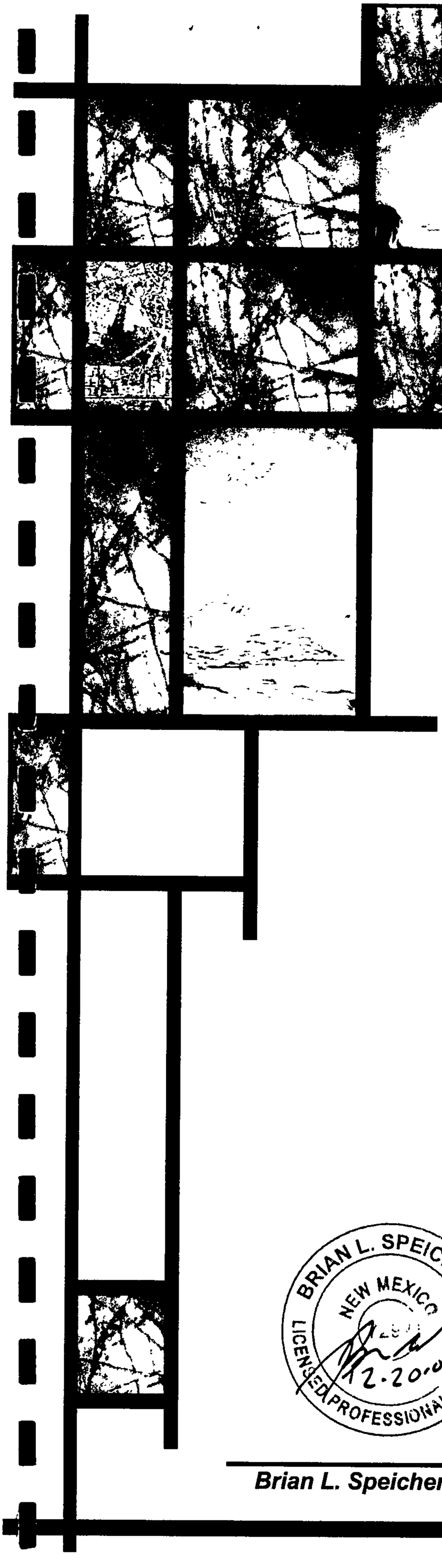


LANDS OF SALAZAR

Drainage Management Plan

October 2006
Revised November 13, 2006

Prepared for:

Salazar Family Trust

Prepared By:

Community Sciences Corporation
P.O. Box 1328 / 4481 Corrales Road
Corrales, New Mexico 87048
505/897.0000

BRIAN L. SPEICHER
NEW MEXICO
12-20-06
LICENSED PROFESSIONAL ENGINEER

Brian L. Speicher, P.E.

TABLE OF CONTENTS

SITE DESCRIPTION

The property, Lands comprised of 189.96 acres zoned RD, is to be developed on the west side of Unser Blvd. and east of Morrissey Street, near the intersection of 98th Street and Dennis Chavez Blvd. The site location is shown in figure 1 within atlas map N-9.

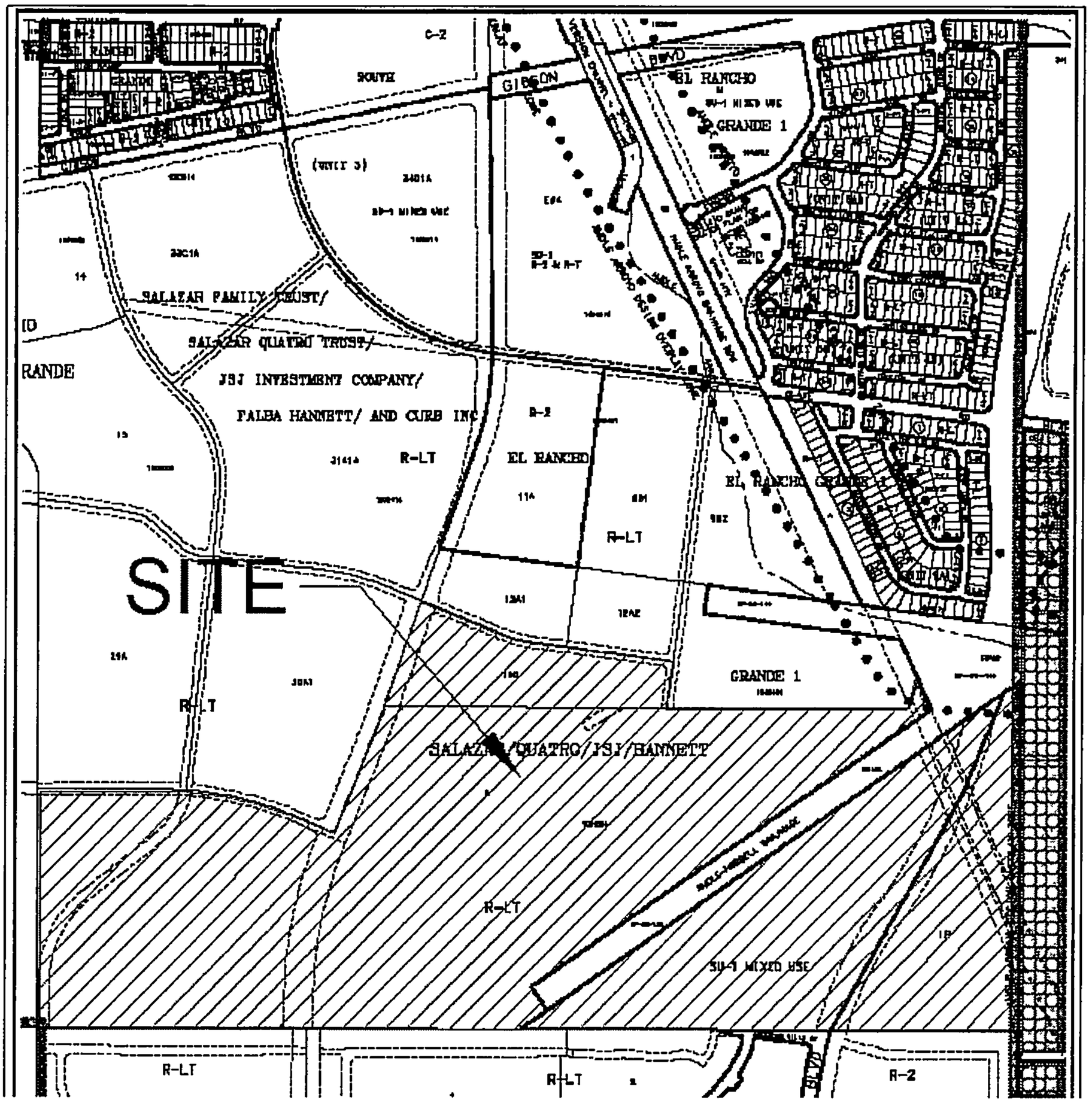


FIGURE 1 Zone Atlas Map N-9

The report represents an overall drainage management and conceptual hydraulic analysisi for approval by the City of Albuquerque in order that the subsequent platting actions may proceed. The site is not located in a designated Flood Hazard Zone per FEMA – FIRM Map 35001C0336E and C0338E, but the site has several designated transport channels that cross the site that are designated on the FIRM Map. Per the USDA Soil Conservation Services (SCS), the soils type for this site is "Type A".

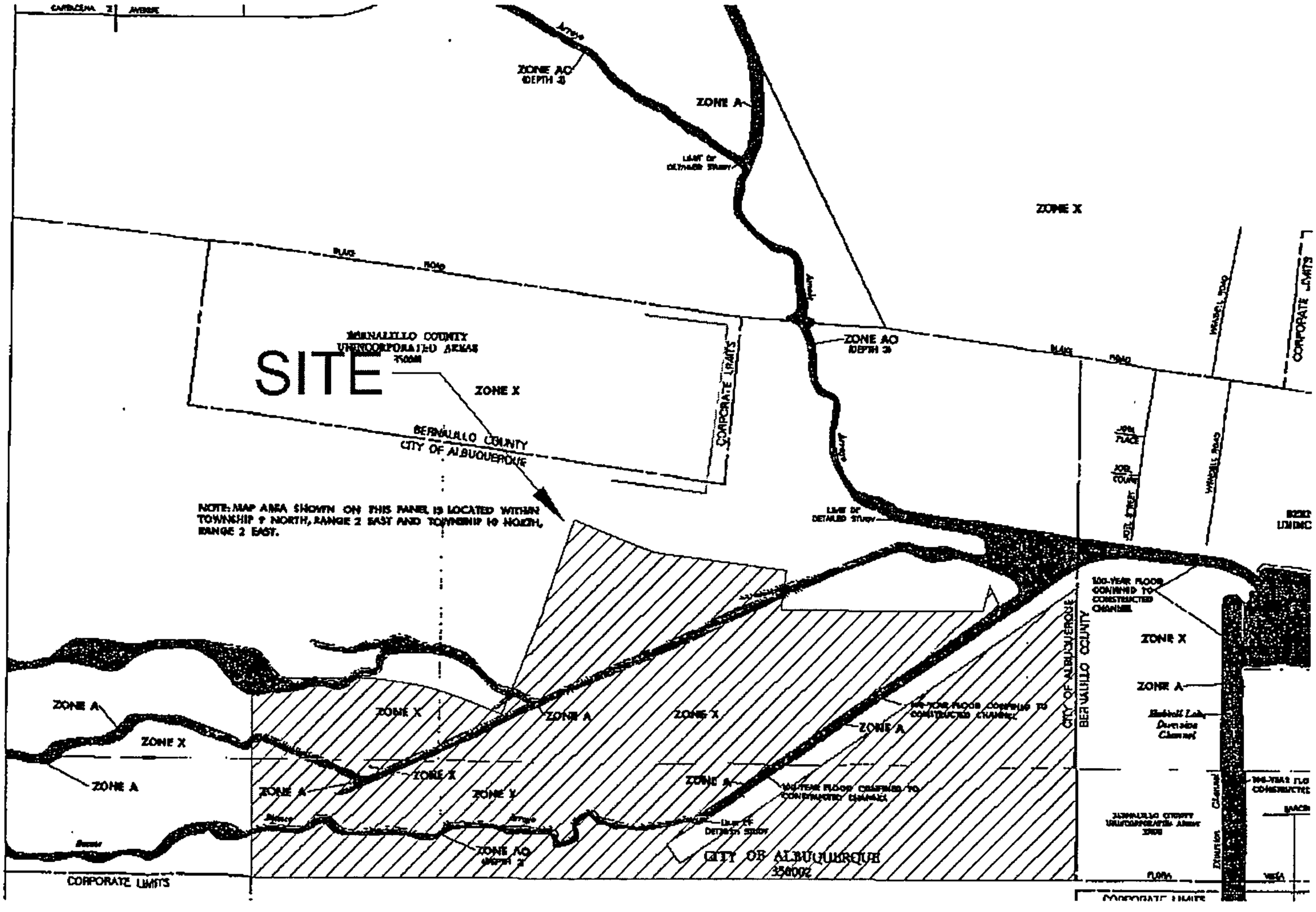


FIGURE 2 FEMA – FIRM Map 35001C0336E and C0338E

PREVIOUS STUDIES

There are no previous studies which encompass the site. However, the drainage improvements to the west of the property included in the development of Anderson Heights has intercepted all flows entering the site. The analysis of the hydrology and design of the improvements were conducted by Mark Goodwin & Assoc. A portion of the site was included in the analysis of Anderson Heights by Wilson & Company. This area is designated as Basin 3 in this study. The flows from this basin have been designated as 108.03 for discharge to a storm drain at the northeast corner of the basin.

EXISTING DRAINAGE CONDITIONS

The site is currently undeveloped. Topography consists of sparse vegetation and native grasses and chamisa. It has as cross-slope of approximately 3.0 to 4.5%. The site accepts no offsite flows. The existing drainage is to the east via surface flow to the Amole Detention Basin with a discharge into the detention basin of 366.5 cfs and a volume of 9.3 ac.ft. The existing discharge computations from AHYMO for the site are summarized in table 3.

RAINFALL

The hydrological analysis is based on the 100-year frequency, 6-hour duration storm, as represented in Section 22, Part A, Hydrology, of the Development Process Manual. Rainfall intensities per this report are as follows:

Zone	P60	P360	P1440
1	1.87	2.20	2.66

TABLE 1 Rainfall Intensitiy

HYDROLOGY

AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE
0.29681	366.45	9.298	0.58737	1.532	1.9296

TABLE 2 Existing AHYMO Summary

268.66 cfs 45m, Comput LT TP .45 included

CL
1-11-0

DEVELOPED DRAINAGE CONDITIONS

DESIGN-CRITERIA

The drainage plan presented in this report has been prepared in accordance with the City of Albuquerque Drainage Ordinances and Chapter 22 of the Development Process Manual DPM. The analysis included in this report is to support the submission of Bulk Land Plats for the Site, and to outline the overall management plan of storm runoff in future developments.

LAND TREATMEMNT

Residential DPM-Eqn. a-4, Section 22 -- $\%D = 7((N*N)+(5*N))^{1/2}$

Treatment Type /Basin	A	B	C	D
1	9%	18%	26%	47%
2	9%	18%	26%	47%
3	0%	0%	10%	90%
4	9%	17%	26%	48%
5	0%	0%	43%	57%
6	8%	16%	24%	~~51%~~ 52
7	8%	16%	24%	~~51%~~ 52
8	8%	16%	24%	~~51%~~ 52
9	0%	0%	10%	90%
10	0%	100%	0%	0%

TABLE 3 Developed Land Treatments

HYDROLOGY

BASIN	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE
1	0.00590	12.69	0.439	1.39530	1.499	3.36
2	0.05455	117.17	4.059	1.39530	1.499	3.356
3	0.00548	15.12	0.573	1.95764	1.499	4.307
4	0.03791	81.99	2.848	1.40878	1.499	3.38
5	0.02262	55.30	1.959	1.62353	1.499	3.82
6	~~0.02262~~	50.02	1.754	1.45366	1.499	3.455
7	0.03791	83.81	2.939	1.45366	1.499	3.455
8	0.03789	83.78	2.938	1.45366	1.499	3.455
9	~~0.05689~~	156.68	5.94	1.95765	1.499	4.303
10	0.00458	6.29	0.174	0.7134	~~1.499~~	2.146

TABLE 4 Proposed AHYMO Summary

1030126

41.74 cfs

1.532

Begins 6 → 10 wrong

See AHYMO or plan cc 1-11-07

HYDRAULICS

The developed runoff from the site will be transported via surface flow i.e. curb & gutter until street capacity is reached. The flows will then be collected in storm drain systems for outfall to the Amole Detention Pond. This system would begin with the collection of the runoff from basins west of 98th Street The system runs east it and will collect the flows from the subsequent drainage areas. The system is anticipated to be constructed adjacent to or within existing drainage ways.

An additional storm drain system was designed, with adjacent development, and serves a portion of the site (Basins 1, B & C).

The final discharge point for both of these systems is the Amole Pond. The capacity of this pond has not been analyzed as part of this report, but it is anticipated the analysis would show adequate capacity within the facility.

Basin 12 discharge is based on undeveloped conditions. It is anticipated that this parcel will be deeded to AMAFCA in a land exchange and included in future detention expansion. If the tract is developed instead, the development would be required to retain the flows pending design of downstream improvements.

CONCLUSION

No adverse impact will result due to the future conditions of the site. The final discharge point has adequate capacity to accommodate the runoff volume. The majority of the runoff that historically reached the facility has been diverted away from the site.

The data included in this report quantifies the runoff potential from the site and the feasibility of drainage transport. The analysis demonstrates the overall drainage impact and the ability of the site to develop. Additional analysis and design will be required to show how future developments reference and adhere to the recommendations of this report.

ARROWOOD REPORT EXCERPTS

Off-Site The land treatments for the remaining off-site basins were assumed to be 6.0 DU's per acre, based on surrounding development and land treatments considered in Reference 1, 2 & 4.

Under developed conditions, the development will discharge as it has historically, to the Amole Detention Basin. Free discharge is allowed from the proposed development with the conveyance of offsite flows. On-site flows will be collected by a storm drain system and conveyed into the Amole Detention Basin.

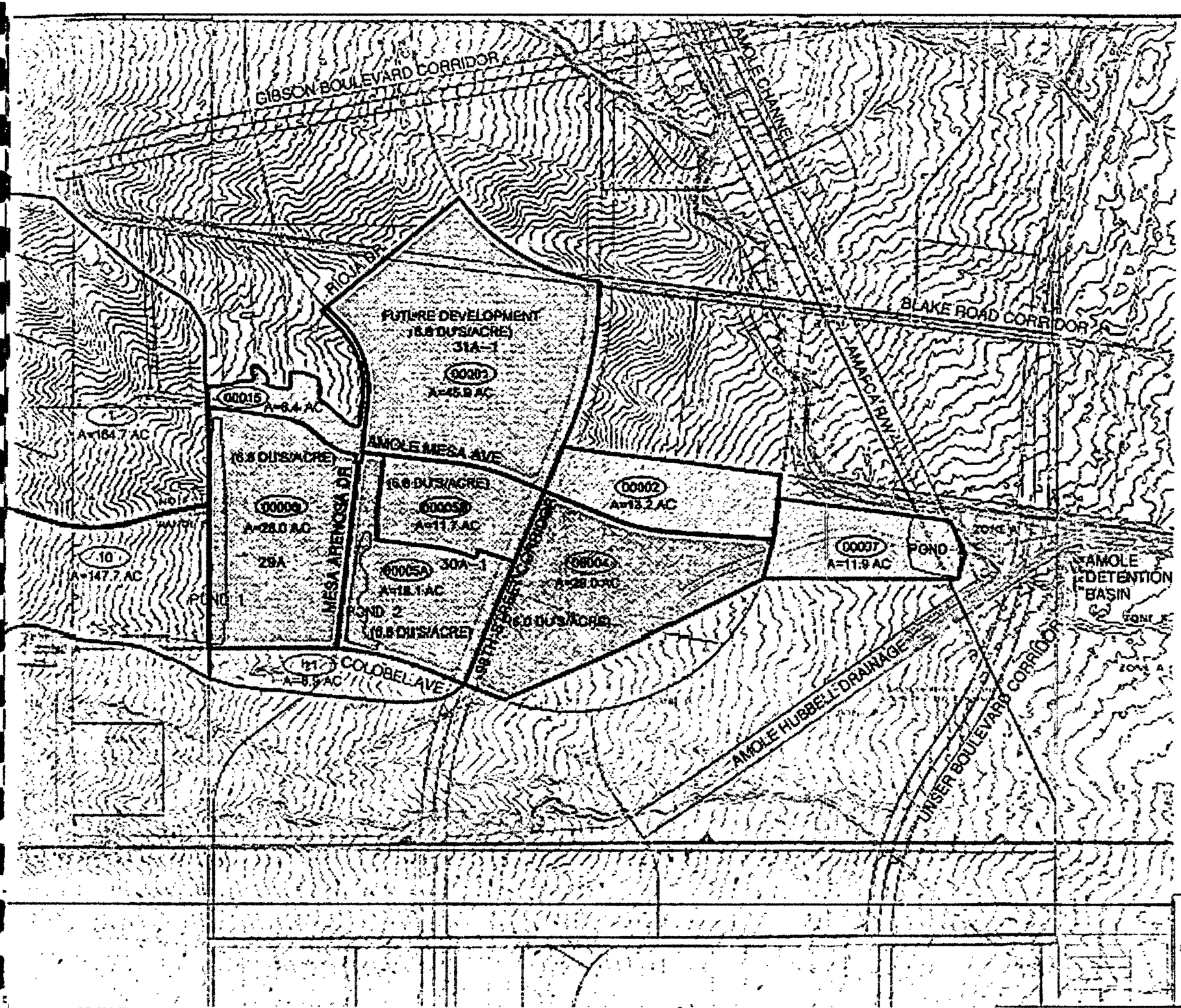
In the developed condition, off-site flows will be routed through the proposed storm drain system. The off-site area includes portions of El Rancho Grande Subdivision as obtained from reference 2. Basin areas labeled 00002 and 00003, which were obtained from reference 2, were modified for this analysis to accommodate proposed infrastructure. Peak flows were interpolated based on the basin areas and peak flows given in Exhibit 4 of the report entitled "Master Storm Drain Basin Map". Except for small bypass flows established in Reference 3, Anderson Heights Subdivision (designated as Basins 60122 and 60123 in reference 1) runoff will be retained on-site in the interim condition, while it will be routed through the South Power Line Diversion into the Rio Bravo Channel in the future condition. See Figures 3 and 4 for the existing and proposed basin boundaries.

MODELING RESULTS

Table 1 shows land treatment applied to on-site and off-site basins, along with peak discharges and the total runoff volumes resulting from the 100-year, 6-hour storm event.

Table 1. Hydrologic Summary

Sub-basin	Acres	DUs/Acre	T_p	Land Treatments A	B	C	D	Q_{peak} (cfs)	Volume (ac-ft)	Methodology/Notes
ONSITE										
00001	45.9	6.8	0.13	0	0	37	63	174.87	6.132	DPM for D
00005A	18.1	6.8	0.13	0	0	37	63	69.00	2.418	DPM for D
00005B	11.7	6.8	0.13	0	0	37	63	44.61	1.563	DPM for D
00006	28.0	6.8	0.13	0	0	37	63	106.70	3.740	
OFFSITE										
15	6.4	---	0.13	0	0	20	80	24.00	0.848	DERIVED FROM REFERENCE 2
00002	13.2	---	0.13	0	19	26	55	46.42	1.599	DERIVED FROM REFERENCE 4
00004	29.0	6.0	0.13	0	0	43	57	108.03	3.738	DPM for D
00007	11.9	---	0.13	80	0	20	0	19.06	0.529	School Site
1	164.69	---	0.1992	95	0	5	0	175.77	6.299	DERIVED FROM REFERENCE 3
10	147.68	---	0.2812	95	0	5	0	112.96	5.648	DERIVED FROM REFERENCE 3
11	8.92	---	0.13	95	0	5	0	12.14	0.341	DERIVED FROM REFERENCE 3



LEGEND

- REFERENCE: ONSITE, CREATED IN THIS REPORT
- REFERENCE: OFFSITE, DRAINAGE STUDY FOR EL RANCHO GRANDE UNITS 14 AND 15, DATED OCTOBER 15, 2003 REVISED NOVEMBER 20, 2003*
- REFERENCE: OFFSITE, CREATED IN THIS REPORT
- 00004 — BASIN I.D. LABEL
- PROPOSED BASIN BOUNDARY
- PROPOSED INTERIM DETENTION POND
- * BASIN BOUNDARY REVISED IN THIS REPORT TO ACCOMODATE PROPOSED INFRASTRUCTURE

BASIN TABLE

BASIN	AREA	Q 100 (CFS)	V 100D AC-FT
00004	28.0 AC	45.46	7.3
00005A	15.1 AC	36.8	2.1
00005B	11.7 AC	44.7	1.8
00006	38.5 AC	93.8	3.5
00007	11.9 AC	16.1	0.23
00015	6.4 AC	21.0	0.85
1	164.7 AC	175.8	6.3
10	147.7 AC	113.0	5.6
11	8.9 AC	12.1	0.34

*NOTE: BASINS 0001 AND 0002 WERE NOT MODELED IN INTERIM CONDITIONS. UNDER EXISTING CONDITIONS, DRAINAGE FROM THESE BASINS DOES NOT ENTER THE PROJECT SITE.

N

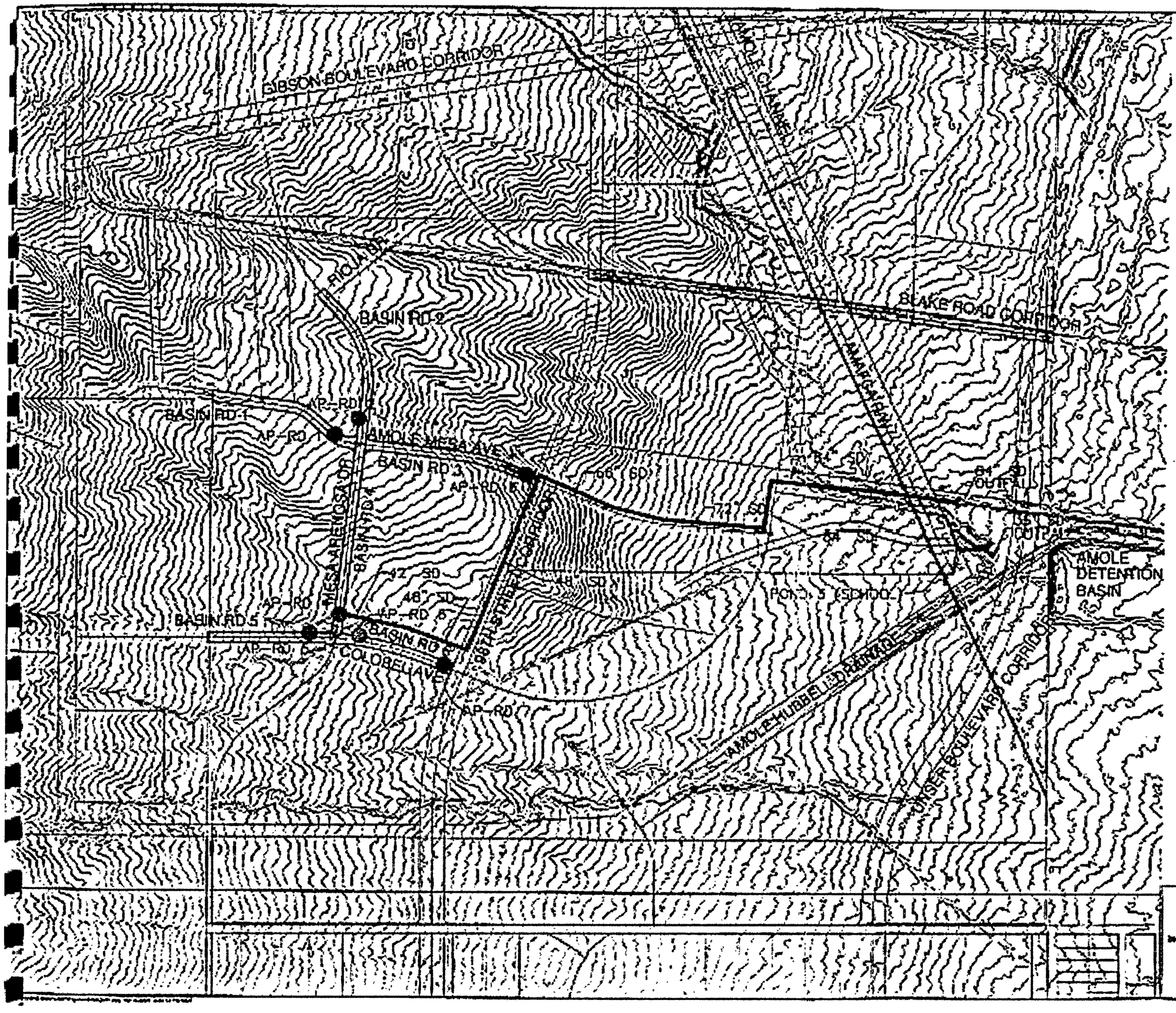
600 0 300 600 1200

GRAPHIC SCALE (IN FEET) 1" = 600 ft.

WILSON & COMPANY

TRACTS 29, 30, 31
DRAINAGE MANAGEMENT PLAN
FIGURE 3A

PROPOSED INTERIM CONDITIONS



LEGEND

- PROJECT SITE
- FEMA MAP PANEL 35001C0336D
- PROPOSED STORM DRAIN LINE
- PROPOSED STREET BASINS

OFFSITE STREET CAPACITY TABLE

AP	SLOPE (%)	Q_{100} (cfs)	D_{100} (ft)
AP-RD 1	2.17	3.2	0.29
AP-RD 2	3.60	2.8	0.34
AP-RD 3	3.45	25.0	0.41
AP-RD 4	0.89	6.8	0.34
AP-RD 5	2.70	2.2	0.25
AP-RD 6	2.70	9.0	0.38
AP-RD 7	3.18	11.4	0.40

WILSON & COMPANY

TRACTS 29, 30, 31
DRAINAGE MANAGEMENT PLAN
FIGURE 5

OFFSITE STREET CAPACITY

AHYMO CALCULATION

EXISTING DRAINAGE CONDITIONS

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COMMAND          IDENTIFICATION   NO.   NO.     (SQ MI)          (CFS)         (AC-FT)     (INCHES)     (HOURS)  ACRE     NOTATION

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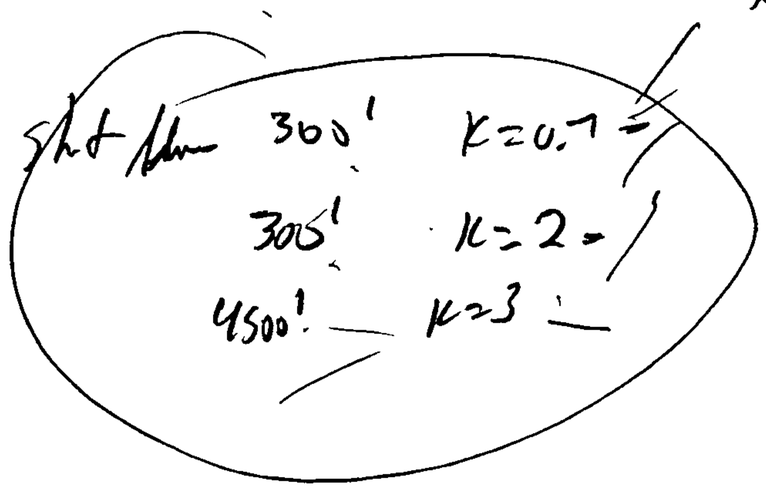

365 cfs

267.8 cfs

+ 29681 mi²/640 ac/mi² = 189.95 ac

A_{10} = 132.96 ac × 1.29 cfs/ac = 171.52

B_{10} = 18.99 ac × 2.03 cfs/ac = 38.55

C_{10} = 37.9 ac × 2.87 cfs/ac = 109.0

319 cfs

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   *                  *100 YEAR EXISTING CONDITIONS
   *                  **********************************
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                      RAIN DAY=2.80 IN DT=0.0333 HRS

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                    .0631   .0672   .0715   .0760   .0806   .0855   .0906
                    .0960   .1016   .1076   .1128   .1184   .1244   .1368
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                   1.8900  1.9213  1.9511  1.9794  2.0062  2.0139  2.0202
                   2.0261  2.0318  2.0373  2.0425  2.0476  2.0524  2.0571
                   2.0617 2.0661 2.0704 2.0745 2.0786 2.0825 2.0864
                   2.0901  2.0938  2.0974  2.1009  2.1043  2.1077  2.1110
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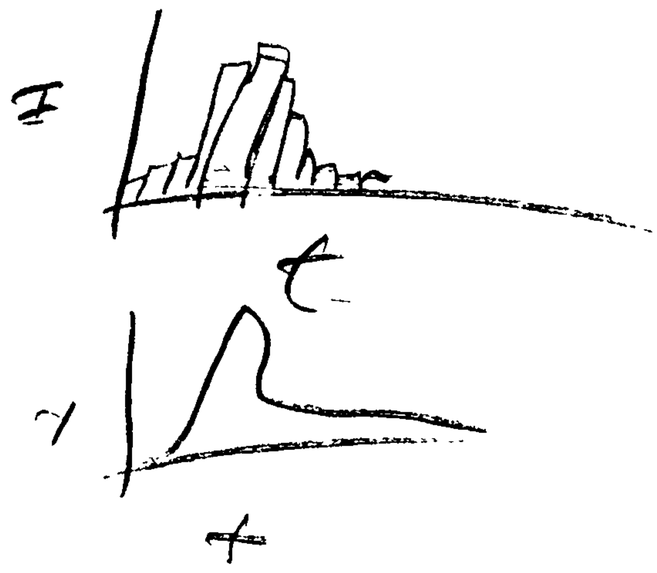

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*                    ***********
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                                   HYDROGRAPH FROM AREA EXISTING


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FINISH
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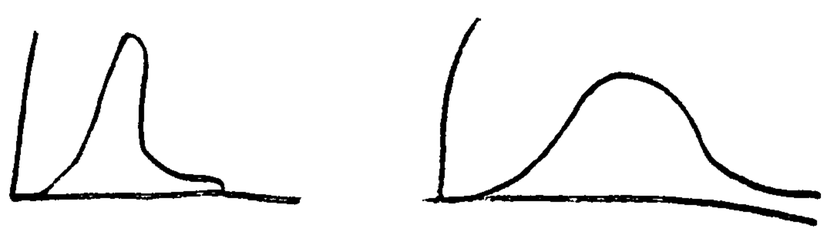

PROPOSED DRAINAGE CONDITIONS

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COMPUTE NM HYD	DA2	-	2	.05455	117.17	4.059	1.39530	1.499	3.356	PER IMP= 47.00
COMPUTE NM HYD	DA3	-	3	.00548	15.12	.573	1.95764	1.499	4.307	PER IMP= 90.00
COMPUTE NM HYD	DA4	-	4	.03791	81.99	2.848	1.40878	1.499	3.380	PER IMP= 48.00
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ADD HYD	AP2	20& 4	21	.09794	214.27	7.480	1.43200	1.499	3.419	
ADD HYD	DA5DA6	5& 6	22	.03879	91.17	3.218	1.55522	1.499	3.672	
ADD HYD	AP3	22&21	23	.13673	305.44	10.697	1.46696	1.499	3.490	
ADD HYD	AP4	23& 7	24	.15951	355.95	12.471	1.46592	1.499	3.487	
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FINISH										

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*                    *100 YEAR ULTIMATE CONDITIONS
*                    **********************************
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                     .0631   .0672   .0715   .0760   .0806   .0855   .0906
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                    2.1142  2.1174  2.1205  2.1235  2.1265  2.1295  2.1324
                    2.1353  2.1381  2.1408  2.1436  2.1463  2.1489  2.1515
                    2.1541  2.1566  2.1592  2.1616  2.1641  2.1665  2.1689
                    2.1712  2.1736  2.1759  2.1781  2.1804  2.1826  2.1848
                    2.1870  2.1891  2.1913  2.1934  2.1955  2.1975  2.1996
                    2.2016  2.2036  2.2056  2.2076  2.2095  2.2114  2.2134
                    2.2153  2.2171  2.2190  2.2209  2.2227  2.2245  2.2263
                    2.2281  2.2299  2.2316  2.2334  2.2351  2.2369  2.2386
                    2.2403  2.2419  2.2436  2.2453  2.2469  2.2486  2.2502
                    2.2518  2.2534  2.2550  2.2566  2.2581  2.2597  2.2613
                    2.2628  2.2643  2.2658  2.2674  2.2689  2.2703  2.2718
                    2.2733  2.2748  2.2762  2.2777  2.2791  2.2805  2.2820
                    2.2834  2.2848  2.2862  2.2876  2.2890  2.2903  2.2917
                    2.2931  2.2944  2.2958  2.2971  2.2984  2.2998

*                    **********************************
COMPUTE NM HYD       ID=1 HYD NO=DA1 DA=0.005903 SQ MI
                     PER A=9 PER B=18 PER C=26 PER D=47
```

```
                    TP=-0.1333 HR MASS RAIN=-1

     K =   .072649HR    TP =   .133300HR    K/TP RATIO =   .545000     SHAPE CONSTANT, N =  7.106420
     UNIT PEAK =   10.954    CFS   UNIT VOLUME =    .9984       B =   526.28      P60 = 1.9000
     AREA =      .002774 SQ MI    IA =    .10000 INCHES    INF =    .04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300


     K =   .124177HR    TP =   .133300HR    K/TP RATIO =   .931559     SHAPE CONSTANT, N =  3.797308
     UNIT PEAK =   8.0133    CFS   UNIT VOLUME =    .9986       B =   341.42      P60 = 1.9000
     AREA =      .003129 SQ MI    IA =    .45189 INCHES    INF =   1.11528 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300

*                   ***********
PRINT HYD           ID=1 CODE=1

                                    HYDROGRAPH FROM AREA DA1


    RUNOFF VOLUME =     1.39530 INCHES     =        .4393 ACRE-FEET
    PEAK DISCHARGE RATE =      12.69 CFS  AT   1.499 HOURS   BASIN AREA =   .0059 SQ. MI.


*                   **********************************
COMPUTE NM HYD      ID=2 HYD NO=DA2 DA=0.054545 SQ MI
                    PER A=9 PER B=18 PER C=26 PER D=47
                    TP=-0.1333 HR MASS RAIN=-1

     K =   .072649HR    TP =   .133300HR    K/TP RATIO =   .545000     SHAPE CONSTANT, N =  7.106420
     UNIT PEAK =   101.21    CFS   UNIT VOLUME =    .9992       B =   526.28      P60 = 1.9000
     AREA =      .025636 SQ MI    IA =    .10000 INCHES    INF =    .04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300


     K =   .124177HR    TP =   .133300HR    K/TP RATIO =   .931559     SHAPE CONSTANT, N =  3.797308
     UNIT PEAK =   74.044    CFS   UNIT VOLUME =    1.000       B =   341.42      P60 = 1.9000
     AREA =      .028909 SQ MI    IA =    .45189 INCHES    INF =   1.11528 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300

*                   ***********
PRINT HYD           ID=2 CODE=1

                                    HYDROGRAPH FROM AREA DA2


    RUNOFF VOLUME =     1.39530 INCHES     =       4.0590 ACRE-FEET
    PEAK DISCHARGE RATE =     117.17 CFS  AT   1.499 HOURS   BASIN AREA =   .0545 SQ. MI.
```

```
*                       **********************************
COMPUTE NM HYD          ID=3 HYD NO=DA3 DDA=0.005484 SQ MI
                        PER A=0 PER B=0 PER C=10 PER D=90
                        TP=-0.1333 HR MASS RAIN=-1

     K =   .072649HR    TP =   .133300HR    K/TP RATIO =   .545000     SHAPE CONSTANT, N =  7.106420
     UNIT PEAK =   19.486    CFS   UNIT VOLUME =    .9987       B =   526.28      P60 = 1.9000
     AREA =      .004936 SQ MI    IA =    .10000 INCHES    INF =    .04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300


     K =   .106205HR    TP =   .133300HR    K/TP RATIO =   .796738     SHAPE CONSTANT, N =  4.498737
     UNIT PEAK =   1.5927    CFS   UNIT VOLUME =    .9925       B =   387.15      P60 = 1.9000
     AREA =      .000548 SQ MI    IA =    .35000 INCHES    INF =    .83000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300

*                       ***********
PRINT HYD               ID=3 CODE=1

                                HYDROGRAPH FROM AREA DA3

     RUNOFF VOLUME =     1.95764 INCHES     =       .5726 ACRE-FEET
     PEAK DISCHARGE RATE =      15.12 CFS  AT   1.499 HOURS   BASIN AREA =    .0055 SQ. MI.


*                       ******************************
COMPUTE NM HYD          ID=4 HYD NO=DA4 DA=0.037906 SQ MI
                        PER A=9 PER B=17 PER C=26 PER D=48
                        TP=-0.1333 HR MASS RAIN=-1

     K =   .072649HR    TP =   .133300HR    K/TP RATIO =   .545000     SHAPE CONSTANT, N =  7.106420
     UNIT PEAK =   71.834    CFS   UNIT VOLUME =    .9992       B =   526.28      P60 = 1.9000
     AREA =      .018195 SQ MI    IA =    .10000 INCHES    INF =    .04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300


     K =   .124050HR    TP =   .133300HR    K/TP RATIO =   .930610     SHAPE CONSTANT, N =  3.801356
     UNIT PEAK =   50.527    CFS   UNIT VOLUME =    1.000       B =   341.70      P60 = 1.9000
     AREA =      .019711 SQ MI    IA =    .45096 INCHES    INF =   1.11269 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300

*                       ***********
PRINT HYD               ID=4 CODE=1
```

HYDROGRAPH FROM AREA DA4

RUNOFF VOLUME = 1.40878 INCHES = 2.8480 ACRE-FEET
PEAK DISCHARGE RATE = 81.99 CFS AT 1.499 HOURS BASIN AREA = .0379 SQ. MI.

```
*                    ******************************
COMPUTE NM HYD       ID=5 HYD NO=DA5 DA=0.022621 SQ MI
                     PER A=0 PER B=0 PER C=43 PER D=57
                     TP=-0.1333 HR MASS RAIN=-1
```

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420
UNIT PEAK = 50.906 CFS UNIT VOLUME = .9991 B = 526.28 P60 = 1.9000
AREA = .012894 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .106205HR TP = .133300HR K/TP RATIO = .796738 SHAPE CONSTANT, N = 4.498737
UNIT PEAK = 28.251 CFS UNIT VOLUME = 1.000 B = 387.15 P60 = 1.9000
AREA = .009727 SQ MI IA = .35000 INCHES INF = .83000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

```
*                    ***********
PRINT HYD            ID=5 CODE=1
```

HYDROGRAPH FROM AREA DA5

RUNOFF VOLUME = 1.62353 INCHES = 1.9587 ACRE-FEET
PEAK DISCHARGE RATE = 55.30 CFS AT 1.499 HOURS BASIN AREA = .0226 SQ. MI.

```
*                    ******************************
COMPUTE NM HYD       ID=6 HYD NO=DA6 DA=0.016173 SQ MI
                     PER A=8 PER B=16 PER C=24 PER D=52
                     TP=-0.1333 HR MASS RAIN=-1
```

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420
UNIT PEAK = 33.203 CFS UNIT VOLUME = .9990 B = 526.28 P60 = 1.9000
AREA = .008410 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .123844HR TP = .133300HR K/TP RATIO = .929063 SHAPE CONSTANT, N = 3.807983

```
     UNIT PEAK =   19.926     CFS   UNIT VOLUME =     .9997       B =   342.16      P60 = 1.9000
     AREA =       .007763 SQ MI     IA =     .45000 INCHES     INF =   1.11000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =    .033300

*                    ***********
PRINT HYD            ID=6 CODE=1

                                        HYDROGRAPH FROM AREA DA6


    RUNOFF VOLUME =      1.45971 INCHES      =       1.2591 ACRE-FEET
    PEAK DISCHARGE RATE =       35.87 CFS  AT   1.499 HOURS   BASIN AREA =    .0162 SQ. MI.


*                    ******************************
COMPUTE NM HYD       ID=7 HYD NO=DA7 DA=0.022779 SQ MI
                     PPER A=8 PER B=16 PER C=24 PER D=52
                     TP=-0.1333 HR MASS RAIN=-1

     K =    .072649HR     TP =    .133300HR     K/TP RATIO =    .545000     SHAPE CONSTANT, N =  7.106420
     UNIT PEAK =   46.765     CFS   UNIT VOLUME =     .9991       B =   526.28      P60 = 1.9000
     AREA =       .011845 SQ MI     IA =     .10000 INCHES     INF =    .04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =    .033300

     K =    .123844HR     TP =    .133300HR     K/TP RATIO =    .929063     SHAPE CONSTANT, N =  3.807983
     UNIT PEAK =   28.065     CFS   UNIT VOLUME =     .9999       B =   342.16      P60 = 1.9000
     AREA =       .010934 SQ MI     IA =     .45000 INCHES     INF =   1.11000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =    .033300

*                    ***********
PRINT HYD            ID=7 CODE=1

                                        HYDROGRAPH FROM AREA DA7


    RUNOFF VOLUME =      1.45971 INCHES      =       1.7734 ACRE-FEET
    PEAK DISCHARGE RATE =       50.51 CFS  AT   1.499 HOURS   BASIN AREA =    .0228 SQ. MI.


*                    ******************************
COMPUTE NM HYD       ID=8 HYD NO=DA8 DA=0.060377 SQ MI
                     PER A=8 PER B=16 PER C=24 PER D=52
                     TP=-0.1333 HR MASS RAIN=-1

     K =    .072649HR     TP =    .133300HR     K/TP RATIO =    .545000     SHAPE CONSTANT, N =  7.106420
```

```
     UNIT PEAK =   123.95    CFS   UNIT VOLUME =    .9993       B =   526.28      P60 = 1.9000
     AREA =      .031396 SQ MI    IA =    .10000 INCHES    INF =    .04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300


     K =   .123844HR    TP =   .133300HR    K/TP RATIO =   .929063     SHAPE CONSTANT, N =  3.807983
     UNIT PEAK =   74.389    CFS   UNIT VOLUME =    1.000       B =   342.16      P60 = 1.9000
     AREA =      .028981 SQ MI    IA =    .45000 INCHES    INF =   1.11000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300

*                    ***********
PRINT HYD            ID=8 CODE=1

                                    HYDROGRAPH FROM AREA DA8


    RUNOFF VOLUME =     1.45971 INCHES     =       4.7004 ACRE-FEET
    PEAK DISCHARGE RATE =     133.87 CFS  AT   1.499 HOURS   BASIN AREA =   .0604 SQ. MI.


*                    ******************************
COMPUTE NM HYD       ID=9 HYD NO=DA9 DA=0.010633 SQ MI
                     PER A=0 PER B=0 PER C=10 PER D=90
                     TP=-0.1333 HR MASS RAIN=-1

     K =   .072649HR    TP =   .133300HR    K/TP RATIO =   .545000     SHAPE CONSTANT, N =  7.106420
     UNIT PEAK =   37.782    CFS   UNIT VOLUME =    .9991       B =   526.28      P60 = 1.9000
     AREA =      .009570 SQ MI    IA =    .10000 INCHES    INF =    .04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300


     K =   .106205HR    TP =   .133300HR    K/TP RATIO =   .796738     SHAPE CONSTANT, N =  4.498737
     UNIT PEAK =   3.0882    CFS   UNIT VOLUME =    .9962       B =   387.15      P60 = 1.9000
     AREA =      .001063 SQ MI    IA =    .35000 INCHES    INF =    .83000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300

*                    ***********
PRINT HYD            ID=9 CODE=1

                                    HYDROGRAPH FROM AREA DA9


    RUNOFF VOLUME =     1.95764 INCHES     =       1.1102 ACRE-FEET
    PEAK DISCHARGE RATE =      29.30 CFS  AT   1.499 HOURS   BASIN AREA =   .0106 SQ. MI.
```

```
*                       ******************************
COMPUTE NM HYD          ID=10 HYD NO=DA10 DA=0.030426 SQ MI
                        PER A=0 PER B=100 PER C=0 PER D=0
                        TP=-0.1333 HR MASS RAIN=-1

     K =   .130761HR    TP =   .133300HR    K/TP RATIO =   .980950     SHAPE CONSTANT, N =  3.599930
     UNIT PEAK =   74.764    CFS   UNIT VOLUME =    1.000       B =   327.55      P60 = 1.9000
     AREA =      .030426 SQ MI    IA =    .50000 INCHES    INF =   1.25000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =   .033300

*                       ***********
PRINT HYD               ID=10 CODE=1

                                        HYDROGRAPH FROM AREA DA10


    RUNOFF VOLUME =      .71340 INCHES     =       1.1576 ACRE-FEET
    PEAK DISCHARGE RATE =    41.74 CFS  AT   1.532 HOURS   BASIN AREA =   .0304 SQ. MI.


*
ADD HYD                 ID=20   HYD NO=AP1   IDS=2 AND 3
*
PRINT HYD                   ID=20 CODE=1

                                        HYDROGRAPH FROM AREA AP1


    RUNOFF VOLUME =     1.44666 INCHES     =       4.6315 ACRE-FEET
    PEAK DISCHARGE RATE =     132.28 CFS  AT   1.499 HOURS   BASIN AREA =   .0600 SQ. MI.


*
ADD HYD                 ID=21   HYD NO=AP2   IDS=20 AND 4
*
PRINT HYD                   ID=21 CODE=1

                                        HYDROGRAPH FROM AREA AP2


    RUNOFF VOLUME =     1.43200 INCHES     =       7.4796 ACRE-FEET
    PEAK DISCHARGE RATE =     214.27 CFS  AT   1.499 HOURS   BASIN AREA =   .0979 SQ. MI.


*
```

```
ADD HYD              ID=22   HYD NO=DA5DA6   IDS=5 AND 6
*
PRINT HYD                ID=22 CODE=1

                                        HYDROGRAPH FROM AREA DA5DA6


    RUNOFF VOLUME =     1.55522 INCHES     =       3.2177 ACRE-FEET
    PEAK DISCHARGE RATE =      91.17 CFS  AT   1.499 HOURS   BASIN AREA =   .0388 SQ. MI.


*
ADD HYD              ID=23   HYD NO=AP3   IDS=22 AND 21
*
PRINT HYD                ID=23 CODE=1

                                        HYDROGRAPH FROM AREA AP3


    RUNOFF VOLUME =     1.46696 INCHES     =      10.6973 ACRE-FEET
    PEAK DISCHARGE RATE =     305.44 CFS  AT   1.499 HOURS   BASIN AREA =   .1367 SQ. MI.


*
ADD HYD              ID=24   HYD NO=AP4   IDS=23 AND 7
*
PRINT HYD                ID=24 CODE=1

                                        HYDROGRAPH FROM AREA AP4


    RUNOFF VOLUME =     1.46592 INCHES     =      12.4707 ACRE-FEET
    PEAK DISCHARGE RATE =     355.95 CFS  AT   1.499 HOURS   BASIN AREA =   .1595 SQ. MI.


*
ADD HYD              ID=25   HYD NO=AP5   IDS=8 AND 9
*
PRINT HYD                ID=25 CODE=1

                                        HYDROGRAPH FROM AREA AP5


    RUNOFF VOLUME =     1.53426 INCHES     =       5.8105 ACRE-FEET
    PEAK DISCHARGE RATE =     163.16 CFS  AT   1.499 HOURS   BASIN AREA =   .0710 SQ. MI.
```

```
    *
    FINISH

        NORMAL PROGRAM FINISH          END TIME (HR:MIN:SEC) = 11:22:56
□(s0p10h4099T□&l6D
```

DRAINAGE PLANS

```
********
* FILE: EXPRESSSIONS OF LIFE.TXT
**************************************
START                  TIME = 0.0    PUNCH CODE = 0
*S THE IA/INF METHOD
**********************
RAINFALL               TYPE=1    QUARTER=0.0    HOUR=1.90 IN        SIX=2.30 IN
                       DAY=2.80        DT=0.033333

COMPUTE LT TP          LCODE=1     NK=2     ISLOPE=0
                       LENGTH=300     FT    SLOPE=.03          K=.7
                       LENGTH=5000      FT  SLOPE=.03          K=2

COMPUTE NM HYD         ID=1  HYD NO=existing  DA=0.296810
                       PER A=70     PER B=10     PER C=20     PER D=0
                       TP=0.0    MASSRAIN=-1
PRINT HYD              ID=1  CODE=10

PLOT HYD               ID=1

FINISH
```

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09 2015)

Project Title: Tracts A-1-A & A-1-B Lands of Salazar Family Trust, Et AL Building Permit #: _____ City Drainage #: N09D011

DRB#: _____ EPC#: _____ Work Order#: _____

Legal Description: Tracts A-1-A & A-1-B Lands of Salazar Family Trust, Et AL

City Address: 98th St at Colobel Ave

Engineering Firm: Mark Goodwin & Associates Contact: Doug hughes

Address: PO Box 90606 Albuquerque NM 87199

Phone#: 505-828-2200 Fax#: 505-897-7539 E-mail: doug@goodwinengineers.com

Owner: Salazar Family Trust Contact: Keith Meyer

Address: 700 Carlisle Blvd SE, Albuquerque Nm 87106

Phone#: (505) 878-0001 Fax#: _____ E-mail: keithmeyer@gotspaceusa.com

Architect: n/a Contact: _____

Address: _____

Phone#: _____ Fax#: _____ E-mail: _____

Other Contact: _____ Contact: _____

Address: _____

Phone#: _____ Fax#: _____ E-mail: _____

Check all that Apply:

In Rudy's office

DEPARTMENT:

x HYDROLOGY/ DRAINAGE

_____ TRAFFIC/ TRANSPORTATION

_____ MS4/ EROSION & SEDIMENT CONTROL

TYPE OF SUBMITTAL:

_____ ENGINEER/ ARCHITECT CERTIFICATION

_____ CONCEPTUAL G & D PLAN

_____ GRADING PLAN

_____ DRAINAGE MASTER PLAN

_____ DRAINAGE REPORT

x CLOMR/LOMR

_____ TRAFFIC CIRCULATION LAYOUT (TCL)

_____ TRAFFIC IMPACT STUDY (TIS)

_____ EROSION & SEDIMENT CONTROL PLAN (ESC)

_____ OTHER (SPECIFY) _____

CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

_____ BUILDING PERMIT APPROVAL

_____ CERTIFICATE OF OCCUPANCY

_____ PRELIMINARY PLAT APPROVAL

_____ SITE PLAN FOR SUB'D APPROVAL

_____ SITE PLAN FOR BLDG. PERMIT APPROVAL

_____ FINAL PLAT APPROVAL

_____ SIA/ RELEASE OF FINANCIAL GUARANTEE

_____ FOUNDATION PERMIT APPROVAL

_____ GRADING PERMIT APPROVAL

_____ SO-19 APPROVAL

_____ PAVING PERMIT APPROVAL

_____ GRADING/ PAD CERTIFICATION

_____ WORK ORDER APPROVAL

x CLOMR/LOMR

_____ PRE-DESIGN MEETING

_____ OTHER (SPECIFY) _____

IS THIS A RESUBMITTAL?: _____ Yes x No

DATE SUBMITTED: 5/03/2016 By: Doug Hughes

Rec'd 5/4/2016

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: ___

Abiel X. Carrillo

From: Abiel X. Carrillo
Sent: Tuesday, May 24, 2016 9:05 AM
To: 'Doug Hughes'
Subject: LOMR - Lands of Salazar N09D011

Doug,

Like I mentioned over the phone, these are our only comments, so I will mark the submittal as approved. If you'd like, next time you are at the City you can just drop off a revised Annotated FIRM and write up for item 1 instead of making a new submittal.

1. I recommend (not require) that you speak to the fact that the Detention Pond 17 is completely detached from the property.
2. Include the removal of the 98th Street ROW from the Floodplain.

Any question just let me know.

Abiel Carrillo, PE, CFM
Principal Engineer - Hydrology
Planning Department
Development Review Services Division
City of Albuquerque
505-924-3986
acarrillo@cabq.gov
600 2nd Street NW
Albuquerque, NM 87102

Hardcopy in Rudy Rael's office

Highlands at Anderson Hills Unit 7 W6 708163

Anderson Hughes Unit 2 753984

built SD Ent av mem of Inc

pour print 2003

Take photos of slds

N, S, E, W

CITY OF ALBUQUERQUE

January 11, 2007

Brian L. Speicher, P.E.
Community Sciences Corporation
P.O. Box 1328
Corrales, NM 87048

Re: Lands of Salazar Drainage Management Plan
Engineer's Stamp dated 12-20-06 (N9/D11)

Dear Mr. Speicher,

Based upon the information provided in your submittal dated 12-26-06, the above referenced plan is approved for Conceptual Drainage Management Plan.

If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne, E.I.
Engineering Associate, Planning Dept.
Development and Building Services

C: file

P.O. Box 1293
Albuquerque
New Mexico 87103
www.cabq.gov

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/28/2003rd)

PROJECT TITLE: Lands of Salazar
DRB #: 1005224 EPC#:_____

ZONE MAP/DRG. FILE #: N-9 / D11
WORK ORDER#:_____

LEGAL DESCRIPTION: _____
CITY ADDRESS:_____

ENGINEERING FIRM: Community Sciences Corporation
ADDRESS: PO Box 1328
CITY, STATE: Corrales, NM

CONTACT: Brian L. Speicher
PHONE: 923-9552
ZIP CODE: 87048

OWNER: Salazar Family Trust ET AL
ADDRESS: 1016 Summit NE
CITY, STATE: Albuquerque, NM

CONTACT: Tom Salazar
PHONE: 266-3025
ZIP CODE: 87106

ARCHITECT: _____
ADDRESS: _____
CITY, STATE: _____

CONTACT: _____
PHONE:
ZIP CODE: _____

SURVEYOR: Community Sciences Corporation
ADDRESS: PO Box 1328
CITY, STATE: Corrales, NM

CONTACT: Tom Patrick
PHONE: 923-9558
ZIP CODE: 87048

CONTRACTOR: _____
ADDRESS: _____
CITY, STATE: _____

CONTACT: _____
PHONE: _____
ZIP CODE: _____

CHECK TYPE OF SUBMITTAL:

- ☐ DRAINAGE REPORT
- ☐ DRAINAGE PLAN 1st SUBMITTAL, ***REQUIRES TCL or equal***
- ☐ DRAINAGE PLAN RESUBMITTAL
- ☒ CONCEPTUAL GRADING & DRAINAGE PLAN
- ☐ GRADING PLAN
- ☐ EROSION CONTROL PLAN
- ☐ ENGINEER'S CERTIFICATION (HYDROLOGY)
- ☐ CLOMR/LOMR
- ☐ TRAFFIC CIRCULATION LAYOUT (TCL)
- ☐ ENGINEERS CERTIFICATION (TCL)
- ☐ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
- ☐ OTHER

CHECK TYPE OF APPROVAL SOUGHT:

- ☐ SIA / FINANCIAL GUARANTEE RELEASE
- ☐ PRELIMINARY PLAT APPROVAL
- ☐ S. DEV. PLAN FOR SUB'D. APPROVAL
- ☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
- ☐ SECTOR PLAN APPROVAL
- ☐ FINAL PLAT APPROVAL
- ☐ FOUNDATION PERMIT APPROVAL
- ☐ BUILDING PERMIT APPROVAL
- ☐ CERTIFICATE OF OCCUPANCY (PERM.)
- ☐ CERTIFICATE OF OCCUPANCY (TEMP.)
- ☐ GRADING PERMIT APPROVAL
- ☐ PAVING PERMIT APPROVAL
- ☐ WORK ORDER APPROVAL
- ☒ OTHER (SPECIFY)

Concurrence of Conceptual Drainage Master Plan

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☒ YES
- ☐ NO
- ☐ COPY PROVIDED

RECEIVED
DEC 26 2006
HYDROLOGY SECTION

DATE SUBMITTED:12/26/06

BY:Brian L. Speicher

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature , location and scope ot the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plan**: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
2. **Drainage Plans**: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
3. **Drainage Report**: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

CITY OF ALBUQUERQUE

November 30, 2006

Brian L. Speicher, P.E.
Community Sciences Corporation
P.O. Box 1328
Corrales, NM 87048

Re: Salazar Family Trust Conceptual Drainage Plan

Engineer's Stamp dated 11-17-06 (N9/D11)

Dear Mr. Speicher,

Based upon the information provided in your submittal dated 11-17-06, the above referenced plan cannot be approved for Conceptual Drainage Plan until the following comment is addressed:

- It is still not clear which basins are included in the analysis points. Per our telephone conversation, we were to meet to discuss this. Please call to schedule a meeting.

If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne, E.I.
Engineering Associate, Planning Dept.
Development and Building Services

C: file
Brad Bingham

P.O. Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov

DRAINAGE AND TRANSPORTATION INFORMATION SHEET
(Rev. 12/05)

PROJECT TITLE: SALAZAR FAMILY TRUST ZONE MAP/DRG. FILE # N-9/D11
DRB#: _____ EPC#: _____ WORK ORDER#: _____

LEGAL DESCRIPTION: TRACT A SALAZAR FAMILY TRUST
CITY ADDRESS: _____

ENGINEERING FIRM: CSC CONTACT: RON HENSLEY
ADDRESS: P.O. BOX 1328 PHONE: 877-0000 EXT. 111
CITY, STATE: CORRALES, NM ZIP CODE: 87048

OWNER: SALAZAR FAMILY TRUST CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

ARCHITECT: N/A CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

SURVEYOR: CSC CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

CONTRACTOR: N/A CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

TYPE OF SUBMITTAL:
_____ DRAINAGE REPORT
_____ DRAINAGE PLAN 1st SUBMITTAL
_____ DRAINAGE PLAN RESUBMITTAL
_____ CONCEPTUAL G & D PLAN
_____ GRADING PLAN
_____ EROSION CONTROL PLAN
_____ ENGINEER'S CERT (HYDROLOGY)
_____ CLOMR/LOMR
_____ TRAFFIC CIRCULATION LAYOUT
_____ ENGINEER/ARCHITECT CERT (TCL)
_____ ENGINEER/ARCHITECT CERT (DRB S.P.)
_____ ENGINEER/ARCHITECT CERT (AA)
__X__ OTHER (SPECIFY) RESUBMIT

CHECK TYPE OF APPROVAL SOUGHT:
_____ SIA/FINANCIAL GUARANTEE RELEASE
_____ PRELIMINARY PLAT APPROVAL
_____ S. DEV. PLAN FOR SUB'D APPROVAL
_____ S. DEV. FOR BLDG. PERMIT APPROVAL
_____ SECTOR PLAN APPROVAL
_____ FINAL PLAT APPROVAL
_____ FOUNDATION PERMIT APPROVAL
_____ BUILDING PERMIT APPROVAL
_____ CERTIFICATE OF OCCUPANCY (PERM)
_____ CERTIFICATE OF OCCUPANCY (TEMP)
_____ GRADING PERMIT APPROVAL
_____ PAVING PERMIT APPROVAL
_____ WORK ORDER APPROVAL
__X__ OTHER (SPECIFY) OTHER PLATTING

WAS A PRE-DESIGN CONFERENCE ATTENDED:
_____ YES
_____ NO
_____ COPY PROVIDED

RECEIVED
NOV 17 2006
HYDROLOGY SECTION

SUBMITTED BY: Ron E. Hensley DATE: 11/16/06

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope to the proposed development define the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

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3. **Drainage Report:** Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.

CITY OF ALBUQUERQUE

595-02M-610.610

October 23, 2006

RECEIVED
OCT 25 2006

Brian L. Speicher, P.E.
Community Sciences Corporation
P.O. Box 1328
Corrales, NM 87048

Re: Salazar Family Trust Conceptual Drainage Plan
Engineer's Stamp dated 10-10-06 (N9/D11)

Dear Mr. Speicher,

Based upon the information provided in your submittal dated 10-11-06, the above referenced plan cannot be approved for Conceptual Drainage Plan until the following comments are addressed:

- Please provide a table that includes the Basin, API, street flow and pipe flow.

- Add excerpt from Arrowwood justifying Basins 1, 2, and 4 draining into Colobel and 98th.

- Add Sacate Blanco Channel R/W into either Basin 8 or 9 and plan accordingly.

- Basin 11 appears to be made up of mostly Unser Blvd. Please update the land treatment type, corresponding runoff and volume.

- Provide for the case in which Basin 12 is not deeded to AMAFCA.

- Would it be possible to provide a plan that wasn't as dark and also to improve the Vicinity Map and FEMA Map so they are legible?

RECEIVED
NOV 17 2006
HYDROLOGY SECTION

If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne, E.I.
Engineering Associate, Planning Dept.
Development and Building Services

C: file

P.O. Box 1293
Albuquerque
New Mexico 87103
www.cabq.gov

CITY OF ALBUQUERQUE

October 23, 2006

Brian L. Speicher, P.E.
Community Sciences Corporation
P.O. Box 1328
Corrales, NM 87048

Re: Salazar Family Trust Conceptual Drainage Plan

Engineer's Stamp dated 10-10-06 (N9/D11)

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If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne

Curtis A. Cherne, E.I.
Engineering Associate, Planning Dept.
Development and Building Services

BM

C: file

P.O. Box 1293
Albuquerque
New Mexico 87103
www.cabq.gov

DRAINAGE INFORMATION SHEET
(REV. 11/01/2001)

PROJECT TITLE: LANDS OF SALAZAR DRAINAGE MANAGEMENT PLAN ZONE MAP/DRG. FILE #: N-9 / D-11
DRB #: N/A EPC#: N/A WORK ORDER#: N/A
LEGAL DESCRIPTION: RIO BRAVO TRACTS A-1-A, A-1-B, A-1-C & 13-D-1
CITY ADDRESS:

ENGINEERING FIRM: Community Sciences Corp.
ADDRESS: P.O. Box 1328
CITY, STATE: CORRALES, NM
CONTACT: BRIAN L. SPEICHER
PHONE: 923-9552
ZIP CODE: 87048 897-0000 X112

OWNER: TOM SALAZAR
ADDRESS: 1016 SUMMIT DR. NE
CITY, STATE: ALBQ., NM
CONTACT: TOM SALAZAR
PHONE: 977-5372
ZIP CODE: 87106

ARCHITECT:
ADDRESS:
CITY, STATE:
CONTACT:
PHONE:
ZIP CODE:

SURVEYOR: Community Sciences Corp
ADDRESS
CITY, STATE:
CONTACT:
PHONE:
ZIP CODE:

CONTRACTOR:
ADDRESS:
CITY, STATE:
CONTACT:
PHONE:
ZIP CODE:

CHECK TYPE OF SUBMITTAL:
- ___ DRAINAGE REPORT
- X DRAINAGE PLAN
- ___ CONCEPTUAL GRADING & DRAINAGE PLAN
- ___ GRADING PLAN
- ___ EROSION CONTROL PLAN
- ___ ENGINEER'S CERTIFICATION (HYDROLOGY)
- ___ CLOMR/LOMR
- ___ TRAFFIC CIRCULATION LAYOUT (TCL)
- ___ ENGINEERS CERTIFICATION (TCL)
- ___ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
- ___ OTHER

CHECK TYPE OF APPROVAL SOUGHT:
- ___ SIA / FINANCIAL GUARANTEE RELEASE
- ___ PRELIMINARY PLAT APPROVAL
- ___ S. DEV. PLAN FOR SUB'D. APPROVAL
- ___ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
- ___ SECTOR PLAN APPROVAL
- ___ FINAL PLAT APPROVAL
- ___ FOUNDATION PERMIT APPROVAL
- ___ BUILDING PERMIT APPROVAL
- ___ CERTIFICATE OF OCCUPANCY (PERM.)
- ___ CERTIFICATE OF OCCUPANCY (TEMP.)
- ___ GRADING PERMIT APPROVAL
- ___ PAVING PERMIT APPROVAL
- ___ WORK ORDER APPROVAL
- X OTHER (SPECIFY) Drainage Mangt Plan

WAS A PRE-DESIGN CONFERENCE ATTENDED:
- X YES
- ___ NO
- ___ COPY PROVIDED

RECEIVED
OCT 11 2006
HYDROLOGY SECTION

DATE SUBMITTED: OCT. 10, 2006 BY: Brian

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plan**: Required for approval of Site Development Plans greater than five
2. **Drainage Plans**: Required for building permits, grading permits, paving permits and site plans less than five (5)
3. **Drainage Report**: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or