-1
PRINT HYD           ID=2      CODE=1
*S*****
*S          ADD BASINS A AND B FOR DISCHARGE INTO GARDEN GATE LANE EAST STUB
ADD HYD            ID=3      HYD NO=A.B      ID I=1      ID II=2
PRINT HYD          ID=3      CODE=1
*S*****
COMPUTE NM HYD      ID=4      HYD NO=BASIN.C      DA=0.0059 SQ MI
                    PER A=0.0   PER B=22.5   PER C=22.5   PER D=55.0
                    TP=-0.1333 HR    MASSRAIN=-1
PRINT HYD           ID=4      CODE=1
*S*****
COMPUTE NM HYD      ID=5      HYD NO=BASIN.D      DA=0.0074 SQ MI
                    PER A=0.0   PER B=22.5   PER C=22.5   PER D=55.0
                    TP=-0.1333 HR    MASSRAIN=-1
PRINT HYD           ID=5      CODE=1
*S*****
COMPUTE NM HYD      ID=6      HYD NO=BASIN.E      DA=0.009 SQ MI
                    PER A=0.0   PER B=22.5   PER C=22.5   PER D=55.0
                    TP=-0.1333 HR    MASSRAIN=-1
PRINT HYD           ID=6      CODE=1
*S*****
ADD HYD            ID=7      HYD NO=D.E      ID I=5      ID II=6
PRINT HYD          ID=7      CODE=1
*S*****
*S          ADD BASINS C, D, E FOR DISCHARGE INTO GARDEN GATE/STONE GATE INTERSECTION
ADD HYD            ID=8      HYD NO=C.D.E      ID I=4      ID II=7
PRINT HYD          ID=8      CODE=1
*S*****
*S          TOTAL DEVELOPED FLOW FROM SUN GATE SUBDIVISION
ADD HYD            ID=9      HYD NO=TOTAL      ID I=3      ID II=8
PRINT HYD          ID=9      CODE=1
*S*****
FINISH

```

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
---------	---------------------------	-------------	-----------	--------------	----------------------	-----------------------	----------------------	--------------	----------	----------

PROJECT NAME: SUNGATE SUBDIVISION (ERG 12)

DATE: JANUARY 7, 2004

INPUT FILE NAME: SUNGATE.HYM

OUTPUT FILE NAME: SUNGATE.OUT

PROJECT NUMBER: 040154

COMMENTS: 100 YEAR-6 HOUR STORM

TIME= .00
 RAIN6= 2.200

```

RAINFALL TYPE= 1
*****
COMPUTE NM HYD BASIN.A - 1 .00840 18.89 .653
*****
COMPUTE NM HYD BASIN.B - 2 .00370 8.33 .287
*****
BASINS A AND B FOR DISCHARGE INTO GARDEN GATE LANE EAST STUB
A.B 1& 2 3 .01210 27.22 .940
*****
ADD HYD
*****
COMPUTE NM HYD BASIN.C - 4 .00590 13.27 .458
*****
COMPUTE NM HYD BASIN.D - 5 .00740 16.64 .575
*****
COMPUTE NM HYD BASIN.E - 6 .00900 20.24 .699
*****
ADD HYD D.E 5& 6 7 .01640 36.88 1.274
*****
D, E FOR DISCHARGE INTO GARDEN GATE/STONE GATE INTERSECTION
C.D.E 4& 7 8 .02230 50.15 1.733
*****
TOTAL DEVELOPED FLOW FROM SUN GATE SUBDIVISION
TOTAL 3& 8 9 .03440 77.36 2.673
*****
FINISH
  
```

APPENDIX B

STREET CAPACITY AND STORM DRAIN INLET ANALYSIS

SUN GATE SUBDIVISION
Internal Street Capacity Calculations
January 2004

1. **Corral Gate Lane**
(See Basin Map)
 $Q = 8.3$ cfs

The amount of developed runoff produced from Basin B does not exceed the street capacity. Therefore, inlets are not required on this street. Roll curb may be installed to the eastern edge of the basin. Flow will continue on the surface east towards Bridal Gate Trail. See PC stream output.

2. **Bridal Gate Trail**
(See Basin Map)
 $Q = 27.2$ cfs

The total flow produced from Basins A and B does not exceed the street capacity. Therefore, inlets are not required on this street. Standard curb will be installed throughout Basin A. Flow will continue towards the east stub of Garden Gate Lane. See PC stream output.

3. **Garden Gate Lane**
(See Basin Map)
 $Q = 27.2$ cfs

The total runoff from Basins A and B will be captured by an inlet at the east end of Garden Gate Lane. This inlet discharges into a temporary pond on the east side of Blake Road. The pond will be removed once the master planned storm drain in Blake Road is constructed. See PC stream output and inlet nomograph.

4. **Basin C**
(See Basin Map)
 $Q = 13.3$ cfs

The runoff produced from Basin C does not exceed the street capacity for Iron Gate Trail and a portion of Garden Gate Lane. However, inlets will be placed at the intersection of Garden Gate Lane and Stone Gate Way to capture the flow produced from Basins C, D, and E. See PC stream output and inlet nomograph.

5. **Basins D and E**
(See Basin Map)
 $Q = 36.9$ cfs

The flow in Meadow Gate Trail, Sun Gate Trail, and the west end of Garden Gate Lane does not exceed the street capacity for those roads, therefore, inlets are not required. Inlets will be placed in Garden Gate Lane near the intersection with Sun Gate Trail. Bypass runoff will be captured by a sump inlet at the intersection of Garden Gate Lane and Stone Gate Way.

PC PROGRAM STREAM

SEPTEMBER 1994

CORRAL GATE LANE - BASIN B

MANNING'S N= .017 SLOPE= .0188

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.01	0.01	0.00	0.0	0.33	0.34	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.54	0.63	0.00	0.02
0.03	0.03	0.01	0.0	0.99	0.70	0.94	0.01	0.04
0.04	0.04	0.03	0.0	1.32	0.85	1.25	0.01	0.05
0.05	0.05	0.04	0.0	1.64	0.99	1.56	0.02	0.07
0.06	0.06	0.06	0.1	1.97	1.12	1.88	0.02	0.08
0.07	0.07	0.08	0.1	2.30	1.24	2.19	0.02	0.09
0.08	0.08	0.10	0.1	2.63	1.36	2.50	0.03	0.11
0.09	0.09	0.13	0.2	2.96	1.47	2.81	0.03	0.12
0.10	0.10	0.16	0.2	3.29	1.57	3.13	0.04	0.14
0.11	0.11	0.19	0.3	3.62	1.68	3.44	0.04	0.15
0.12	0.12	0.23	0.4	3.95	1.78	3.75	0.05	0.17
0.13	0.13	0.26	0.5	4.28	1.87	4.07	0.05	0.18
0.14	0.14	0.31	0.6	5.15	1.84	4.93	0.05	0.19
0.15	0.15	0.36	0.7	6.03	1.84	5.79	0.05	0.20
0.16	0.16	0.43	0.8	6.91	1.87	6.65	0.05	0.21
0.17	0.17	0.50	0.9	7.79	1.91	7.51	0.06	0.23
0.18	0.18	0.58	1.1	8.67	1.97	8.38	0.06	0.24
0.19	0.19	0.66	1.3	9.54	2.03	9.24	0.06	0.25
0.20	0.20	0.76	1.6	10.42	2.09	10.10	0.07	0.27
0.21	0.21	0.87	1.9	11.30	2.16	10.96	0.07	0.28
0.22	0.22	0.98	2.2	12.18	2.23	11.83	0.08	0.30
0.23	0.23	1.10	2.5	13.06	2.31	12.69	0.08	0.31
0.24	0.24	1.23	2.9	13.93	2.38	13.55	0.09	0.33
0.25	0.25	1.37	3.4	14.81	2.45	14.41	0.09	0.34
0.26	0.26	1.52	3.8	15.69	2.53	15.27	0.10	0.36
0.27	0.27	1.68	4.4	16.57	2.60	16.14	0.11	0.38
0.28	0.28	1.84	4.9	17.45	2.68	17.00	0.11	0.39
0.29	0.29	2.02	5.6	18.32	2.75	17.86	0.12	0.41
0.30	0.30	2.20	6.2	19.20	2.83	18.72	0.12	0.42
0.31	0.31	2.39	6.9	20.08	2.90	19.59	0.13	0.44
0.32	0.32	2.59	7.7	20.96	2.98	20.45	0.14	0.46
0.33	0.33	2.80	8.5	21.84	3.05	21.31	0.14	0.47
0.34	0.34	3.02	9.4	22.71	3.12	22.17	0.15	0.49
0.35	0.35	3.25	10.4	23.59	3.19	23.03	0.16	0.51
0.36	0.36	3.48	11.4	24.47	3.27	23.90	0.17	0.53
0.37	0.37	3.72	12.4	25.35	3.34	24.76	0.17	0.54
0.38	0.38	3.98	13.5	26.23	3.41	25.62	0.18	0.56
0.39	0.39	4.24	14.7	27.10	3.48	26.48	0.19	0.58
0.40	0.40	4.50	16.0	27.98	3.55	27.35	0.20	0.60
0.41	0.41	4.78	17.3	28.86	3.62	28.21	0.20	0.61
0.42	0.42	5.06	19.0	28.88	3.75	28.21	0.22	0.64
0.43	0.43	5.35	20.8	28.90	3.89	28.22	0.24	0.67
0.44	0.44	5.64	22.6	28.92	4.02	28.23	0.26	0.70
0.45	0.45	5.91	24.6	28.94	4.16	28.23	0.27	0.72
0.46	0.46	6.19	26.5	28.96	4.29	28.23	0.29	0.75
0.47	0.47	6.48	28.6	28.98	4.41	28.24	0.30	0.77
0.48	0.48	6.76	30.7	29.01	4.54	28.24	0.32	0.80
0.49	0.49	7.04	32.8	29.03	4.66	28.25	0.34	0.83
0.50	0.50	7.32	35.0	29.05	4.78	28.25	0.36	0.86
0.51	0.51	7.61	37.3	29.07	4.90	28.26	0.37	0.88
0.52	0.52	7.89	39.6	29.09	5.02	28.26	0.39	0.91

ACTUAL FLOW +
ROLL CURB LIMIT

STREET CAPACIT

PC PROGRAM STREAM

SEPTEMBER 1994

CORRAL GATE LANE - BASINS A+B

MANNING'S N= .017 SLOPE= .03

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.01	0.01	0.00	0.0	0.33	0.43	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.68	0.63	0.01	0.03
0.03	0.03	0.01	0.0	0.99	0.89	0.94	0.01	0.04
0.04	0.04	0.03	0.0	1.32	1.08	1.25	0.02	0.06
0.05	0.05	0.04	0.0	1.64	1.25	1.56	0.02	0.07
0.06	0.06	0.06	0.1	1.97	1.41	1.88	0.03	0.09
0.07	0.07	0.08	0.1	2.30	1.57	2.19	0.04	0.11
0.08	0.08	0.10	0.2	2.63	1.71	2.50	0.05	0.13
0.09	0.09	0.13	0.2	2.96	1.85	2.81	0.05	0.14
0.10	0.10	0.16	0.3	3.29	1.99	3.13	0.06	0.16
0.11	0.11	0.19	0.4	3.62	2.12	3.44	0.07	0.18
0.12	0.12	0.23	0.5	3.95	2.24	3.75	0.08	0.20
0.13	0.13	0.26	0.6	4.28	2.37	4.07	0.09	0.22
0.14	0.14	0.31	0.7	5.15	2.32	4.93	0.08	0.22
0.15	0.15	0.36	0.8	6.03	2.32	5.79	0.08	0.23
0.16	0.16	0.43	1.0	6.91	2.36	6.65	0.09	0.25
0.17	0.17	0.50	1.2	7.79	2.41	7.51	0.09	0.26
0.18	0.18	0.58	1.4	8.67	2.48	8.38	0.10	0.28
0.19	0.19	0.66	1.7	9.54	2.56	9.24	0.10	0.29
0.20	0.20	0.76	2.0	10.42	2.64	10.10	0.11	0.31
0.21	0.21	0.87	2.4	11.30	2.73	10.96	0.12	0.33
0.22	0.22	0.98	2.8	12.18	2.82	11.83	0.12	0.34
0.23	0.23	1.10	3.2	13.06	2.91	12.69	0.13	0.36
0.24	0.24	1.23	3.7	13.93	3.01	13.55	0.14	0.38
0.25	0.25	1.37	4.3	14.81	3.10	14.41	0.15	0.40
0.26	0.26	1.52	4.9	15.69	3.20	15.27	0.16	0.42
0.27	0.27	1.68	5.5	16.57	3.29	16.14	0.17	0.44
0.28	0.28	1.84	6.2	17.45	3.38	17.00	0.18	0.46
0.29	0.29	2.02	7.0	18.32	3.48	17.86	0.19	0.48
0.30	0.30	2.20	7.9	19.20	3.57	18.72	0.20	0.50
0.31	0.31	2.39	8.8	20.08	3.67	19.59	0.21	0.52
0.32	0.32	2.59	9.7	20.96	3.76	20.45	0.22	0.54
0.33	0.33	2.80	10.8	21.84	3.85	21.31	0.23	0.56
0.34	0.34	3.02	11.9	22.71	3.94	22.17	0.24	0.58
0.35	0.35	3.25	13.1	23.59	4.03	23.03	0.25	0.60
0.36	0.36	3.48	14.4	24.47	4.12	23.90	0.26	0.62
0.37	0.37	3.72	15.7	25.35	4.21	24.76	0.28	0.65
0.38	0.38	3.98	17.1	26.23	4.30	25.62	0.29	0.67
0.39	0.39	4.24	18.6	27.10	4.39	26.48	0.30	0.69
0.40	0.40	4.50	20.2	27.98	4.48	27.35	0.31	0.71
0.41	0.41	4.78	21.8	28.86	4.57	28.21	0.32	0.73
0.42	0.42	5.06	24.0	28.88	4.74	28.21	0.35	0.77
0.43	0.43	5.35	26.3	28.90	4.92	28.22	0.38	0.81
0.45	0.45	5.91	31.0	28.94	5.25	28.23	0.43	0.88
0.46	0.46	6.19	33.5	28.96	5.41	28.23	0.46	0.92
0.47	0.47	6.48	36.1	28.98	5.57	28.24	0.48	0.95
0.48	0.48	6.76	38.7	29.01	5.73	28.24	0.51	0.99
0.49	0.49	7.04	41.5	29.03	5.89	28.25	0.54	1.03
0.50	0.50	7.32	44.3	29.05	6.04	28.25	0.57	1.07
0.51	0.51	7.61	47.1	29.07	6.19	28.26	0.60	1.11
0.52	0.52	7.89	50.0	29.09	6.34	28.26	0.62	1.14

Roll CURB LIMIT

STREET CAPACITY
+ ACTUAL FLOW

PC PROGRAM STREAM

SEPTEMBER 1994

BRIDAL GATE TRAIL - BASIN A

MANNING'S N= .017 SLOPE= .0328

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.01	0.01	0.00	0.0	0.33	0.45	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.71	0.63	0.01	0.03
0.03	0.03	0.01	0.0	0.99	0.93	0.94	0.01	0.04
0.04	0.04	0.03	0.0	1.32	1.13	1.25	0.02	0.06
0.05	0.05	0.04	0.1	1.64	1.31	1.56	0.03	0.08
0.06	0.06	0.06	0.1	1.97	1.48	1.88	0.03	0.09
0.07	0.07	0.08	0.1	2.30	1.64	2.19	0.04	0.11
0.08	0.08	0.10	0.2	2.63	1.79	2.50	0.05	0.13
0.09	0.09	0.13	0.2	2.96	1.94	2.81	0.06	0.15
0.10	0.10	0.16	0.3	3.29	2.08	3.13	0.07	0.17
0.11	0.11	0.19	0.4	3.62	2.21	3.44	0.08	0.19
0.12	0.12	0.23	0.5	3.95	2.35	3.75	0.09	0.21
0.13	0.13	0.26	0.7	4.28	2.47	4.07	0.10	0.23
0.14	0.14	0.31	0.8	5.15	2.43	4.93	0.09	0.23
0.15	0.15	0.36	0.9	6.03	2.43	5.79	0.09	0.24
0.16	0.16	0.43	1.0	6.91	2.47	6.65	0.09	0.25
0.17	0.17	0.50	1.3	7.79	2.52	7.51	0.10	0.27
0.18	0.18	0.58	1.5	8.67	2.60	8.38	0.10	0.28
0.19	0.19	0.66	1.8	9.54	2.68	9.24	0.11	0.30
0.20	0.20	0.76	2.1	10.42	2.76	10.10	0.12	0.32
0.21	0.21	0.87	2.5	11.30	2.85	10.96	0.13	0.34
0.22	0.22	0.98	2.9	12.18	2.95	11.83	0.14	0.36
0.23	0.23	1.10	3.4	13.06	3.05	12.69	0.14	0.37
0.24	0.24	1.23	3.9	13.93	3.14	13.55	0.15	0.39
0.25	0.25	1.37	4.5	14.81	3.24	14.41	0.16	0.41
0.26	0.26	1.52	5.1	15.69	3.34	15.27	0.17	0.43
0.27	0.27	1.68	5.8	16.57	3.44	16.14	0.18	0.45
0.28	0.28	1.84	6.5	17.45	3.54	17.00	0.19	0.47
0.29	0.29	2.02	7.3	18.32	3.64	17.86	0.21	0.50
0.30	0.30	2.20	8.2	19.20	3.74	18.72	0.22	0.52
0.31	0.31	2.39	9.2	20.08	3.83	19.59	0.23	0.54
0.32	0.32	2.59	10.2	20.96	3.93	20.45	0.24	0.56
0.33	0.33	2.80	11.3	21.84	4.03	21.31	0.25	0.58
0.34	0.34	3.02	12.4	22.71	4.12	22.17	0.26	0.60
0.35	0.35	3.25	13.7	23.59	4.22	23.03	0.28	0.63
0.36	0.36	3.48	15.0	24.47	4.31	23.90	0.29	0.65
0.37	0.37	3.72	16.4	25.35	4.41	24.76	0.30	0.67
0.38	0.38	3.98	17.9	26.23	4.50	25.62	0.31	0.69
0.39	0.39	4.24	19.5	27.10	4.59	26.48	0.33	0.72
0.40	0.40	4.50	21.1	27.98	4.68	27.35	0.34	0.74
0.41	0.41	4.78	22.8	28.86	4.78	28.21	0.35	0.76
0.42	0.42	5.06	25.1	28.88	4.96	28.21	0.38	0.80
0.43	0.43	5.35	27.5	28.90	5.14	28.22	0.41	0.84
0.45	0.45	5.91	32.5	28.94	5.49	28.23	0.47	0.92
0.46	0.46	6.19	35.1	28.96	5.66	28.23	0.50	0.96
0.47	0.47	6.48	37.7	28.98	5.83	28.24	0.53	1.00
0.48	0.48	6.76	40.5	29.01	5.99	28.24	0.56	1.04
0.49	0.49	7.04	43.4	29.03	6.16	28.25	0.59	1.08
0.50	0.50	7.32	46.3	29.05	6.32	28.25	0.62	1.12
0.51	0.51	7.61	49.3	29.07	6.48	28.26	0.65	1.16
0.52	0.52	7.89	52.3	29.09	6.63	28.26	0.68	1.20

ROLL CURB LIMIT

STREET CAPACITY.
ACTUAL FLOW

PC PROGRAM STREAM

SEPTEMBER 1994

GARDEN GATE LANE - BASIN A

MANNING'S N= .017 SLOPE= .007

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00
□ WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
	INC	AREA	RATE	PER	VEL		HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.01	0.01	0.00	0.0	0.33	0.21	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.33	0.63	0.00	0.02
0.03	0.03	0.01	0.0	0.99	0.43	0.94	0.00	0.03
0.04	0.04	0.03	0.0	1.32	0.52	1.25	0.00	0.04
0.05	0.05	0.04	0.0	1.64	0.60	1.56	0.01	0.06
0.06	0.06	0.06	0.0	1.97	0.68	1.88	0.01	0.07
0.07	0.07	0.08	0.1	2.30	0.76	2.19	0.01	0.08
0.08	0.08	0.10	0.1	2.63	0.83	2.50	0.01	0.09
0.09	0.09	0.13	0.1	2.96	0.89	2.81	0.01	0.10
0.10	0.10	0.16	0.2	3.29	0.96	3.13	0.01	0.11
0.11	0.11	0.19	0.2	3.62	1.02	3.44	0.02	0.13
0.12	0.12	0.23	0.2	3.95	1.08	3.75	0.02	0.14
0.13	0.13	0.26	0.3	4.28	1.14	4.07	0.02	0.15
0.14	0.14	0.31	0.3	5.15	1.12	4.93	0.02	0.16
0.15	0.15	0.36	0.4	6.03	1.12	5.79	0.02	0.17
0.16	0.16	0.43	0.5	6.91	1.14	6.65	0.02	0.18
0.17	0.17	0.50	0.6	7.79	1.17	7.51	0.02	0.19
0.18	0.18	0.58	0.7	8.67	1.20	8.38	0.02	0.20
0.19	0.19	0.66	0.8	9.54	1.24	9.24	0.02	0.21
0.20	0.20	0.76	1.0	10.42	1.28	10.10	0.03	0.23
0.21	0.21	0.87	1.1	11.30	1.32	10.96	0.03	0.24
0.22	0.22	0.98	1.3	12.18	1.36	11.83	0.03	0.25
0.23	0.23	1.10	1.6	13.06	1.41	12.69	0.03	0.26
0.24	0.24	1.23	1.8	13.93	1.45	13.55	0.03	0.27
0.25	0.25	1.37	2.1	14.81	1.50	14.41	0.03	0.28
0.26	0.26	1.52	2.3	15.69	1.54	15.27	0.04	0.30
0.27	0.27	1.68	2.7	16.57	1.59	16.14	0.04	0.31
0.28	0.28	1.84	3.0	17.45	1.63	17.00	0.04	0.32
0.29	0.29	2.02	3.4	18.32	1.68	17.86	0.04	0.33
0.30	0.30	2.20	3.8	19.20	1.73	18.72	0.05	0.35
0.31	0.31	2.39	4.2	20.08	1.77	19.59	0.05	0.36
0.32	0.32	2.59	4.7	20.96	1.82	20.45	0.05	0.37
0.33	0.33	2.80	5.2	21.84	1.86	21.31	0.05	0.38
0.34	0.34	3.02	5.8	22.71	1.90	22.17	0.06	0.40
0.35	0.35	3.25	6.3	23.59	1.95	23.03	0.06	0.41
0.36	0.36	3.48	6.9	24.47	1.99	23.90	0.06	0.42
0.37	0.37	3.72	7.6	25.35	2.04	24.76	0.06	0.43
0.38	0.38	3.98	8.3	26.23	2.08	25.62	0.07	0.45
0.39	0.39	4.24	9.0	27.10	2.12	26.48	0.07	0.46
0.40	0.40	4.50	9.7	27.98	2.16	27.35	0.07	0.47
0.41	0.41	4.78	10.6	28.86	2.21	28.21	0.08	0.49
0.42	0.42	5.06	11.6	28.88	2.29	28.21	0.08	0.50
0.43	0.43	5.35	12.7	28.90	2.37	28.22	0.09	0.52
□ WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
	INC	AREA	RATE	PER	VEL		HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.45	0.45	5.91	15.0	28.94	2.54	28.23	0.10	0.55
0.46	0.46	6.19	16.2	28.96	2.62	28.23	0.11	0.57
0.47	0.47	6.48	17.4	28.98	2.69	28.24	0.11	0.58
0.48	0.48	6.76	18.7	29.01	2.77	28.24	0.12	0.60
0.49	0.49	7.04	20.0	29.03	2.84	28.25	0.13	0.62
0.50	0.50	7.32	21.4	29.05	2.92	28.25	0.13	0.63
0.51	0.51	7.61	22.8	29.07	2.99	28.26	0.14	0.65
0.52	0.52	7.89	24.2	29.09	3.06	28.26	0.15	0.67

0.53	0.53	8.17	25.6	29.11	3.14	28.27	0.15	0.68	ACTUAL FLOW
0.54	0.54	8.45	27.1	29.13	3.21	28.27	0.16	0.70	
0.55	0.55	8.74	28.6	29.15	3.28	28.28	0.17	0.72	
0.56	0.56	9.02	30.2	29.17	3.34	28.28	0.17	0.73	
0.57	0.57	9.30	31.7	29.19	3.41	28.29	0.18	0.75	
0.58	0.58	9.59	33.3	29.21	3.48	28.29	0.19	0.77	
0.59	0.59	9.87	35.0	29.23	3.55	28.30	0.20	0.79	
0.60	0.60	10.15	36.7	29.25	3.61	28.30	0.20	0.80	
0.61	0.61	10.43	38.4	29.27	3.68	28.31	0.21	0.82	
0.62	0.62	10.72	40.1	29.29	3.74	28.31	0.22	0.84	STREET CAPACITY
0.63	0.63	11.00	41.9	29.31	3.80	28.32	0.22	0.85	
0.64	0.64	11.28	43.6	29.34	3.87	28.32	0.23	0.87	
0.65	0.65	11.57	45.5	29.36	3.93	28.33	0.24	0.89	
0.66	0.66	11.85	47.3	29.38	3.99	28.33	0.25	0.91	
0.67	0.67	12.13	49.2	29.40	4.05	28.34	0.26	0.93	
0.68	0.68	12.42	50.0	30.44	4.02	30.30	0.25	0.93	
0.69	0.69	12.72	50.8	31.49	4.00	31.34	0.25	0.94	
0.70	0.70	13.06	51.0	33.45	3.91	32.39	0.24	0.94	
0.71	0.71	13.39	52.1	34.50	3.89	33.44	0.24	0.95	
0.72	0.72	13.73	53.2	35.54	3.88	34.48	0.23	0.95	
0.73	0.73	14.08	54.5	36.59	3.87	35.53	0.23	0.96	
0.74	0.74	14.44	55.7	37.64	3.86	36.58	0.23	0.97	
0.75	0.75	14.81	57.1	38.68	3.86	37.62	0.23	0.98	
0.76	0.76	15.19	58.5	39.73	3.85	38.67	0.23	0.99	
0.77	0.77	15.58	60.0	40.78	3.85	39.72	0.23	1.00	
0.78	0.78	15.98	61.6	41.83	3.85	40.77	0.23	1.01	
0.79	0.79	16.40	63.2	42.87	3.85	41.81	0.23	1.02	
0.80	0.80	16.82	64.9	43.92	3.86	42.86	0.23	1.03	
0.81	0.81	17.25	66.6	44.97	3.86	43.91	0.23	1.04	
0.82	0.82	17.70	68.5	46.01	3.87	44.95	0.23	1.05	
0.83	0.83	18.15	70.4	47.06	3.88	46.00	0.23	1.06	

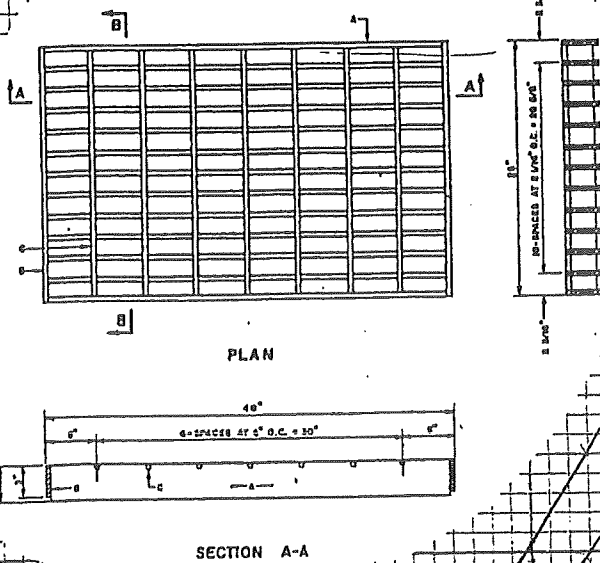
90

60" WING 40" @ TC
18.5" 35.5" 18.5" NET OPEN AREA
GRATE @ 10" (0.83') BELOW TC

EQUATION	SGL A	DBL A	TRPL A	QDPL A
1) ORIFICE	$22.0 h^{0.5} + 16.5 h^{1.5}$	$43.9 h^{0.5} + 16.5 h^{1.5}$	$65.9 h^{0.5} + 16.5 h^{1.5}$	$87.8 h^{0.5} + 16.5 h^{1.5}$
2) 3-SIDED WEIR ($h \leq 0.83'$)	$36.4 h^{1.5}$	$46.2 h^{1.5}$	$56.0 h^{1.5}$	$65.7 h^{1.5}$
3) 4-SIDED WEIR ($h > 0.83'$)	ADD TO 2): $11(h - 0.83)^{1.5}$	ADD TO 2): $22(h - 0.83)^{1.5}$	ADD TO 2): $33(h - 0.83)^{1.5}$	ADD TO 2): $44(h - 0.83)^{1.5}$

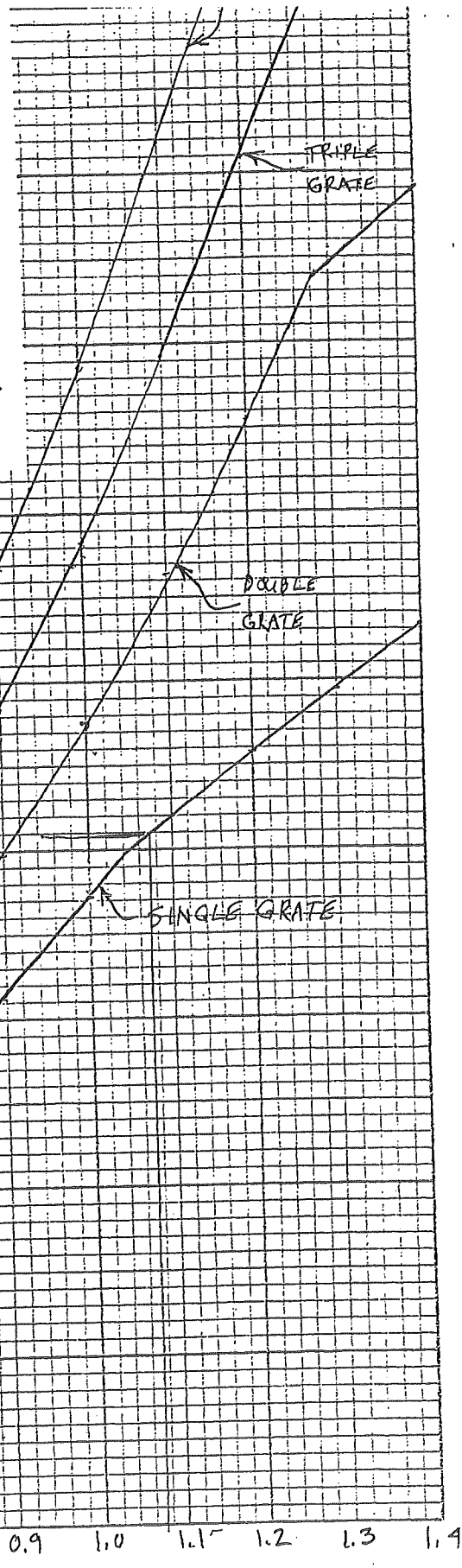
1) $Q = 0.6 A \sqrt{2gh} + 3.3(5') h^{1.5}$
2) & 3) $Q = 3.3 P h^{1.5}$

Q = 27.2 GRATE CAPACITY (Q) IN CFS



SECTION B-B

SECTION A-A



TC 0.83'
HEAD (h) IN FEET

BOHANNAN-HUSTON INC.

PROJECT NAME Sun Gate SHEET OF
PROJECT NO. Garden Gate Inlet BY DATE
SUBJECT RATING CURVE FOR TYPE A INLETS CH'D DATE

B-7

PC PROGRAM STREAM

SEPTEMBER 1994

IRON GATE TRAIL - BASIN C

MANNING'S N= .017 SLOPE= .034

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.01	0.01	0.00	0.0	0.33	0.46	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.72	0.63	0.01	0.03
0.03	0.03	0.01	0.0	0.99	0.95	0.94	0.01	0.04
0.04	0.04	0.03	0.0	1.32	1.15	1.25	0.02	0.06
0.05	0.05	0.04	0.1	1.64	1.33	1.56	0.03	0.08
0.06	0.06	0.06	0.1	1.97	1.50	1.88	0.04	0.10
0.07	0.07	0.08	0.1	2.30	1.67	2.19	0.04	0.11
0.08	0.08	0.10	0.2	2.63	1.82	2.50	0.05	0.13
0.09	0.09	0.13	0.2	2.96	1.97	2.81	0.06	0.15
0.10	0.10	0.16	0.3	3.29	2.12	3.13	0.07	0.17
0.11	0.11	0.19	0.4	3.62	2.25	3.44	0.08	0.19
0.12	0.12	0.23	0.5	3.95	2.39	3.75	0.09	0.21
0.13	0.13	0.26	0.7	4.28	2.52	4.07	0.10	0.23
0.14	0.14	0.31	0.8	5.15	2.47	4.93	0.09	0.23
0.15	0.15	0.36	0.9	6.03	2.47	5.79	0.10	0.25
0.16	0.16	0.43	1.1	6.91	2.51	6.65	0.10	0.26
0.17	0.17	0.50	1.3	7.79	2.57	7.51	0.10	0.27
0.18	0.18	0.58	1.5	8.67	2.64	8.38	0.11	0.29
0.19	0.19	0.66	1.8	9.54	2.72	9.24	0.12	0.31
0.20	0.20	0.76	2.1	10.42	2.81	10.10	0.12	0.32
0.21	0.21	0.87	2.5	11.30	2.91	10.96	0.13	0.34
0.22	0.22	0.98	2.9	12.18	3.00	11.83	0.14	0.36
0.23	0.23	1.10	3.4	13.06	3.10	12.69	0.15	0.38
0.24	0.24	1.23	3.9	13.93	3.20	13.55	0.16	0.40
0.25	0.25	1.37	4.5	14.81	3.30	14.41	0.17	0.42
0.26	0.26	1.52	5.2	15.69	3.40	15.27	0.18	0.44
0.27	0.27	1.68	5.9	16.57	3.50	16.14	0.19	0.46
0.28	0.28	1.84	6.6	17.45	3.60	17.00	0.20	0.48
0.29	0.29	2.02	7.5	18.32	3.70	17.86	0.21	0.50
0.30	0.30	2.20	8.4	19.20	3.80	18.72	0.22	0.52
0.31	0.31	2.39	9.3	20.08	3.90	19.59	0.24	0.55
0.32	0.32	2.59	10.4	20.96	4.00	20.45	0.25	0.57
0.33	0.33	2.80	11.5	21.84	4.10	21.31	0.26	0.59
0.34	0.34	3.02	12.7	22.71	4.20	22.17	0.27	0.61
0.35	0.35	3.25	13.9	23.59	4.30	23.03	0.29	0.64
0.36	0.36	3.48	15.3	24.47	4.39	23.90	0.30	0.66
0.37	0.37	3.72	16.7	25.35	4.49	24.76	0.31	0.68
0.38	0.38	3.98	18.2	26.23	4.58	25.62	0.33	0.71
0.39	0.39	4.24	19.8	27.10	4.68	26.48	0.34	0.73
0.40	0.40	4.50	21.5	27.98	4.77	27.35	0.35	0.75
0.41	0.41	4.78	23.3	28.86	4.86	28.21	0.37	0.78
0.42	0.42	5.06	25.6	28.88	5.05	28.21	0.40	0.82
0.43	0.43	5.35	28.0	28.90	5.23	28.22	0.43	0.86
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.45	0.45	5.91	33.0	28.94	5.59	28.23	0.49	0.94
0.46	0.46	6.19	35.7	28.96	5.76	28.23	0.52	0.98
0.47	0.47	6.48	38.4	28.98	5.93	28.24	0.55	1.02
0.48	0.48	6.76	41.2	29.01	6.10	28.24	0.58	1.06
0.49	0.49	7.04	44.1	29.03	6.27	28.25	0.61	1.10
0.50	0.50	7.32	47.1	29.05	6.43	28.25	0.64	1.14
0.51	0.51	7.61	50.2	29.07	6.59	28.26	0.68	1.19
0.52	0.52	7.89	53.3	29.09	6.75	28.26	0.71	1.23

(LOT 77)
ROLL CURB LIMIT

ACTUAL Q

STREET CAPACITY

PC PROGRAM STREAM

SEPTEMBER 1994

GARDEN GATE LANE — BASIN C

MANNING'S N= .017 SLOPE= .007

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00
□ WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
	INC	AREA	RATE	PER	VEL		HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.01	0.01	0.00	0.0	0.33	0.21	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.33	0.63	0.00	0.02
0.03	0.03	0.01	0.0	0.99	0.43	0.94	0.00	0.03
0.04	0.04	0.03	0.0	1.32	0.52	1.25	0.00	0.04
0.05	0.05	0.04	0.0	1.64	0.60	1.56	0.01	0.06
0.06	0.06	0.06	0.0	1.97	0.68	1.88	0.01	0.07
0.07	0.07	0.08	0.1	2.30	0.76	2.19	0.01	0.08
0.08	0.08	0.10	0.1	2.63	0.83	2.50	0.01	0.09
0.09	0.09	0.13	0.1	2.96	0.89	2.81	0.01	0.10
0.10	0.10	0.16	0.2	3.29	0.96	3.13	0.01	0.11
0.11	0.11	0.19	0.2	3.62	1.02	3.44	0.02	0.13
0.12	0.12	0.23	0.2	3.95	1.08	3.75	0.02	0.14
0.13	0.13	0.26	0.3	4.28	1.14	4.07	0.02	0.15
0.14	0.14	0.31	0.3	5.15	1.12	4.93	0.02	0.16
0.15	0.15	0.36	0.4	6.03	1.12	5.79	0.02	0.17
0.16	0.16	0.43	0.5	6.91	1.14	6.65	0.02	0.18
0.17	0.17	0.50	0.6	7.79	1.17	7.51	0.02	0.19
0.18	0.18	0.58	0.7	8.67	1.20	8.38	0.02	0.20
0.19	0.19	0.66	0.8	9.54	1.24	9.24	0.02	0.21
0.20	0.20	0.76	1.0	10.42	1.28	10.10	0.03	0.23
0.21	0.21	0.87	1.1	11.30	1.32	10.96	0.03	0.24
0.22	0.22	0.98	1.3	12.18	1.36	11.83	0.03	0.25
0.23	0.23	1.10	1.6	13.06	1.41	12.69	0.03	0.26
0.24	0.24	1.23	1.8	13.93	1.45	13.55	0.03	0.27
0.25	0.25	1.37	2.1	14.81	1.50	14.41	0.03	0.28
0.26	0.26	1.52	2.3	15.69	1.54	15.27	0.04	0.30
0.27	0.27	1.68	2.7	16.57	1.59	16.14	0.04	0.31
0.28	0.28	1.84	3.0	17.45	1.63	17.00	0.04	0.32
0.29	0.29	2.02	3.4	18.32	1.68	17.86	0.04	0.33
0.30	0.30	2.20	3.8	19.20	1.73	18.72	0.05	0.35
0.31	0.31	2.39	4.2	20.08	1.77	19.59	0.05	0.36
0.32	0.32	2.59	4.7	20.96	1.82	20.45	0.05	0.37
0.33	0.33	2.80	5.2	21.84	1.86	21.31	0.05	0.38
0.34	0.34	3.02	5.8	22.71	1.90	22.17	0.06	0.40
0.35	0.35	3.25	6.3	23.59	1.95	23.03	0.06	0.41
0.36	0.36	3.48	6.9	24.47	1.99	23.90	0.06	0.42
0.37	0.37	3.72	7.6	25.35	2.04	24.76	0.06	0.43
0.38	0.38	3.98	8.3	26.23	2.08	25.62	0.07	0.45
0.39	0.39	4.24	9.0	27.10	2.12	26.48	0.07	0.46
0.40	0.40	4.50	9.7	27.98	2.16	27.35	0.07	0.47
0.41	0.41	4.78	10.6	28.86	2.21	28.21	0.08	0.49
0.42	0.42	5.06	11.6	28.88	2.29	28.21	0.08	0.50
0.43	0.43	5.35	12.7	28.90	2.37	28.22	0.09	0.52
□ WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
	INC	AREA	RATE	PER	VEL		HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.45	0.45	5.91	15.0	28.94	2.54	28.23	0.10	0.55
0.46	0.46	6.19	16.2	28.96	2.62	28.23	0.11	0.57
0.47	0.47	6.48	17.4	28.98	2.69	28.24	0.11	0.58
0.48	0.48	6.76	18.7	29.01	2.77	28.24	0.12	0.60
0.49	0.49	7.04	20.0	29.03	2.84	28.25	0.13	0.62
0.50	0.50	7.32	21.4	29.05	2.92	28.25	0.13	0.63
0.51	0.51	7.61	22.8	29.07	2.99	28.26	0.14	0.65
0.52	0.52	7.89	24.2	29.09	3.06	28.26	0.15	0.67

ACTUAL Q

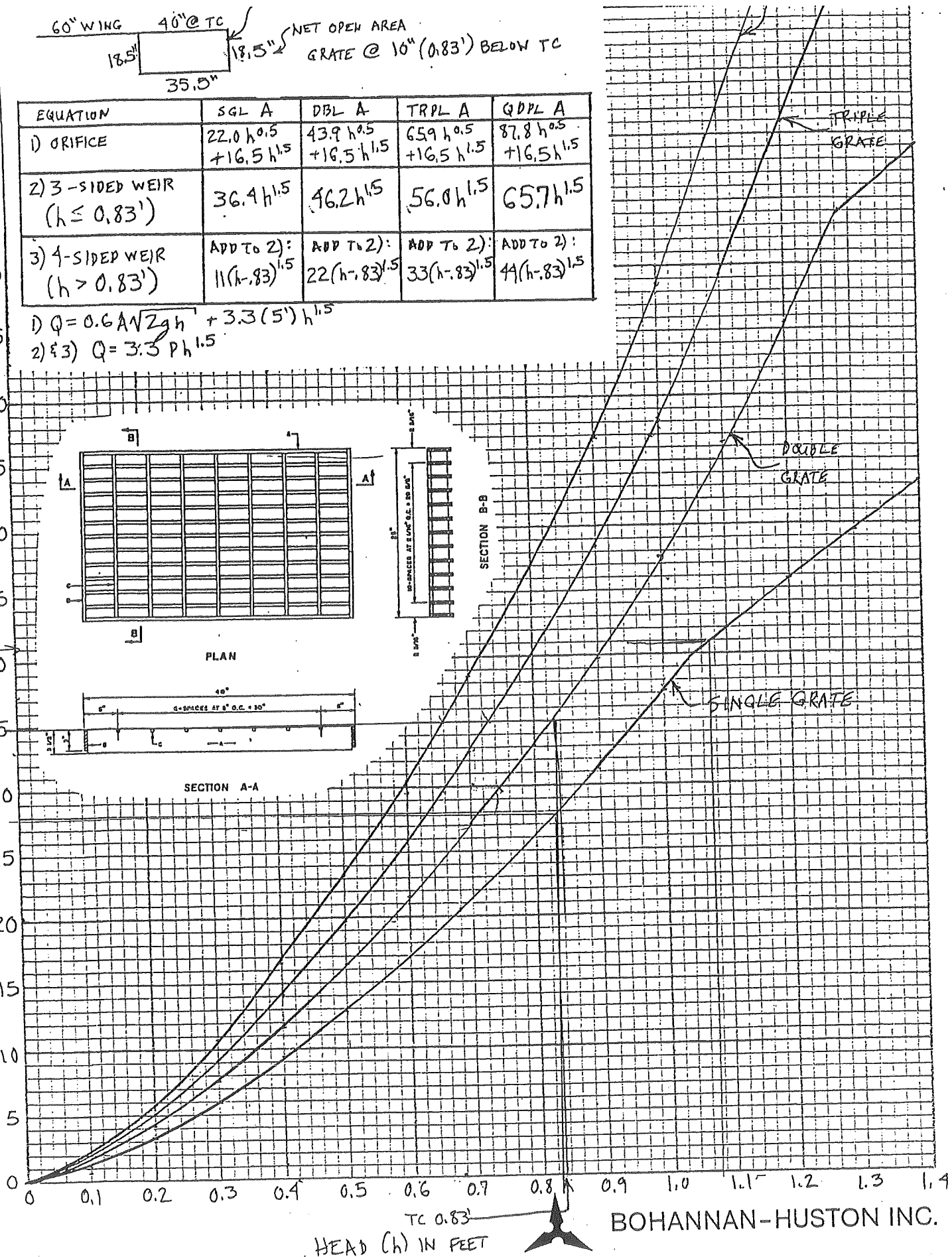
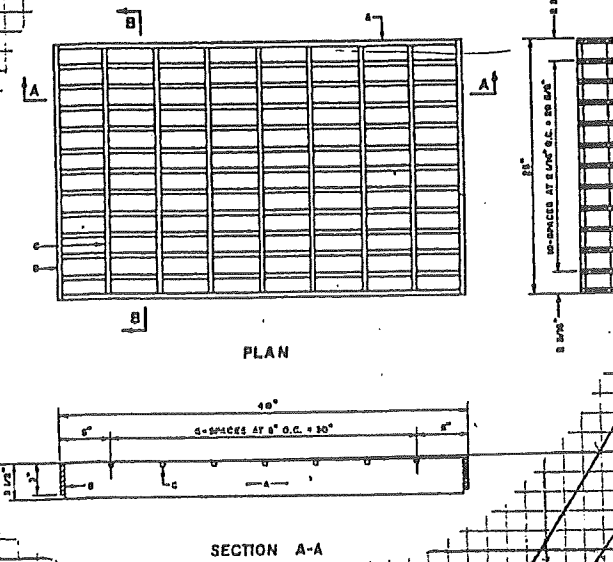
60" WING 40" @ TC
 18.5" 18.5" NET OPEN AREA
 35.0" GRATE @ 10" (0.83') BELOW TC

EQUATION	SGL A	DBL A	TRPL A	QDPL A
1) ORIFICE	$22.0 h^{0.5} + 16.5 h^{1.5}$	$43.9 h^{0.5} + 16.5 h^{1.5}$	$65.9 h^{0.5} + 16.5 h^{1.5}$	$87.8 h^{0.5} + 16.5 h^{1.5}$
2) 3-SIDED WEIR ($h \leq 0.83'$)	$36.4 h^{1.5}$	$46.2 h^{1.5}$	$56.0 h^{1.5}$	$65.7 h^{1.5}$
3) 4-SIDED WEIR ($h > 0.83'$)	ADD TO 2): $11(h-0.83)^{1.5}$	ADD TO 2): $22(h-0.83)^{1.5}$	ADD TO 2): $33(h-0.83)^{1.5}$	ADD TO 2): $44(h-0.83)^{1.5}$

1) $Q = 0.6 A \sqrt{2gh} + 3.3(5') h^{1.5}$
 2) & 3) $Q = 3.3 P h^{1.5}$

GRATE CAPACITY (Q) IN CFS

Q=35 cfs



BOHANNAN-HUSTON INC.

PROJECT NAME Sun Gate SHEET _____ OF _____
 PROJECT NO. Garden Gate + Stone Gate BY _____ DATE _____
 SUBJECT RATING CURVE FOR TYPE A INLETS CH'D _____ DATE _____

PC PROGRAM STREAM

SEPTEMBER 1994

SUN GATE TRAIL

- BASIN D

MANNING'S N= .017 SLOPE= .0395

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00

WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)

0.01	0.01	0.00	0.0	0.33	0.49	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.78	0.63	0.01	0.03
0.03	0.03	0.01	0.0	0.99	1.02	0.94	0.02	0.05
0.04	0.04	0.03	0.0	1.32	1.24	1.25	0.02	0.06
0.05	0.05	0.04	0.1	1.64	1.44	1.56	0.03	0.08
0.06	0.06	0.06	0.1	1.97	1.62	1.88	0.04	0.10
0.07	0.07	0.08	0.1	2.30	1.80	2.19	0.05	0.12
0.08	0.08	0.10	0.2	2.63	1.96	2.50	0.06	0.14
0.09	0.09	0.13	0.3	2.96	2.13	2.81	0.07	0.16
0.10	0.10	0.16	0.4	3.29	2.28	3.13	0.08	0.18
0.11	0.11	0.19	0.5	3.62	2.43	3.44	0.09	0.20
0.12	0.12	0.23	0.6	3.95	2.57	3.75	0.10	0.22
0.13	0.13	0.26	0.7	4.28	2.72	4.07	0.11	0.24
0.14	0.14	0.31	0.8	5.15	2.66	4.93	0.11	0.25
0.15	0.15	0.36	1.0	6.03	2.67	5.79	0.11	0.26
0.16	0.16	0.43	1.2	6.91	2.71	6.65	0.11	0.27
0.17	0.17	0.50	1.4	7.79	2.77	7.51	0.12	0.29
0.18	0.18	0.58	1.6	8.67	2.85	8.38	0.13	0.31
0.19	0.19	0.66	1.9	9.54	2.94	9.24	0.13	0.32
0.20	0.20	0.76	2.3	10.42	3.03	10.10	0.14	0.34
0.21	0.21	0.87	2.7	11.30	3.13	10.96	0.15	0.36
0.22	0.22	0.98	3.2	12.18	3.24	11.83	0.16	0.38
0.23	0.23	1.10	3.7	13.06	3.34	12.69	0.17	0.40
0.24	0.24	1.23	4.3	13.93	3.45	13.55	0.18	0.42
0.25	0.25	1.37	4.9	14.81	3.56	14.41	0.20	0.45
0.26	0.26	1.52	5.6	15.69	3.67	15.27	0.21	0.47
0.27	0.27	1.68	6.3	16.57	3.78	16.14	0.22	0.49
0.28	0.28	1.84	7.2	17.45	3.88	17.00	0.23	0.51
0.29	0.29	2.02	8.1	18.32	3.99	17.86	0.25	0.54

ROLL CURB LIMIT
(LOT 55)

ACTUAL Q

STREET CAPACITY

WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.45	0.45	5.91	35.6	28.94	6.03	28.23	0.56	1.01
0.46	0.46	6.19	38.5	28.96	6.21	28.23	0.60	1.06
0.47	0.47	6.48	41.4	28.98	6.40	28.24	0.64	1.11
0.48	0.48	6.76	44.5	29.01	6.58	28.24	0.67	1.15
0.49	0.49	7.04	47.6	29.03	6.76	28.25	0.71	1.20
0.50	0.50	7.32	50.8	29.05	6.93	28.25	0.75	1.25
0.51	0.51	7.61	54.1	29.07	7.11	28.26	0.78	1.29
0.52	0.52	7.89	57.4	29.09	7.28	28.26	0.82	1.34

PC PROGRAM STREAM

SEPTEMBER 1994

MEADOW GATE TRAIL - BASIN E

MANNING'S N= .017 SLOPE= .0361

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00

WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)

0.01	0.01	0.00	0.0	0.33	0.47	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.75	0.63	0.01	0.03
0.03	0.03	0.01	0.0	0.99	0.98	0.94	0.01	0.04
0.04	0.04	0.03	0.0	1.32	1.18	1.25	0.02	0.06
0.05	0.05	0.04	0.1	1.64	1.37	1.56	0.03	0.08
0.06	0.06	0.06	0.1	1.97	1.55	1.88	0.04	0.10
0.07	0.07	0.08	0.1	2.30	1.72	2.19	0.05	0.12
0.08	0.08	0.10	0.2	2.63	1.88	2.50	0.05	0.13
0.09	0.09	0.13	0.3	2.96	2.03	2.81	0.06	0.15
0.10	0.10	0.16	0.3	3.29	2.18	3.13	0.07	0.17
0.11	0.11	0.19	0.4	3.62	2.32	3.44	0.08	0.19
0.12	0.12	0.23	0.6	3.95	2.46	3.75	0.09	0.21
0.13	0.13	0.26	0.7	4.28	2.60	4.07	0.10	0.23
0.14	0.14	0.31	0.8	5.15	2.55	4.93	0.10	0.24
0.15	0.15	0.36	0.9	6.03	2.55	5.79	0.10	0.25
0.16	0.16	0.43	1.1	6.91	2.59	6.65	0.10	0.26
0.17	0.17	0.50	1.3	7.79	2.65	7.51	0.11	0.28
0.18	0.18	0.58	1.6	8.67	2.72	8.38	0.12	0.30
0.19	0.19	0.66	1.9	9.54	2.81	9.24	0.12	0.31
0.20	0.20	0.76	2.2	10.42	2.90	10.10	0.13	0.33
0.21	0.21	0.87	2.6	11.30	3.00	10.96	0.14	0.35
0.22	0.22	0.98	3.0	12.18	3.09	11.83	0.15	0.37
0.23	0.23	1.10	3.5	13.06	3.20	12.69	0.16	0.39
0.24	0.24	1.23	4.1	13.93	3.30	13.55	0.17	0.41
0.25	0.25	1.37	4.7	14.81	3.40	14.41	0.18	0.43
0.26	0.26	1.52	5.3	15.69	3.51	15.27	0.19	0.45
0.27	0.27	1.68	6.1	16.57	3.61	16.14	0.20	0.47
0.28	0.28	1.84	6.8	17.45	3.71	17.00	0.21	0.49
0.29	0.29	2.02	7.7	18.32	3.82	17.86	0.23	0.52
0.30	0.30	2.20	8.6	19.20	3.92	18.72	0.24	0.54
0.31	0.31	2.39	9.6	20.08	4.02	19.59	0.25	0.56
0.32	0.32	2.59	10.7	20.96	4.12	20.45	0.26	0.58
0.33	0.33	2.80	11.8	21.84	4.23	21.31	0.28	0.61
0.34	0.34	3.02	13.1	22.71	4.33	22.17	0.29	0.63
0.35	0.35	3.25	14.4	23.59	4.43	23.03	0.30	0.65
0.36	0.36	3.48	15.7	24.47	4.52	23.90	0.32	0.68
0.37	0.37	3.72	17.2	25.35	4.62	24.76	0.33	0.70
0.38	0.38	3.98	18.8	26.23	4.72	25.62	0.35	0.73
0.39	0.39	4.24	20.4	27.10	4.82	26.48	0.36	0.75
0.40	0.40	4.50	22.1	27.98	4.91	27.35	0.38	0.78
0.41	0.41	4.78	24.0	28.86	5.01	28.21	0.39	0.80
0.42	0.42	5.06	26.4	28.88	5.20	28.21	0.42	0.84
0.43	0.43	5.35	28.8	28.90	5.39	28.22	0.45	0.88

ROLL CURB LIMIT
(LOT 20)

ACTUAL Q

STREET CAPACITY

WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.45	0.45	5.91	34.0	28.94	5.76	28.23	0.52	0.97
0.46	0.46	6.19	36.8	28.96	5.94	28.23	0.55	1.01
0.47	0.47	6.48	39.6	28.98	6.12	28.24	0.58	1.05
0.48	0.48	6.76	42.5	29.01	6.29	28.24	0.61	1.09
0.49	0.49	7.04	45.5	29.03	6.46	28.25	0.65	1.14
0.50	0.50	7.32	48.5	29.05	6.63	28.25	0.68	1.18
0.51	0.51	7.61	51.7	29.07	6.79	28.26	0.72	1.23
0.52	0.52	7.89	54.9	29.09	6.96	28.26	0.75	1.27

PC PROGRAM STREAM

SEPTEMBER 1994

GARDEN GATE LANE

- BASIN E

MANNING'S N= .017 SLOPE= .02

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.01	0.01	0.00	0.0	0.33	0.35	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.55	0.63	0.00	0.02
0.03	0.03	0.01	0.0	0.99	0.73	0.94	0.01	0.04
0.04	0.04	0.03	0.0	1.32	0.88	1.25	0.01	0.05
0.05	0.05	0.04	0.0	1.64	1.02	1.56	0.02	0.07
0.06	0.06	0.06	0.1	1.97	1.15	1.88	0.02	0.08
0.07	0.07	0.08	0.1	2.30	1.28	2.19	0.03	0.10
0.08	0.08	0.10	0.1	2.63	1.40	2.50	0.03	0.11
0.09	0.09	0.13	0.2	2.96	1.51	2.81	0.04	0.13
0.10	0.10	0.16	0.3	3.29	1.62	3.13	0.04	0.14
0.11	0.11	0.19	0.3	3.62	1.73	3.44	0.05	0.16
0.12	0.12	0.23	0.4	3.95	1.83	3.75	0.05	0.17
0.13	0.13	0.26	0.5	4.28	1.93	4.07	0.06	0.19
0.14	0.14	0.31	0.6	5.15	1.89	4.93	0.06	0.20
0.15	0.15	0.36	0.7	6.03	1.90	5.79	0.06	0.21
0.16	0.16	0.43	0.8	6.91	1.93	6.65	0.06	0.22
0.17	0.17	0.50	1.0	7.79	1.97	7.51	0.06	0.23
0.18	0.18	0.58	1.2	8.67	2.03	8.38	0.06	0.24
0.19	0.19	0.66	1.4	9.54	2.09	9.24	0.07	0.26
0.20	0.20	0.76	1.6	10.42	2.16	10.10	0.07	0.27
0.21	0.21	0.87	1.9	11.30	2.23	10.96	0.08	0.29
0.22	0.22	0.98	2.3	12.18	2.30	11.83	0.08	0.30
0.23	0.23	1.10	2.6	13.06	2.38	12.69	0.09	0.32
0.24	0.24	1.23	3.0	13.93	2.45	13.55	0.09	0.33
0.25	0.25	1.37	3.5	14.81	2.53	14.41	0.10	0.35
0.26	0.26	1.52	4.0	15.69	2.61	15.27	0.11	0.37
0.27	0.27	1.68	4.5	16.57	2.69	16.14	0.11	0.38
0.28	0.28	1.84	5.1	17.45	2.76	17.00	0.12	0.40
0.29	0.29	2.02	5.7	18.32	2.84	17.86	0.13	0.42
0.30	0.30	2.20	6.4	19.20	2.92	18.72	0.13	0.43
0.31	0.31	2.39	7.2	20.08	2.99	19.59	0.14	0.45
0.32	0.32	2.59	8.0	20.96	3.07	20.45	0.15	0.47
0.33	0.33	2.80	8.8	21.84	3.14	21.31	0.15	0.48
0.34	0.34	3.02	9.7	22.71	3.22	22.17	0.16	0.50
0.35	0.35	3.25	10.7	23.59	3.29	23.03	0.17	0.52
0.36	0.36	3.48	11.7	24.47	3.37	23.90	0.18	0.54
0.37	0.37	3.72	12.8	25.35	3.44	24.76	0.18	0.55
0.38	0.38	3.98	14.0	26.23	3.51	25.62	0.19	0.57
0.39	0.39	4.24	15.2	27.10	3.59	26.48	0.20	0.59
0.40	0.40	4.50	16.5	27.98	3.66	27.35	0.21	0.61
0.41	0.41	4.78	17.8	28.86	3.73	28.21	0.22	0.63
0.42	0.42	5.06	19.6	28.88	3.87	28.21	0.23	0.65
0.43	0.43	5.35	21.5	28.90	4.01	28.22	0.25	0.68
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.45	0.45	5.91	25.3	28.94	4.29	28.23	0.29	0.74
0.46	0.46	6.19	27.4	28.96	4.42	28.23	0.30	0.76
0.47	0.47	6.48	29.5	28.98	4.55	28.24	0.32	0.79
0.48	0.48	6.76	31.6	29.01	4.68	28.24	0.34	0.82
0.49	0.49	7.04	33.9	29.03	4.81	28.25	0.36	0.85
0.50	0.50	7.32	36.1	29.05	4.93	28.25	0.38	0.88
0.51	0.51	7.61	38.5	29.07	5.06	28.26	0.40	0.91
0.52	0.52	7.89	40.9	29.09	5.18	28.26	0.42	0.94

ACTUAL Q

STREET CAPACITY

PC PROGRAM STREAM

SEPTEMBER 1994

GARDEN GATE LANE

- BASIN D

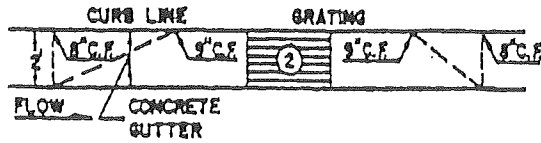
MANNING'S N= .017 SLOPE= .033

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	0.00	0.83	5	11.00	0.13	9	37.17	0.67
2	8.38	0.67	6	23.00	0.41	10	37.63	0.67
3	8.83	0.67	7	35.00	0.13	11	46.00	0.83
4	9.00	0.00	8	37.00	0.00	12	0.00	0.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.01	0.01	0.00	0.0	0.33	0.45	0.31	0.00	0.01
0.02	0.02	0.01	0.0	0.66	0.71	0.63	0.01	0.03
0.03	0.03	0.01	0.0	0.99	0.93	0.94	0.01	0.04
0.04	0.04	0.03	0.0	1.32	1.13	1.25	0.02	0.06
0.05	0.05	0.04	0.1	1.64	1.31	1.56	0.03	0.08
0.06	0.06	0.06	0.1	1.97	1.48	1.88	0.03	0.09
0.07	0.07	0.08	0.1	2.30	1.64	2.19	0.04	0.11
0.08	0.08	0.10	0.2	2.63	1.80	2.50	0.05	0.13
0.09	0.09	0.13	0.2	2.96	1.94	2.81	0.06	0.15
0.10	0.10	0.16	0.3	3.29	2.08	3.13	0.07	0.17
0.11	0.11	0.19	0.4	3.62	2.22	3.44	0.08	0.19
0.12	0.12	0.23	0.5	3.95	2.35	3.75	0.09	0.21
0.13	0.13	0.26	0.7	4.28	2.48	4.07	0.10	0.23
0.14	0.14	0.31	0.8	5.15	2.43	4.93	0.09	0.23
0.15	0.15	0.36	0.9	6.03	2.44	5.79	0.09	0.24
0.16	0.16	0.43	1.1	6.91	2.47	6.65	0.10	0.26
0.17	0.17	0.50	1.3	7.79	2.53	7.51	0.10	0.27
0.18	0.18	0.58	1.5	8.67	2.60	8.38	0.11	0.29
0.19	0.19	0.66	1.8	9.54	2.68	9.24	0.11	0.30
0.20	0.20	0.76	2.1	10.42	2.77	10.10	0.12	0.32
0.21	0.21	0.87	2.5	11.30	2.86	10.96	0.13	0.34
0.22	0.22	0.98	2.9	12.18	2.96	11.83	0.14	0.36
0.23	0.23	1.10	3.4	13.06	3.06	12.69	0.14	0.37
0.24	0.24	1.23	3.9	13.93	3.15	13.55	0.15	0.39
0.25	0.25	1.37	4.5	14.81	3.25	14.41	0.16	0.41
0.26	0.26	1.52	5.1	15.69	3.35	15.27	0.17	0.43
0.27	0.27	1.68	5.8	16.57	3.45	16.14	0.18	0.45
0.28	0.28	1.84	6.5	17.45	3.55	17.00	0.20	0.48
0.29	0.29	2.02	7.4	18.32	3.65	17.86	0.21	0.50
0.30	0.30	2.20	8.2	19.20	3.75	18.72	0.22	0.52
0.31	0.31	2.39	9.2	20.08	3.85	19.59	0.23	0.54
0.32	0.32	2.59	10.2	20.96	3.94	20.45	0.24	0.56
0.33	0.33	2.80	11.3	21.84	4.04	21.31	0.25	0.58
0.34	0.34	3.02	12.5	22.71	4.14	22.17	0.27	0.61
0.35	0.35	3.25	13.7	23.59	4.23	23.03	0.28	0.63
0.36	0.36	3.48	15.1	24.47	4.33	23.90	0.29	0.65
0.37	0.37	3.72	16.5	25.35	4.42	24.76	0.30	0.67
0.38	0.38	3.98	17.9	26.23	4.51	25.62	0.32	0.70
0.39	0.39	4.24	19.5	27.10	4.61	26.48	0.33	0.72
0.40	0.40	4.50	21.2	27.98	4.70	27.35	0.34	0.74
0.41	0.41	4.78	22.9	28.86	4.79	28.21	0.36	0.77
0.42	0.42	5.06	25.2	28.88	4.97	28.21	0.38	0.80
0.43	0.43	5.35	27.6	28.90	5.16	28.22	0.41	0.84
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
(FT)	INC	AREA	RATE	PER	VEL	(FT)	HEAD	HEAD
(FT)	(FT)	SQ. FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.45	0.45	5.91	32.6	28.94	5.51	28.23	0.47	0.92
0.46	0.46	6.19	35.2	28.96	5.68	28.23	0.50	0.96
0.47	0.47	6.48	37.9	28.98	5.85	28.24	0.53	1.00
0.48	0.48	6.76	40.6	29.01	6.01	28.24	0.56	1.04
0.49	0.49	7.04	43.5	29.03	6.18	28.25	0.59	1.08
0.50	0.50	7.32	46.4	29.05	6.34	28.25	0.62	1.12
0.51	0.51	7.61	49.4	29.07	6.50	28.26	0.66	1.17
0.52	0.52	7.89	52.5	29.09	6.65	28.26	0.69	1.21

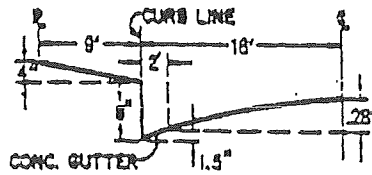
STREET CAPACITY

ACTUAL Q = 36.8

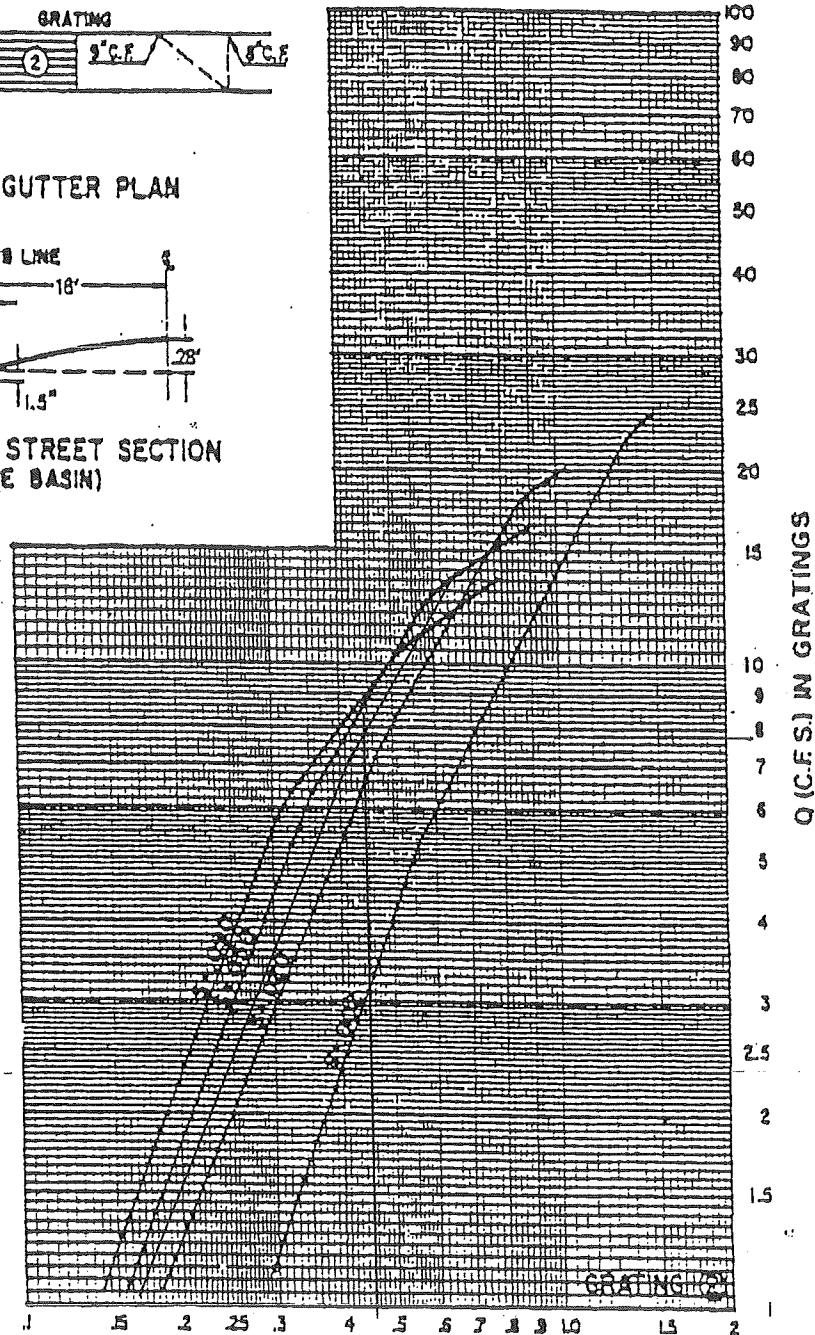
GRATING CAPACITIES FOR TYPE 'A' , 'C' and 'D'



GRATING & GUTTER PLAN



TYPICAL HALF STREET SECTION
(ABOVE BASIN)



D = DEPTH OF FLOW (FT.) ABOVE NORMAL GUTTER GRADE

$$D = 0.46'$$

PLATE 22.3 D-5

Garden Gate Lane

Basin D

Slope = 3.3%

B-15/15

APPENDIX C

INFRASTRUCTURE LIST

Current DRC
Project No.

Date Submitted: January 8, 2004

Date Site Plan Approved:

Date Preliminary Plat Approved:

Date Preliminary Plat Expires:

DRB Project No.

APPLICATION NO.

Figure 12

INFRASTRUCTURE LIST

EXHIBIT "A"
TO SUBDIVISION IMPROVEMENTS AGREEMENT
DEVELOPMENT REVIEW BOARD (D.R.B.) REQUIRED INFRASTRUCTURE LIST

SUN GATE SUBDIVISION
PRELIMINARY PLAT

Following is a summary of PUBLIC/PRIVATE Infrastructure required to be constructed or financially guaranteed for the above development. This Listing is not necessarily a complete listing. During the SIA process and/or in the review of the construction drawings, if the DRC Chair determines that appurtenant items and/or unforeseen items have not been included in the Infrastructure listing, the DRC Chair may include those items in the listing and related financial guarantee. Likewise, if the DRC Chair determines that appurtenant or non-essential items can be deleted from the listing, those items may be deleted as well as the related portions of the financial guarantees. All such revisions require approval by the DRC Chair, the User Department and agent/owner. If such approvals are obtained, these revisions to the listing will be incorporated administratively. In addition, any unforeseen items which arise during construction which are necessary to complete the project and which normally are the Subdivider's responsibility will be required as a condition of project acceptance and close out by the City.

SIA Sequence #	COA DRC Project #	Size	Type of Improvement	Location	From	To	Private Inspector	City Inspector	City Cnst Engineer
ONSITE PUBLIC ROADWAY IMPROVEMENTS									
		30' EOA-F	ARTERIAL PAVING W/ PCC CURB & GUTTER AND PCC 6' WIDE SIDEWALK ON SOUTH SIDE ONLY	GIBSON BLVD.	MESA ARENOSO DR.	BLAKE ROAD	/	/	/
		24' EOA-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON WEST SIDE ONLY	BLAKE ROAD	GIBSON BOULEVARD	OPEN RANGE AVENUE	/	/	/
		16' EOA-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON EAST SIDE ONLY	MESA ARENOSO DR	GIBSON BOULEVARD	OPEN RANGE AVENUE	/	/	/
		24' EOA-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON NORTH SIDE ONLY	OPEN RANGE AVE.	BLAKE ROAD	MESA ARENOSO DRIVE	/	/	/
		24' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON NORTH SIDE ONLY*	CORRAL GATE LANE	WEST STUB TERMINUS	MEADOW GATE TRAIL	/	/	/
		28' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON BOTH SIDES*	CORRAL GATE LANE	MEADOW GATE TRAIL	BRIDAL GATE TRAIL	/	/	/
		24' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON NORTH SIDE ONLY*	CORRAL GATE LANE	BRIDAL GATE TRAIL	EAST STUB TERMINUS	/	/	/
		28' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON BOTH SIDES*	MEADOW GATE TRAIL	CORRAL GATE LANE	GARDEN GATE LANE	/	/	/
		28' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON BOTH SIDES*	SUN GATE TRAIL	CORRAL GATE LANE	GARDEN GATE LANE	/	/	/

SIA Sequence #	COA DRC Project #	Size	Type of Improvement	Location	From	To	Private Inspector	City Inspector	City Crst Engineer
		28' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON BOTH SIDES*	IRON GATE TRAIL	CORRAL GATE LANE	GARDEN GATE LANE	/	/	/
		24' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON SOUTH SIDE ONLY*	GARDEN GATE LANE	WEST STUB TERMINUS	MEADOW GATE TRAIL	/	/	/
		28' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON BOTH SIDES*	GARDEN GATE LANE	MEADOW GATE TRAIL	BRIDAL GATE TRAIL	/	/	/
		24' F-F	RESIDENTIAL PAVING W/ PCC CURB & GUTTER AND PCC 4' WIDE SIDEWALK ON SOUTH SIDE ONLY*	GARDEN GATE LANE	BRIDAL GATE TRAIL	EAST STUB TERMINUS	/	/	/

* SIDEWALKS TO BE DEFERRED PER DEFERRAL EXHIBIT 'B'.

STREET LIGHTS AS PER COA DPM

ONSITE PUBLIC STORM DRAIN IMPROVEMENTS

18"-36" D/A	RCP W/ NEC. MH'S, LATERALS & INLETS	GARDEN GATE LANE	EAST STUB TERMINUS	RETENTION POND	/	/	/
66" D/A	RCP W/ NEC. MH'S, LATERALS & INLETS	OPEN RANGE AVENUE	MESA ARENOSA DR.	BLAKE ROAD	/	/	/
18" - 36" D/A	RCP W/ NEC. MH'S, LATERALS & INLETS	GARDEN GATE LANE	SUN GATE TRAIL	STONE GATE WAY	/	/	/
	2.7 ACRE- FEET TEMPORARY RETENTION POND WITH PUBLIC EASEMENT AND COVENANT AND AGREEMENT	TRACT 34D-1-A			/	/	/

NOTE: A GRADING AND DRAINAGE CERTIFICATION OF THE APPROVED GRADING PLAN IS REQUIRED
PRIOR TO THE RELEASE OF FINANCIAL GUARANTEES.

NINA LEUNG		DRB CHAIR	DATE	PARKS & RECREATION DEPARTMENT	DATE
PREPARED BY: PRINT NAME					
BOHANNAN HUSTON INC.		TRANSPORTATION DEVELOPMENT	DATE	AMAFCA	DATE
FIRM:					
SIGNATURE		UTILITY DEVELOPMENT	DATE	CITY ENGINEER	DATE
MAXIMUM TIME ALLOW TO CONSTRUCT					
IMPROVEMENTS WITHOUT A DRB EXTENSION		NEW MEXICO UTILITIES INC.	DATE		DATE

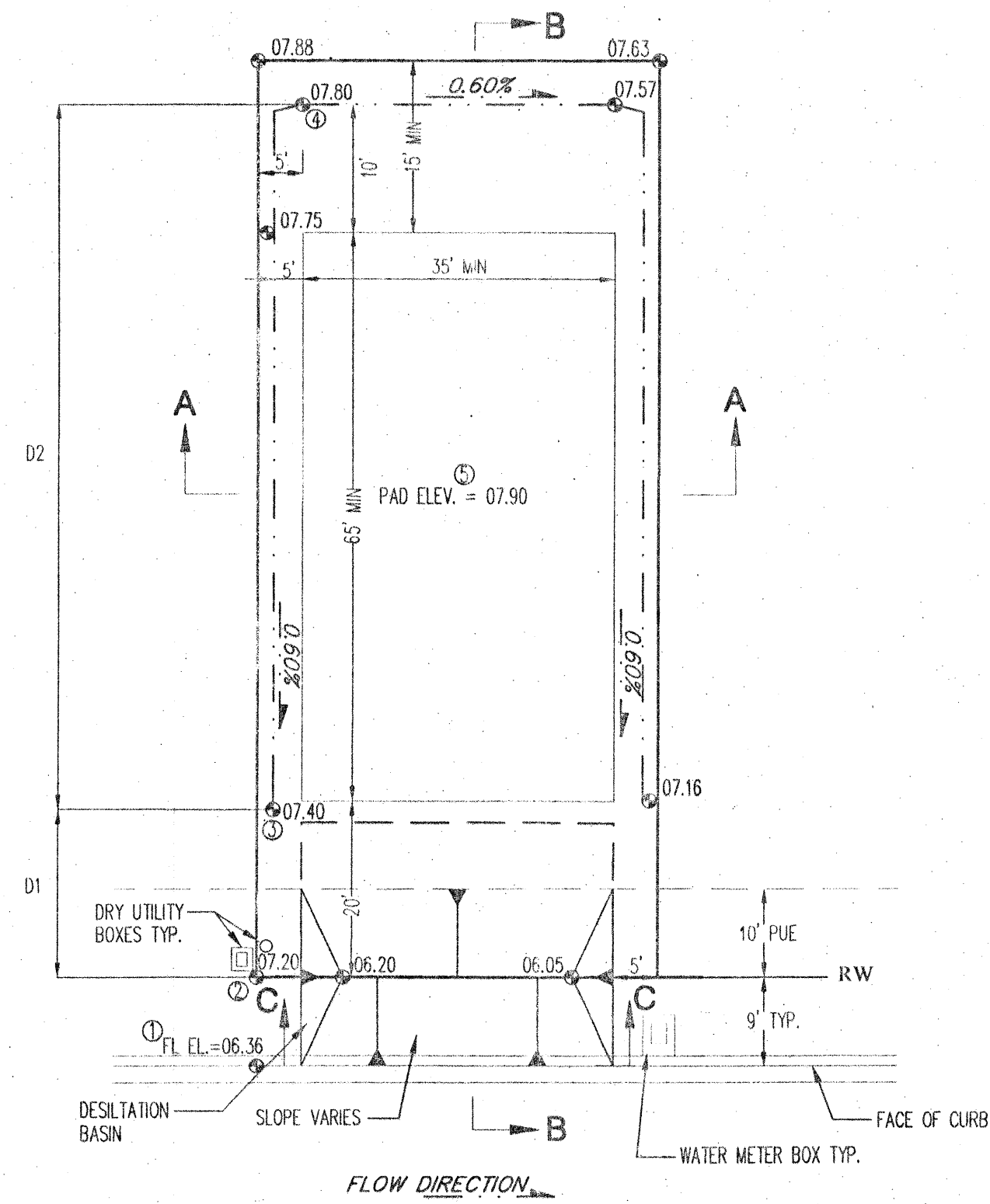
DESIGN REVIEW COMMITTEE REVISIONS					AGENT/OWNER
REVISION	DATE	DRC CHAIR	USER DEPARTMENT		

EXHIBITS

- EXHIBIT 1 - PRELIMINARY PLAT
- EXHIBIT 2 - GRADING PLAN
- EXHIBIT 3 - DEVELOPED CONDITIONS BASIN MAP
- EXHIBIT 4 - MASTER STORM DRAIN BASIN MAP

EXHIBIT 1

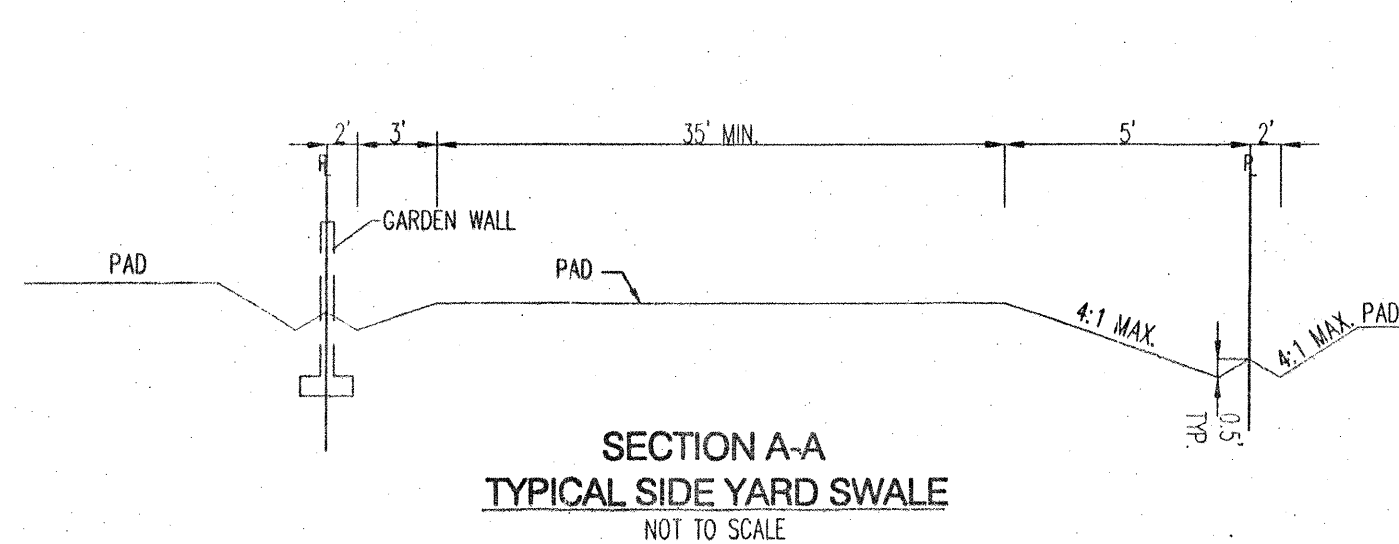
PRELIMINARY PLAT



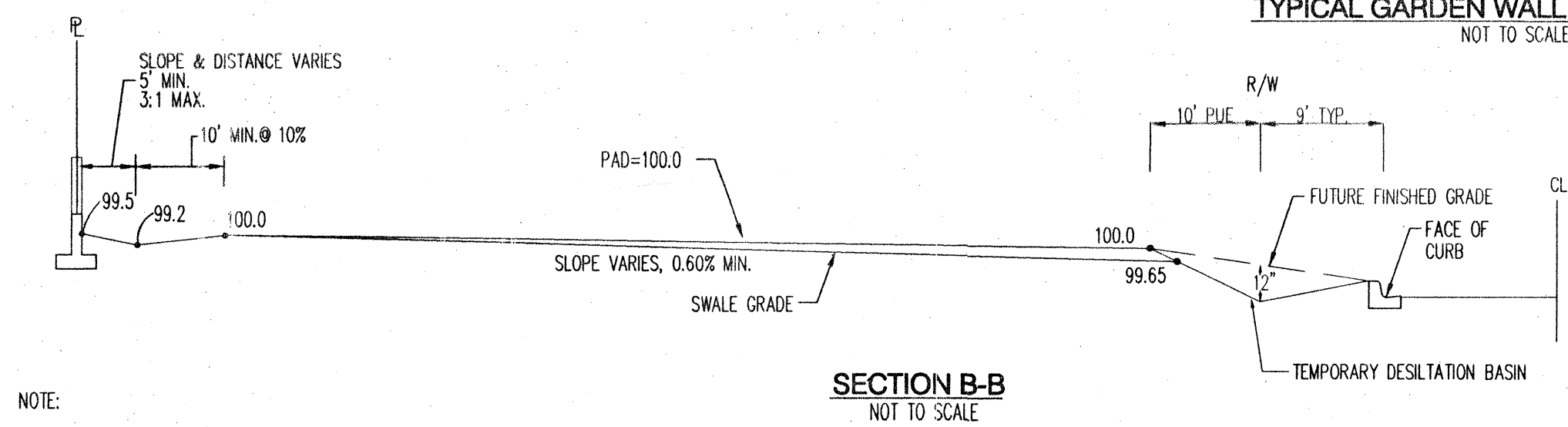
**SUN GATE SUBDIVISION
TYPICAL LOT GRADE DETAIL
WITH DESILITATION BASIN FOR SEDIMENTATION CONTROL**
NOT TO SCALE

TO SET SPOT ② - ADD 0.84' TO SPOT ①
TO SET SPOT ③ - MULTIPLY D1 BY 1.1% AND ADD TO SPOT ②
TO SET SPOT ④ - MULTIPLY D2 BY 0.3% AND ADD TO SPOT ③
TO SET SPOT ⑤ - ADD 0.2' TO SPOT ④

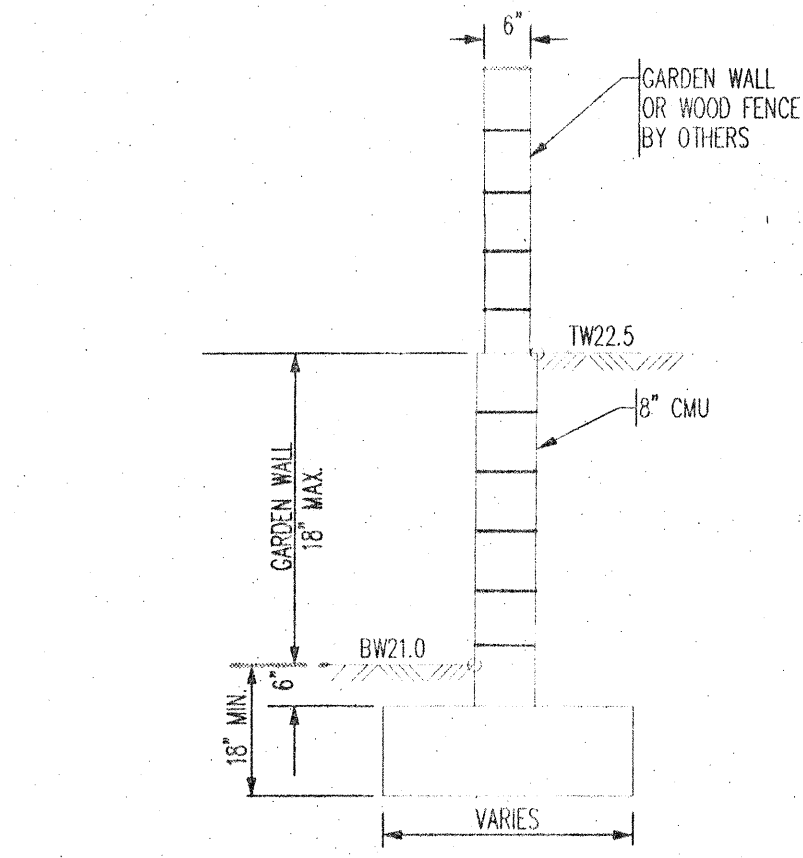
BOTTOM OF BASIN IS 1' BELOW PROPERTY LINE ELEVATION.
SEE GRADING PLANS FOR EXACT ELEVATIONS.
CONTRACTOR SHALL CONSTRUCT TEMPORARY DESILITATION BASIN AT EACH LOT.



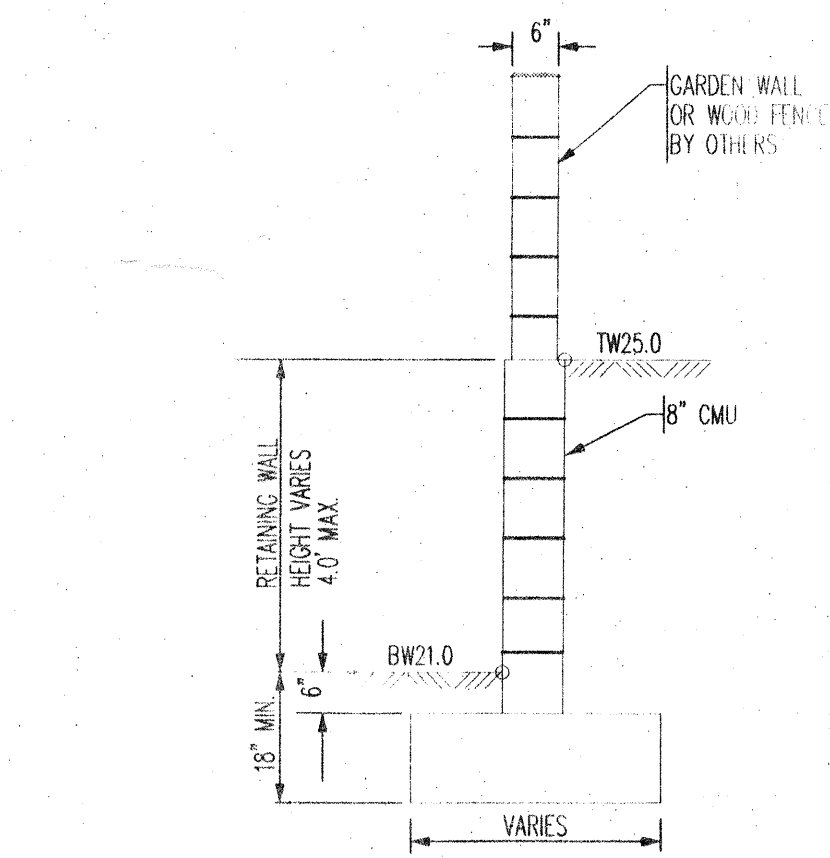
**SECTION A-A
TYPICAL SIDE YARD SWALE**
NOT TO SCALE



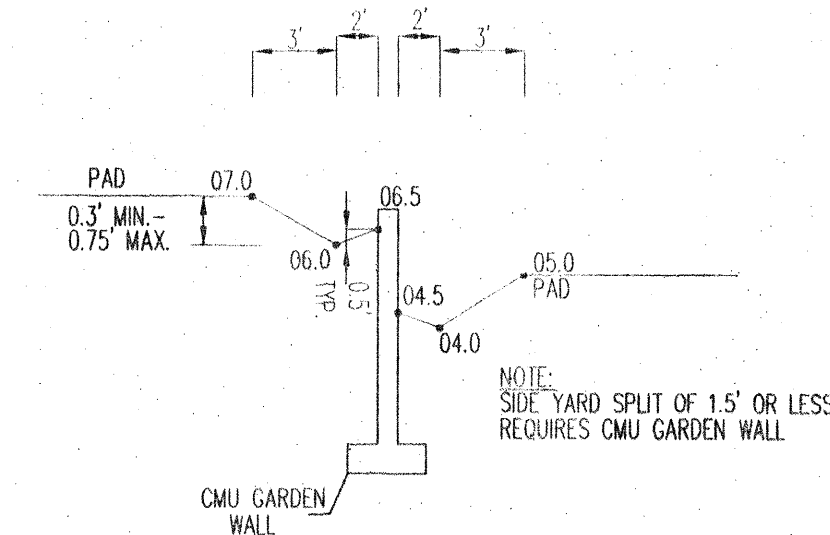
**SECTION B-B
NOT TO SCALE**



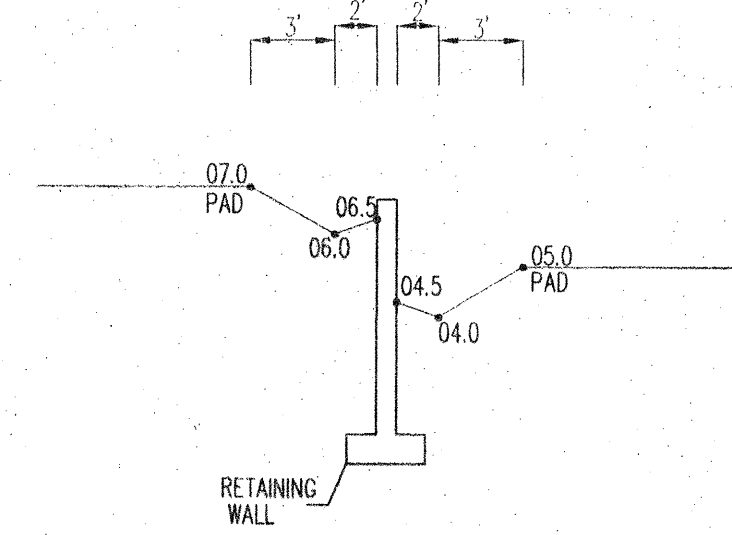
TYPICAL GARDEN WALL NOMENCLATURE
NOT TO SCALE
(RETAINING HEIGHT IS TAKEN TO BE DIFFERENCE IN FINISHED GRADES ON LEFT AND RIGHT SIDE OF WALL.)



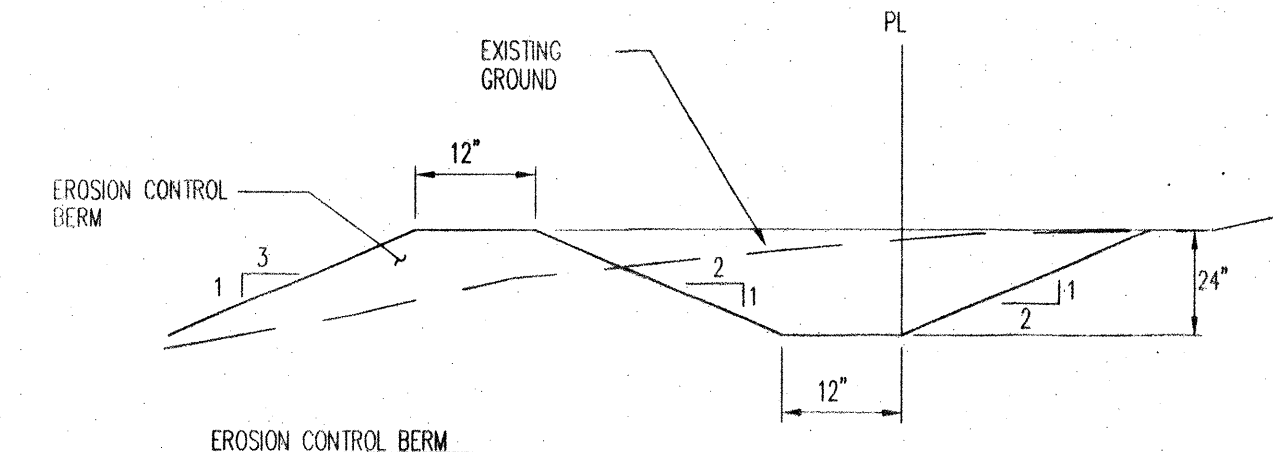
TYPICAL RETAINING WALL NOMENCLATURE
NOT TO SCALE
(RETAINING HEIGHT IS TAKEN TO BE DIFFERENCE IN FINISHED GRADES ON LEFT AND RIGHT SIDE OF WALL.)



SIDE YARD GARDEN WALL DETAIL
NOT TO SCALE



SIDE YARD RETAINING WALL DETAIL
NOT TO SCALE



1. TO PREVENT EROSION FROM THE SITE, DURING GRADING & CONSTRUCTION OPERATIONS, A BERM OF THE ABOVE DIMENSIONS SHALL BE MAINTAINED ON THE WEST WEST BOUNDARY OF THE SITE.
2. WIND EROSION SHALL BE PREVENTED BY MAINTAINING AN ADEQUATELY MOISTENED SITE.
3. AFTER CONSTRUCTION ALL SURFACES WILL BE PAVED OR LANDSCAPED TO PREVENT EROSION.
4. CONTRACTOR MUST OBTAIN A TOP SOIL DISTURBANCE PERMIT FROM ENVIRONMENTAL HEALTH DIVISION PRIOR TO CONSTRUCTION

EROSION CONTROL PLAN
NOT TO SCALE

- NOTE:
1. CONTRACTOR IS TO MASS GRADE ROADS TO 2' BEYOND FUTURE CURB. EXCESS FROM DRY UTILITY TRENCH IS TO BE USED TO BACK FILL BEHIND CURB.
 2. FRONT YARDS ARE TO BE GRADED AS SHOWN ON THIS DETAIL FOR FINAL GRADING AND CERTIFICATION THIS DETAIL TO BE COORDINATED WITH.
 3. HOME BUILDER TO BRING FRONT YARD TO ULTIMATE FRONT YARD GRADES AFTER HOME CONSTRUCTION IS COMPLETED. SEE ULTIMATE FRONT YARD GRADING DETAIL ON THIS SHEET.

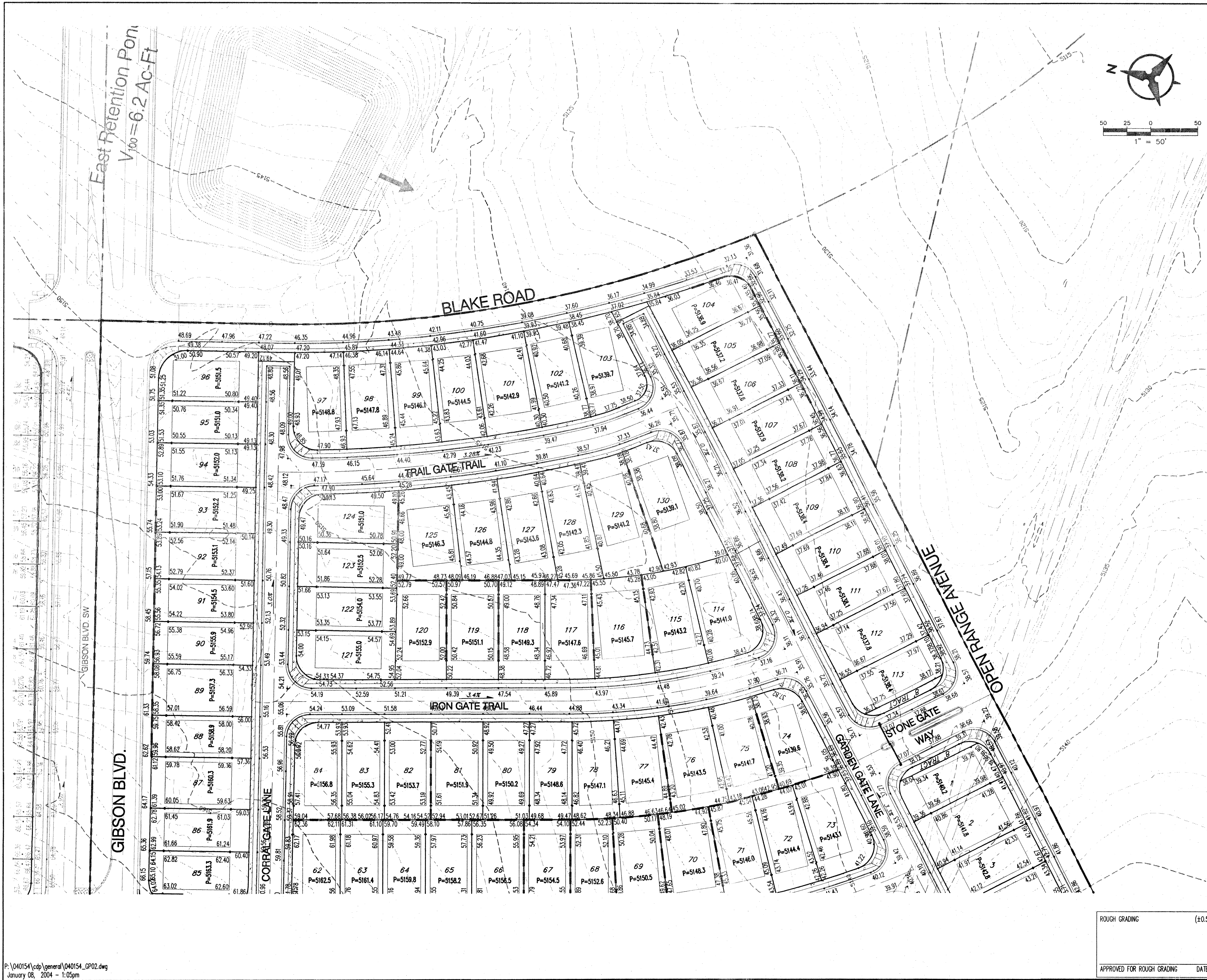
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**CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT**

**SUN GATE SUBDIVISION
GRADING AND DRAINAGE PLAN
GRADING DETAILS**

Design Review Committee	City Engineer Approval	Last Design Update	Mo./Day/Yr.	Mo./Day/Yr.
City Project No.	Zone Map No.	Sheet	Of	
	N-9-Z	3	3	

ENGINEER'S SEAL		SURVEY INFORMATION		BENCH MARKS		AS-BUILT INFORMATION		
	FIELD NOTES		DATE		CONTRACTOR		DATE	
	NO.	BY			STAGED BY		DATE	
					DESIGNED BY		DATE	
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LEGEND

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- === PROPOSED MOUNTABLE CURB & GUTTER
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- PROPOSED CATTLE GUARD INLET
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Bohannon & Huston

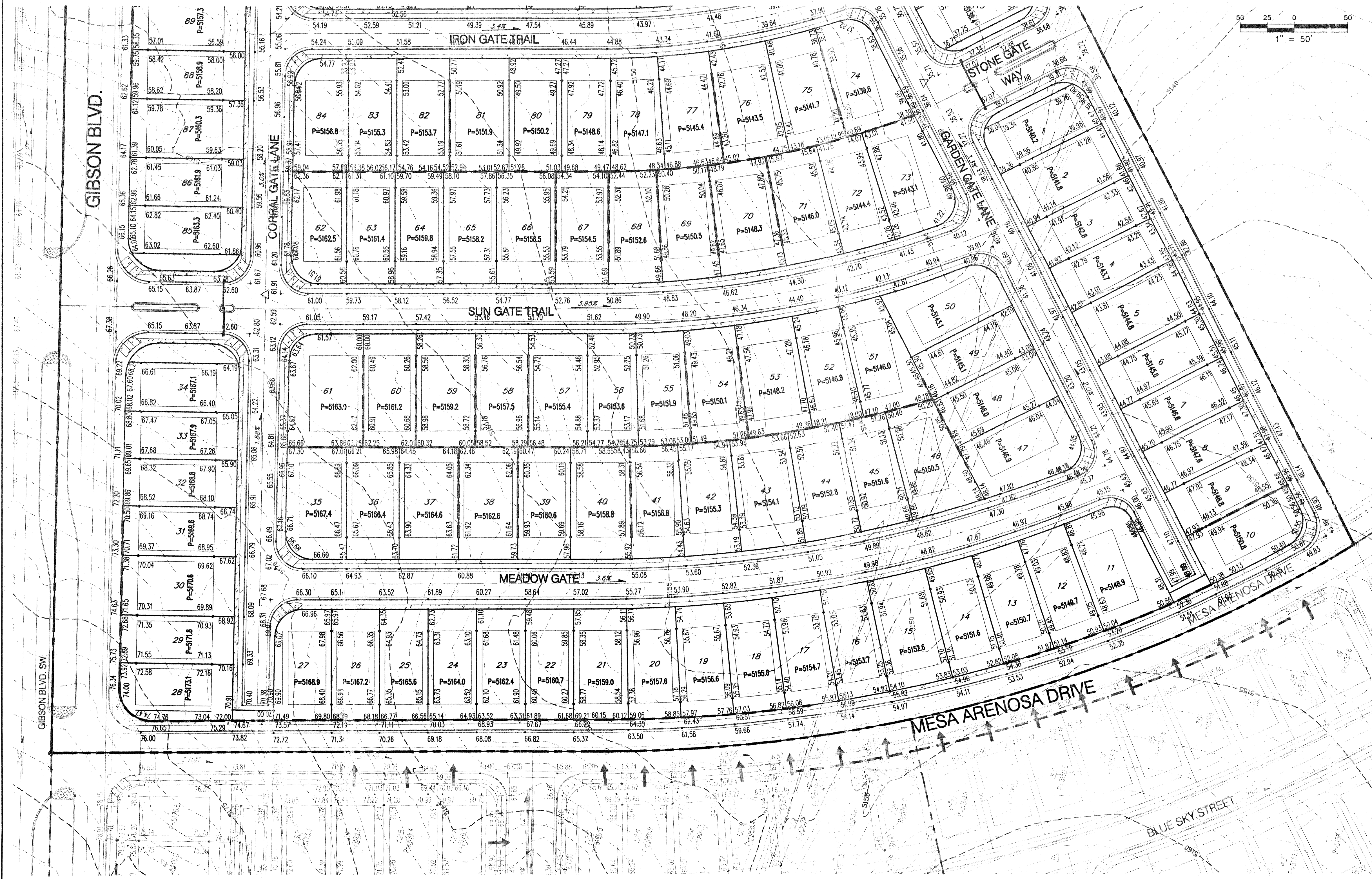
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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT

SUN GATE SUBDIVISION GRADING AND DRAINAGE PLAN

Design Review Committee	City Engineer Approval	Last Design Update	Mo./Day/Yr.	Mo./Day/Yr.
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	N-9-Z	2	3	



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CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT	
SUN GATE SUBDIVISION GRADING AND DRAINAGE PLAN	
Design Review Committee	City Engineer Approval
City Project No.	Zone Map No.
	N-9-Z
Sheet	Of
1	3

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EXHIBIT 2

GRADING PLAN

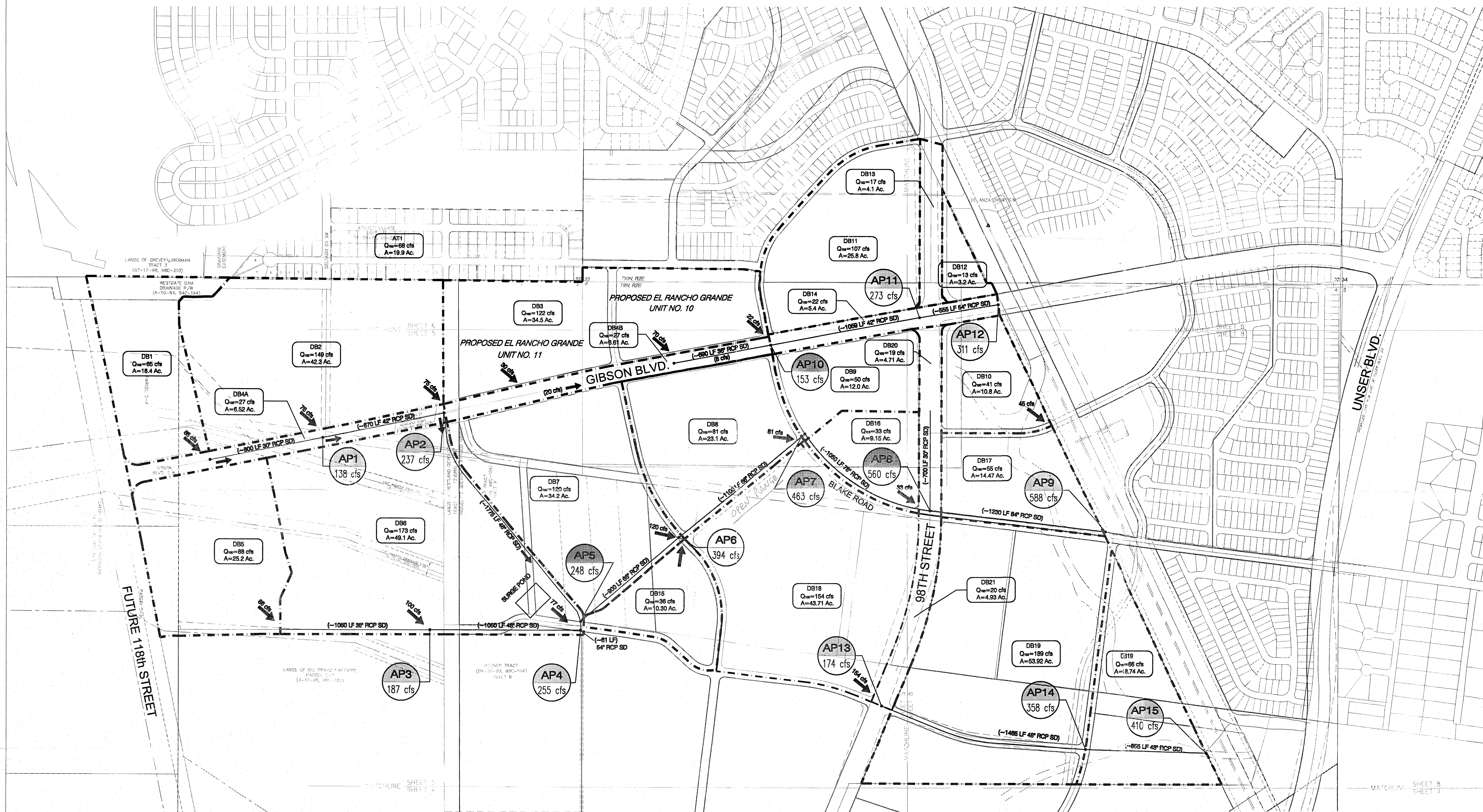
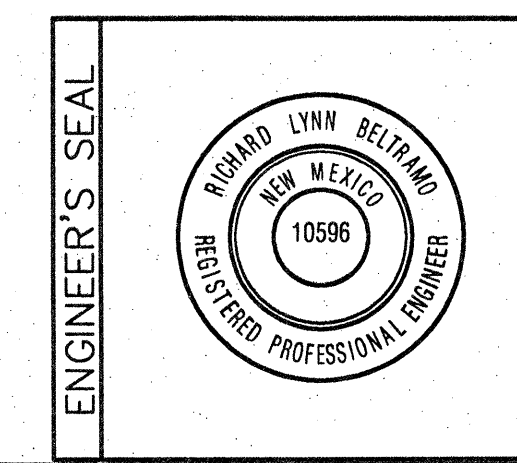
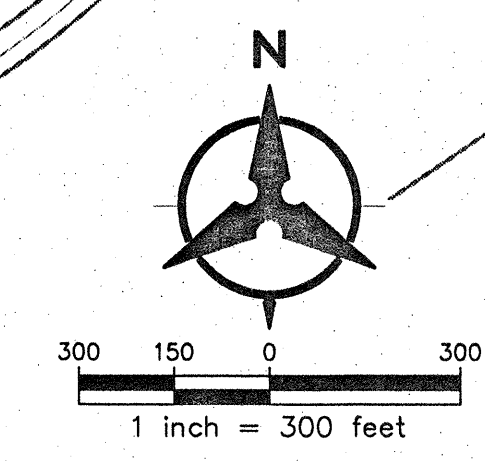


EXHIBIT 4 MASTER STORM DRAIN BASIN MAP

PIPE	FROM AP	TO AP	SIZE (IN)	LENGTH (FT)	SLOPE (%)	FLOW (CFS)	PIPE CAP (CFS)
P1	2	4	54	1775	2.30	237	301
P2	5	6	60	900	1.40	245	307
P3	6	7	66	1100	1.50	394	411
P4	7	8	78	1050	1.00	463	524
P5	8	9	78	1230	2.50	560	829



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EXHIBIT 3

DEVELOPED CONDITIONS BASIN MAP

EXHIBIT 4

MASTER STORM DRAIN BASIN MAP

PRELIMINARY PLAT FOR SUN GATE SUBDIVISION TRACT 33C-1-A LANDS OF SALAZAR FAMILY TRUST, SALAZAR QUATRO TRUST, JSJ INVESTMENT COMPANY AND FALBA HANNETT

LEGAL DESCRIPTION

TRACT 33C-1-A LANDS OF SALAZAR FAMILY TRUST, SALAZAR QUATRO TRUST, JSJ INVESTMENT COMPANY AND FALBA HANNETT.
(BOOK 2003C, PAGE 357, DATE 10-05-03)

GENERAL NOTES

- EXISTING ZONING: R-2 ALLOWING R-T AND R-LT
(RIO BRAVO SECTOR PLAN)
PROPOSED DEVELOPMENT: R-2 ALLOWING R-T AND R-LT
- PROPOSED NET ACREAGE: 20.78 AC
NUMBER OF LOTS: 130
PROPOSED DENSITY: 6.26 DU/AC
- MIN. LOT DIMENSIONS: 45' X 105'
MINIMUM LOT AREA: 4725 SQ FT
- ALL UTILITIES AND STORM DRAIN IMPROVEMENTS ARE TO BE PUBLIC, AND TO BE DEDICATED TO THE CITY OF ALBUQUERQUE FOR MAINTENANCE.
PROPOSED DEVELOPMENT: R-2 ALLOWING R-T AND R-LT
- PRIVATE ACCESS: THIS SUBDIVISION IS PLANNED TO BE A GATED COMMUNITY. ALL ROADS WITHIN THE DEVELOPMENT EXIST WITHIN TRACT "A". PRIVATE ACCESS, PUBLIC DRAINAGE, WATER, AND SANITARY SEWER EASEMENT ON TRACT "A".
- HOMEOWNERS ASSOCIATION: THIS SUBDIVISION WILL HAVE A HOMEOWNERS ASSOCIATION. THE HOMEOWNERS ASSOCIATION WILL OWN TRACTS "A", "B", "C", "D", "E", "F", "G", "H", AND "J" AND BE RESPONSIBLE FOR MAINTENANCE INCLUDING STREET AND LANDSCAPE IMPROVEMENTS.
- TRACTS "B", "C", "D", "E", "F", "G", "H", AND "J" ARE LANDSCAPE TRACTS ENCUMBERED BY PRIVATE PEDESTRIAN ACCESS EASEMENTS
- LOT SETBACKS SHALL BE 20' TO GARAGE, 5' SIDEYARD, AND 15' BACKYARD
- NO INDIVIDUAL LOTS SHALL BE ALLOWED DIRECT ACCESS TO MESA ARENOSA DRIVE, OPEN RANGE AVENUE, AND/OR BLAKE ROAD.

SITE DATA

ZONE ATLAS NO.	N-9-Z
ZONING	R-2 ALLOWING R-T AND R-LT
MILES OF FULL WIDTH STREETS CREATED	.92 MILES
NO. OF EXISTING PARCELS	1
NO. OF LOTS CREATED	130
DENSITY	6.26 DU/AC

SURVEY NOTES:

- ALL BOUNDARY CORNERS SHOWN (●) ARE FOUND REBAR W/CAP.
- ALL STREET CENTERLINE MONUMENTATION SHALL BE INSTALLED AT ALL CENTERLINE PC'S, PTS, ANGLE POINTS, AND STREET INTERSECTIONS AND SHOWN THUS (▲) AND WILL BE MARKED BY (4") ALUMINUM CAP STAMPED "CITY OF ALBUQUERQUE CENTERLINE MONUMENTATION MARKED, DO NOT DISTURB PLS 2219".
- THE SUBDIVISION BOUNDARY WILL BE TIED TO THE NEW MEXICO STATE PLANE COORDINATE SYSTEM AS SHOWN.
- BASIS OF BEARINGS WILL BE NEW MEXICO STATE PLANE BEARINGS.
- DISTANCES SHALL BE GROUND DISTANCES.
- MANHOLES WILL BE OFFSET AT ALL POINTS OF CURVATURE, TANGENCY STREET INTERSECTIONS, AND ALL OTHER ANGLE POINTS TO ALLOW USE OF CENTERLINE MONUMENTATION.

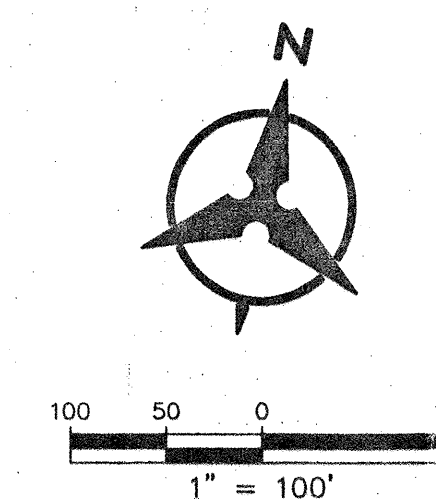
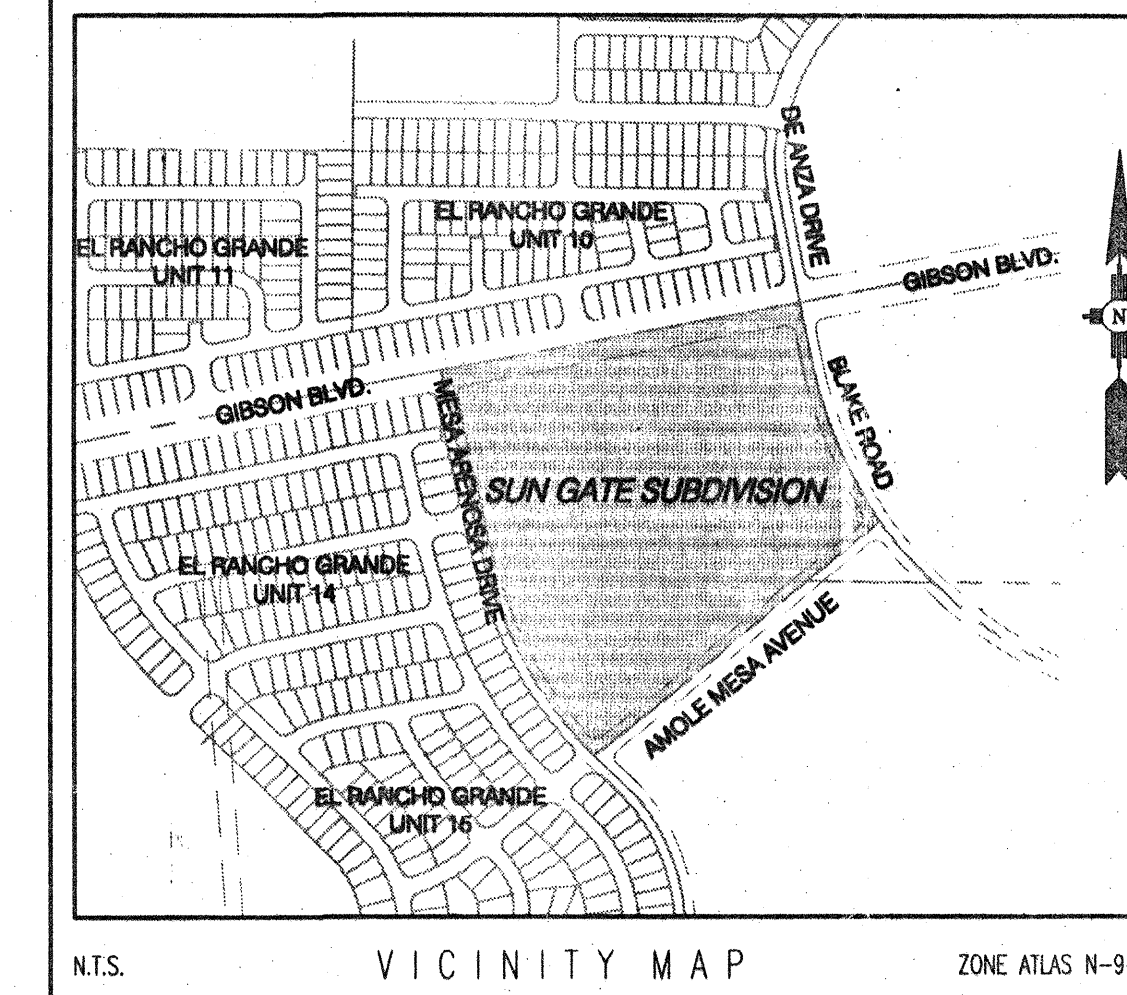
APPROVED

[Signature] 1-8-04
CITY SURVEYOR DATE

[Signature] 1/8/04
BO JOHNSON DATE
VICE PRESIDENT, CURB INC.

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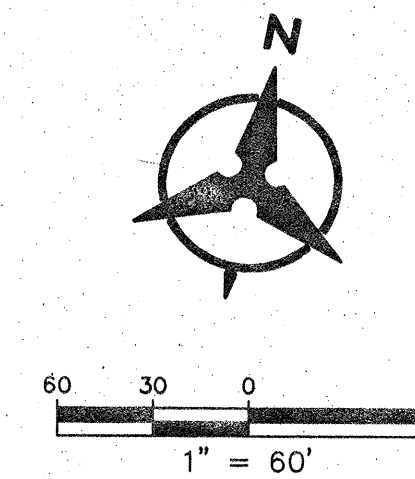
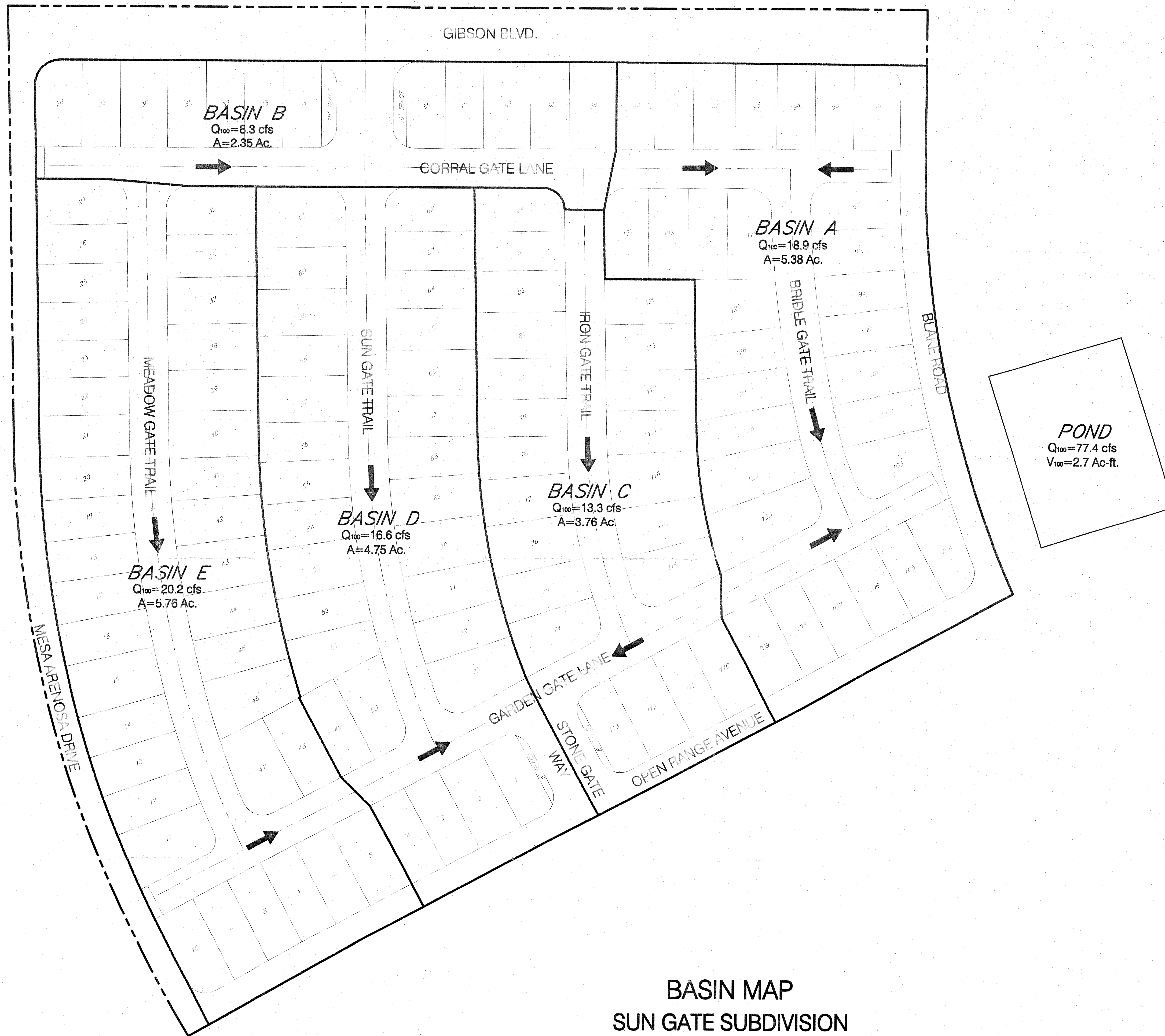


LEGEND	
—	SUBDIVISION BOUNDARY LINE
- - -	EXISTING SUBDIVISION BOUNDARY
—	NEW LOT LINE
- - -	ADJOINING PROPERTY LINE
▲	CENTERLINE MONUMENT TO BE INSTALLED
△	CITY OF ALBUQUERQUE SURVEY CONTROL MONUMENT

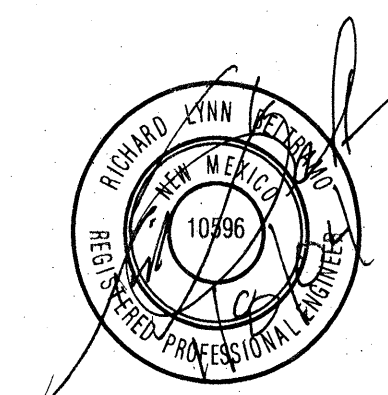
KEYED NOTES

- (A) 10' PUBLIC UTILITY EASEMENT





BASIN MAP
SUN GATE SUBDIVISION
JANUARY, 2004



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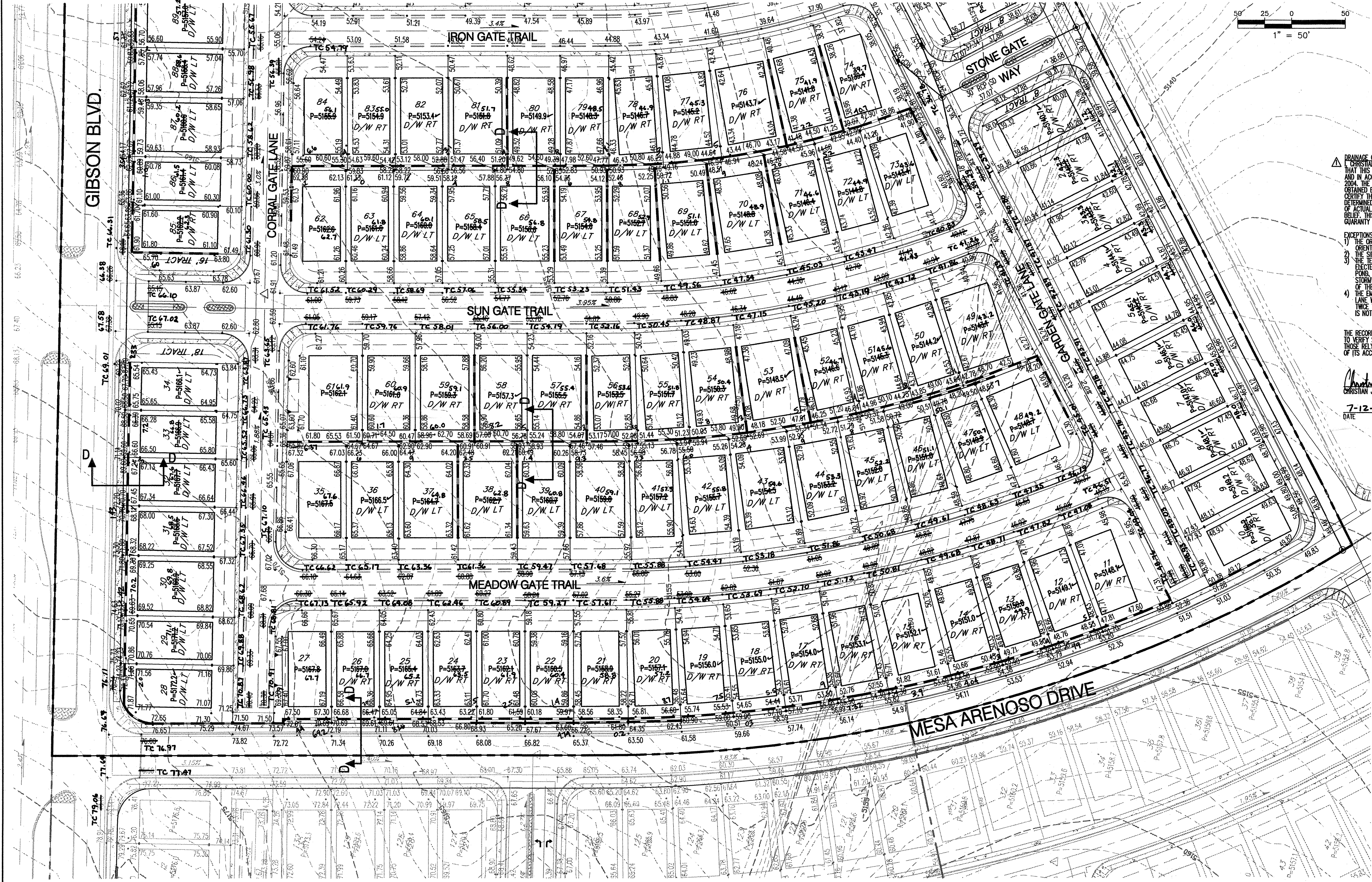
Courtyard I
7500 Jefferson St. NE
Albuquerque, NM
87109-4335

www.bhinc.com

voice: 505.823.1000

facsimile: 505.798.7988

toll free: 800.877.5332



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DRAINAGE CERTIFICATION
I, CHRISTIAN J. SULLIVAN, N.E.P.E. 16244, OF THE FIRM BOHANNAN HUSTON, INC. HEREBY CERTIFY THAT THIS PROJECT HAS BEEN GRADED AND DRAINAGE AS SHOWN ON THE APPROVED PLAN DATED JANUARY 4, 2004. THE RECORD INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY TWENTY ALBUQUERQUE MAPS 7774 OF THE FIRM BOHANNAN HUSTON, INC. I FURTHER CERTIFY THAT I HAVE PERSONALLY VISITED THE PROJECT SITE ON JULY 11, 2005, AND HAVE DETERMINED BY VISUAL INSPECTION THAT THE SURVEY DATA PROVIDED IS REPRESENTATIVE OF ACTUAL SITE CONDITIONS AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR SIA FINANCIAL GUARANTEE RELEASE.

EXCEPTIONS TO THE APPROVED PLAN ARE NOTED BELOW:
1) THE ORIENTATION OF LOTS 47-50 AND 121-124 WERE ROTATED 90 DEGS TO MATCH THE ORIENTATION OF THE REMAINING LOTS ON THE BLOCK.
2) THE SIDEWALK SLOPES WERE REVISED TO UTILIZE 1% SLOPES INSTEAD OF 0.5% SLOPES.
3) THE TEMPORARY STORM WATER POND AND TRAIL WAS NOT BUILT. THE DEVELOPER ELECTED TO BUILD A PUBLIC STORM DRAIN IN BLAKE ROAD IN LOT 12, 2005, AND HAD THE STORM DRAIN BUILT BY PHIL NO. 73870.
4) THE EMERGENCY OVERFLOW THRU THE NEW WALL AT THE NORTH END OF GARDEN GATE LANE WAS NOT BUILT. THE STORM INLET AT THIS LOCATION HAS CAPACITY TO ACCEPT TWICE THE 100-YEAR DEVELOPED FLOWRATE; THEREFORE, THE EMERGENCY OVERFLOW IS NOT NEEDED.

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE ASPECTS OF THIS PROJECT. THOSE RELYING ON THIS RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE.

Christian J. Sullivan
CHRISTIAN J. SULLIVAN, N.E.P.E. 16244
7-12-05
DATE



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 - ===== PROPOSED SLOPE
 - PROPOSED STORM DRAIN
 - PROPOSED STORM DRAIN MANHOLE
 - PROPOSED STORM DRAIN INLET
 - PROPOSED CATTLE GUARD INLET
 - WALL DRAIN
 - P=5154.1 D/W RT PAD ELEVATION WITH DRIVEWAY LOCATION SPECIFIED, SEE NOTE 8 ABOVE

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- ===== PROPOSED STORM DRAIN INLET
- ===== PROPOSED CATTLE GUARD INLET
- ===== WALL DRAIN
- P=5154.1 D/W RT PAD ELEVATION WITH DRIVEWAY LOCATION SPECIFIED, SEE NOTE 8 ABOVE

ENGINEER'S SEAL

CRISTIAN J. SHULTS
LICENSED PROFESSIONAL ENGINEER
16244

5-5-04

Bohannon & Huston

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CITY OF ALBUQUERQUE

PUBLIC WORKS DEPARTMENT

SUN GATE SUBDIVISION

GRADING AND DRAINAGE PLAN

Design Review Committee	City Engineer Approval	Mo./Day/Yr.	Mo./Day/Yr.

City Project No.	Zone Map No.	Sheet	Of
	N-9-Z	2	3

AS-BUILT INFORMATION

CONTRACTOR	DATE

INSPECTOR'S	DATE
STAMPED BY	

FIELD	DATE
VERIFICATION BY	

MICROFILM	DATE
RECORDED BY	

FIELD NOTES

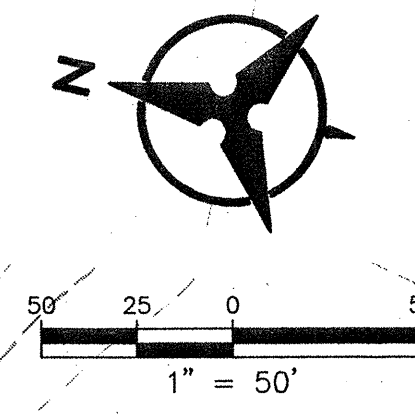
NO.	BY	DATE

EXCEPTIONS TO THE APPROVED PLAN ARE NOTED BELOW:

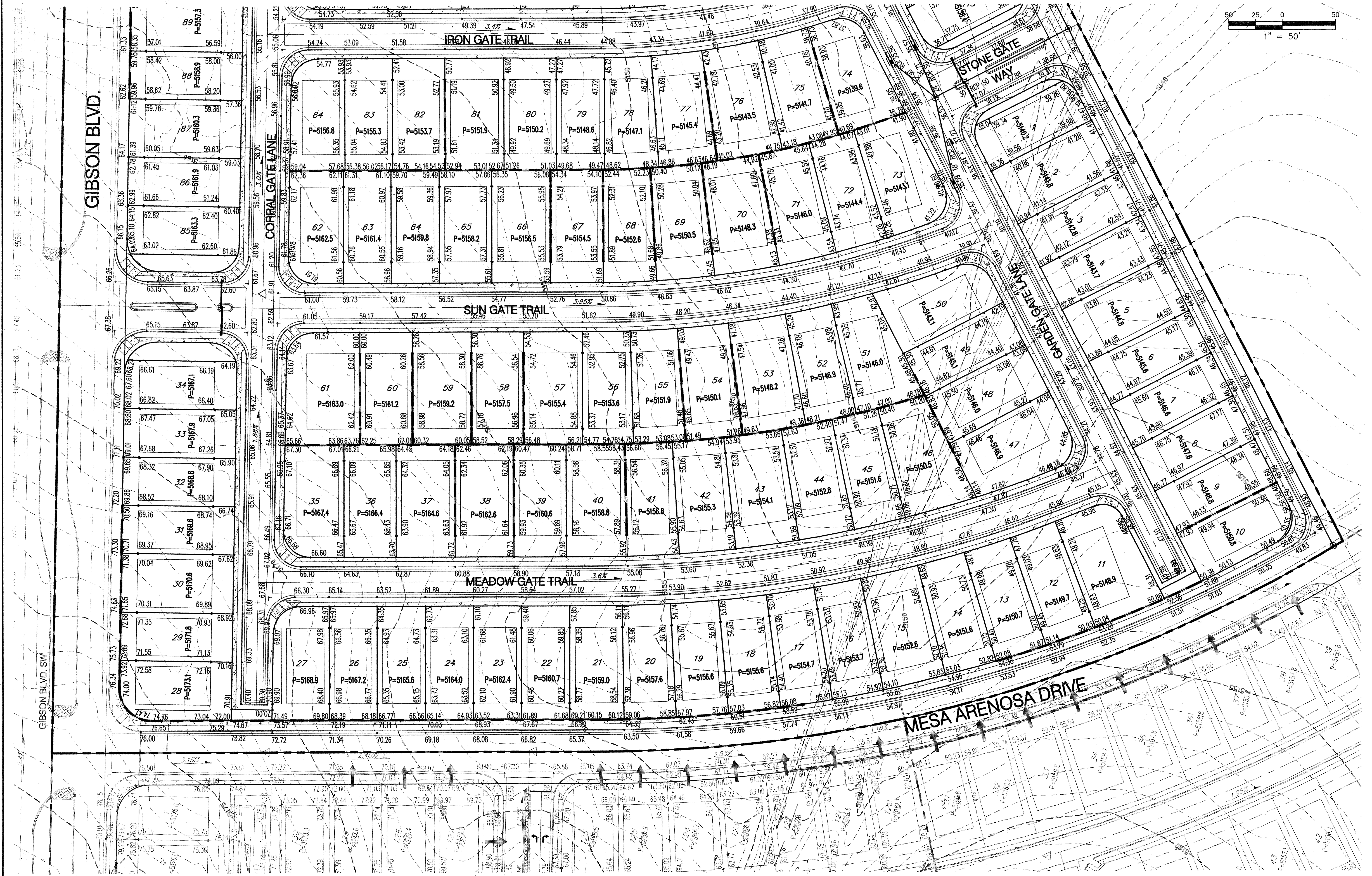
- THE ORIENTATION OF LOTS 47-50 AND 121-124 WERE ROTATED 90 DEG TO MATCH THE ORIENTATION OF THE REMAINING LOTS ON THE BLOCK.
- THE SLOPE SLOPES WERE REVISED TO UTILIZE 1% SLOPES INSTEAD OF 0.5% SLOPES.
- THE TEMPORARY STORM WATER POND ON TRACT 34-D-1-A WAS NOT BUILT. THE DEVELOPER ELECTED TO BUILD A PUBLIC STORM DRAIN IN BLAKE ROAD IN LEFT OF THE TEMPORARY POND. AS OF 7-11-05, THIS STORM DRAIN IS IN PLACE AND OPERATIONAL. CONVEYS STORMWATER FROM THE PROJECT SITE TO THE EXISTING AMULE ARROYO LOCATED EAST OF THE SITE. THE STORM DRAIN HAS BEEN BUILT BY PERM. NO. 736782.
- THE EMERGENCY OVERFLOW THRU THE NEW WALL AT THE NORTH END OF GARDEN GATE LANE WAS NOT BUILT. THE STORM INLET AT THIS LOCATION HAS CAPACITY TO ACCEPT TWICE THE 100-YEAR DEVELOPED FLOWRATE; THEREFORE, THE EMERGENCY OVERFLOW IS NOT NEEDED.

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE ASPECTS OF THIS PROJECT. THOSE RELYING ON THIS RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE.

CRISTIAN J. SHULTS
7-12-05
DATE



- GENERAL NOTES**
1. CONTRACTOR MUST OBTAIN A TOPSOIL DISTURBANCE PERMIT FROM THE ENVIRONMENTAL HEALTH DIVISION PRIOR TO CONSTRUCTION.
 2. THE CONTRACTOR IS TO REFER TO EARTHWORK SPECIFICATION AS NOTED IN THE SOILS REPORT.
 3. THE CONTRACTOR SHALL CONFORM TO ALL CITY, COUNTY, STATE, AND FEDERAL DUST CONTROL MEASURES & REQUIREMENTS AND WILL BE RESPONSIBLE FOR PREPARING AND OBTAINING ALL NECESSARY APPLICATIONS AND APPROVALS.
 4. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE LOTS INTO PUBLIC RIGHT-OF-WAY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AS PER DETAIL, SHEET 3, AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
 5. ALL SPOT ELEVATIONS ARE TO FLOWLINE UNLESS OTHERWISE NOTED.
 6. BOULDERS GREATER THAN 3 FEET IN DIAMETER EXCAVATED DURING GRADING ACTIVITIES SHALL BE STOCKPILED AND DISPOSED OF AT THE DISCRETION OF THE OWNER.
 7. ALL WALLS SHOWN ARE TO BE PLACED ALONG PROPERTY LINE. WALLS ARE SHOWN OFFSET FOR VISUAL PURPOSE ONLY.



- LEGEND**
- (91.62) FUTURE SPOT ELEVATION
 - 91.62 PROPOSED SPOT ELEVATION
 - × 92.46 EXISTING SPOT ELEVATION (GRID & TC)
 - EXISTING CURB & GUTTER
 - == PROPOSED MOUNTABLE CURB & GUTTER
 - == PROPOSED STANDARD CURB & GUTTER
 - 5470 EXISTING CONTOUR W/ INDEX ELEVATION
 - FLOW ARROW
 - PROPOSED RETAINING WALL
 - PROPOSED GARDEN WALL, CONCRETE FILLED TO 1-FOOT DEPTH
 - PROPOSED SLOPE
 - PROPOSED STORM DRAIN
 - PROPOSED STORM DRAIN MANHOLE
 - PROPOSED STORM DRAIN INLET
 - PROPOSED CATTLE GUARD INLET
 - ↑ WALL DRAIN

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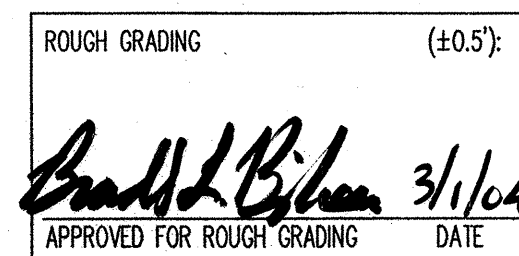
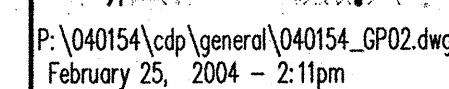
ROUGH GRADING (±0.5')
Bradley B. Bly
APPROVED FOR ROUGH GRADING DATE

REMARKS
REVISIONS
DESIGN

No. Date By

Designed By: JVS
Drawn By: DH
Checked By: RLB

DATE: 01/04
DATE: 01/04
DATE: 01/04



LEGEND

-

Map No. 9-7 Sheet 2 of 3

DRB APPROVED PLAN