CITY OF ALBUQUERQUE

Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller

November 5, 2021

Dean Cardwell, P.E. Bohler 6017 Main St. Frisco, TX 75034

RE: KABQ Cargo Facility 3724 Spirit Dr. SE Grading and Drainage Plan Stamp Date: 11/5/21 Drainage File: P15D004

Dear Mr. Cardwell:

Based on the submittal received on 11/5/21 the above-referenced Grading and Drainage Plan is approved for Building Permit by Hydrology.

Albuquerque As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

NM 87103

www.cabq.gov

If you have any questions, please contact me at 924-3986 or earmijo@cabq.gov.

Sincerely,

Ernest Armijo, P.E. Principal Engineer, Planning Dept. Development Review Services

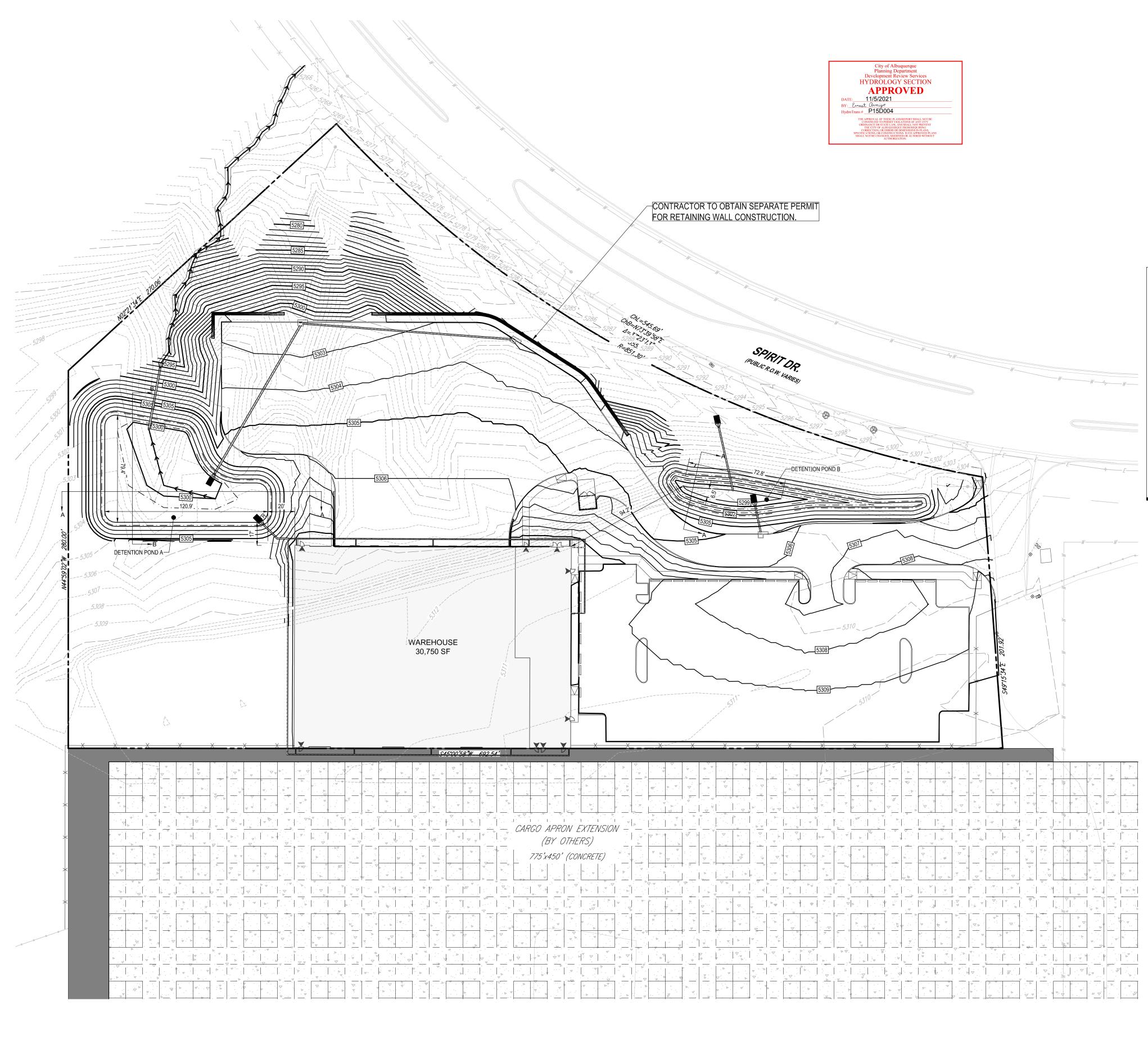


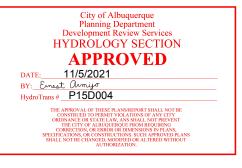
City of Albuquerque

Planning Department Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title:	_ Building Perm	nit #: 2021-43107	Hydrology File #:
DRB#: A 5.00 ACRE PARCEL KNOW TRACT A-1, SUNPORT MUN	AS A PORTION OF	TRACT A-1 OF SUNPOR	T MUNICIPAL GROUND LEASE WITHIN THE
City Address: 2200 Sunport Boulevard, Albu	uquerque, NM 87	7106	
Applicant: Bohler			Contact: Dean Cardwell
Address: 6017 Main Street, Frisco TX 7503			
Phone#: 469-458-7300			
Other Contact: Method Architecture			_ Contact: Tom Bartoo
Address: 2140 Rossville Avenue, Suite 101,			
Phone#: 423-718-8663	Fax#:		E-mail: tbartoo@method-architecture.com
TYPE OF DEVELOPMENT: PLAT	(# of lots)	_ RESIDENCE	DRB SITE ADMIN SITE
IS THIS A RESUBMITTAL? Yes	No		
DEPARTMENT TRANSPORTATION	X HYDE	ROLOGY/DRAINAGE	
Check all that Apply: TYPE OF SUBMITTAL: ENGINEER/ARCHITECT CERTIFICATIO PAD CERTIFICATION CONCEPTUAL G & D PLAN X GRADING PLAN DRAINAGE REPORT DRAINAGE MASTER PLAN FLOODPLAIN DEVELOPMENT PERMIT ELEVATION CERTIFICATE CLOMR/LOMR TRAFFIC CIRCULATION LAYOUT (TCI TRAFFIC IMPACT STUDY (TIS) STREET LIGHT LAYOUT OTHER (SPECIFY)	APPLIC .)	X BUILDING PE CERTIFICATE PRELIMINAR SITE PLAN FO SITE PLAN FO FINAL PLAT SIA/ RELEAS FOUNDATION X GRADING PE SO-19 APPRO PAVING PER GRADING/ PA WORK ORDER CLOMR/LOM	E OF OCCUPANCY Y PLAT APPROVAL OR SUB'D APPROVAL OR BLDG. PERMIT APPROVAL APPROVAL E OF FINANCIAL GUARANTEE N PERMIT APPROVAL ERMIT APPROVAL OVAL MIT APPROVAL AD CERTIFICATION & APPROVAL
11/5/2021		OTHER (SPEC	CIFY)
DATE SUBMITTED: <u>18/40/2024</u>		r (Dean Cardwell)	
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THE ABOVE DESCRIBED LEASE CONTAINS 5 MORE OR LESS.

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THE PROJECT IS LOCATED ON A 5-ACRE LEASE TRACT WITHIN THE SUNPORT AIRPORT PROPERTY AND IS ON THE SOUTH SIDE OF SPIRIT DRIVE ADJACENT TO SECURITY GATE F-1F. IT WAS USED PREVIOUSLY AS AN AIRPORT VIEWING AREA AND HAS A SMALL GRAVEL DRIVE AND PARKING LOT WHICH ARE LOCATED ON A 'FLAT' AREA ON TOP OF A LARGE SLOPED HILLSIDE DOWN TO SPIRIT DRIVE THE GRADES OF WHICH EXCEED 20% WITH A VERTICAL DROP OF APPROXIMATELY 30'-50'. THE PROJECT IS LOCATED IN ZONE 2 WITH AREAS IN LAND TREATMENT AREAS C AND D. THE EXISTING STORMWATER DISCHARGES UNDETAINED TO A ROADSIDE SWALE ON THE SOUTH SIDE OF SPIRIT DRIVE WHICH IS DIRECTED TO AN INLET NEAR THE INTERSECTION OF SPIRIT DRIVE AND UNIVERSITY BOULEVARD. IN REVIEW OF THE PREVIOUSLY APPROVED DRAINAGE REPORT FOR THE ABQ AIRPORT THE PROJECT IS LOCATED IN SUB-BASIN #1202 WHICH IS THE HEADWATERS FOR DRAINAGE BASIN UE AND HAS A MAXIMUM ALLOWABLE RELEASE RATE OF 2.06 CFS/ACRE FOR THE 100-YEAR 6-HOUR STORM EVENT.

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GRADING NOTES:

- 1. SEE PLAN SHEET C-701 FOR DETENTION POND CROSS-SECTIONS.
- (MUST MEET CGP 2.2.14b).
- MINIMAL FLOOD HAZARD).
- 4. ALL CURBS ARE 6" TALL UNLESS NOTED OTHERWISE.



VICINITY MAP **IDO ZONE ATLAS NUMBER P-15**

SCALE: 1" : 1000'

IECEND

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OF THE LEASE, ATELY	PROPERTY LINE/LEASE LINE	
1 SURVEY ALCULATED A	PROPOSED CONTOURS	85
54' TO THE	MAJOR EXISTING CONTOUR	<i> 85</i>
44°59'41"W A	MINOR EXISTING CONTOUR	84
IDENTIFIED	GROUND SPOT ELEVATION	<u> </u>
2°20'55"E A CORNER	LOW/HIGH POINT ELEVATION	HP 83.00 LP 83.00
VITH A DELTA TH OF 555.44'	MATCH EXST. GRADE	MATCH EX. G 527.1+/-
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IORTHEAST	DRAINAGE ARROW	
5.00 ACRES	SWALE	$\rightarrow \rightarrow \rightarrow \rightarrow$

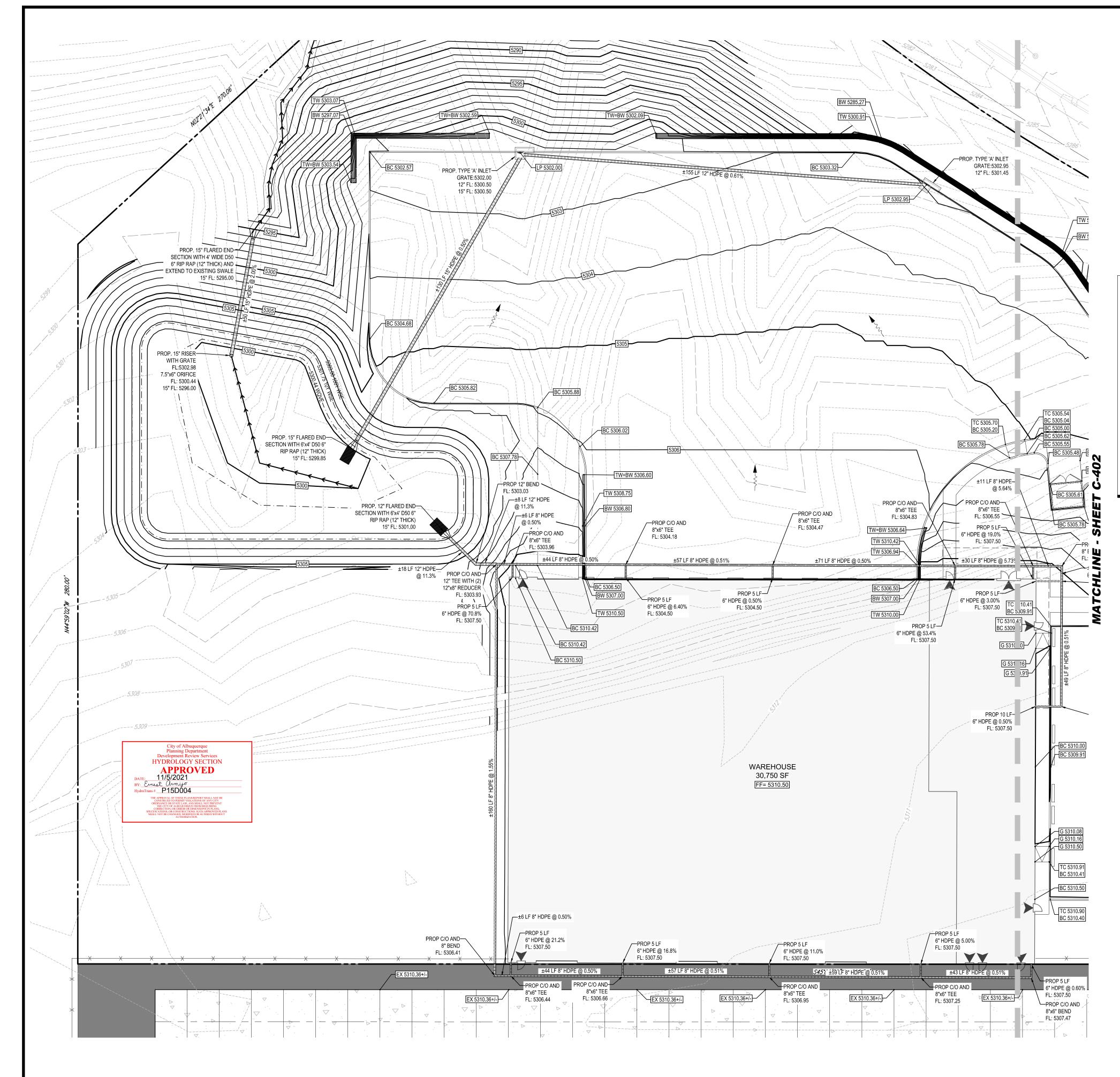
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FEDERAL EMERGENCY MANAGEMENT AGENCY, FEMA FIRMETTE PUBLISHED 09/14/2021. REFERENCING FLOOD INSURANCE RATE MAP, MAP NUMBER 35001C0344G EFFECTIVE DATE 09/26/2008, INDICATES THIS PARCEL OF LAND IS LOCATED IN ZONE X (AREA OF

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DEPARTMENT OF MU ENGINEER	DRAWN BY: AAA CHECKED BY: DOC DATE 11/2021 BUQUERQUE NICIPAL DEVELOPMENT ING DIVISION

OVERALL DRAINAGE PLAN DESIGN REVIEW COMMITTEE CITY ENGINEER APPROVAL ZONE MAP NO. NR-SU CITY PROJECT NO. 000000 SHEET NO. C - 300





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54' TO THE	MAJOR EXISTING CONTOUR	<i> 85</i>
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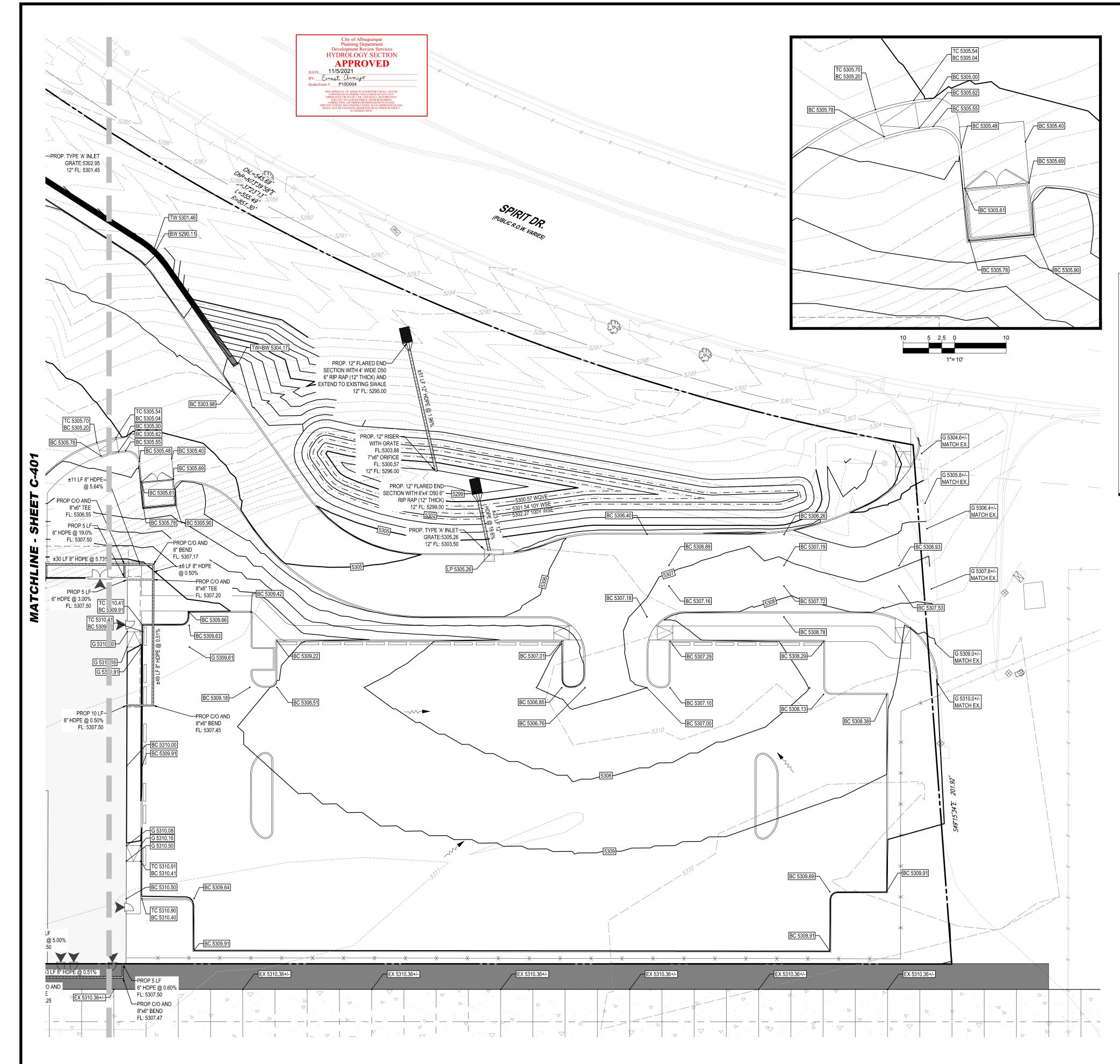
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(1 OF 2) DESIGN REVIEW COMMITTEE CITY ENGINEER APPROVAL ZONE MAP NO. NR-SU CITY PROJECT NO. 000000 SHEET NO. C-301





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BASIN AREA A: THIS AREA GENERALLY INCLUDES THE BUILDING, TRUCK COURT, TRUCK DOCKS AND THE MAIN ACCESS DRIVE WHICH IS COMPRISED OF APPROXIMATELY 2.54-ACRES OF THE 5.00-ACRE PROJECT AREA AND WHICH INCLUDES 0.82-ACRES OF TREATMENT C AND 1.72-ACRES OF TREATMENT D LAND AREAS. FOR THE 100-YEAR STORM EVENT THIS GENERATES 9.97 CFS OF STORMWATER FLOW AND REQUIRES 0.40 AC-FT OF DETENTION STORAGE IN POND A OF WHICH 0.059 AC-FT IS RESERVED FOR STORMWATER QUALITY. BASIN AREA B: THIS AREA GENERALLY INCLUDES THE PARKING LOT AND A PORTION OF THE MAIN ACCESS ROADWAY WHICH IS COMPRISED OF

APPROXIMATELY 1.09-ACRES OF THE 5.00-ACRE PROJECT AREA ALL OF WHICH IS TREATMENT D LAND AREA. FOR THE 100-YEAR STORM EVENT THIS GENERATES 4.75 CFS OF STORMWATER FLOW AND REQUIRES 0.21 AC-FT OF DETENTION STORAGE IN POND B OF WHICH 0.031 AC-FT IS RESERVED FOR STORMWATER QUALITY.

OFFSITE RUNOFF: THIS IS THE AREA IS LOCATED BETWEEN THE SITE IMPROVEMENTS AND SPIRIT DRIVE AND WILL GENERALLY CONTINUE TO RUNOFF TO THE ROADSIDE DRAINAGE SWALE ALONG SPIRIT DRIVE SIMILAR TO THE EXISTING CONDITION. IN TOTAL, THERE ARE FOUR SUB-BASINS CONTAINING APPROXIMATELY 1.09-ACRES OF THE 5.00-ACRE PROJECT AREA ALL OF WHICH IS TREATMENT D LAND AREA. FOR THE 100-YEAR STORM EVENT THIS GENERATES 4.18 CFS OF STORMWATER FLOW AND REQUIRES 0.12 AC-FT OF DETENTION STORAGE, OF WHICH THERE IS NO STORMWATER QUALITY REQUIREMENT AS THERE IS NO PERVIOUS AREA IN THIS BASIN. ADDITIONALLY, THERE IS NO DETENTION FACILITY FOR THIS

BASIN, THEREFORE, POND A AND POND B HAVE BEEN OVERSIZED TO ACCOUNT FOR THE VOLUME REQUIREMENTS OF THIS BASIN. ONE OTHER ITEM OF NOTE, THE PREVIOUSLY APPROVED STUDY FOR THE AIRPORT RESTRICTS THE RELEASE RATE FOR THIS SITE TO 2.06 CFS/ACRE, OR 10.30 CFS FOR THE 5-ACRE PROJECT. AS NOTED ABOVE, THE CALCULATED RUNOFF TOTALS 18.89 CFS. THIS HAS BEEN ACCOUNTED FOR IN THE POND ROUTING MODEL FOR POND A AND POND B AS PERFORMED WITH HEC_HMS.

GRADING NOTES:

- 1. SEE PLAN SHEET C-701 FOR DETENTION POND CROSS-SECTIONS.
- (MUST MEET CGP 2.2.14b).
- MINIMAL FLOOD HAZARD).
- 4. ALL CURBS ARE 6" TALL UNLESS NOTED OTHERWISE.



VICINITY MAP **IDO ZONE ATLAS NUMBER P-15**

SCALE: 1" : 1000'

IECENID

N:	LEG	END
OF THE LEASE, ATELY	PROPERTY LINE/LEASE LINE	
1 SURVEY ALCULATED A	PROPOSED CONTOURS	85
54' TO THE	MAJOR EXISTING CONTOUR	<i> 85</i>
44°59'41"W A	MINOR EXISTING CONTOUR	84
IDENTIFIED	GROUND SPOT ELEVATION	<u> </u>
2°20'55"E A CORNER	LOW/HIGH POINT ELEVATION	<u>HP 83.00</u> LP 83.00
VITH A DELTA TH OF 555.44'	MATCH EXST. GRADE	MATCH EX. G 527.1+/-
49°16'13"E A	TOP CURB / BOTTOM CURB	TC 83.00 BC 83.00
IORTHEAST	DRAINAGE ARROW	
5.00 ACRES	SWALE	$\rightarrow\rightarrow\rightarrow\rightarrow\rightarrow$

GRADING NARRATIVE

2. STABILIZE ALL POND SLOPES WITH NATIVE SEED AND AGGREGATE MULCH OR EQUAL

FEDERAL EMERGENCY MANAGEMENT AGENCY, FEMA FIRMETTE PUBLISHED 09/14/2021. REFERENCING FLOOD INSURANCE RATE MAP, MAP NUMBER 35001C0344G EFFECTIVE DATE 09/26/2008, INDICATES THIS PARCEL OF LAND IS LOCATED IN ZONE X (AREA OF

	CONSULTANTS	M			SITE CIVIL AND CONSULTING ENGINEERING			FERMIT LING SERVICES TRANSPORTATION SERVICES	THE INFORMATION DESIGNAND CONTERF OF THIS PLAVARE PROPERTIENVE VAIO SAUL MOT BE CORED ON LEED FOR ANY PLACES WITHOUT PRORE MATTER AUTHORIZATION FROM EXALER, ON V APPROVED SIGNED AND SEALED FLANS SHALL BE UTILIZED FOR CONSTRUCTION PLAPOSES.	
BEND			220	F A 50 S	Cl Sun	CA LI por e, N	TY t B	r Ivd,		
	BENCH MARKS	BM#1 TOP OF SANITARY	SEWEK MANHOLE ELEVATION = 5284.30'	BM#2 NO. 4 REBAR W/ CAP IN RAW LAND	NORTHING: 465944.03' EASTING: 386672.45' FLEVATION: 5298.69'	BM #3	NO. 4 REBAK W/ CAP IN KAW LANU NORTHING: 466213.90' FASTING: 386683 57'	ELEVATION: 5270.95	BM #4 NO. 4 REBAR W/CAP IN RAW LAND NORTHING: 466367.43'	EASTING: 387207.10' ELEVATION: 5304.16'
			PROS	ANTESS	24	AR NEX 039	W CO	CAREFO TO		1
E SOUTH SIDE OF SPIRIT DRIVE LL GRAVEL DRIVE AND PARKING LOT ADES OF WHICH EXCEED 20% WITH A MENT AREAS C AND D. THE EXISTING IS DIRECTED TO AN INLET NEAR THE INAGE REPORT FOR THE ABQ AIRPORT 5 A MAXIMUM ALLOWABLE RELEASE	SEAL				ВҮ	-	ü	ü	ü	ü
TH THE ADDITION OF A BUILDING, THAT THE STORMWATER RUNOFF IS TREAM AREAS WITHIN DRAINAGE ATER QUALITY EVENT. THE PROJECT IS RATE FOR THE PROJECT IS 18.89 CFS SINS AREAS WITH DETAILS AS ACCESS DRIVE WHICH IS COMPRISED TMENT C AND 1.72-ACRES OF DW AND REQUIRES 0.40 AC-FT OF					DESCRIPTION	CONTRACTOR:	DATE:	DATE	DATE	DATE
DWAY WHICH IS COMPRISED OF OR THE 100-YEAR STORM EVENT THIS OF WHICH 0.031 AC-FT IS RESERVED ILL GENERALLY CONTINUE TO RUNOFF ERE ARE FOUR SUB-BASINS AND AREA. FOR THE 100-YEAR STORM OF WHICH THERE IS NO IS NO DETENTION FACILITY FOR THIS NTS OF THIS BASIN. TE FOR THIS SITE TO 2.06 CFS/ACRE, OR BEEN ACCOUNTED FOR IN THE POND	CITY COMMENTS				DE			PTANCE BY:	I BY:	TED BY:
	1 10/18/21			D BY	NO. DATE	AS-BUILT INFORMATION	WORK STAKED BY:	INSPECTOR'S ACCEPTANCE	FIELD VERIFICATION BY	DRAWINGS CORRECTED
CALL NM ONE-CALL SYSTEM SEVEN (7) DAYS PRIOR TO ANY EXCAVATION	DR CH	AW	N B		Α	AA OC	02	1		
CITY OF A DEPARTMENT OF MU ENGINEEF	LBUC	QU IPA		DE	JE VE	/			IT	

(2 OF 2) DESIGN REVIEW COMMITTEE CITY ENGINEER APPROVAL ZONE MAP NO. NR-SU CITY PROJECT NO. 000000 SHEET NO. C-302

Existing Basin

											1	LOO-year			10-year	
		Area	Treatn	nent A	Treatr	nent B	Treatr	nent C	Treatn	nent D	Weighted E	Volume	Flow	Weighted E	Volume	Flow
Basin	Area (sf)	(acres)	(%)	(acres)	(%)	(acres)	(%)	(acres)	(%)	(acres)	(in)	(ac-ft)	(cfs)	(in)	(ac-ft)	(cfs)
Α	217,944	5.00	0.0%	0.00	0.0%	0.00	100.0%	5.00	0.0%	0.00	2.030	0.85	15.26	0.946	0.39	7.96

Proposed Basins

											100-year			10-year		
		Area	Treatr	nent A	Treatr	nent B	Treatn	nent C	Treatn	nent D	Weighted E	Volume	Flow	Weighted E	Volume	Flow
Basin	Area (sf)	(acres)	(%)	(acres)	(%)	(acres)	(%)	(acres)	(%)	(acres)	(in)	(ac-ft)	(cfs)	(in)	(ac-ft)	(cfs)
А	110,580	2.54	0.0%	0.00	0.0%	0.00	32.3%	0.82	67.8%	1.72	1.911	0.40	9.97	1.178	0.25	5.97
В	47,634	1.09	0.0%	0.00	0.0%	0.00	0.0%	0.00	100.0%	1.09	2.330	0.21	4.75	1.510	0.14	2.96
OFS	59,730	1.37	0.0%	0.00	0.0%	0.00	100.0%	1.37	0.0%	0.00	1.030	0.12	4.18	0.480	0.05	2.18
	Total =	5.00									Total =	0.73	18.89	Total =	0.44	11.11

Peak Discharge, Q (cfs/acre)Zone 2100-year10-year

 Qa
 1.71
 0.41

 Qb
 2.36
 0.95

 Qc
 3.05
 1.59

 Qd
 4.34
 2.71

Excess Preceipitation, E (in)Zone 2100-year10-year

Ea 0.62 0.15

Eb 0.80 0.30 Ec 1.03 0.48

Ed 2.33 1.51

Equations

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted E * Total Area

Flow = Qa*Aa + Qb*Ab + Qc*Ac + Qd*Ad

Stormwater Quality

	Impervious Area		Stormwater Quality	Stormwater Quality	
Basin	(sf)	SWQV (in)	Volume (cf)	Volume (ac-ft)	
A1	30,789	0.42	1,078	0.025	
A2	17,714	0.42	620	0.014	
A3	24,565	0.42	860	0.020	
A4	0	0.42	0	0.000	
Total =	73,067		2,557	0.059	0.060 Provided
B1	38,515	0.42	1,348	0.031	0.031 Provided
					City of Albuquerque
OS1	0	0.42	0	0.000	City of Albuquerque Planning Department Development Review Services
OS2	0	0.42	0	0.000	HYDROLOGY SECTION
OS3	0	0.42	0	0.000	APPROVED
OS4	1,840	0.42	64	0.001	BY: Errest Armijo HydroTrans #_P15D004
Total =	1,840		64	0.001	THE APPROVAL OF THESE PLANS/REPORT SHALL NOT BE CONSTRUED TO PERMIT VIOLATIONS OF ANY CITY

	Inlet A2	
Project Description		
Solve For	Spread	
Input Data		
Discharge	2.42 cfs	
Gutter Width	0.0 in	
Gutter Cross Slope	0.020 ft/ft	
Road Cross Slope	0.020 ft/ft	Dinin Dini Di Songin (jintresis) in 16 di
Grate Width	25.0 in	
Grate Length	3.3 ft	
Local Depression	0.0 in	
Local Depression Width	0.0 in	
Grate Type	P-50 mm (P-1 -7/8")	
Clogging	41.0 %	
Results		
Spread	15.1 ft	
Depth	3.6 in	
Gutter Depression	0.0 in	
Total Depression	0.0 in	
Open Grate Area	3.7 ft²	
Active Grate Weir Length	5.8 ft	

		Inlet A3
Project Description		
Solve For	Spread	
Input Data		
Discharge	3.35 cfs	
Gutter Width	0.0 in	
Gutter Cross Slope	0.020 ft/ft	
Road Cross Slope	0.020 ft/ft	
Grate Width	25.0 in	
Grate Length	3.3 ft	
Local Depression	0.0 in	
Local Depression Width	0.0 in	
Grate Type	P-50 mm (P-1 -7/8")	
Clogging	41.0 %	
Results		
Spread	18.3 ft	
Depth	4.4 in	
Gutter Depression	0.0 in	
Total Depression	-0.0 in	
Open Grate Area	3.7 ft ²	
Active Grate Weir Length	5.8 ft	

		12"' Full	Flow @ 0.50%
	Project Description		
	Friction Method	Manning Formula	
	Solve For	Full Flow Capacity	
	Input Data		
45.P S(XP)	Roughness Coefficient	0.010	
	Channel Slope	0.005 ft/ft	
	Normal Depth	12.0 in	
	Diameter	12.0 in	
	Discharge	3.27 cfs	
	Results		
	Discharge	3.27 cfs	
	Normal Depth	12.0 in	
	Flow Area	0.8 ft²	
	Wetted Perimeter	37.7 in	
	Hydraulic Radius	3.0 in	
	Top Width	0.0 in	
	Critical Depth	9.3 in	
	Percent Full	100.0 %	
	Critical Slope	0.006 ft/ft	
	Velocity	4.17 ft/s	
	Velocity Head	3.24 in	
	Specific Energy	1.27 ft	
	Froude Number	(N/A)	
	Maximum Discharge	3.52 cfs	
	Discharge Full	3.27 cfs	
	Slope Full	0.005 ft/ft	
	Flow Type	Supercritical	
		Supercificati	
	GVF Input Data		
	Downstream Depth	0.0 in	
	Length	0.0 in	
	Number Of Steps	0	
	GVF Output Data		
	Upstream Depth	0.0 in	
	Profile Description	N/A	
	Profile Headloss	0.00 ft	
	Average End Depth Over Rise	0.0 %	
	Normal Depth Over Rise	100.0 %	
	Downstream Velocity	Infinity ft/s	
	Upstream Velocity	Infinity ft/s	
	Normal Depth	12.0 in	
	Critical Depth	9.3 in	
	Channel Slope	0.005 ft/ft	
	Critical Slope	0.006 ft/ft	

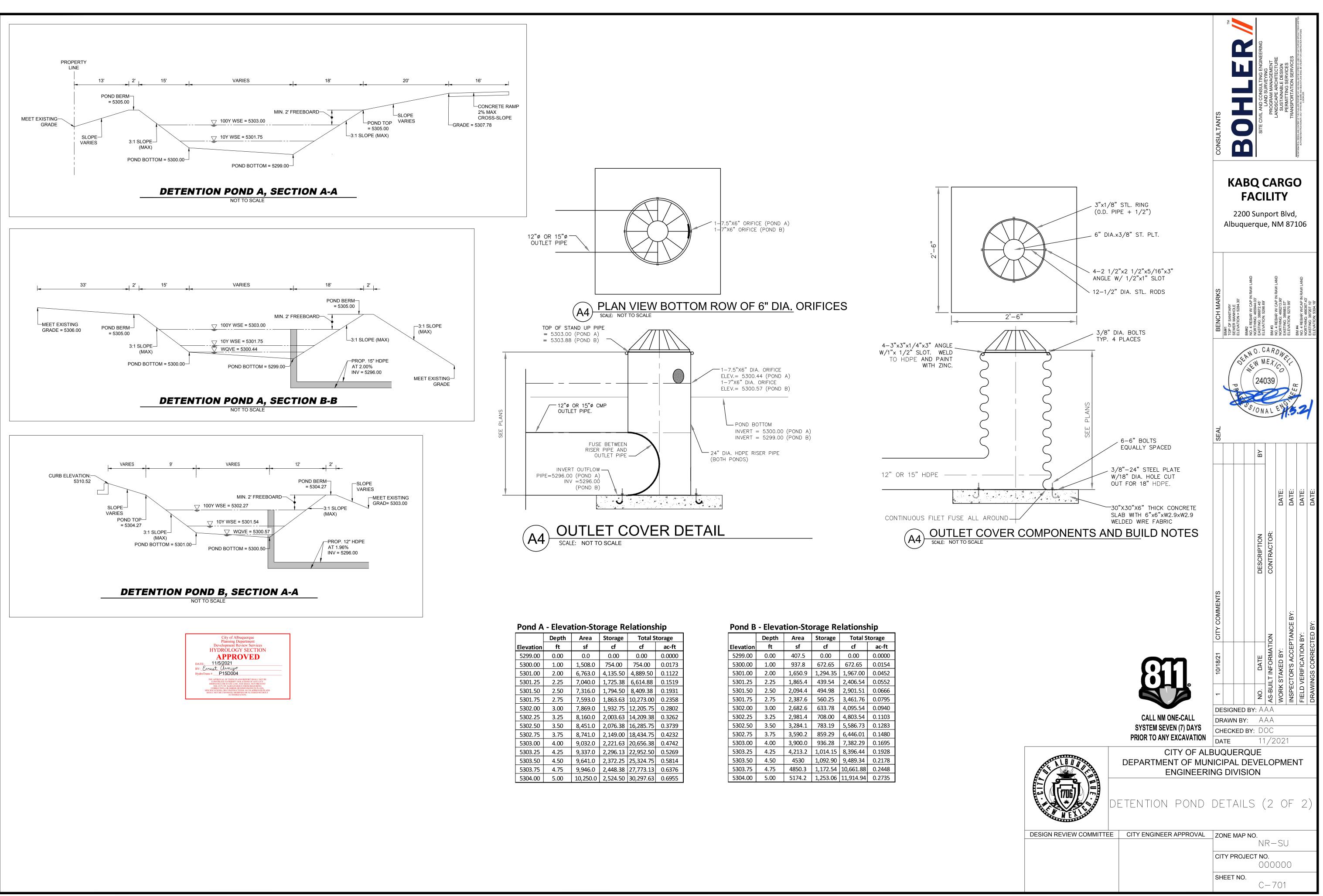
-			15"
1	Project Desc	ription	
n	Friction Met	nod	Manning Formula
	Solve For		Full Flow Capacity
	Input Data		
2 -	Roughness (Coefficient	0.010
1	Channel Slop	be	0.005 ft/1
	Normal Dept	th	15.0 in
.1	Diameter		15.0 in
	Discharge		5.94 cfs
	Results		
1	Discharge		5.94 cfs
-	Normal Dept	th	15.0 in
	Flow Area		1.2 ft ²
	Wetted Perir	neter	47.1 in
<i>3</i>	Hydraulic Ra	dius	3.8 in
	Top Width		0.0 in
-0	Critical Dept	h	11.8 in
5	Percent Full		100.0 %
	Critical Slope	2	0.005 ft/i
	Velocity		4.84 ft/s
	Velocity Hea	d	4.37 in
	Specific Ener	rgy	1.61 ft
	Froude Num	ber	(N/A)
	Maximum Di	scharge	6.39 cfs
	Discharge Fu	111	5.94 cfs
	Slope Full		0.005 ft/1
	Flow Type		Supercritical
	GVF Input D	ata	
	Downstream	Depth	0.0 in
	Length	-	0.0 in
	Number Of S	Steps	0
	GVF Output	Data	
	Upstream De		0.0 in
	Profile Descr		N/A
	Profile Head	-	0.00 ft
		Depth Over Rise	0.0 %
	Normal Dept		100.0 %
	Downstream		Infinity ft/s
	Upstream Ve	-	Infinity ft/
	Normal Dept		15.0 in
	Critical Dept		11.8 in
	Channel Slop		0.005 ft/1
	Critical Slope		0.005 ft/i

	Pe	ond A Orifice	
Project Description			
Solve For	Discharge		
Input Data			
Headwater Elevation Centroid Elevation Tailwater Elevation Discharge Coefficient Opening Width Opening Height	60.00 in 3.00 in 0.00 in 0.610 7.5 in 6.0 in		
Results			
Discharge Headwater Height Above Centroid Tailwater Height Above	3.33 cfs 57.00 in -3.00 in		
Centroid Flow Area Velocity	0.3 ft² 10.66 ft/s		

Project Description	
Solve For	Discharge
Input Data	
Headwater Elevation	48.00 in
Centroid Elevation	3.00 in
Tailwater Elevation	0.00 in
Discharge Coefficient	0.610
Opening Width	7.0 in
Opening Height	6.0 in
Results	
Discharge	2.76 cf
Headwater Height Above Centroid	45.00 in
Tailwater Height Above Centroid	-3.00 in
Flow Area	0.3 ft
Velocity	9,48 ft

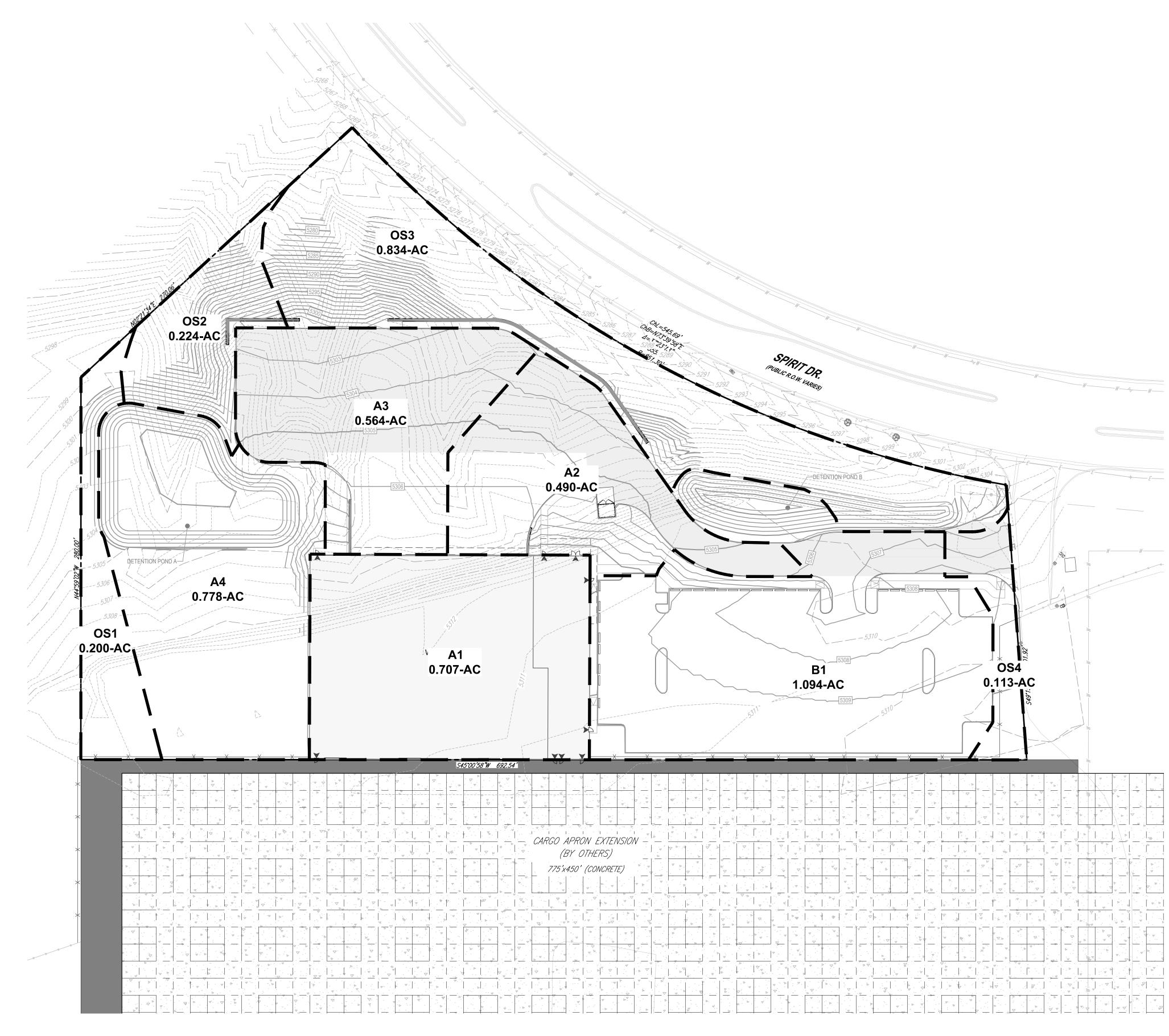
5		Inlet B1
Project Description		
Solve For	Spread	
Input Data		
Discharge	4.75 cfs	
Gutter Width	0.0 in	
Gutter Cross Slope	0.020 ft/ft	
Road Cross Slope	0.020 ft/ft	
Grate Width	25.0 in	
Grate Length	3.3 ft	
Local Depression	0.0 in	
Local Depression Width	0.0 in	
Grate Type	P-50 mm (P-1 -7/8")	
Clogging	41.0 %	
Results		
Spread	22.7 ft	
Depth	5.4 in	
Gutter Depression	0.0 in	
Total Depression	0.0 in	
Open Grate Area	3.7 ft ²	
Active Grate Weir Length	5.8 ft	

				M		Ű				JRPOSE WITHOUT PRIOR WRITTEN LUCTION PURPOSES	
" Full Flow	2 @ 0.50%					SITE CIVIL AND CONSULTING ENGINEERING	LANU SURVEYING PROGRAM MANAGEMENT ANDSCADE ADCHITECTI IDE	SUSTAINABLE DESIGN	TRANSPORTATION SERVICES	THE INFORMATION, DESIGN AND CONTENT OF THE PARAM REFEARCH AND SHALL NOT BE CORED ON USED FOR AN PLATED WITHIN THE INFORMATION PURCHARTERS, AND FUNCTION PURCHARTERS, AND FUNCH	
t/ft n n cfs			CONSULTANTS			SITE CIVIL AND CC		SUSTAI	TRANSPOR	DESIGN AND CONTENT OF THIS PLAN ARE PROPRIE ORIZATION FROM BOHLER. ONLY APPROVED, SIGNE © BOHL	
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n			ĸĸs		N RAW LAND					N RAW LAND	
n tu %			BENCH MARKS	TOP OF SANITARY SEWER MANHOLE ELEVATION = 5284.30'	BM#2 NO. 4 REBAR W/ CAP IN RAW LAND	RUCK I TING: 403944.03 EASTING: 386672.45 ELEVATION: 5298.69	BM #3 NO 4 DERAD W// CAD II	NOLTHING: 466213.90' NORTHING: 466213.90' EASTING: 386683.57'	ELEVATION: 5270.95' BM #4	NO. 4 REBAR W/CAP IN NORTHING: 466367.43'	EASTING: 387207.10' ELEVATION: 5304.16'
% t/s t/s n t/ft				/	AN	D. C		ni			
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						DESCRIPTION	CONTRACTOR				
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t/s		<u> </u>	CITY COMMENTS				NOI.		PTANCE BY:	I BY:	TED BY:
		81.	10/18/21			DATE	AS-BUILT INFORMATION	WORK STAKED BY:	NSPECTOR'S ACCEPTANCE	FIELD VERIFICATION BY:	DRAWINGS CORRECTED
		CALL NM ONE-CALL		BIGNEI			4 A	WORI	INSPE	FIELD	DRAV
		SYSTEM SEVEN (7) DAYS PRIOR TO ANY EXCAVATION		AWN B ECKED E		D	АА ЭС /2	02	1		
		CITY OF AL DEPARTMENT OF MUI ENGINEER	VICI	PAL	DE۱	/El	_0	PM	EN	T	
		DETENTION POND	DE	ETA		$\hat{}$	(1	C)F	2	2)
	DESIGN REVIEW COMMITTEE	E CITY ENGINEER APPROVAL	ZON	NE MA	P NC		R-	SL	J		
				Y PRO) . 00	00	0		
			SHE	EET NO).	С	-7	00)		

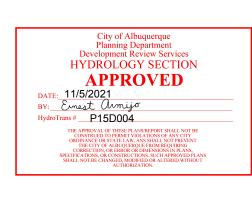


	Depth Area Storage Total Stora			torage		
Elevation	ft	sf	cf	cf	ac-ft	
5299.00	0.00	0.0	0.00	0.00	0.0000	
5300.00	1.00	1,508.0	754.00	754.00	0.0173	
5301.00	2.00	6,763.0	4,135.50	4,889.50	0.1122	
5301.25	2.25	7,040.0	1,725.38	6,614.88	0.1519	
5301.50	2.50	7,316.0	1,794.50	8,409.38	0.1931	
5301.75	2.75	7,593.0	1,863.63	10,273.00	0.2358	
5302.00	3.00	7 <i>,</i> 869.0	1,932.75	12,205.75	0.2802	
5302.25	3.25	8,160.0	2,003.63	14,209.38	0.3262	
5302.50	3.50	8,451.0	2,076.38	16,285.75	0.3739	
5302.75	3.75	8,741.0	2,149.00	18,434.75	0.4232	
5303.00	4.00	9,032.0	2,221.63	20,656.38	0.4742	
5303.25	4.25	9,337.0	2,296.13	22,952.50	0.5269	
5303.50	4.50	9,641.0	2,372.25	25,324.75	5 0.5814	
5303.75	4.75	9,946.0	2,448.38	27,773.13	0.6376	
5304.00	5.00	10,250.0	2,524.50	30,297.63	0.6955	

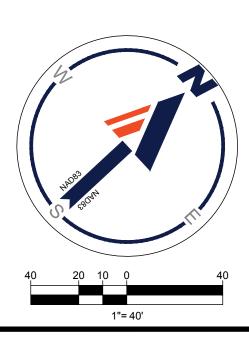
Pond B - Elevation-Storage Relationship							
	Depth	Area	Storage	Total S	torage		
Elevation	ft	sf	cf	cf	ac-ft		
5299.00	0.00	407.5	0.00	0.00	0.0000		
5300.00	1.00	937.8	672.65	672.65	0.0154		
5301.00	2.00	1,650.9	1,294.35	1,967.00	0.0452		
5301.25	2.25	1,865.4	439.54	2,406.54	0.0552		
5301.50	2.50	2,094.4	494.98	2,901.51	0.0666		
5301.75	2.75	2,387.6	560.25	3,461.76	0.0795		
5302.00	3.00	2,682.6	633.78	4,095.54	0.0940		
5302.25	3.25	2,981.4	708.00	4,803.54	0.1103		
5302.50	3.50	3,284.1	783.19	5,586.73	0.1283		
5302.75	3.75	3,590.2	859.29	6,446.01	0.1480		
5303.00	4.00	3,900.0	936.28	7,382.29	0.1695		
5303.25	4.25	4,213.2	1,014.15	8,396.44	0.1928		
5303.50	4.50	4530	1,092.90	9,489.34	0.2178		
5303.75	4.75	4850.3	1,172.54	10,661.88	0.2448		
5304.00	5.00	5174.2	1,253.06	11,914.94	0.2735		



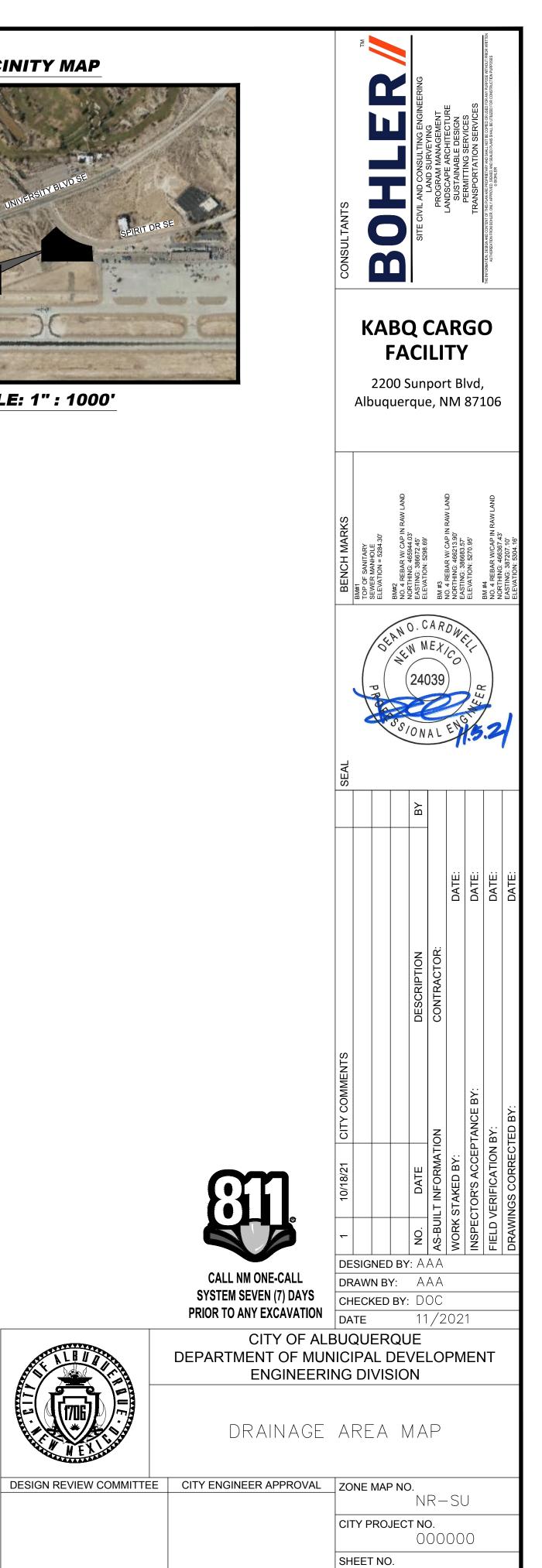




PETE)			



SCALE: 1" : 1000'



C-800