CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



Mayor Timothy M. Keller

November 2, 2023

Dave Thompson, P.E. Thompson Engineering Consultants P O Box 65760 Albuquerque, NM 87193

RE: Roses SW Papers TR D-6 PLAT OF TRACTS D-1 THRU D-7 MESA DEL SOL INNOVATIONPARK II Grading and Drainage Plan Engineer's Stamp Date: 10/17/2023 Hydrology File: Q16DA5000C

Dear Mr. Thompson:

PO Box 1293 Based upon the information provided in your submittal received 10/18/2023, the Grading & Drainage Plan **is not** approved for action by the DFT on Site Plan for Building Permit. The following comments need to be addressed for approval of the above referenced project:

Albuquerque

1. Please provide legend and benchmark information (location, description and elevation) for the survey contour information provided. Create drainage easements for the retention pond and future pond.

NM 87103

3. Sheet CD-2: Add spillway with erosion protection plan to the retention pond.

2. Sheet CD-0: what do the circled numbers mean in "Easement Notes".

www.cabq.gov

- 4. Provide details for the SD pipes, e.g. material, size, length, drop inlets and manhole locations and rip-rap size and dimension information. Add rip-rap symbol to the legend.
 - 5. Sheet CD-3, notes IV & V: Use the 100-yaer 10-day storm event for hydrologic calculations per the City of Albuquerque DPM section 6-11(I)2.

IV. PRECIPITATION

THE STORM EVENT USED FOR THE FOLLOWING CALCULATIONS IS THE 100YR-6HR STORM. THE PROJECT SITE IS LOCATED IN ZONE 3 (EAST OF RIO GRANDE, SOUTH

HISTORY PORTION, THE MASTER PLAN FOR MESA DEL SOL REQUIRES THAT ALL PARCELS PROVIDE FULL RETENTION OF EXCESS FOR OFF FOR THE 100 YEAR - 24 HOUR STORM RUNOFF EVENT (PROVIDED INFILTRATION WILL DRAIN THE POND

CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



Mayor Timothy M. Keller

6. Note VI: Change the new building size form 100,G00 SF to 100,600 SF.

VI. PROPOSED DRAINAGE CONDITIONS

THE NEW BUILDING WILL BE 100.G00 SF WAREHOUSE/MANUFACTURING BUILDING.

If you have any questions, please contact me at 505-924-3695 or tchen@cabq.gov.

Sincerely,

Tieque Cha

Tiequan Chen, P.E. Principal Engineer, Hydrology Planning Department, Development Review Services

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

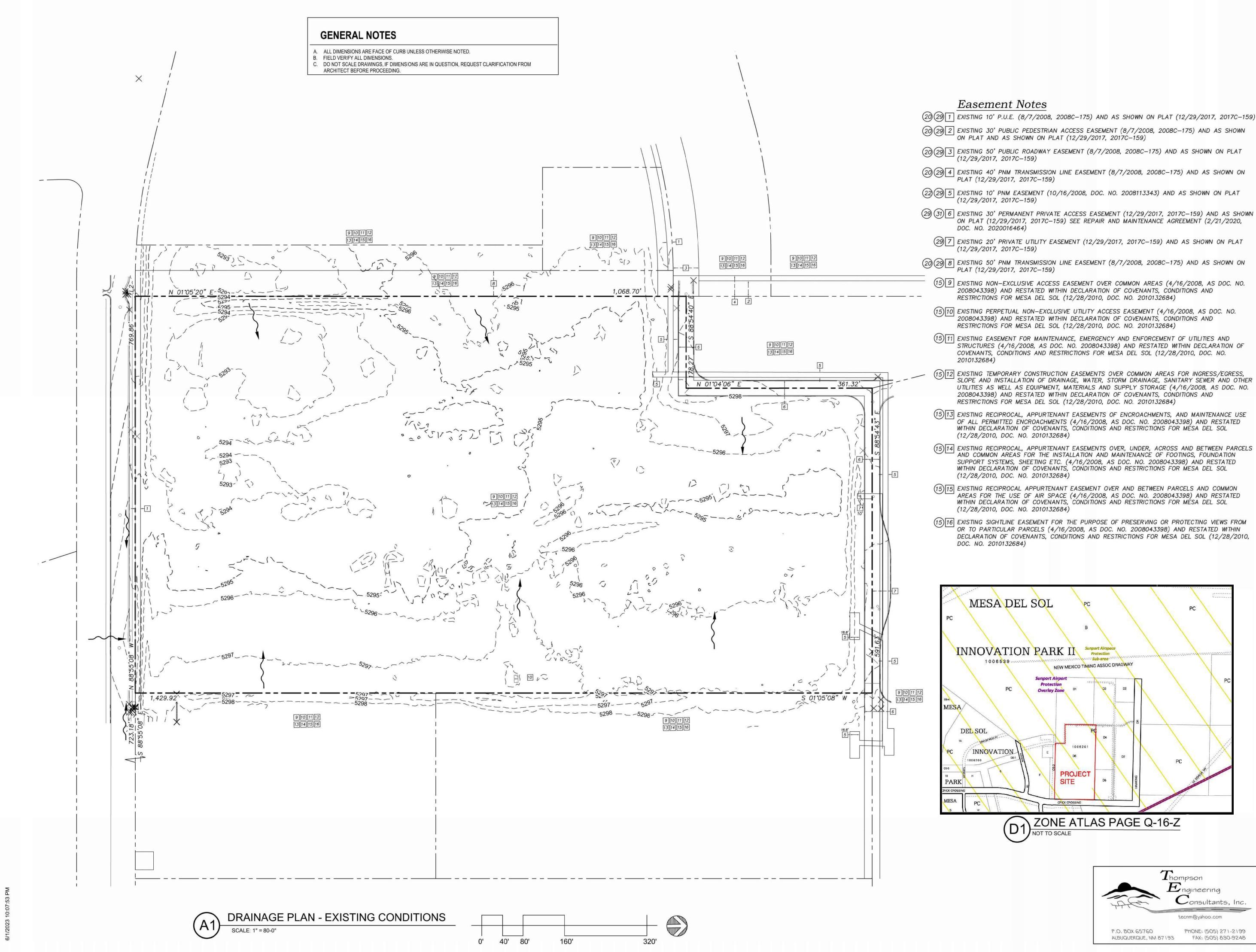


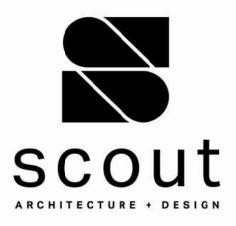
City of Albuquerque

Planning Department Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: Roses SW Papers	Building Permit #	Hydrology File #: <u>Q16DA5</u> 000
DRB#:	EPC#:	Work Order#:
Legal Description: TR D-6 PLAT C	F TRACTS D-1 THRU	D-7 MESA DEL SOL INNOVATIONPARK II
City Address: Not yet established	UPC 101605227405	540202
Applicant: Thompson Engineering	Consultants	Contact: Dave Thompson
Address: PO Box 65760, Albuque	rque, NM 87193	
Phone#: 505-271-2199	Fax#: <u>505-830-</u>	E-mail: tecnm@yahoo.com
Other Contact:		Contact:
Address:		
Phone#:	Fax#:	E-mail:
TYPE OF DEVELOPMENT:	PLAT (# of lots) RE	SIDENCE DRB SITE X ADMIN SITE
IS THIS A RESUBMITTAL?	Yes X No	
DEPARTMENT TRANSPORTAT	ION <u>X</u> HYDROLO	DGY/DRAINAGE
Check all that Apply: TYPE OF SUBMITTAL: ENGINEER/ARCHITECT CERTIFIC	-	YPE OF APPROVAL/ACCEPTANCE SOUGHT: X BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY
 PAD CERTIFICATION CONCEPTUAL G & D PLAN X GRADING PLAN X DRAINAGE REPORT DRAINAGE MASTER PLAN FLOODPLAIN DEVELOPMENT PEI ELEVATION CERTIFICATE CLOMR/LOMR TRAFFIC CIRCULATION LAYOUT TRAFFIC IMPACT STUDY (TIS) STREET LIGHT LAYOUT OTHER (SPECIFY) PRE-DESIGN MEETING? 		 PRELIMINARY PLAT APPROVAL SITE PLAN FOR SUB'D APPROVAL SITE PLAN FOR BLDG. PERMIT APPROVAL FINAL PLAT APPROVAL SIA/ RELEASE OF FINANCIAL GUARANTEE FOUNDATION PERMIT APPROVAL GRADING PERMIT APPROVAL SO-19 APPROVAL PAVING PERMIT APPROVAL GRADING/ PAD CERTIFICATION WORK ORDER APPROVAL CLOMR/LOMR FLOODPLAIN DEVELOPMENT PERMIT OTHER (SPECIFY)
DATE SUBMITTED: <u>10-18-2023</u>		ube
COA STAFF:	ELECTRONIC SUBMI FEE PAID:	TTAL RECEIVED:



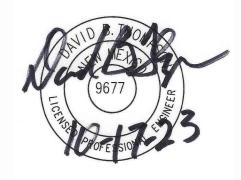


- ON PLAT (12/29/2017, 2017C-159) SEE REPAIR AND MAINTENANCE AGREEMENT (2/21/2020,

- STRUCTURES (4/16/2008, AS DOC. NO. 2008043398) AND RESTATED WITHIN DECLARATION OF
- UTILITIES AS WELL AS EQUIPMENT, MATERIALS AND SUPPLY STORAGE (4/16/2008, AS DOC. NO.
- (15) [14] EXISTING RECIPROCAL, APPURTENANT EASEMENTS OVER, UNDER, ACROSS AND BETWEEN PARCELS AND COMMON AREAS FOR THE INSTALLATION AND MAINTENANCE OF FOOTINGS, FOUNDATION
- AREAS FOR THE USE OF AIR SPACE (4/16/2008, AS DOC. NO. 2008043398) AND RESTATED
- OR TO PARTICULAR PARCELS (4/16/2008, AS DOC. NO. 2008043398) AND RESTATED WITHIN DECLARATION OF COVENANTS, CONDITIONS AND RESTRICTIONS FOR MESA DEL SOL (12/28/2010,

_____ PHONE: (505) 271-2199

ARCHITECT/ ENGINEER



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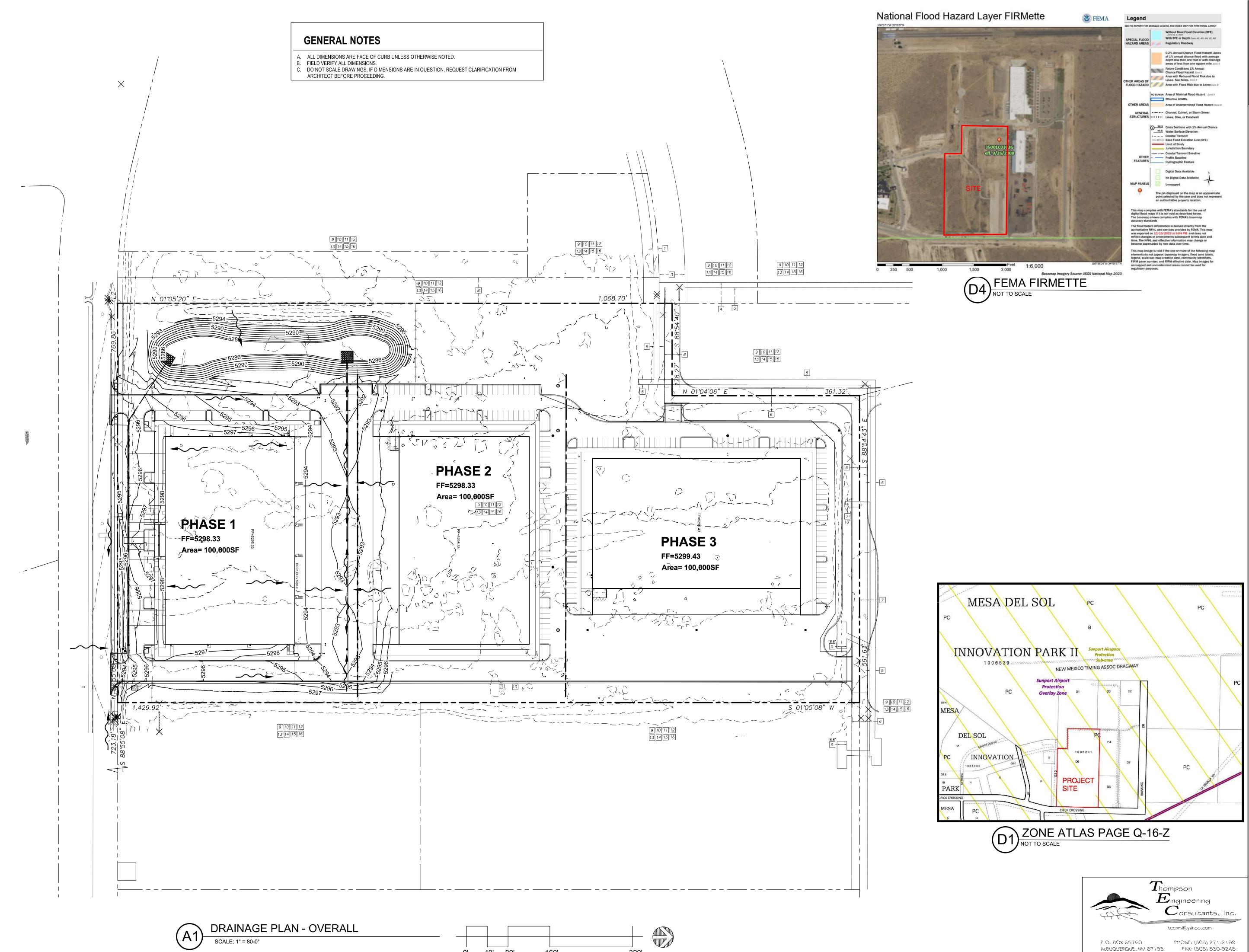
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BUILDING PERMIT SET

REVISION	DATE
DATE	10/16/23
PROJECT NO	
DRIANAGE PLAN - EXISTING	
CONDITIONS	
SHEET NO.	

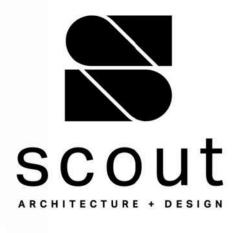
CD-0



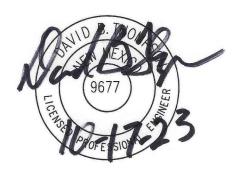
0' 40' 80'

160'

320'



ARCHITECT/ ENGINEER



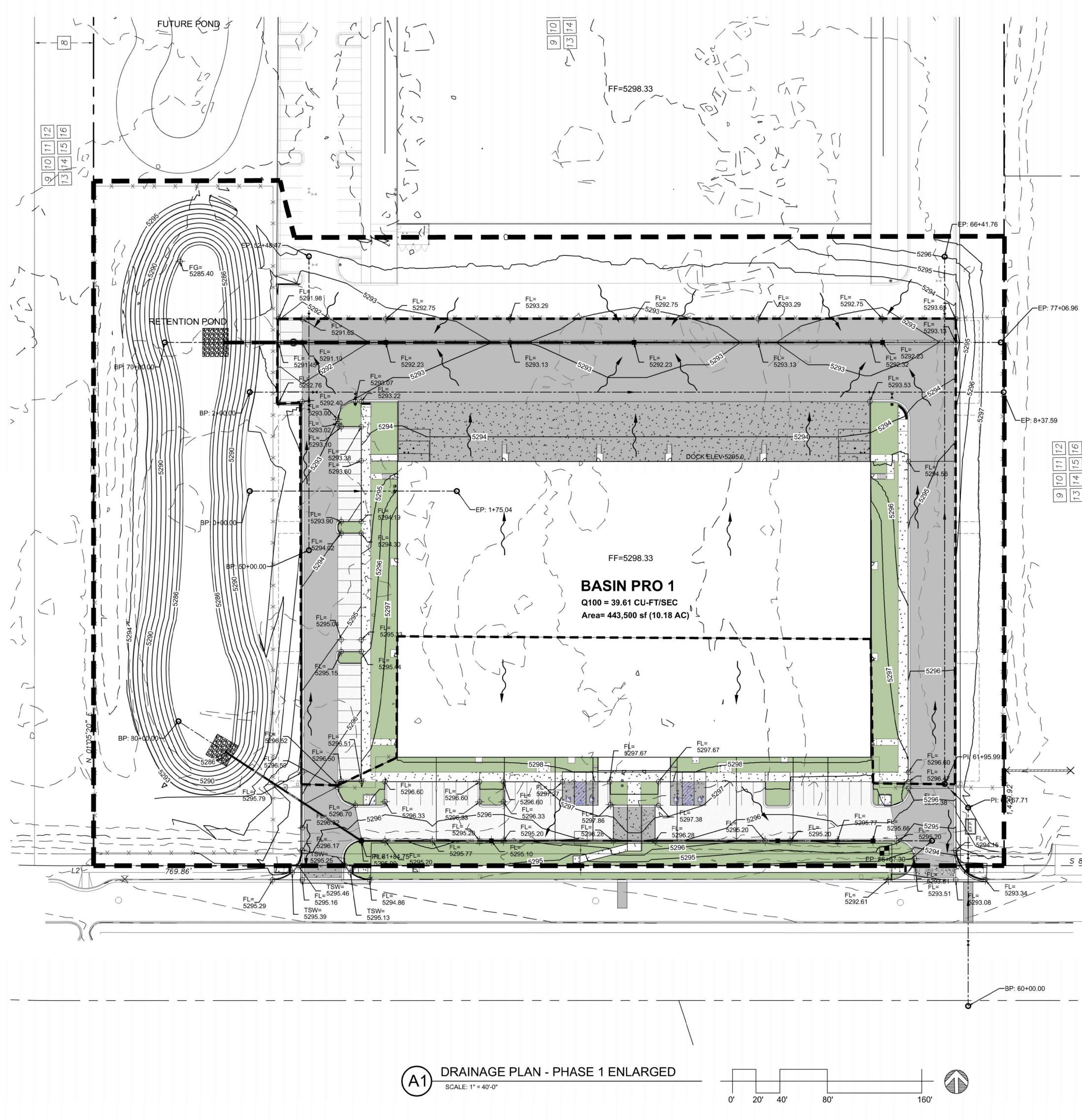
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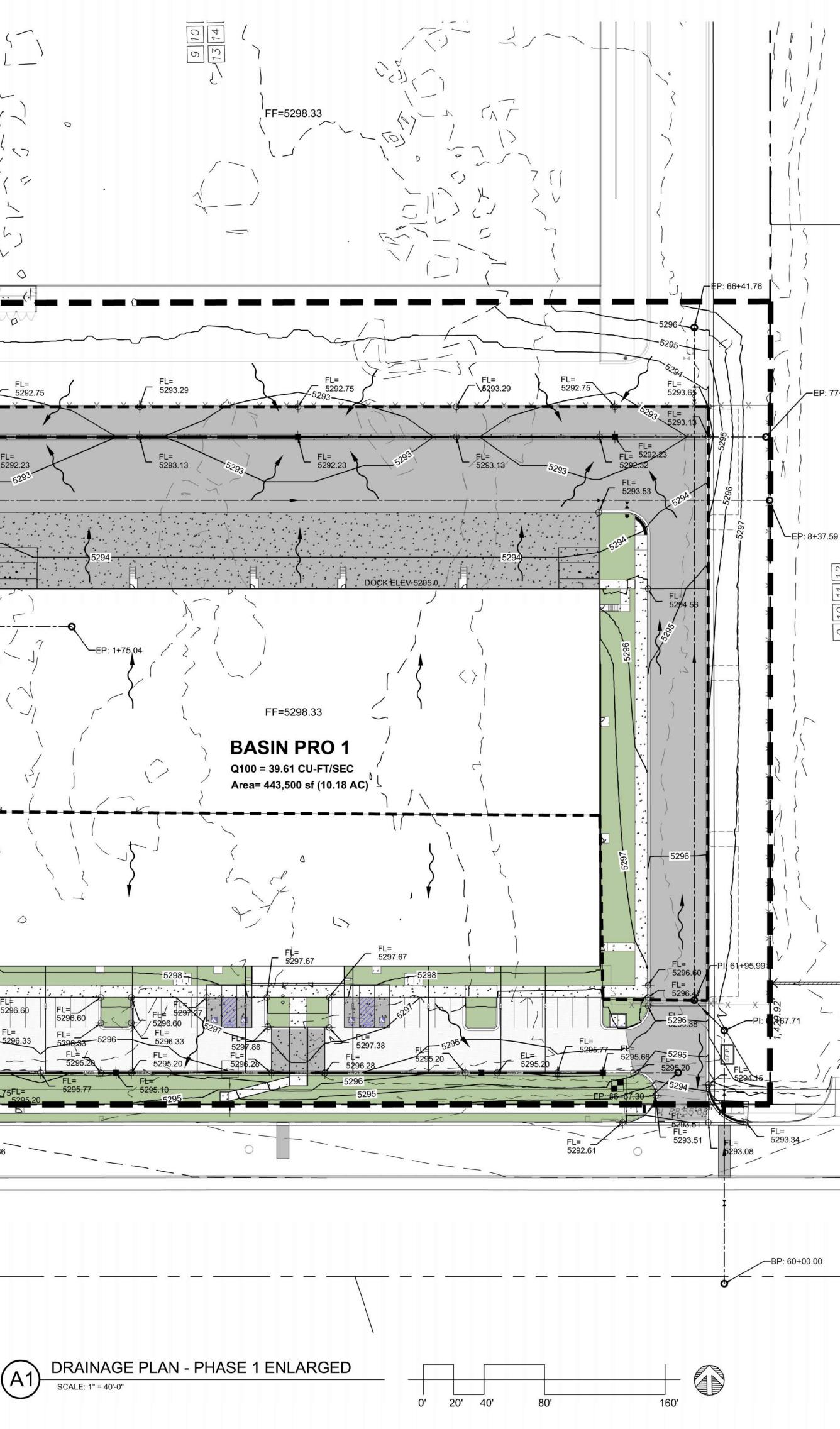
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BUILDING PERMIT SET DATE REVISION

10/16/23 DATE PROJECT NO DRIANAGE PLAN -OVERALL

SHEET NO. CD-1



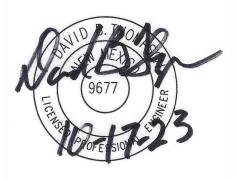


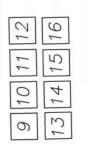
GENERAL NOTES

- A. ALL DIMENSIONS ARE FACE OF CURB UNLESS OTHERWISE NOTED.
 B. FIELD VERIFY ALL DIMENSIONS.
 C. DO NOT SCALE DRAWINGS, IF DIMENSIONS ARE IN QUESTION, REQUEST CLARIFICATION FROM ARCHITECT BEFORE PROCEEDING.



ARCHITECT/ ENGINEER







S THWE SO S ROSE

S 88'55'08" E 723.18'





HEAVY DUTY ASPHALT PER DETAIL D1/C-501



4 Å Å 4



4 4 4 4 -4





PARKING LOT ASPHALT PER DETAIL D1/C-501

HEAVY DUTY CONCRETE PAVEMENT PER DETAIL D4/C-501

4" THICK CONCRETE SIDEWALK PER DETAIL B4/C-501

EROSION PROTECTION, SEE GRADING AND DRAINAGE PLANS (SHEET C-201 AND C-202)

EARTHEN POND, SEE GRADING AND DRAINAGE PLANS (SHEET C-201 AND C-202)

LANDSCAPING AREA.



BUILDING PERMIT SET

REVISION	DATE				
DATE	10/16/23				

DATE	
PROJECT NO	

DRAINAGE PLAN -PHASE 1 ENLARGED

SHEET NO.

DATE

CD-2

I. PURPOSE AND SCOPE

THE PURPOSE OF THIS DRAINAGE PLAN IS TO PRESENT THE EXISTING AND PROPOSED DRAINAGE CONDITIONS FOR THE PROPOSED ROSES SOUTHWEST PAPERS PROJECT. THE NEW BUILDING IS LOCATED ON CRICK CROSSING SW, IN MESA DEL SOL, IN SOUTHWEST ALBUQUERQUE. THE ZONE ATLAS PAGE FOR THE SITE IS Q-16-Z.

II. SITE DESCRIPTION AND HISTORY

THE PROJECT SITE IS LOCATED ON THE NORTH SIDE OF CRICK CROSSING SW, WEST OF HAWKING DRIVE SW, IN MESA DEL SOL THE SITE IS CURRENTLY VACANT. PROPERTIES TO THE EAST AND NORTH HAVE BEEN DEVELOPED AND CONTAIN STORMWATER MANAGEMENT PONDS TO CONTROL RUNOFF. THE MESA DEL SOL COMMUNITY MASTER PLAN REQUIRES THAT ALL PARCELS CONTAIN THE 100 YEAR STORM RUNOFF EVENT IN RETENTION PONDS.

III. COMPUTATIONAL PROCEDURES

HYDROLOGIC ANALYSIS WAS PERFORMED UTILIZING THE DESIGN CRITERIA BASED ON CHAPTER 6, HYDROLOGY, OF THE DEVELOPMENT PROCESS MANUAL RELEASED 2020. TABLES WITHIN CHAPTER 6, WERE USED TO AID IN THE STUDY OF THE SITE HYDROLOGY.

IV. PRECIPITATION

THE STORM EVENT USED FOR THE FOLLOWING CALCULATIONS IS THE 100YR-6HR STORM. THE PROJECT SITE IS LOCATED IN ZONE 3 (EAST OF RIO GRANDE, SOUTH OF I40 AND WITHIN TOWNSHIP 4E).

V. EXISTING DRAINAGE CONDITIONS

THE SITE IS CURRENTLY UNDEVELOPED. THE SITE TO THE EAST IS FULLY DEVELOPED. THE SITE IS BOUNDED ON THE NORTH BY A PRIVATE DRIVE THAT IS UTILIZED BY KAIROS POWER FOR ACCESS. THE SITE IS BOUNDED ON THE SOUTH BY CRICK CROSSING THAT HAS THE NORTH HALF STREET SECTION DEVELOPED. NO RUNOFF FROM ADJACENT SITES IS ANTICIPATED.

THE SITE GENERALLY DRAINS FROM EAST TO WEST BUT HAS VERY LITTLE SLOPE. A MAJORITY OF EXCESS RUNOFF WOULD CURRENTLY BE CAUGHT IN LOCAL DEPRESSION AND ALLOWED TO SOAK INTO THE GROUND. INFILTRATION RATES WERE MEASURED BY THE GEOTECHNICAL ENGINEER AT 6" PER HOUR.

CRICK CROSSING CONTAINS PUBLIC STORMWATER COLLECTION AND CONVEYANCE PIPING BUT DISCHARGE INTO THE SYSTEM IF LIMITED TO THE ROADWAY AND MINOR DISCHARGE AT DRIVEWAYS. AS MENTIONED IN THE SITE DESCRIPTION AND HISTORY PORTION, THE MASTER PLAN FOR MESA DEL SOL REQUIRES THAT ALL PARCELS PROVIDE FULL RETENTION OF EXCESS FOR OFF FOR THE 100 YEAR - 24 HOUR STORM RUNOFF EVENT (PROVIDED INFILTRATION WILL DRAIN THE POND PRIOR TO THE NEXT STORM EVENT).

VI. PROPOSED DRAINAGE CONDITIONS

THE NEW BUILDING WILL BE 100,G00 SF WAREHOUSE/MANUFACTURING BUILDING. THE FRONT AND SIDES WILL BE GENERALLY AT GRADE TO ALLOW FOR EASY ACCESS FOR PEDESTRIANS, AND DUE TO NFPA REQUIRING ACCESS DOORS AT 100' INTERVALS. THE NORTH SIDE OF THE BUILDING WILL BE A LOADING DOCK, SO THERE WILL BE A 4' DROP FROM FINISHED FLOOR TO DOCK GRADE.

THE SITE FOR PHASE 1 ALL DRAINS TO THE RETENTION POND ON THE WEST SIDE OF THE SITE. THE ROOF WILL DRAIN BOTH NORTH AND SOUTH. 60% OF THE ROOF WILL DRAIN TO THE DOCK SIDE. ALONG THE SOUTH SIDE OF THE SITE IN THE PARKING AREA, CATCH BASINS WILL COLLECT RUNOFF AND CONVEY STORM WATER TO THE RETENTION POND. CATCH BASINS HAVE BEEN SET AT CLOSE AS POSSIBLE TO THE DRIVEWAYS TO CAPTURE STORM RUNOFF. A SMALL AREA AT EACH DRIVEWAY WILL SPILL OUT INTO CRICK CROSSING AND WILL BE COLLECTED IN THE SUMP CONDITION TYPE A CATCH BASIN NEAR THE EASTERN ENTRANCE DRIVE. THE INCREASE TO THE STREET RUNOFF IS LESS THAN 2,000 SF AND THEREFORE LESS THAN 0.1 CFS. THIS IS A NEGLIGIBLE INCREASE. AN 18" HDPE STORM PIPE WILL CONVEY THE STORM RUNOFF FROM THE FOUR CATCH BASINS TO THE RETENTION POND.

APPROXIMATELY 25% OF BASIN PRO #1 FLOWS TO THE SOUTH TO THE CATCH BASINS. THERE ARE A TOTAL OF FOUR CATCH BASINS AND PER FIGURE 6.9.9 THE CAPACITY OF EACH TYPE "C" INLET FLOWING AT A MAXIMUM OF 0.5' DEEP (AND 1% SLOPE GOING TOWARD INLET ALONG CURB LINE) THE COLLECTION RATE IS APPROXIMATELY 6.6 CFS. WITH THE REQUIRED COLLECTION OF 8 CFS (25% OF 39.61 CFS) ONLY TWO INLETS WOULD BE REQUIRED. FOUR INLETS WILL ELIMINATE THE BYPASS OUT INTO THE PUBLIC STREET VIA THE DRIVEWAYS, EXCEPT FOR WHAT RAIN FALLS BEYOND THE CATCH BASINS.

NORTHERN (60% OF ROOF) ROOF RUNOFF WILL BE COLLECTED IN INTERNAL ROOF DRAINS AND ROUTED INTERNALLY TO DISCHARGE THROUGH THE 4' TALL STEM WALL AT THE LOADING DOCK. FROM THERE THE RUNOFF WILL SHEET FLOW OVER TO A SERIES OF SUMP CONDITION MEDIAN TYPE CATCH BASINS. HDPE STORM PIPES WILL CONVEY RUNOFF TO THE POND.

THERE ARE A TOTAL OF 3 TYPE "D" (MEDIAN TYPE) CATCH BASINS LOCATED IN THE TRUCK DOCK AREA. EACH AREA LOCATED IN A SUMP CONDITION WITH A MAXIMUM DEPTH OF 0.90'. BEYOND THAT DEPTH, THE WATER OVERTOPS FROM ONE CATCH BASIN TO THE NEXT AND FLOW WEST TOWARD THE RETENTION POND.

THE WEIR EQUATION (Q=CLH³) WAS USED TO EVALUATE THE CAPACITY OF THE TYPE "D" CATCH BASINS. THE GRATE PERIMETER IS (25"X40") = 10.83'. AT A MAXIMUM DEPTH OF 0.90' THE CAPACITY OF THE INLET IS (Q=2.7*10.83'*.9³) = 24.9 CFS. THE FUTURE PHASE 2 WILL DOUBLE THE REQUIRED FLOW RATE OF THE BASIN CONTRIBUTING TO THE LINE OF TYPE "D" CATCH BASINS. THE TOTAL RUNOFF TO BE COLLECTED IN THIS SYSTEM WILL BE = 32.4 CFS (AFTER PHASE 2 IS CONSTRUCTED, AND ONLY 16.64 IN THIS PHASE, SEE NORTH SUB BASIN COMPUTATIONS). THE THREE INLETS WILL HAVE CAPACITY OF 74.7 CFS. PIPES HAVE BEEN SIZED TO KEEP THE ENERGY GRADE LINE BELOW GRADE.

CONVEYANCE PIPING WILL START WITH 18" HDPE AND BE UP-SIZED TO 30" HDPE AT THE SECOND CATCH BASIN, AND 36" HDPE AT THE THIRD INLET TO BE ABLE TO CONVEY THE EXCESS STORM RUNOFF TO THE RETENTION POND. THE ENERGY GRADE LINE WILL REMAIN BELOW THE SURFACE AT ALL INLETS.

INFILTRATION WAS CONFIRMED BY THE GEOTECHNICAL ENGINEER AS 6" PER HOUR. THE EXCESS RUNOFF FROM THE 100 YEAR 6 HOUR EVENT IS 39.61 CFS. THE RUNOFF VOLUME AS REQUIRED BY THE MESA DEL SOL COMMUNITY MASTERPLAN IS 2.1 ACRE-FEET FOR THE 100 YEAR - 24 HOUR EVENT. THE PROPOSED POND IS 3.0' DEEP TO THE MWSEL (PLUS OVER 2 OF FREEBOARD) . THE INFILTRATION OF 6" PER HOUR (4.16 CFS) WOULD REQUIRED OVER 11.2 HOURS TO SOAK IN COMPLETELY. THEREFORE THE POND HAS BEEN SIZED TO CONTAIN SINGLE 100 YEAR. THE EXCESS RUNOFF VOLUME ENTERING THE POND IS 2.1 ACRE FEET.

VII. CONCLUSIONS

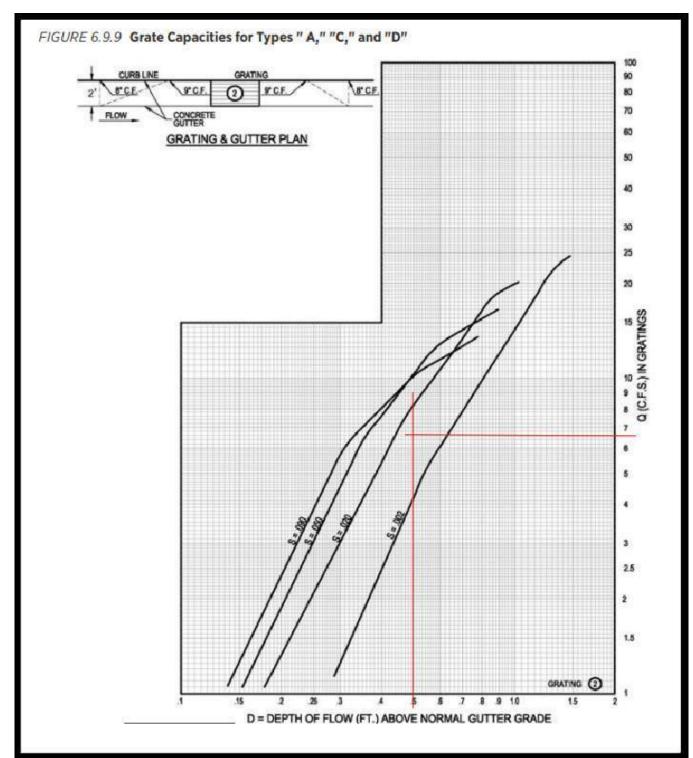
THE SITE HAS BEEN DESIGNED TO COLLECT AND CONVEY THE 100-YEAR, 6-HOUR PEAK RUNOFF RATE OF 39.61 CFS. THE FULL RETENTION POND AS REQUIRED BY THE MESA DEL SOL COMMUNITY MASTER PLAN HAS BEEN SIZED TO FULL CONTAIN SINGLE 100-YEAR EVENT SINCE THE INFILTRATION RATE IS 6" PER HOUR AND WILL BE FULLY ABSORBED PRIOR TO THE NEXT POTENTIAL MONSOON EVENT.

FUTURE PHASES WILL REQUIRE THE EXPANSION OF THE STORM POND. COMPUTATIONS HAVE BEEN INCLUDED FOR THE FULL BUILDOUT AS CURRENTLY PLANNED, AND THE FULL RETENTION POND CAN EASILY BE ACCOMMODATED.

Pond Routing and V	olumes	Full Buildout Ph	nase 1	
Incoming Flow Rate	Qin	94.93	39.61	cfs
Allowable Discharge Rate	Qout	3.5	4.19	cfs
Hyrdology Zone		3	3	per Figure A-1
Area Total	At	23.792	10.181	acres
Area Type A	Aa	0		%
Area Type B	Ab	25	30	%
Area Type C	Ac	0		%
Area Type D Impervious	Ad	75	70	%
Excess runoff rates	А	0.67	0.67	
	В	0.86	0.86	
	С	1.09	1.09	
	D	2.58	2.58	
Weighted E (Exces Runoff)		2.15	2.06	
Time of Concentration		0.2		hours
Time to Peak		0.211	0.215	hours
=0.7*Tc + ((1.6-(Ad/At)/12)				
		0.948	0.943	hours
=2.107*E*At/Qp-(.25*Ad/At) Duration of Peak		0.188	0.175	houro
Time for end of peak		0.398	0.175	
Time when storage begins		0.008	0.390	
Time incoming is less that discharge		0.928	0.884	
Volume Required during storm	acre-inch	50.926	18.648	acre inch
Volume Required during storm	cf	184,862	67,691	cubic feet
Volume Stored in Pond during storm	cf	220,780	70,825	cubic feet

TABLE 6.2.7 Precipitation Zones								
Zone	Location							
1	West of the Rio Grande							
2	Between the Rio Grande and San Mateo							
3	Between San Mateo and Eubank, North of Interstate 40 and between San Mateo and the East boundary of Range 4 East, South of Interstate 40							
4	East of Eubank, North of Interstate 40 and East of the East boundary of Range 4 East, South of Interstate 40							
	Not including the Cibola National Forest							

Infiltration Rates		
Per GeoMAT Report Perc Rate	6" per hour	inches per hour
	.5 ft / hour	ft/ hour
	0.0001389	ft/sec
Pond area	29,931	sf
Infiltration Flow Rate	4.16	cfs



	Drainage Su	mmary	
Project: Project Numbe: Date: By:	Roses Southwest TEC Roses SW P 09/16/23 MTD	•	
Site Location			
Precipitaion Zone	3	Per COA DP	M Chapter 6
Existing summary			
Basin Name	Ex Basin 1		
Area (sf)	1036392.4		
Area (acres) %A Land treatment %B Land treatment %C Land treatment %D Land treatment	23.79 0 80 20 0		
Soil Treatment (acres)			
Area "A" Area "B" Area "C" Area "D"	0.00 19.03 4.76 0.00		
Excess Runoff (acre-feet)		_	
100yr. 6hr. 10yr. 6hr.	1.7963 0.7455	acre-ft. acre-ft.	
2yr. 6hr.	0.1546	acre-ft.	
100yr. 24hr.	1.7963	acre-ft.	
Peak Discharge (cfs) 100 yr. 10yr. 2yr.	62.48 28.41 6.23	cfs cfs cfs	
Proposed summary			
Basin Name Area (sf)	Overall Site 1036392.4	Phase 1 443500	North SubBasin 168977.5
Area (acres) %A Land treatment	23.792	10.181	3.879
%B Land treatment %C Land treatment	25	30	10
%D Land treatment	75	70	90
Soil Treatment (acres)	0.00	0.00	0.00
Area "A" Area "B"	0.00 5.95	0.00 3.05	0.00 0.39
Area "C"	0.00	0.00	0.00
Area "D"	17.84	7.13	3.49
Excess Runoff (acre-feet) 100yr. 6hr.	4.2628	1.7512	0.7784
10yr. 6hr.	2.6072	1.0606	0.4881
2yr. 6hr. 100yr. 24hr.	1.5862 5.1342	0.6363 2.0992	0.3071 0.9489
100yr, 10day	6.7461	2.7430	1.2643
Peak Discharge (cfs)			
100 yr.	94.93	39.61	16.64
10yr. 2yr	56.51 31.76	23.29 12.79	10.23 6.10
2yr.	31.70	12.19	0.10
Water Quality Ponding Voulme (cf) Wter Quality Acre Feet	22023.3 0.5056	8796.1 0.2019	4308.9 0.0989

HYDRAULIC GRADELINE CALCULATIONS

STORM LINE ON LOADING DOCK SIDE

LINE: Loading Dock

1	2	3	4	5	6	7	8	9	10		13	14		15	16	19	20	21	22
											LOSSES								
STATION	STRUCT	D	Q	A	V	K	Sf	L	DELTA	Q	hf	hb	Aavg	hj	hmh	SUM	E.G.	hv	H.G.
		Inches															5285.40	1.50	5285.40
1+00.00	POND																5287.23	0.33	5286.90
		36	32.44	7.069	4.589	667	0.002	90			0.21					0.21	5287.44	0.33	5286.90
1+60.00	MH #1								10	0.00		0.02	7.07	0.00	0.02	0.04	5287.48	0.33	5287.11
		36	32.44	7.069	4.589	667	0.002	78			0.18					0.18	5287.66	0.33	5287.15
3+38.00	CB #1								0	10.81		0.00	5.99	0.28	0.00	0.28	5287.94	0.33	5287.34
		30	21.63	4.909	4.406	410	0.003	210			0.58					0.58	5288.52	0.30	5287.61
5+48.00	CB #2									10.81		0.00	2.45	0.73	0.00	0.73	5289.26	0.30	5288.22
		18	10.81	1.767	6.119	105	0.011	210			2.23					2.23	5291.48	0.58	5288.96
7+58.00	CB #3									10.81		0.00	2.45	0.00	0.02	0.02	5291.50	0.58	5290.90

Drainage Summary

90 0.00 0.39 0.00 3.49 0.7784 acre-ft. 0.4881 acre-ft. 0.3071 acre-ft. 0.9489 acre-ft. 1.2643 acre-ft.

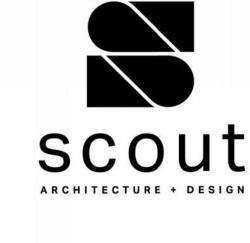
16.64 cfs 10.23 cfs 6.10 cfs 4308.9 cf

0.0989 acre-ft

Roses Southwest Papers

BY: DAA

DATE: REVISED 9-27-23



ARCHITECT/ ENGINEER



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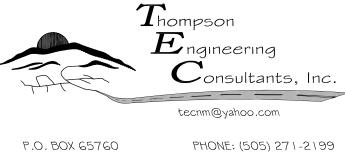
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BUILDING PERMIT SET

REVISION	DATE
DATE	10/16/23
PROJECT NO	
SITE DRAINA PLAN - NARRATIVE	

CALCULATIONS

SHEET NO.



P.O. BOX 65760 ALBUQUERQUE, NM 87193 FAX: (505) 830-9248 CD-3