CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



Mayor Timothy M. Keller

October 1, 2024

Dave Thompson, P.E. Thompson Engineering Consultants P O Box 65760 Albuquerque, NM 87193

RE: Roses SW Papers TR D-6 PLAT OF TRACTS D-1 THRU D-7 MESA DEL SOL INNOVATIONPARK II Revised Grading and Drainage Plan Engineer's Stamp Date: 9/16/2024 Hydrology File: Q16DA5000C

Dear Mr. Thompson:

Based upon the information provided in your submittal received 9/19/2024, the Grading Plan is approved for Building Permit. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter.

PO Box 1293

PRIOR TO CERTIFICATE OF OCCUPANCY:

- Albuquerque 1. Engineer's Certification, per the DPM Part 6-14 (F): *Engineer's Certification Checklist For Non-Subdivision* is required.
 - 2. Please provide the executed paper Drainage Covenant (latest revision) printed on one-side only with Exhibit A and a check for **\$25.00** made out to "**Bernalillo County**" for the stormwater quality pond per Article 6-15(C) of the DPM to Hydrology for review.

www.cabq.gov

NM 87103

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 505-924-3695 or tchen@cabq.gov.

Sincerely,

Teque Cha

Tiequan Chen, P.E. Principal Engineer, Hydrology Planning Department, Development Review Services



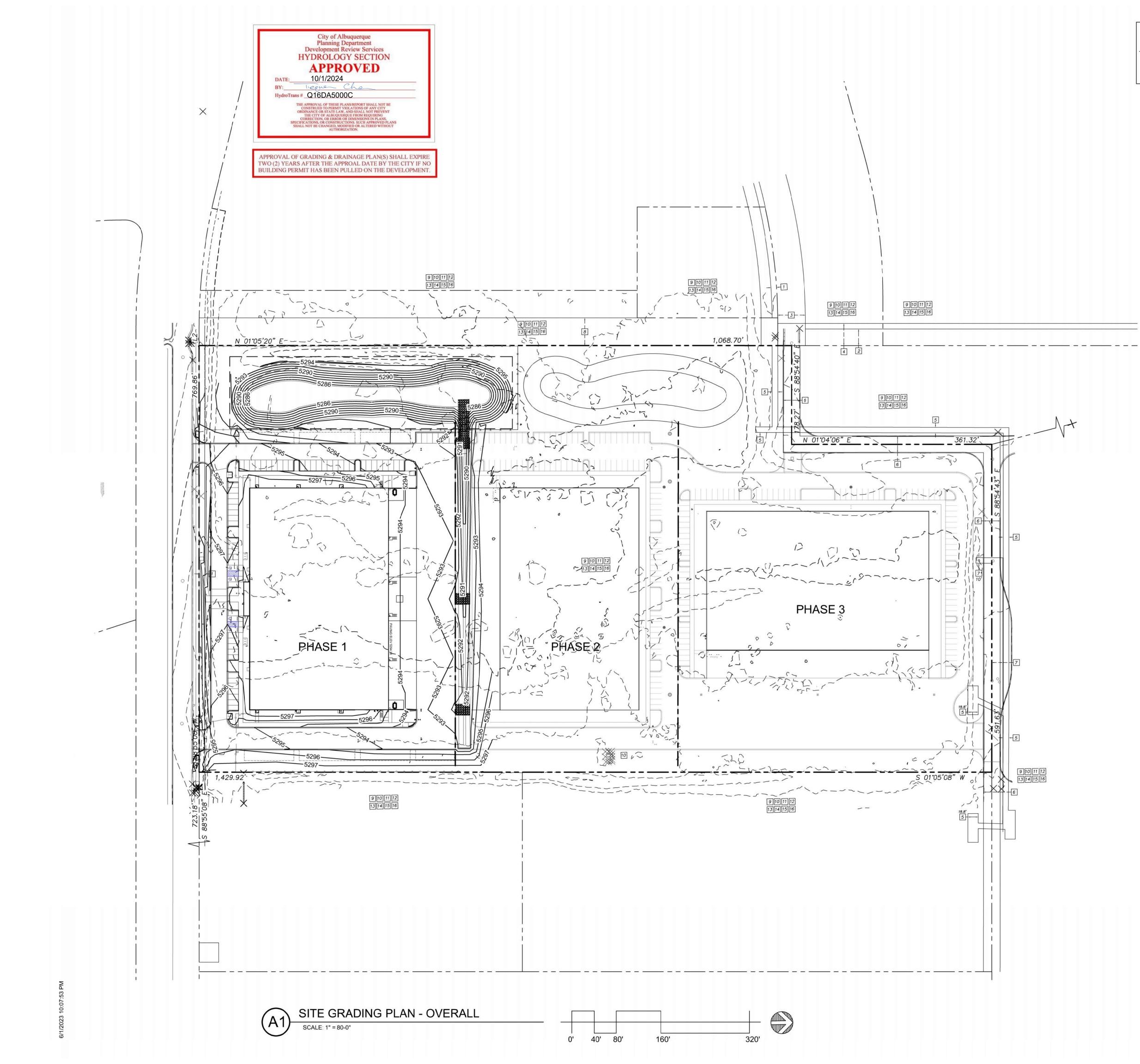
City of Albuquerque

Planning Department Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (DTIS)

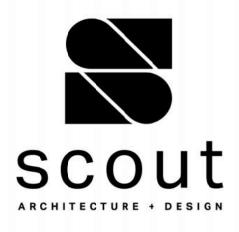
Project Title:	Hydrology File #
egal Description:	
City Address, UPC, OR Parcel:	
Applicant/Agent:	Contact:
	Phone:
Email:	
Applicant/Owner:	Contact:
Address:	Phone:
Email:	
(Please note that a DFT SITE is one that need	ds Site Plan Approval & ADMIN SITE is one that does not need it.)
TYPE OF DEVELOPMENT: PLAT	(#of lots) RESIDENCE
DFT	SITE ADMIN SITE
RE-SUBMITTAL: YES NO	
DEPARTMENT: TRANSPORTA	TION HYDROLOGY/DRAINAGE
Check all that apply under Both the Type	of Submittal and the Type of Approval Sought:
TYPE OF SUBMITTAL:	TYPE OF APPROVAL SOUGHT:
ENGINEER/ARCHITECT CERTIFICA	TION BUILDING PERMIT APPROVAL
PAD CERTIFICATION	CERTIFICATE OF OCCUPANCY
CONCEPTUAL G&D PLAN	CONCEPTUAL TCL DFT APPROVAL
GRADING & DRAINAGE PLAN	PRELIMINARY PLAT APPROVAL
DRAINAGE REPORT	FINAL PLAT APPROVAL
DRAINAGE MASTER PLAN	SITE PLAN FOR BLDG PERMIT DFT
CLOMR/LOMR	APPROVAL
TRAFFIC CIRCULATION LAYOUT (7	SIA/RELEASE OF FINANCIAL GUARANTEE
ADMINISTRATIVE	FOUNDATION PERMIT APPROVAL
TRAFFIC CIRCULATION LAYOUT F APPROVAL	OR DFT GRADING PERMIT APPROVAL
TRAFFIC IMPACT STUDY (TIS)	SO-19 APPROVAL
STREET LIGHT LAYOUT	PAVING PERMIT APPROVAL
OTHER (SPECIFY)	GRADING PAD CERTIFICATION
omer(billen i)	WORK ORDER APPROVAL
	CLOMR/LOMR
	OTHER (SPECIFY)

DATE SUBMITTED: ____



GENERAL NOTES

- A. ALL DIMENSIONS ARE FACE OF CURB UNLESS OTHERWISE NOTED.B. FIELD VERIFY ALL DIMENSIONS.
- C. DO NOT SCALE DRAWINGS, IF DIMENSIONS ARE IN QUESTION, REQUEST CLARIFICATION FROM ARCHITECT BEFORE PROCEEDING.



ARCHITECT/ ENGINEER



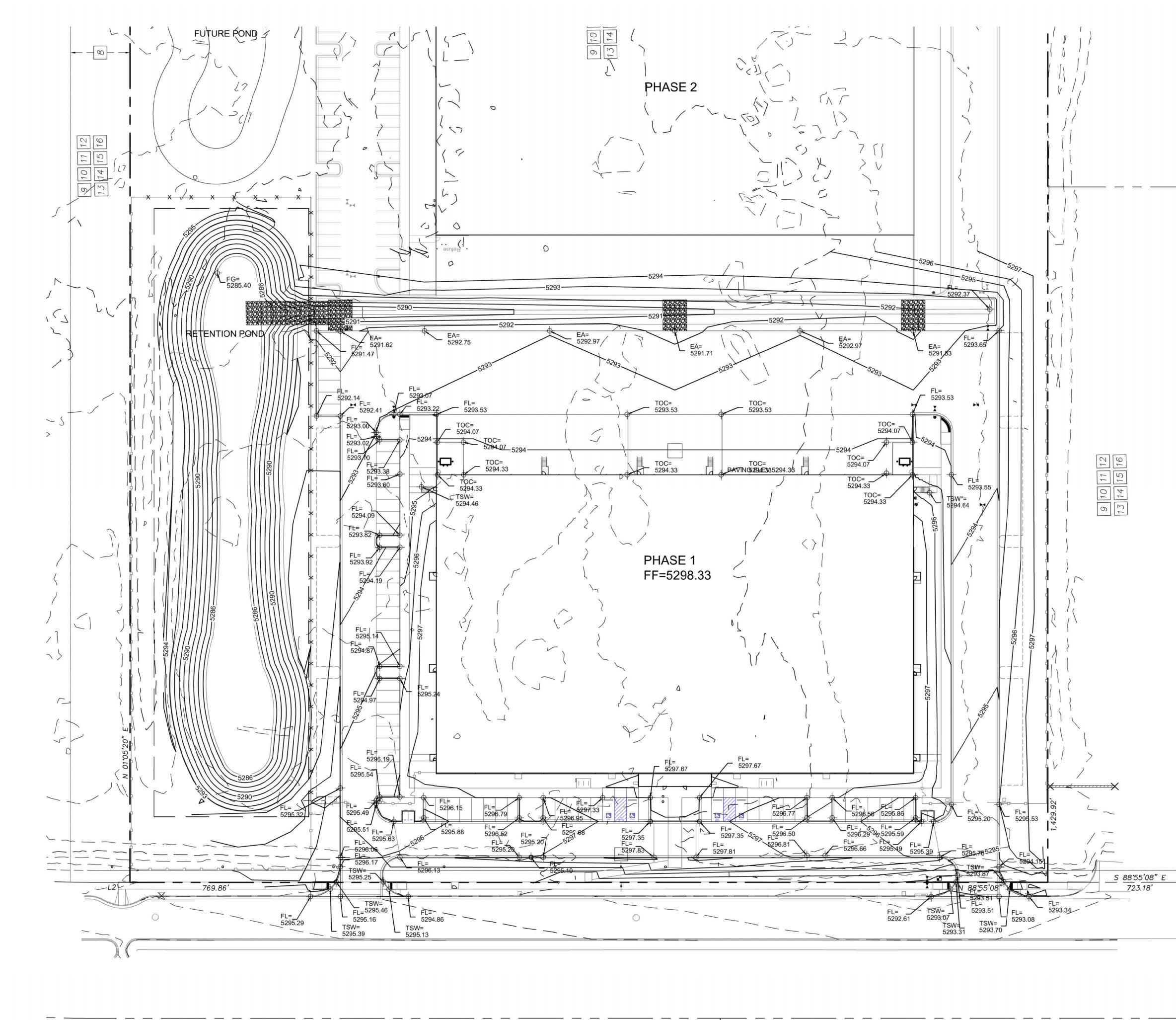
CRICK CROSSING

ROSES SOUTHWEST PAPERS

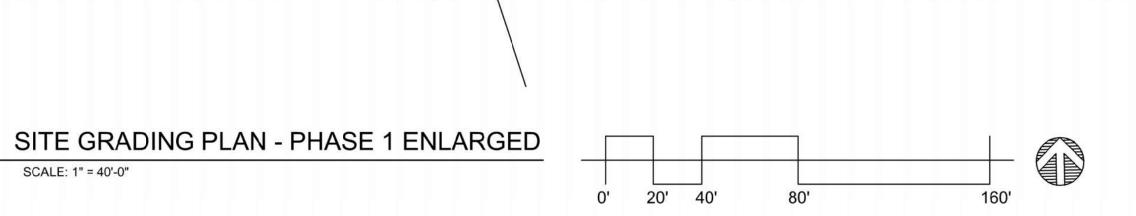
BUILDING PERMIT SET

REVISIO	ON	DATE
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DATE		10/16/23
PROJE	CT NO	
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SHEET	NO.	
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Tho E	mpson ngineering Consultants, Inc.
	tecnm@yahoo.com
P.O. BOX 65760 ALBUQUERQUE, NM 87193	PHONE: (505) 271-2199 FAX: (505) 830-9248



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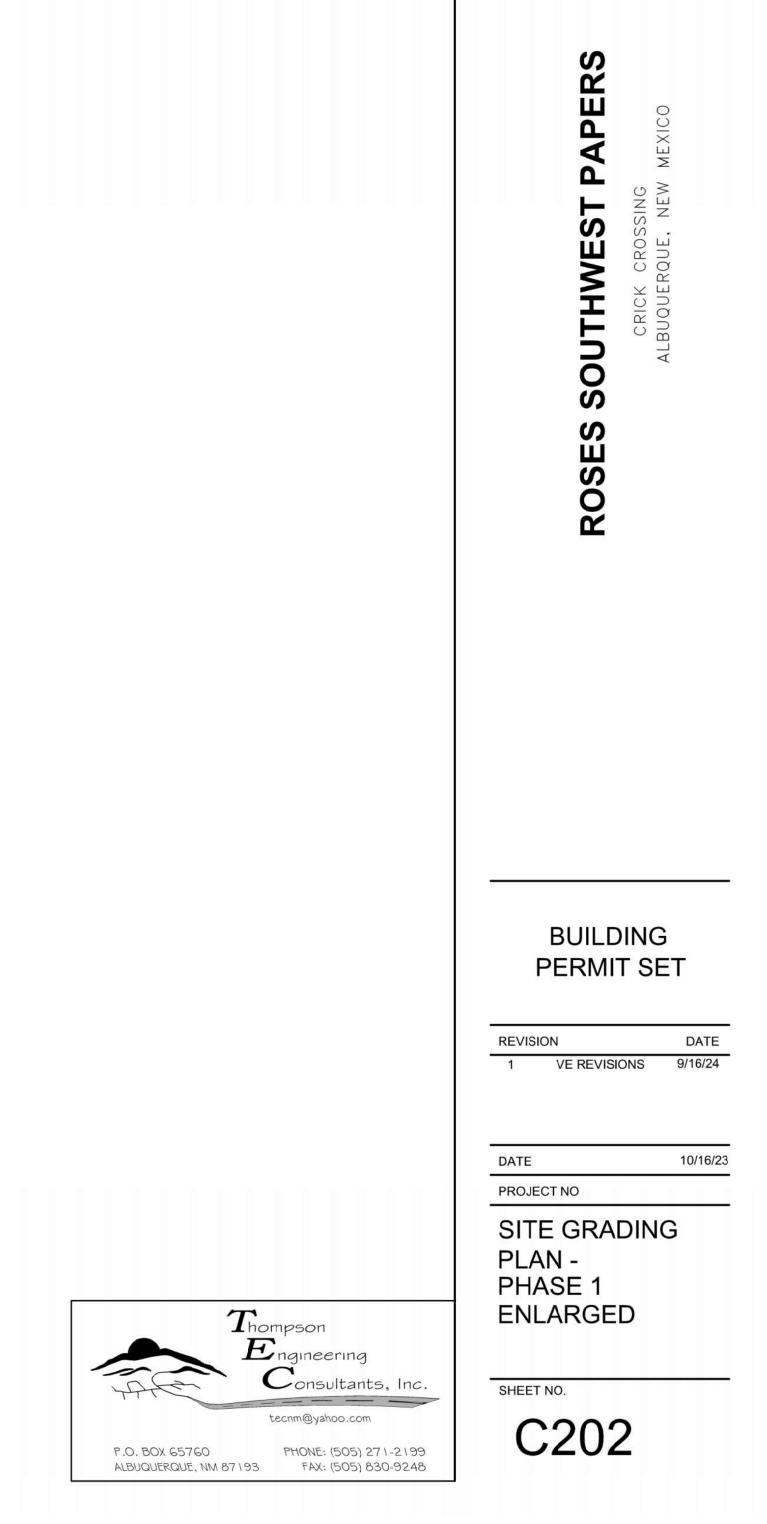


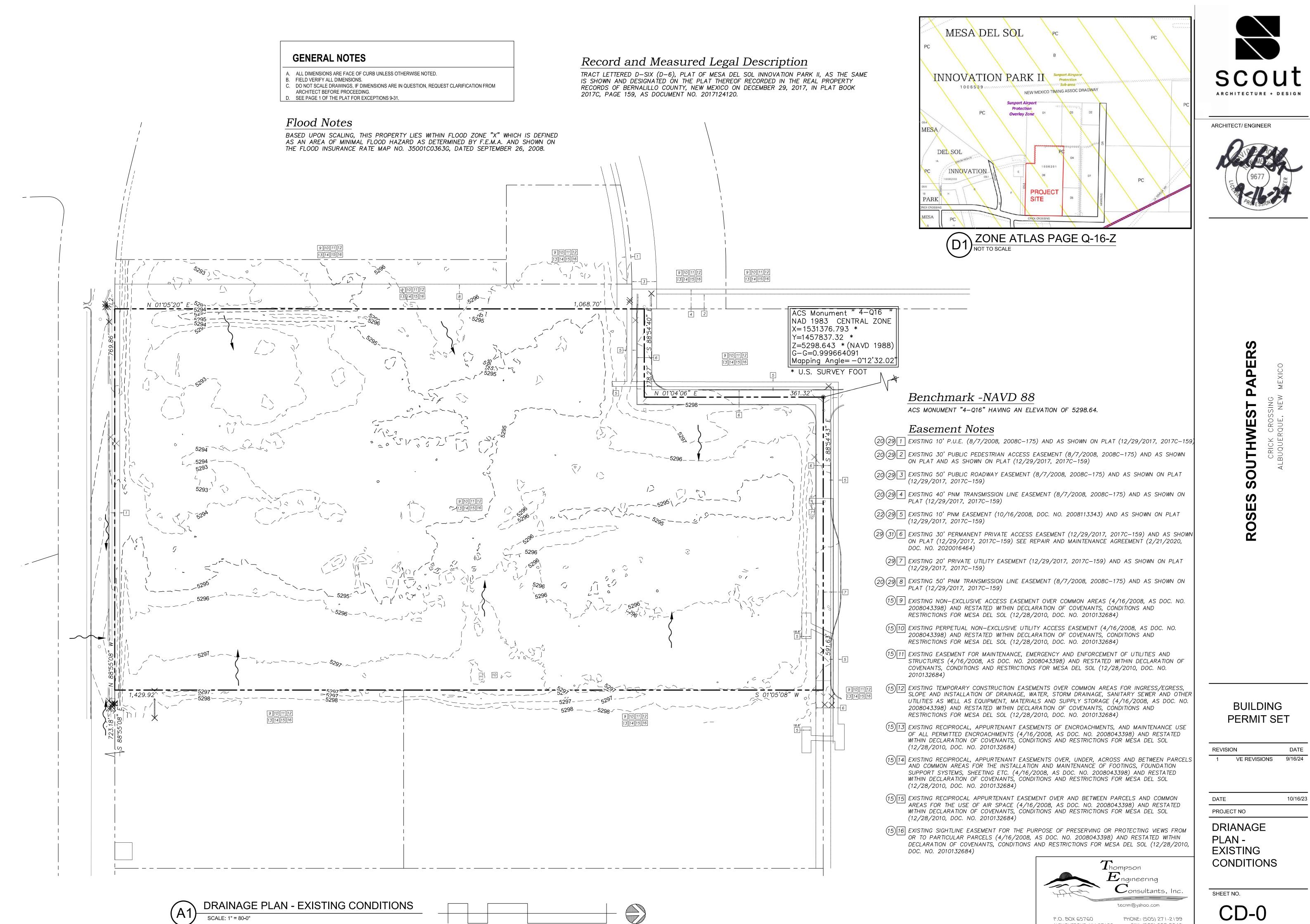
ARCHITECT/ ENGINEER

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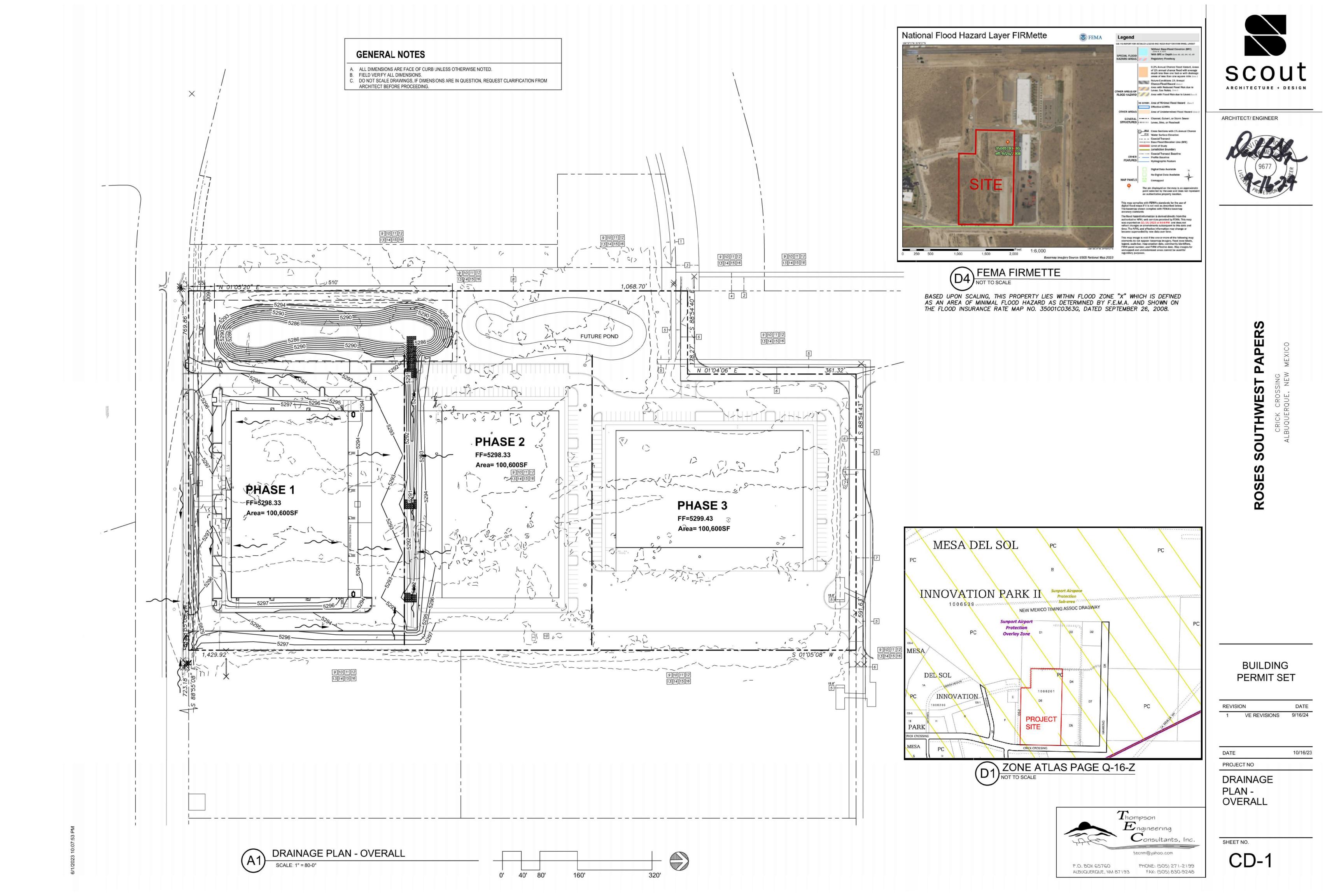
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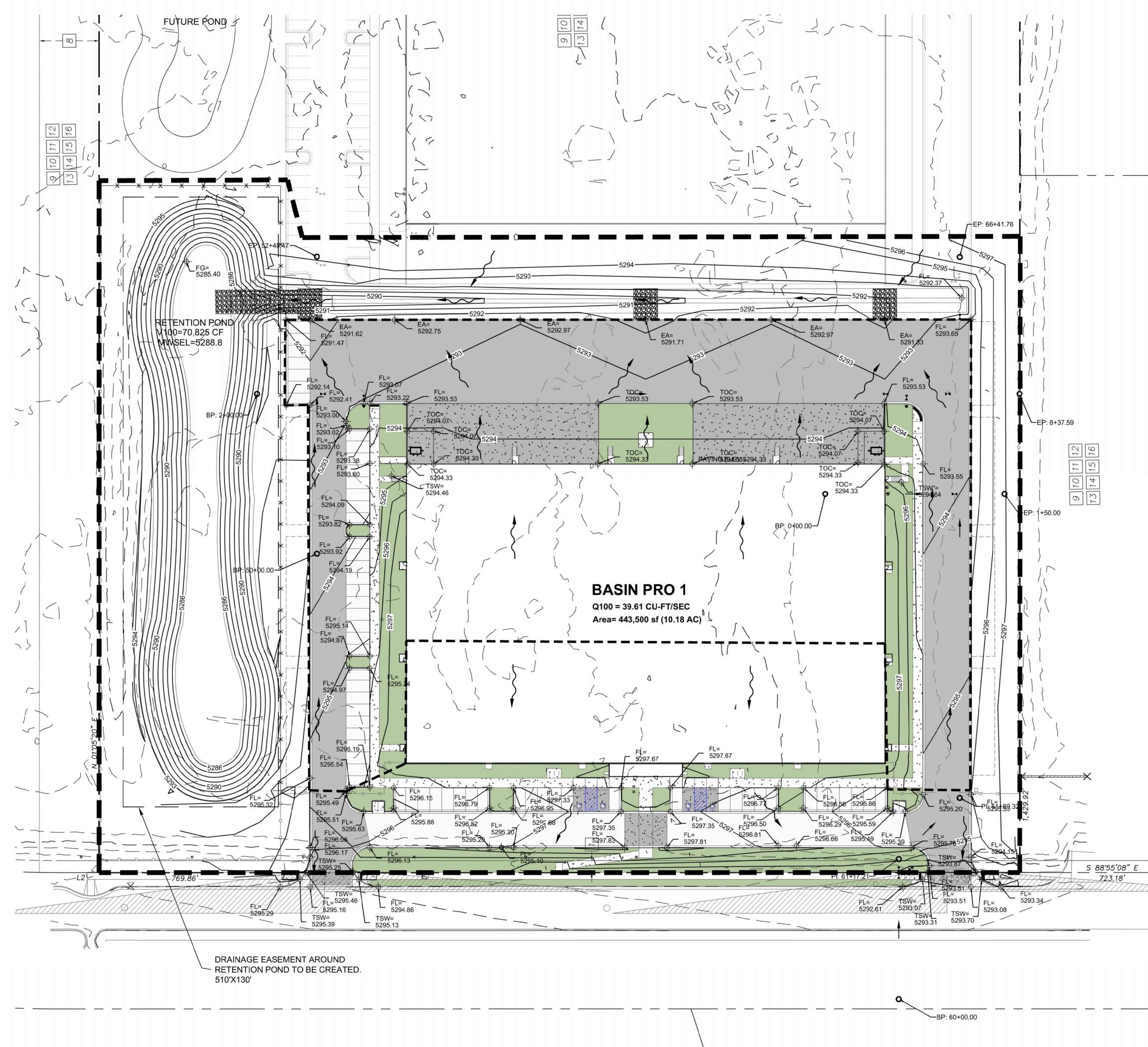
160'



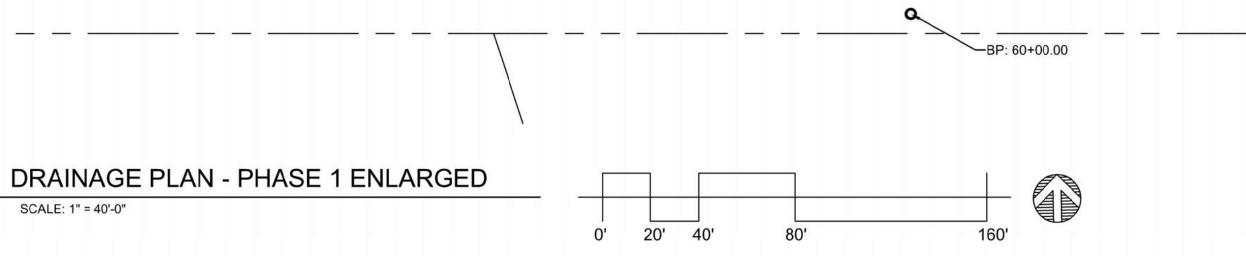
ALBUQUERQUE, NM 87193 FAX: (505) 830-9248

DATE





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ARCHITECT/ ENGINEER





S 88.55'08" E

SO S ROSE

MATERIAL LEGEND

4	4 4	đ	4 4
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HEAVY DUTY ASPHALT PER DETAIL D1/C-501

PARKING LOT ASPHALT PER DETAIL D1/C-501

HEAVY DUTY CONCRETE PAVEMENT PER DETAIL D4/C-501

4" THICK CONCRETE SIDEWALK PER DETAIL B4/C-501

RIPRAP EROSION PROTECTION, SEE GRADING AND DRAINAGE PLANS (SHEET C-201 AND C-202)

EARTHEN POND, SEE GRADING AND DRAINAGE PLANS (SHEET C-201 AND C-202)

LANDSCAPING AREA.



BUILDING PERMIT SET

REVISI	ON	DATE
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DATE		10/16/23
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SHEET NO.

CD-2

I. PURPOSE AND SCOPE

THE PURPOSE OF THIS DRAINAGE PLAN IS TO PRESENT THE EXISTING AND PROPOSED DRAINAGE CONDITIONS FOR THE PROPOSED ROSES SOUTHWEST PAPERS PROJECT. THE NEW BUILDING IS LOCATED ON CRICK CROSSING SW, IN MESA DEL SOL, IN SOUTHWEST ALBUQUERQUE. THE ZONE ATLAS PAGE FOR THE SITE IS Q-16-Z.

II. SITE DESCRIPTION AND HISTORY

THE PROJECT SITE IS LOCATED ON THE NORTH SIDE OF CRICK AVENUE SW, WEST OF HAWKING DRIVE SW. IN MESA DEL SOL THE SITE IS CURRENTLY VACANT. PROPERTIES TO THE EAST AND NORTH HAVE BEEN DEVELOPED AND CONTAIN STORMWATER MANAGEMENT PONDS TO CONTROL RUNOFF. THE MESA DEL SOL COMMUNITY MASTER PLAN REQUIRES THAT ALL PARCELS CONTAIN THE 100 YEAR STORM RUNOFF EVENT IN RETENTION PONDS.

III. COMPUTATIONAL PROCEDURES

HYDROLOGIC ANALYSIS WAS PERFORMED UTILIZING THE DESIGN CRITERIA BASED ON CHAPTER 6, HYDROLOGY, OF THE DEVELOPMENT PROCESS MANUAL RELEASED 2020. TABLES WITHIN CHAPTER 6, WERE USED TO AID IN THE STUDY OF THE SITE HYDROLOGY.

IV. PRECIPITATION

THE STORM EVENT USED FOR THE FOLLOWING CALCULATIONS IS THE 100YR-24HR STORM. THE PROJECT SITE IS LOCATED IN ZONE 3 (EAST OF RIO GRANDE, SOUTH OF I40 AND WITHIN TOWNSHIP 4E).

V. EXISTING DRAINAGE CONDITIONS

THE SITE IS CURRENTLY UNDEVELOPED. THE SITE TO THE EAST IS FULLY DEVELOPED. THE SITE IS BOUNDED ON THE NORTH BY A PRIVATE DRIVE THAT IS UTILIZED BY KAIROS POWER FOR ACCESS. THE SITE IS BOUNDED ON THE SOUTH BY CRICK CROSSING THAT HAS THE NORTH HALF STREET SECTION DEVELOPED. NO RUNOFF FROM ADJACENT SITES IS ANTICIPATED.

THE SITE GENERALLY DRAINS FROM EAST TO WEST BUT HAS VERY LITTLE SLOPE. A MAJORITY OF EXCESS RUNOFF WOULD CURRENTLY BE CAUGHT IN LOCAL DEPRESSION AND ALLOWED TO SOAK INTO THE GROUND. INFILTRATION RATES WERE MEASURED BY THE GEOTECHNICAL ENGINEER AT 6" PER HOUR.

CRICK CROSSING CONTAINS PUBLIC STORMWATER COLLECTION AND CONVEYANCE PIPING BUT DISCHARGE INTO THE SYSTEM IF LIMITED TO THE ROADWAY AND MINOR DISCHARGE AT DRIVEWAYS. AS MENTIONED IN THE SITE DESCRIPTION AND HISTORY PORTION, THE MASTER PLAN FOR MESA DEL SOL REQUIRES THAT ALL PARCELS PROVIDE FULL RETENTION OF EXCESS FOR OFF FOR THE 100 YEAR - 24 HOUR STORM RUNOFF EVENT (PROVIDED INFILTRATION WILL DRAIN THE POND PRIOR TO THE NEXT STORM EVENT).

VI. PROPOSED DRAINAGE CONDITIONS

THE NEW BUILDING WILL BE 100,600 SF WAREHOUSE/MANUFACTURING BUILDING. THE FRONT AND SIDES WILL BE GENERALLY BE AT GRADE TO ALLOW FOR EASY ACCESS FOR PEDESTRIANS, AND DUE TO NFPA REQUIRING ACCESS DOORS AT 100' INTERVALS. THE NORTH SIDE OF THE BUILDING WILL BE A LOADING DOCK, SO THERE WILL BE A 4' DROP FROM FINISHED FLOOR TO DOCK GRADE.

THE SITE FOR PHASE 1, ALL DRAINS TO THE RETENTION POND ON THE WEST SIDE OF THE SITE. THE ROOF WILL DRAIN BOTH NORTH AND SOUTH. 60% OF THE ROOF WILL DRAIN TO THE DOCK SIDE. ALONG THE SOUTH SIDE OF THE SITE IN THE PARKING AREA, A CONCRETE VALLEY GUTTER WILL CONVEY RUNOFF TO THE ACCESS DRIVES THAT DRAIN NORTH. A WATER BLOCK HAS BEEN SET AT CLOSE AS POSSIBLE TO THE DRIVEWAYS TO CAPTURE STORM RUNOFF. A SMALL AREA AT EACH DRIVEWAY WILL SPILL OUT INTO CRICK CROSSING AND WILL BE COLLECTED IN THE SUMP CONDITION TYPE A CATCH BASIN NEAR THE EASTERN ENTRANCE DRIVE. THE INCREASE TO THE STREET RUNOFF IS LESS THAN 2,000 SF AND THEREFORE LESS THAN 0.1 CFS. THIS IS A NEGLIGIBLE INCREASE. AN 18" HDPE STORM PIPE WILL CONVEY THE STORM RUNOFF FROM THE FOUR CATCH BASINS TO THE RETENTION POND.

NORTHERN (60% OF ROOF) ROOF RUNOFF WILL BE COLLECTED IN INTERNAL ROOF DRAINS AND ROUTED INTERNALLY TO DISCHARGE THROUGH THE 4' TALL STEM WALL AT THE LOADING DOCK. FROM THERE THE RUNOFF WILL SHEET FLOW OVER TO AN EARTHEN CHANNEL THAT IS JUST BEYOND THE PHASE 1 PAVING LIMITS. IN THE SUBSEQUENT PHASE, AN UNDERGROUND CONVEYANCE SYSTEM WILL BE DESIGNED AND INSTALLED TO CONVEY STORM RUNOFF TO THE RETENTION POND TO THE WEST.

THE EARTHEN CHANNEL WILL HAVE A 3' WIDE BOTTOM AND 3:1 SIDE SLOPE AND DRAIN TO THE WEST WITH A 0.8% SLOPE. CAPACITY OF THE OVERALL CHANNEL IS 140CFS. DEPTH WILL BE APPROXIMATELY 1.2-FEET TO CONVEY THE 100-YEAR, 6-HOUR STORM EVENT RUNOFF OF 39.61CFS.

INFILTRATION WAS CONFIRMED BY THE GEOTECHNICAL ENGINEER AS 6" PER HOUR. THE EXCESS RUNOFF FROM THE 100 YEAR 6 HOUR EVENT IS 39.61 CFS. THE RUNOFF VOLUME AS REQUIRED BY THE MESA DEL SOL COMMUNITY MASTERPLAN IS 2.1 ACRE-FEET FOR THE 100 YEAR - 24 HOUR EVENT. THE PROPOSED POND IS 3.0' DEEP TO THE MWSEL (PLUS OVER 2 OF FREEBOARD). THE INFILTRATION OF 6" PER HOUR (4.16 CFS) WOULD REQUIRED OVER 11.2 HOURS TO SOAK IN COMPLETELY. THEREFORE THE POND HAS BEEN SIZED TO CONTAIN SINGLE 100 YEAR. THE EXCESS RUNOFF VOLUME ENTERING THE POND IS 2.1 ACRE FEET.

VII. CONCLUSIONS

THE SITE HAS BEEN DESIGNED TO COLLECT AND CONVEY THE 100-YEAR, 6-HOUR PEAK RUNOFF RATE OF 39.61 CFS. THE FULL RETENTION POND AS REQUIRED BY THE MESA DEL SOL COMMUNITY MASTER PLAN HAS BEEN SIZED TO FULL CONTAIN SINGLE 100-YEAR EVENT SINCE THE INFILTRATION RATE IS 6" PER HOUR AND WILL BE FULLY ABSORBED PRIOR TO THE NEXT POTENTIAL MONSOON EVENT.

FUTURE PHASES WILL REQUIRE THE EXPANSION OF THE STORM POND. COMPUTATIONS HAVE BEEN INCLUDED FOR THE FULL BUILDOUT AS CURRENTLY PLANNED, AND THE FULL RETENTION POND CAN EASILY BE ACCOMMODATED.

Pond Routing and Volu	Full Buildout	Phase 1		
r ond Routing and von			FildSe I	
Incoming Flow Rate	Qin	94.93	39.61	cfs
Allowable Discharge Rate	Qout	3.5	4.19	cfs
Hyrdology Zone		3	3	per Figure A-1
Area Total	At	23.792	10.181	acres
Area Type A	Aa	0	0	%
Area Type B	Ab	25	30	%
Area Type C	Ac	0	0	%
Area Type D Impervious	Ad	75	70	%
Excess runoff rates	A	0.67	0.67	
	В	0.86	0.86	
	С	1.09	1.09	
	D	2.58	2.58	
Weighted E (Exces Runoff)		2.15	2.06	
Time of Concentration		0.2	0.2	hours
Time to Peak		0.211	0.215	hours
=0.7*Tc + ((1.6-(Ad/At)/12)				
Time of Base		0.948	0.943	hours
=2.107*E:*At/Qp-(.25*Ad/At)				
Duration of Peak		0.188	0.175	hours
Time for end of peak		0.398	0.390	hours
Time when storage begins		0.008	0.023	hours
Time incoming is less that discharge		0.928	0.884	hours
Volume Required during storm	acre-inch	50.926	18.648	acre inch
Volume Required during storm	cf	184,862	67,691	cubic feet
Volume Stored in Pond during storm	cf	220,780	70,825	

Zone Location		
1	West of the Rio Grande	
2	Between the Rio Grande and San Mateo	
3	Between San Mateo and Eubank, North of Interstate 40 and between San Mateo and the East boundary of Range 4 East, South of Interstate 40	
4	East of Eubank, North of Interstate 40 and East of the East boundary of Range 4 East, South of Interstate 40 Not including the Cibola National Forest	

Infiltration Rates		
Per GeoMAT Report Perc Rate	6" per hour	inches per hour
	.5 ft / hour	ft/ hour
	0.0001389	ft/sec
Pond area	29,931	sf
Infiltration Flow Rate	4.16	cfs

Site Location Precipitaion Zone Existing summary Basin Name Area (sf) Area (acres) %A Land treatment %B Land treatment %C Land treatment %D Land treatment Soil Treatment (acres) Area "A" Area "B" Area "C" Area "D" Excess Runoff (acre-feet) 100yr. 6hr. 10yr. 6hr. 2yr. 6hr. 100yr. 24hr. Peak Discharge (cfs) 100 yr. 10yr.

Project∶

)ate:

Project Numbe:

Proposed summary

Basin Name

Area (sf) Area (acres) %A Land treatment %B Land treatment %C Land treatment %D Land treatment Soil Treatment (acres)

Area "A"

Area "B" Area "C" Area "D"

Excess Runoff (acre-feet)

100yr. 6hr. 10yr. 6hr. 2yr. 6hr. 100yr. 24hr. 100yr, 10day

Peak Discharge (cfs) 100 yr.

10yr.

2yr.

Water Quality Ponding Voulme (cf) Wter Quality Acre Feet

Drainage S	ummarv			
Roses Southwe	est Papers			
TEC Roses SW	/ Papers			
09/16/23				
MTD				
	3 Per COA DP	M Chapter 6		
				-
Ex Basin 1				
1036392.4				
23.79				
0				
80				
20				
0				
_				-
0.00	_			
0.00				
19.03	_			_
4.76				_
0.00				_
1.7963	acre-ft.			
0.7455	acre-ft.			
0.1546	acre-ft.			
1.7963	acre-ft.			
				_
62.48	cfs			-
28.41	CIS			-
6.23	cfs			
0.20	015			_
				_
Overall Site	Phase 1	North SubBasin		_
1036392.4	443500	168977.5		
23.792	10.181	3.879		
20.132	10.101	0.019		
25	30	10		-
20		iv.		
75	70	90		
	10			
0.00	0.00	0.00		_
5.95	3.05	0.39		
0.00	0.00	0.00		
17.84	7.13	3.49		
4.2628	1.7512	0.7784	ac re-ft.	
2.6072	1.0606	0.4881	ac re-ft.	
1.5862	0.6363	0.3071	ac re-ft.	
5.1342	2.0992	0.9489	ac re-ft.	
6.7461	2.7430	1.2643	ac re-ft.	
94.93	39.61	16.64	c fs	
56.51	23.29	10.23	c fs	
31.76	12.79	6.10	c fs	
22023.	3 8796.1	4308.9	cf	



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SITE DRA PLAN - NARRATI CALCULA	VE AND

SHEET NO.

CD-3

