CITY OF ALBUQUERQUE



July 24, 2017

David Aube, PE Hartman + Majewski Design Group 120 Vassar Dr. SE Suite 100 Albuquerque, NM 87106

Re: GAHP Casa Feliz

421 Espanola Street SE

Request for Permanent C. O. – Not Accepted Engineers Stamp Date 12/18/15 (L19D073F)

Certification dated: 7/18/2017

Dear Mr. Aube,

Based on the Engineer's Certification provided in your submittal received 7/18/2017, Hydrology cannot approve the issuance of the Certificate of Occupancy until the following comments are addressed:

PO Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov

- 1. Even though your certification and associated survey clams a 6" deep channel on the south side of the site, at our site meeting on 7/20/2017 it appeared that most of that depth is loose landscape rock that won't hold water. So the Grading Plan must be revised and resubmitted to hydrology to include a design of a wall or curb along the south property line as necessary to prevent cross lot drainage. The revised plan should include a new engineer's stamp date.
- 2. The revised plan should also include a cross-section through the swale and property line and show this sit 1' higher that the adjacent site as shown at section D on the surveyed sections. Manning's "n" value in the channel calculations also needs correction from 0.025to 0.045 for rip-rap.

After the revised G&D Plan is approved by Hydrology and the contractor has completed the construction, a new Engineer's Certification should be submitted to hydrology for CO. The following should be on that plan.

1. The pipe that was added in the side yard swale can either be added to the revised G&D Plan or added to the subsequent Engineer's Certification, but it must include invert elevations at both ends and the flow depth must be revised and the pipe must be analyzed as a culvert instead of the normal depth calculation in 6/28/2017 submittal.

2. As-built survey information should be added to the plan to include spot elevations throughout the parking lot, the swale, and the finished floor elevations as well as the top of the new wall to be added on the south side of the property.

An inspection by our office will need to take place after these corrections are made.

If you have any questions, you can contact me at 924-3986 or Totten Elliott at 924-3982.

Sincerely, Januar D. Benjar

James D. Hughes, P.E.

Principal Engineer, Planning Dept. Development and Review Services

TE/JH C: email

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/28/2003rd)

PROJECT TITLE: GAHP Casa Feliz (L19D073F) DRB #: EPC#:	ZONE MAP/DRG. FILE #: <u>L-19-Z</u> WORK ORDER#:
LEGAL DESCRIPTION: Lot 13-18, 18-20 Block 4, Lots 4-8, 17-18 Block 5, CITY ADDRESS: Bldg H at 421 Espanola SE	etc. Emil Mann Addition
ENGINEERING FIRM: <u>Hartman + Majewski Design Group</u> ADDRESS: <u>120 Vassar Dr SE, Suite 100</u> CITY, STATE: <u>Albuquerque, NM 87106</u>	CONTACT: <u>David Aube</u> PHONE: <u>505-998-6430</u> ZIP CODE: <u>87106</u>
OWNER: Greater Albuquerque Housing Partnership ADDRESS: 320 Gold SW, Suite 918 CITY, STATE: Albuquerque, NM	CONTACT: <u>Felipe Rael</u> PHONE: <u>505-244-1614</u> ZIP CODE: <u>87102</u>
ARCHITECT: Hartman + Majewski Design Group ADDRESS: 120 Vassar Dr SE, Suite 100 CITY, STATE: Albuquerque, NM	CONTACT: Mark Wade PHONE: 505-998-6442 ZIP CODE: <u>87106</u>
SURVEYOR: Community Sciences. ADDRESS: CITY, STATE: Albuquerque, NM	CONTACT: PHONE: <u>505-</u> ZIP CODE:
CONTRACTOR: ADDRESS: CITY, STATE:	CONTACT: PHONE: ZIP CODE:
CHECK TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:
 □ DRAINAGE REPORT □ DRAINAGE PLAN 1st SUBMITTAL, REQUIRES TCL or equal □ DRAINAGE PLAN RESUBMITTAL □ CONCEPTUAL GRADING & DRAINAGE PLAN □ GRADING PLAN □ EROSION CONTROL PLAN □ ENGINEER'S CERTIFICATION (HYDROLOGY) □ CLOMR/LOMR □ TRAFFIC CIRCULATION LAYOUT (TCL) □ ENGINEERS CERTIFICATION (TCL) □ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN) □ OTHER 	SIA / FINANCIAL GUARANTEE RELEASE PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D. APPROVAL S. DEV. PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY (PERM.) CERTIFICATE OF OCCUPANCY (TEMP.) GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL WORK ORDER APPROVAL OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED: ☐ YES ☐ NO ☐ COPY PROVIDED	

DATE SUBMITTED: July 18, 2017

BY: David Aube P.E.

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. **Conceptual Grading and Drainage Plan**: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. **Drainage Plans**: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. **Drainage Report**: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

II. SITE DESCRIPTION AND HISTORY

THE PROJECT CONTAINS MANY SCATTERED SITES, LOCATED AROUND A PREVIOUS PROJECT BY GREATER ALBUQUERQUQ HOUSING AUTHORITY CALLED PLAZA FELIZ. THE SITES ARE LOCATED ON ESPANOLA STREET SE, SAN PABLO STREET SE, GROVE STREET SE, BELL AVENUE SE, AND TRUMBELL AVENUE SE.

THIS AREA WAS AT ONE TIME FULLY DEVELOPED WITH A FOURPLEX ON EACH OF THE LOTS BEING REDEVELOPED BY THIS PROJECT. THE LAND WAS ORIGINALLY SUBDIVIDED IN 1944 AND WAS FULLY DEVELOPED PRIOR TO THE IMPLEMENTATION OF THE DRAINAGE ORDINANCE RESTRICTING FLOW FROM THE SITES INTO THE PUBLIC WAY. DEMOLITION OF THESE PRIOR FOURPLEXES WAS COMPLETED BY 2010 WITH THE EXCEPTION OF ONE LOT THAT STILL NEEDS TO HAVE THE BUILDING REMOVED FOR THIS PROJECT.

THE SITES WERE ALL FREE DISCHARGE INTO THE STREET, OR IN SOME CASES INTO THE ADJACENT PROPERTIES. EACH OF THE SITES WERE ANALYZED AS TYPE C SOIL TO ACCOUNT FOR THE PREVIOUSLY COMPACTED SOIL CONDITIONS. THIS INCLUDES THE LOT WITH THE EXISTING BUILDING TO BE REMOVED.

III. COMPUTATIONAL PROCEDURES

HYDROLOGIC ANALYSIS WAS PERFORMED UTILIZING THE DESIGN CRITERIA BASED ON SECTION 22.2, HYDROLOGY, OF THE DEVELOPMENT PROCESS MANUAL.

IV. PRECIPITATION

THE STORM EVENT USED FOR THE FOLLOWING CALCULATIONS IS THE 100YR-6HR STORM. THE PROJECT SITE IS LOCATED IN ZONE 3.

V. EXISTING DRAINAGE CONDITIONS (REFER TO CD EX1)

CURRENTLY THE SITES FLOW FROM EAST TO WEST AND TOWARD BELL AVENUE FROM BOTH NORTH AND SOUTH. WITH THE PRIOR DEVELOPMENT, MANY OF THE LOTS CREATED CROSS LOT DRAINAGE PATTTERNS THAT WILL BE CORRECTED WITH THIS PROJECT. HISTORICALLY THE SITES HAD BEEN DEVELOPED AS FOURPLEX UNITS WITH APPROXIMATELY 6 PARKING SPACES ON SITE. THE SITES CONTAINED MINIMAL LANDSCAPING AND WERE LIKELY 85% IMPERVIOUS (USING SURROUNDING UNITS AS A TYPICAL DEVELOPMENT DENSITY).

USING THE 85% D AND 15% C SOIL TREATMENTS THE TYPCIAL 50X135 LOT (6750 SF) CREATES A PEAK RUNOFF RATE OF 0.74 CFS AND AN EXCESS RUNOFF VOLUME OF 0.0284 ACRE FEET DURING THE 100 YEAR 6 HOUR EVENT.

TO BE MORE CONSERVATIVE. THE SITES WERE ANALYZED AS 100% C SOIL AS REQUIRED FOR SOIL COMPACTED BY HUMAN ACTIVITY. THE REDUCES THE PEAK RUNOFF RATE TO 0.53 CFS AND AN EXCESS RUNOFF VOLUME OF 0.0167 ACRE FEET DURING THE 100 YEAR 6 HOUR EVENT.

VI. PROPOSED DRAINAGE CONDITIONS

THE SCATTERED SITE HAVE BEEN ANALYZED INDIVIDUALLY. BUILDINGS HAVE BEEN ASSIGNED LETTERS AND THIS REPORT IS ORGANIZED TO FOLLOW THAT SAME ORDER.

BUILDING/SITE A IS LOCATED IN A SINGLE LOT THAT WILL HAVE A PORTION OF TRUMBELL VACATED AND IS THEREFORE SLIGHTLY LARGER THAN THE TYPICAL LOT AND CONTAINS 7425 SF. THIS SITE CURRENTLY CONTAINS AN APARTMENT BUILDING THAT WILL BE DEMOLISHED. THE SITE IS BROKEN UP INTO TWO SUB BASINS, THE FIRST FLOWING WEST TOWARD SAN PABLO AND THE OTHER TO THE EAST AND INTO TRUMBELL ON THE SOUTH. THE COMBINED FLOW RATES FOR THIS SITE 0.70 CFS WHICH IS LESS THAN THE ACTUAL CURRENT CONDITIONS OF 0.82 SF. THESE NUMBERS WERE ADJUSTED BECAUSE THE SITE CONTAINS THE 7425 SF IN LIEU OF THE TYPCIAL 6750 SF. THE INCLUSION OF SHALLOW PONDING AREAS (4" DEEP) THAT HARVEST 113 OF THE FIRST FLUSH VOLUME (91.6 CF REQUIRED) WILL FURTHER REDUCE THE PEAK RUNOFF.

BUILDING/SITE B CONTAINS 7 PARCELS AND WOULD HAVE CREATED A PEAK RUNOFF IN THE PREVIOUSLY DEVELOPED CONDITION OF 5.18 CFS (7 * 0.74 CFS). THE PROPOSED DEVELOPMENT WILL CREATE A PEAK RUNOFF OF 4.32 CSF AND AN EXCESS RUNOFF VOLUME OF 0.1520 ACRE FEET. THE SHALLOW PONDS SURROUNDING THE BUILDING WILL HARVEST 576 CF WHICH EXCEEDS THE FIRST FLUSH REQUIRED VOLUME OF 305.6 CF.

BUILDING/SITE C AND D CONTAINS 5 PARCELS AND WOULD HAVE CREATED A PEAK RUNOFF IN THE PREVIOUSLY DEVELOPED CONDITION OF 3.71 CFS (5 * 0.74 CFS). THE PROPOSED DEVELOPMENT WILL CREATE A PEAK RUNOFF OF 3.28 CSF AND AN EXCESS RUNOFF VOLUME OF 0.1156 ACRE FEET. THE SHALLOW PONDS SURROUNDING THE BUILDING WILL HARVEST 454 CF WHICH EXCEEDS THE FIRST FLUSH REQUIRED VOLUME OF 271.3 CF.

BUILDING/SITE E, F AND G CONTAINS 7 PARCELS AND WOULD HAVE CREATED A PEAK RUNOFF IN THE PREVIOUSLY DEVELOPED CONDITION OF 5.19 CFS (7 * 0.74 CFS). THE PROPOSED DEVELOPMENT WILL CREATE A PEAK RUNOFF OF 4.35 CSF AND AN EXCESS RUNOFF VOLUME OF 0.1527 ACRE FEET. THE SHALLOW PONDS SURROUNDING THE BUILDING WILL HARVEST 393 CF WHICH EXCEEDS THE FIRST FLUSH REQUIRED VOLUME OF 263.0 CF.

EXCESS RUNOFF FROM THESE SITES CAN DISCHARGE INTO ESPANOLA THROUGH THE DRIVEWAY OPENING OR THROUGH A SMALL RUNDOWN CHANNEL LOCATED ON THE SOUTH SIDE OF BUILDING G AND THROUGH A SIDEWALK CULVERT UNDER THE PUBLIC SIDEWALK.

BUILDING/SITE H CONTAINS 3 PARCELS AND WOULD HAVE CREATED A PEAK RUNOFF IN THE PREVIOUSLY DEVELOPED CONDITION OF 2.22 CFS (3 * 0.74 CFS). THE PROPOSED DEVELOPMENT WILL CREATE A PEAK RUNOFF OF 1.81 CSF AND AN EXCESS RUNOFF VOLUME OF 0.0629 ACRE FEET. THE SHALLOW PONDS SURROUNDING THE BUILDING WILL HARVEST 315 CF WHICH EXCEEDS THE FIRST FLUSH REQUIRED VOLUME OF 203.8 CF.

EXCESS RUNOFF FROM THESE SITES CAN DISCHARGE INTO ESPANOLA THROUGH THE DRIVEWAY OPENING OR THROUGH A SMALL RUNDOWN CHANNEL LOCATED ON THE SOUTH SIDE OF BUILDING H AND THROUGH A SIDEWALK CULVERT UNDER THE PUBLIC SIDEWALK.

BUILDING/SITE I CONTAINS 4 PARCELS AND WOULD HAVE CREATED A PEAK RUNOFF IN THE PREVIOUSLY DEVELOPED CONDITION OF 2.97 CFS (4 * 0.74 CFS). THE PROPOSED DEVELOPMENT WILL CREATE A PEAK RUNOFF OF 2.88 CSF AND AN EXCESS RUNOFF VOLUME OF 0.1089 ACRE FEET. THE SHALLOW PONDS SURROUNDING THE BUILDING WILL HARVEST 218.3 CF WHICH EXCEEDS THE FIRST FLUSH REQUIRED VOLUME OF 117.1 CF. THIS SITE DRAINS OUT THE SOUTHERN DRIVEWAY INTO BELL AVENUE, SE.

BUILDING/SITE J CONTAINS 4 PARCELS AND WOULD HAVE CREATED A PEAK RUNOFF IN THE PREVIOUSLY DEVELOPED CONDITION OF 3.03 CFS (SLIGHTLY LARGER DUE TO VACATED ROW ON BELL AVENUE SE THAT IS INCORPORATED INTO THE SITE). THE PROPOSED DEVELOPMENT WILL CREATE A PEAK RUNOFF OF 2.48 CSF AND AN EXCESS RUNOFF VOLUME OF 0.0848 ACRE FEET. THE SHALLOW PONDS SURROUNDING THE BUILDING WILL HARVEST 308 CF WHICH EXCEEDS THE FIRST FLUSH REQUIRED VOLUME OF 151 CF. THIS SITE DRAINS OUT THE NORTHERN DRIVEWAY INTO BELL AVENUE, SE.

BUILDING/SITE K CONTAINS 3 PARCELS AND WOULD HAVE CREATED A PEAK RUNOFF IN THE PREVIOUSLY DEVELOPED CONDITION OF 2.22 CFS (3 * 0.74 CFS). THE PROPOSED DEVELOPMENT WILL CREATE A PEAK RUNOFF OF 1.81 CSF AND AN EXCESS RUNOFF VOLUME OF 0.0629 ACRE FEET. THE SHALLOW PONDS SURROUNDING THE BUILDING WILL HARVEST 338CF WHICH EXCEEDS THE FIRST FLUSH REQUIRED VOLUME OF 102 CF.

BUILDING/SITE L IS LOCATED IN A SINGLE LOT THAT WILL HAVE A PORTION OF TRUMBELL VACATED AND IS THEREFORE SLIGHTLY LARGER THAN THE TYPICAL LOT AND CONTAINS 7425 SF. THE COMBINED FLOW RATES FOR THIS SITE 0.70 CFS WHICH IS LESS THAN THE ACTUAL CURRENT CONDITIONS OF 0.81 SF. THE INCLUSION OF SHALLOW PONDING AREAS (4" DEEP) THAT HARVEST 118 OF THE FIRST FLUSH VOLUME (91.6 CF REQUIRED) WILL FURTHER REDUCE THE PEAK RUNOFF.

VII. CONCLUSIONS

EACH INDIVIDUAL SITE HAS BEEN DESIGNED TO HARVEST MORE THAN IS REQUIRED TO MEET THE MSSSS PERMIT REQUIREMENTS FOR FIRST FLUSH VOLUMES. SITES HAVE BEEN DESIGNED TO DIRECT EXCESS RUNOFF TO THE PUBLIC STREETS WHERE PREVIOUS DEVELOPMENTS DISCHARGED IN A CROSS LOT CONFIGURATION.

EACH LOT IS STILL ALLOWING FOR FREE DISCHARGE FOR RUNOFF EXCEEDING THAT CONTAINED IN THE MSSSS REQUIREMENTS DESCRIBED ABOVE. THIS IS CONSISTENT WITH AND A REDUCTION FROM THE HISTORIC DISCHARGE RATES FROM THE PREVIOUSLY DEVELOPED LOTS.

BECAUSE THERE IS A REDUCTION IN FLOW RATES AND EXCESS RUNOFF IS DIRECTED TOWARD THE PUBLIC STREET INSTEAD OF CROSS LOT DRAINAGE, THERE SHOULD BE BENEFITS TO THE DOWNSTREAM LOTS.

Drainage	Summary

Project: Casa Fleliz Project Numbe: Date: 10/12/15 Dave A

Site Location

Existing summary

3 Per Table A-1 COA DPM Section 22.2 Precipitaion Zone

Basin Name	EXA	EXB	EXC & D	EXE	EXF & G	EXH	EXI	EX J	EXK	EXL
Area (sf)	7425	47202	33742	13504	33742	20250	26999	27556	20250	7425
Area (acres)	0.170	1.084	0.775	0.310	0.775	0.465	0.620	0.633	0.465	0.170
%A Land treatment	0	0	0	0	0	0	0	0	0	0
%B Land treatment	0	0	0	0	0	0	0	0	0	0
%C Land treatment	15	15	15	15	15	15	15	15	15	15
%D Land treatment	85	85	85	85	85	85	85	85	85	85
Soil Treatment (acres)										
Area "A"	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Area "B"	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Area "C"	0.03	0.16	0.12	0.05	0.12	0.07	0.09	0.09	0.07	0.03
Area "D"	0.14	0.92	0.66	0.26	0.66	0.40	0.53	0.54	0.40	0.14
Excess Runoff (acre-feet)										

100yr. 6h 0.0353 0.0707 0.1235 0.0883 0.0883 0.0530 0.0721 0.0530 10yr. 6hr. 0.0508 0.0203 0.0508 0.0305 0.0406 0.0305 0.0415 0.1694 0.0678 0.1694 0.1017 0.1356 100yr. 24hr. 0.1384 Peak Discharge (cfs) 100 yr. 5.18 3.45 0.99 1.97 2.46 1.48 2.01 1.48 0.54

1.**4**3

Proposed summary Basin Name

Area (acres) %A Land treatment %B Land treatment %C Land treatment %D Land treatment Soil Treatment (acres) Area "A" Area "B"

Area "C" 100yr. 6hr. 10yr. 6hr. 2yr. 6hr.

Peak Discharge (cfs) 100 yr. 10vr. 0.30

First Flush Ponding Voulme (cf) 45.8 45.8 152.8 First Flush Acre Feet

0.00

0.08

0.0276

0.57

Pro B2

152.8 46.6 178.1 0.0035 0.0011 0.0041

46.6 0.0011

0.0217

38.4 0.0009

1.15

4525

0.03

0.0055

0.29

1.17

Pro E1

7462

0.03

0.0114

0.0053

0.36

Pro F1

4525

0.03

0.0101

0.0055

0.0197

0.29

0.104

30721

46.6 0.0011

131.5 0.0011 0.0030

46.6

Copper Ave NE

Elementary School

Van Cleave Place Im

Trumbull Ave SE

Susan Ave SE

PROJECT LOCATION

San Joaquin Ave SE

Kathryn Ave SE

0.0545

0.0259

101.9 0.0023

0.0101

0.0055

101.9 0.0023

38.7 0.0009

Pro H1

6275

0.04

0.04

0.0119

0.0061

0.0238

4525

Pro H2

13975

0.03

0.19

0.0229

0.0105

0.0461

0.36 0.22 1.75 0.75 0.19

0.0 0.0000

Typ 85% D Typ 100%C

0.00

0.00

0.02

0.13

0.0284

0.0177

0.0102

0.0339

0.74

0.49

0.29

Pro I1

3257

0.075

0.01

0.0075

0.0042

0.0147

0.00

0.15

0.0080

0.0026

0.53

0.31

3342

0.05

0.0059

0.0028

20400

0.07

0.0534

0.0307

0.1024

1.49

79.7 0.0018 0.0016 0.0018

Pro J2

21808

0.30

0.0358

0.0164

1.17

0.0003

0.02

5469

0.126

0.03

0.0104

0.0053

0.31

0.0 51.0 51.0 0.0000 0.0012 0.0012

0.0063

0.37

0.0104

0.74

GREATER ALBUQUERQUE

441 ESPANOLA STREET SE,

ALBUQUERQUE, NEW MEXICO 87108

HOUSING PATRTNERSHIP

100% CONSTRUCTION

DOCUMENTS

PROJECT NAME

CASA FELIZ

DESIGN GROUP

Planners • Urban Designers • LEED®

120 Vassar Dr SE Suite 100

Albuquerque New Mexico 87106

T 505 242 6880 • F 505 242 6881

CONSULTANT

STAMP

5321

0.00

0.08

0.30

0.00

0.01

0.14

Drainage Certification (L19D073F, DRB#1010666)

Cochiti Rd SE

Acoma Rd SF

Bell Ave SE

Trumbull Ave SE

Southern Ave SE

Aps-Wilson
Middle School Ross Ave SE

Kathryn Ave SE

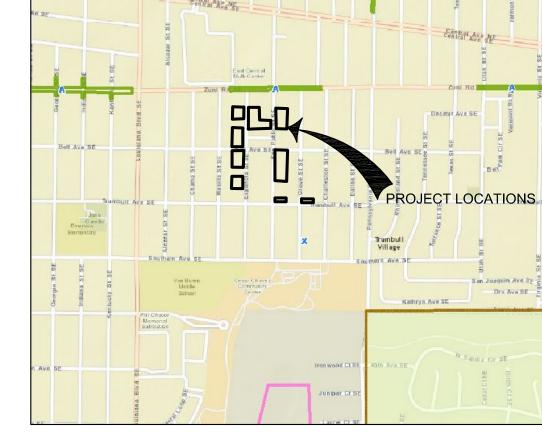
VICINITY MAPS

Zuni Rd SE

I, David A Aube. NMPE 14221, of the firm The Hartman + Majewski Design Group, Inc, hereby certify that portion of the project (Building H located at 421 Espanola St. SE) is in substantial compliance with and in accordance with the design intent of the Grading and Drainage plan approved plan dated 12-18-15. The record information that has been edited onto the original design documents where obtained by Community Sciences Corporation on May 4th, 2017. I further certify that I have personally visited the project site on June 26, 2017 and have determined by visual inspection that the actual site conditions shown on this plan to be true and correct to the best of my knowledge and belief. This certification is submitted in support of a request for Permanent Certificate of Occupancy for Building H located at 421 Espanola St. SE.

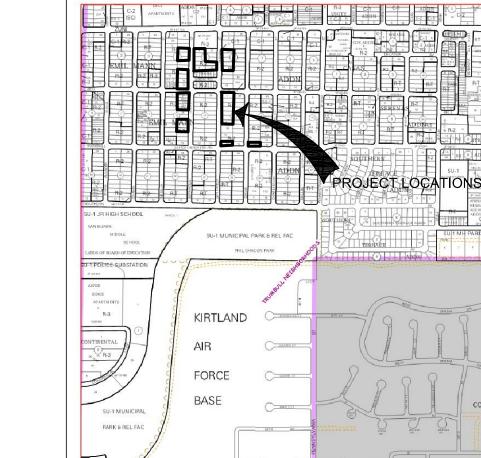
The record information presented hereon is not necessarily complete and intended only to verify substantial compliance of the drainage aspects of this project. Those relying on the record documents are advised to obtain independent verification of its accuracy before using it for any other purpose.



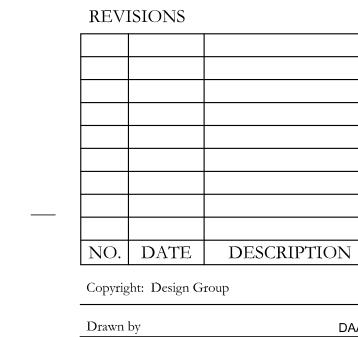








L-19-Z ZONE ATLAS PAGE



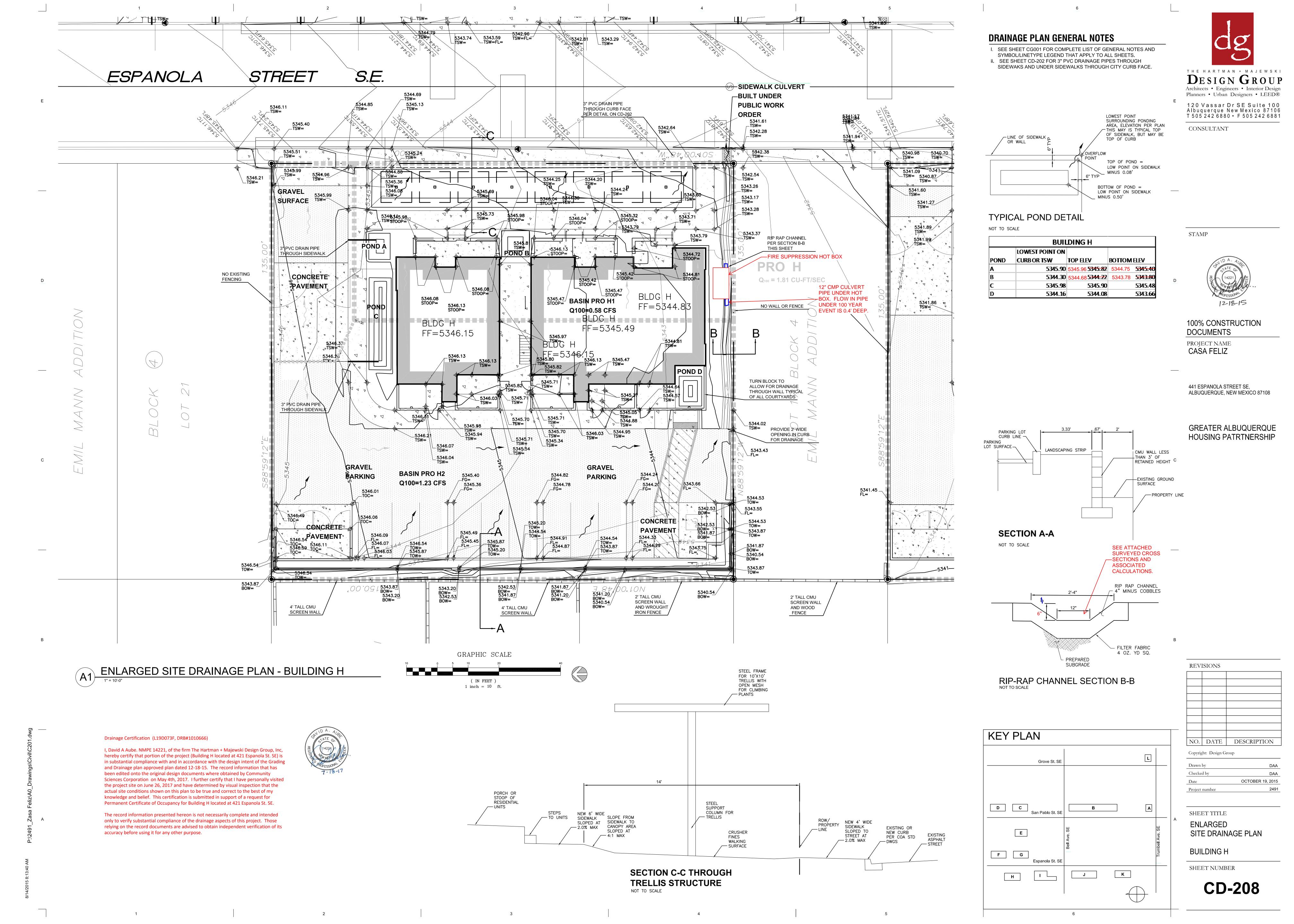
Drawn by	
Checked by	
Date	OCTOBER 19, 2
Project number	2

SHEET TITLE

OVERALL EXISTING SITE DRAINAGE PLAN

CD1

SHEET NUMBER



Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Monday, Jul 17 2017

<Name> Section A

 Trapezoidal

 Bottom Width (ft)
 = 2.00

 Side Slopes (z:1)
 = 0.83, 1.43

 Total Depth (ft)
 = 0.70

 Invert Elev (ft)
 = 2.90

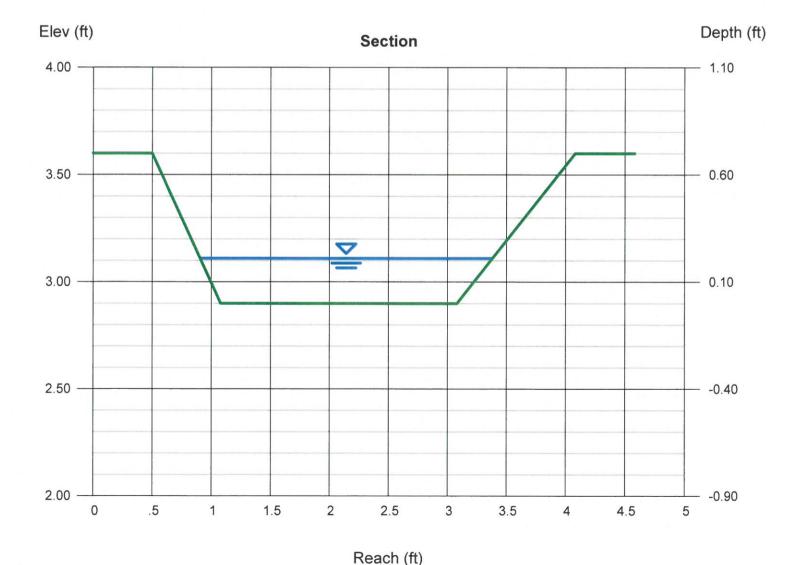
 Slope (%)
 = 2.00

 N-Value
 = 0.025

Calculations

Compute by: Q vs Depth No. Increments = 10

Highlighted Depth (ft) = 0.21= 1.249Q (cfs) Area (sqft) = 0.47Velocity (ft/s) = 2.66 Wetted Perim (ft) = 2.64 Crit Depth, Yc (ft) = 0.23 Top Width (ft) = 2.47EGL (ft) = 0.32



CRECTION A

Depth	Q	Area	Veloc	Wp	Yc	TopWidth	Energy
(ft)	(cfs)	(sqft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
0.07	0.199	0.146	1.37	2.21	0.07	2.16	0.10
0.14	0.633	0.302	2.09	2.43	0.15	2.32	0.21
0.21	1.249	0.470	2.66	2.64	0.23	2.47	0.32
0.28	2.030	0.649	3.13	2.85	0.30	2.63	0.43
0.35	2.968	0.838	3.54	3.07	0.38	2.79	0.54
0.42	4.060	1.039	3.91	3.28	0.47	2.95	0.66
0.49	5.305	1.251	4.24	3.49	0.55	3.11	0.77
0.56	6.703	1.474	4.55	3.70	0.63	3.27	0.88
0.63	8.256	1.708	4.83	3.92	0.70	3.42	0.99
0.70	9.966	1.954	5.10	4.13	0.70	3.58	1.10

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Q vs Depth

= 10

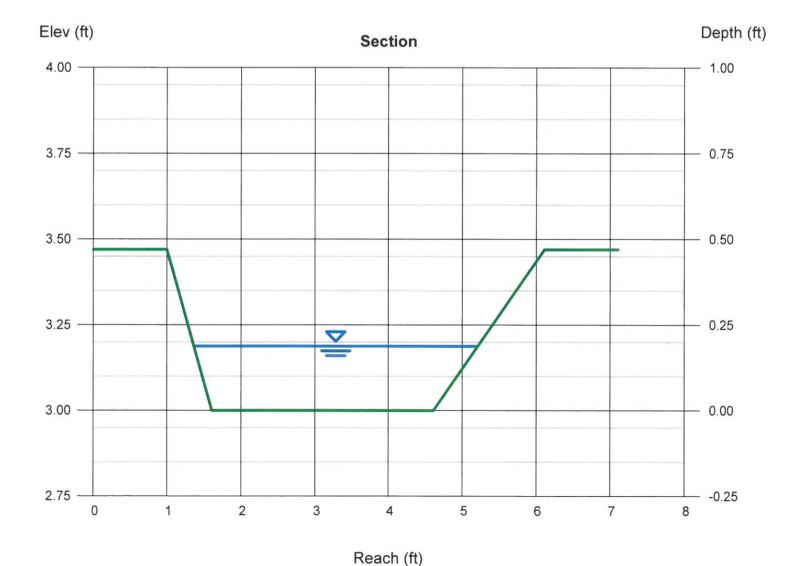
Monday, Jul 17 2017

Section B

Compute by:

No. Increments

Trapezoidal		Highlighted	
Bottom Width (ft)	= 3.00	Depth (ft)	= 0.19
Side Slopes (z:1)	= 1.30, 3.19	Q (cfs)	= 1.616
Total Depth (ft)	= 0.47	Area (sqft)	= 0.64
Invert Elev (ft)	= 3.00	Velocity (ft/s)	= 2.51
Slope (%)	= 2.00	Wetted Perim (ft)	= 3.94
N-Value	= 0.025	Crit Depth, Yc (ft)	= 0.20
		Top Width (ft)	= 3.84
Calculations		EGL (ft)	= 0.29



SECTION B

Depth	Q	Area	Veloc	Wp	Yc	TopWidth	Energy
(ft)	(cfs)	(sqft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
0.05	0.155	0.146	1.06	3.23	0.05	3.21	0.06
0.09	0.498	0.302	1.65	3.47	0.10	3.42	0.14
0.14	0.989	0.468	2.11	3.70	0.15	3.63	0.21
0.19	1.616	0.643	2.51	3.94	0.20	3.84	0.29
0.24	2.372	0.829	2.86	4.17	0.26	4.06	0.36
0.28	3.255	1.025	3.18	4.41	0.31	4.27	0.44
0.33	4.265	1.230	3.47	4.64	0.37	4.48	0.52
0.38	5.401	1.445	3.74	4.87	0.42	4.69	0.59
0.42	6.664	1.671	3.99	5.11	0.47	4.90	0.67
0.47	8.056	1.906	4.23	5.34	0.47	5.11	0.75

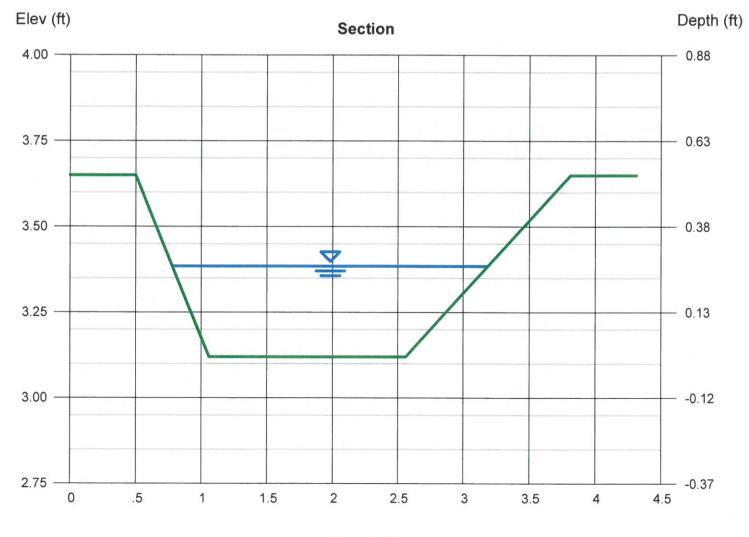
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Monday, Jul 17 2017

Section C

No. Increments

Trapezoidal		Highlighted	
Bottom Width (ft)	= 1.50	Depth (ft)	= 0.27
Side Slopes (z:1)	= 1.06, 2.36	Q (cfs)	= 1.496
Total Depth (ft)	= 0.53	Area (sqft)	= 0.52
Invert Elev (ft)	= 3.12	Velocity (ft/s)	= 2.89
Slope (%)	= 2.00	Wetted Perim (ft)	= 2.57
N-Value	= 0.025	Crit Depth, Yc (ft)	= 0.29
		Top Width (ft)	= 2.41
Calculations		EGL (ft)	= 0.39
Compute by:	Q vs Depth		



Reach (ft)

SECTION C

Depth	Q	Area	Veloc	Wp	Yc	TopWidth	Energy
(ft)	(cfs)	(sqft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
0.05	0.095	0.084	1.13	1.71	0.05	1.68	0.07
0.11	0.306	0.178	1.72	1.93	0.11	1.86	0.15
0.16	0.613	0.282	2.17	2.14	0.17	2.04	0.23
0.21	1.009	0.395	2.56	2.35	0.23	2.23	0.31
0.27	1.496	0.518	2.89	2.57	0.29	2.41	0.39
0.32	2.073	0.650	3.19	2.78	0.35	2.59	0.48
0.37	2.743	0.792	3.46	2.99	0.41	2.77	0.56
0.42	3.508	0.943	3.72	3.20	0.47	2.95	0.64
0.48	4.371	1.105	3.96	3.42	0.53	3.13	0.72
0.53	5.335	1.275	4.18	3.63	0.53	3.31	0.80

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= 0.27 = 1.460 = 0.50 = 2.91 = 2.46 = 0.29 = 2.28

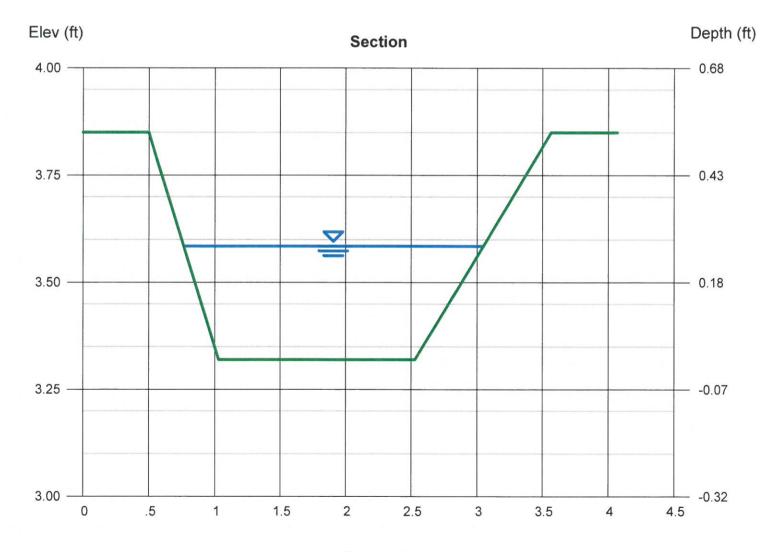
= 0.40

Section D

Trapezoidal		Highlighted
Bottom Width (ft)	= 1.50	Depth (ft)
Side Slopes (z:1)	= 1.00, 1.96	Q (cfs)
Total Depth (ft)	= 0.53	Area (sqft)
Invert Elev (ft)	= 3.32	Velocity (ft/s)
Slope (%)	= 2.00	Wetted Perim (ft)
N-Value	= 0.025	Crit Depth, Yc (ft)
		Top Width (ft)
Calculations		EGL (ft)



No. Increments = 10



Reach (ft)



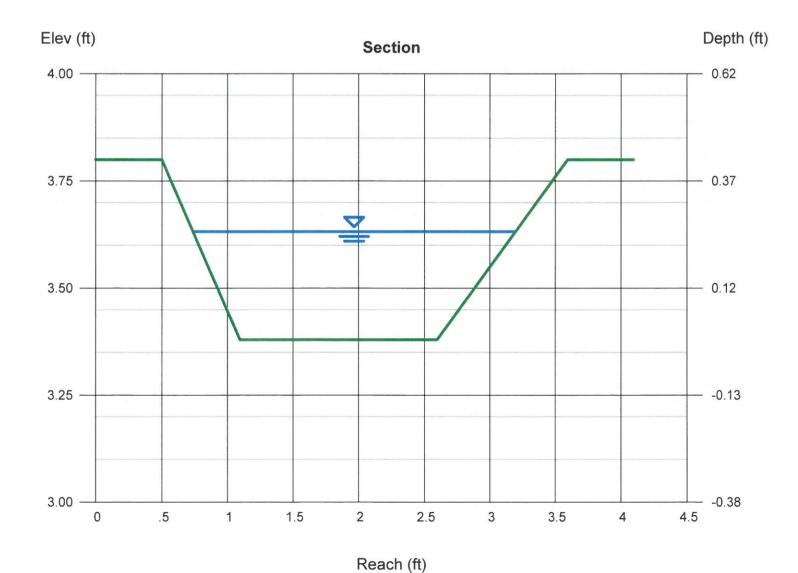
Depth	Q	Area	Veloc	Wp	Yc	TopWidth	Energy
(ft)	(cfs)	(sqft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
0.05	0.095	0.084	1.13	1.69	0.05	1.66	0.07
0.11	0.303	0.176	1.73	1.88	0.11	1.81	0.15
0.16	0.604	0.276	2.19	2.07	0.17	1.97	0.23
0.21	0.990	0.385	2.57	2.27	0.23	2.13	0.32
0.27	1.460	0.501	2.91	2.46	0.29	2.28	0.40
0.32	2.014	0.627	3.21	2.65	0.35	2.44	0.48
0.37	2.652	0.760	3.49	2.84	0.41	2.60	0.56
0.42	3.378	0.902	3.74	3.03	0.47	2.76	0.64
0.48	4.191	1.052	3.98	3.22	0.53	2.91	0.72
0.53	5.096	1.211	4.21	3.42	0.53	3.07	0.81

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Section E

	Highlighted		
= 1.50	Depth (ft)	=	0.25
= 1.42, 2.38	Q (cfs)	=	1.398
= 0.42	Area (sqft)	=	0.50
= 3.38	Velocity (ft/s)	=	2.80
= 2.00	Wetted Perim (ft)	=	2.59
= 0.025	Crit Depth, Yc (ft)	=	0.27
	Top Width (ft)	=	2.46
	EGL (ft)	=	0.37
Q vs Depth			
= 10			
	= 1.42, 2.38 = 0.42 = 3.38 = 2.00 = 0.025	= 1.50	= 1.50



SECTION E

Depth (ft)	Q (cfs)	Area (sqft)	Veloc (ft/s)	Wp (ft)	Yc (ft)	TopWidth (ft)	Energy (ft)
0.08	0.208	0.139	1.49	1.86	0.09	1.82	0.12
0.13	0.415	0.219	1.90	2.04	0.13	1.98	0.18
0.17	0.683	0.306	2.24	2.23	0.18	2.14	0.25
0.21	1.011	0.399	2.53	2.41	0.22	2.30	0.31
0.25	1.398	0.499	2.80	2.59	0.27	2.46	0.37
0.29	1.845	0.605	3.05	2.77	0.32	2.62	0.44
0.34	2.354	0.719	3.28	2.95	0.37	2.78	0.50
0.38	2.926	0.838	3.49	3.13	0.41	2.94	0.57
0.42	3.563	0.965	3.69	3.31	0.42	3.10	0.63

