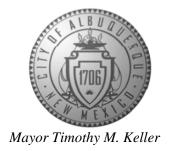
CITY OF ALBUQUERQUE

Planning Department
Brennon Williams, Director



June 24, 2021

Michael Balaskovits Bohannan Huston, Inc. 7500 Jefferson St. NE Albuquerque, NM 87109

RE: MDS OS-7 Storm Drain Relocation

Stryker Rd. and Fellini Blvd.

Grading and Drainage Report Stamp Date: 5/4/21

Hydrology File: R16D097A

Dear Mr. Balaskovits:

Based on the submittal received on 5/5/21, the Grading and Drainage Report is approved for

Grading Permit and Work Order.

Please note, at DRC a note must be added to the pond grading sheet stating that pond slopes are

to be stabilized with "Native Grass Seed with Aggregate Mulch or equal (Must satisfy the "Final

Albuquerque Stabilization criteria" CGP 2.2.14.b.).

NM 87103 If you have any questions, please contact me at 924-3986 or earmijo@cabq.gov.

Sincerely,

www.cabq.gov

PO Box 1293

Ernest Armijo, P.E.

Principal Engineer, Planning Dept.

Development Review Services



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

DRB#: TRACT OS-7 A	EPC#		77ECO4				
TRACT OS-7			Work Order#:				
		OF MESA DEL SOL					
City Address: STRYKER RD & FEL	LINI BLVD						
Applicant: BOHANNAN HUSTON IN			Contact: MICHAEL BALASKOVITS				
Address: 7500 JEFFERSON ST NE	COURTYARD I ALBU	QUERQUE NM 87109					
Phone#: 823-1000	Fax#:		E-mail: MBALASKOVITS@BHINC.COM				
Other Contact:			Contact:				
Address:							
Phone#:	Fax#:		E-mail:				
TYPE OF DEVELOPMENT:	_ PLAT (# of lots)	RESIDENCE _	DRB SITE X ADMIN SITI				
IS THIS A RESUBMITTAL?	Yes X No						
DEPARTMENT TRANSPORTA		DPOLOGV/DPAINAG	36				
DEFACTIVE TRANSFORTA	TIONIII	DROLOG I/DRAINAC					
Check all that Apply:			OVAL/ACCEPTANCE SOUGHT:				
TYPE OF SUBMITTAL:			PERMIT APPROVAL				
ENGINEER/ARCHITECT CERTIF	FICATION	CERTIFICA	TE OF OCCUPANCY				
PAD CERTIFICATION		2227777					
CONCEPTUAL G & D PLAN		PRELIMINARY PLAT APPROVAL					
GRADING PLAN		SITE PLAN FOR SUB'D APPROVAL					
X DRAINAGE REPORT		SITE PLAN FOR BLDG. PERMIT APPROVAL FINAL PLAT APPROVAL					
DRAINAGE MASTER PLAN		FINAL PLA	APPROVAL				
FLOODPLAIN DEVELOPMENT P	PERMIT APPLIC	CIA/DELE	ASE OF FINANCIAL GUARANTEE				
ELEVATION CERTIFICATE			ON PERMIT APPROVAL				
CLOMR/LOMR			PERMIT APPROVAL				
TRAFFIC CIRCULATION LAYOU	JT (TCL)	SO-19 APPI					
TRAFFIC IMPACT STUDY (TIS)			ERMIT APPROVAL				
STREET LIGHT LAYOUT			PAD CERTIFICATION				
OTHER (SPECIFY)		X WORK ORD					
PRE-DESIGN MEETING?		CLOMR/LO					
			IN DEVELOPMENT PERMIT				
		OTHER (SP					
DATE SUBMITTED: 05-05-2021	By: MIC	HAEL BALASKOVITS	3				

FEE PAID:_

DRAINAGE MANAGEMENT PLAN FOR MESA DEL SOL OS-7 STORM DRAINAGE RELOCATION

MAY 2021

Prepared for: MDS Investment, LLC 4020 Vassar Dr. NE, Suite H Albuquerque, NM 87107

Prepared by:

Bohannan A Huston

Engineering
Spatial Data
Advanced Technologies



DRAINAGE REPORT FOR MESA DEL SOL OS-7 STORM DRAINAGE RELOCATION

MAY 4, 2021

Prepared for:

MDS INVESTMENT, LLC 4020 VASSAR DR. NE, SUITE H ALBUQUERQUE, NM 87107

Prepared by:

BOHANNAN HUSTON, INC.
COURTYARD I
7500 JEFFERSON STREET NE
ALBUQUERQUE, NM 87109

PREPARED BY:

UNDER THE SUPERVISION OF:

Mena Ortiz, El

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EXHIBIT B - INFRASTRUCTURE AS-BUILTS

EXHIBIT C - BASIN MAP EXHIBIT

EXHIBIT D - GRADING PLAN

EXHIBIT E - FELLINI BLVD. PNP

I. PURPOSE

The purpose of this report is to establish a drainage management plan to support the relocation of the storm drainage infrastructure and vacation of the public drainage easement over Tract OS-7, in Mesa Del Sol, Albuquerque (see **Exhibit A**). The existing 48" RCP storm drain line from the Stryker Rd and Hawkings Dr roundabout to tract OS-7 will be demolished to support a future development. A new underground storm drainage will be constructed on Fellini Blvd from Stryker Rd to a temporary retention pond at the Mesa Del Sol Blvd. couplet. This proposed retention facility will accommodate existing developed flows intercepted by the storm drain network that currently discharges into tract OS-7, in addition to offsite flows from the north and west. This report is submitted in support of easement vacation and public work order approval by the DRC.

II. CONCEPTS AND METHODOLOGIES

Drainage conditions were analyzed utilizing the City of Albuquerque 2020 DPM Part 6-2(A) Procedure for 40-Acre and Smaller Basins. The site is within zone 2 per Table 6.2.7, Section 6-2(A)(1) of the DPM. The 100-year, 6-hour storm event was utilized to determine peak flow rates for design of the storm drainage improvements. The 100-year, 10-day storm event was used to compute retention ponds required storage volume. The stormwater infrastructure Hydraulic Grade Line (HGL) was analyzed with Stormwater Studio Software in accordance with HEC-22, 3rd edition.

The following documents were referenced in the preparation of this report:

- Drainage Management plan for Drainage Area 2, COA Hydro File #R16/DA2, dated November 2008.
- Drainage Report for Mesa Del Sol Residential Montage Unit 1 and 2, COA
 Hydro File #R16-D003A, dated January 2011.
- Drainage Report for Mesa Del Sol Residential Montage Unit 1 and 2, COA
 Hydro File #R16-D003A, 2018 updated.
- Drainage Report for Mesa Del Sol Community Center, COA Hydro File #R16-DA3005, dated June 2007.
- Drainage Report for Mesa Del Sol Town Center Units 1 and 2, COA Hydro File #R16-D004, dated August 2007.
- Mesa Del Sol Level B Plan, revised September 2012.

III. DEVELOPED HYDRAULIC AND HYDROLOGIC CONDITIONS

A. EXISTING INFRASTRUCTURE

The existing storm drain network in Stryker Rd extends north and west on University Blvd., captures developed flows from the right-of-way as well as a portion of Montage Subdivision basin and the Town Center site at the University Blvd couplet through several inlets and ultimately discharges into tract OS-7 retention pond. See **Exhibit B** for infrastructure as-builts.

Contributing basins to the network, as shown in **Exhibit C**, have been analyzed per the COA 2020 DPM following approved drainage reports land treatment percentages, see **Table 1** for hydrologic results. The 100-year 6-hour design peak flow rate at the connection manhole is computed AP1 $Q_{100vr-6hr} = 77.7$ cfs.

BASINS CONTRIBUTING TO STRYKER STORM DRAINAGE (DISCHARGING TO TRACT OS-7)											
Basin ID	Area (Ac)	%D	$Q_{100yr-6hr}$ (CFS)	$V_{100yr-6hr}(CF)$	$Q_{100yr-10d}(CF)$						
UNIVERSITY 1	5.77	90	24.28	46,046	71,099						
UNIVERSITY 2	1.23	90	5.18	9,822	15,167						
UNIVERSITY 3	1.17	90	4.93	9,350	14,438						
MONTAGE 1	2.75	78	10.95	20,151	30,507						
MONTAGE 2	2.14	25	6.71	9,914	12,500						
STRYKER 1	0.95	90	4.00	7,585	11,712						
STRYKER 2	1.59	90	6.69	12,685	19,587						
TOWN CENTER 1	1.66	90	6.98	13,245	20,452						
TOWN CENTER 2	1.84	100	7.97	15,523	24,384						
TOTAL @ AP1 77.7 144,322 219,846											

Table 1: Hydrologic Analysis of Basins conveyed by the Stryker-University Storm Drainage

An existing retention pond is located within the N Mesa Del Sol Blvd. ROW (See **Exhibit C**). This pond serves the Town Center development and undisturbed area south of Stryker Rd., basin TOWN CENTER 3 $(Q_{100yr-6hr}=18.60\ cfs;\ Q_{100yr-10d}=54,718\ cf)$ and basin M1 $Q_{100yr-6hr}=27.73\ cfs;\ Q_{100yr-10d}=35,726\ cf$ retention requirements. See **Appendix A** for hydrologic analysis.

B. PROPOSED INFRASTRUCTURE

To divert existing developed flows discharging into tract OS-7 to the new retention facility a new storm drainage is proposed in Fellini Blvd. ROW. The new line will connect the existing network in Stryker Rd at the intersection of Stryker Rd and Fellini Blvd and will discharge into the proposed retention pond at the Mesa Del Sol couplet. (See **Exhibit E**) The existing manhole will be rehab in accordance with COA Standard Specifications for Public Works Construction. The new 48" storm drainage is sized to accommodate existing developed flows at AP1($Q_{100yr-6hr} = 77.7$ cfs). See **Appendix B** for the HGL analysis results.

Offsite developed flows from the Town Center development and area south of Stryker Rd (basins Town Center 3 and M1) will be captured by IN1 ($Q_{100yr-6hr}=46.33$ cfs), located on N Mesa del Sol Blvd., west of Fellini Blvd,. This proposed COA Dbl-Type D inlet in sump condition will ultimately discharge to the new retention pond, eliminating the retention requirements of the existing pond in N Mesa Del Sol Blvd. ROW. This will allow for a portion of the existing pond will be rough-graded and the 78' Public Drainage Easement to be vacated.

The proposed retention pond will be located east of Fellini Blvd between North and South Mesa Del Sol Blvd. couplet, within a new Public Drainage Easement. This facility will accommodate existing developed and undeveloped flows captured by the underground storm drainage system. This pond will retain the 100-yr 10-day storm volume $V_{100yr-10d} = 7.12$ ac-ft. It will have a capacity of $V_{Storage} = 8.00$ ac-ft with 2 feet of freeboard. See **Appendix A** for storage calculations and **Exhibit D** for the grading plan. No infiltration is accounted for in the storage calculations.

The 60" pipe outlet discharging into the pond will be constructed with an offset of at least one foot from the pond bottom. An erosion protection rip rap rundown and blanket will be included to prevent erosion during the large storm events.

C. STORMWATER SUSTAINABILTY

In accordance with COA DPM 2020, developments are required to capture and infiltrate the stormwater quality volume (0.42 inches times impervious area). This volume corresponds to the runoff from small storms and the initial portion of larger storms.

The proposed retention pond satisfies stormwater quality requirements since it captures and infiltrates the 100-year 10-day runoff volume. Infiltration will occur thru the

pond bottom and side slopes, evacuating the pond within 96 hours. Soil amendment measures, if necessary, will be determined by the Geotech report.

In addition to retain and infiltrate developed flows, resulting in the recharge of the aquifers, the proposed pond will function as a centralized facility for pollutants removal. Consequently, oils, chemicals, sediments and other types of pollutants conveyed by the public storm drainage system will not reach natural watercourses, protecting and preserving the local resources. These stormwater sustainability goals are pursuant to the Mesa Del Sol Level B Community Master Plan development guidelines.

Due to soil properties, regular, periodic pond maintenance is required to ensure the facility maintains its intended functions. Maintenance activities should include, but is not limited to:

- Repair of slope rills and erosion
- Removal of sediment to design elevations
- o Removal of trash, debris and other pollutants
- Maintenance of clean outlet works conduit
- Inspection of toe of outlet works for scour and undercutting

Maintenance shall occur at least bi-annually and within one week of any significant rainfall event.

IV. CONCLUSION

This drainage management plan provides a detailed study of the proposed public infrastructure to support the removal of the storm drainage and vacation of the public drainage easement on Tract OS-7. The proposed storm drainage in Fellini Blvd. provides an alternative outfall that allows for the safe management of storm runoff in the existing developed conditions. Future drainage analysis reports will be provided upon further development of the Town Center to help develop specific site and roadway drainage schemes. Included is the IDO Zone Atlas Map, infrastructure as-builts, Drainage Management Plan Exhibit, Grading Plan, Fellini Blvd PNP, and all necessary hydrologic and hydraulic analyses. This drainage plan maintains the overall drainage pattern of the area.

APPENDICES

APPENDIX A: EXISTING CONDITIONS

HYDROLOGIC ANALYSIS AND

POND STORAGE CALCULATIONS

APPENDIX B: STORM DRAIN HGL ANALYSIS

APPENDIX A EXISTING CONDITIONS HYDROLOGIC ANALYSIS AND POND STORAGE CALCULATIONS

MESA DEL SOL - TRACT OS7 DRAINAGE RELOCATION												
Basin Data Table												
This table is based on the DPM Part 6-2(A), Zone: 2												
Basin	Area	Lan	d Treatm	ent Percen	tages	Q (100yr)	Q (100yr-6hr)	V (100yr-6hr)	V (100yr-10day)	V (100yr-10day)		
ID	(AC.)	Α	В	С	D	(cfs/ac.)	(CFS)	(CF)	CF	AC-FT		
UNIVERSITY 1	5.77	0.0%	0.0%	10.0%	90.0%	4.21	24.28	46046	71099	1.63		
UNIVERSITY 2	1.23	0.0%	0.0%	10.0%	90.0%	4.21	5.18	9822	15167	0.35		
UNIVERSITY 3	1.17	0.0%	0.0%	10.0%	90.0%	4.21	4.93	9350	14438	0.33		
MONTAGE 1 (1)	2.75	0.0%	11.0%	11.0%	78.0%	3.98	10.95	20151	30507	0.70		
MONTAGE 2 (2)	2.14	0.0%	35.0%	40.0%	25.0%	3.13	6.71	9914	12500	0.29		
STRYKER 1	0.95	0.0%	0.0%	10.0%	90.0%	4.21	4.00	7585	11712	0.27		
STRYKER 2	1.59	0.0%	0.0%	10.0%	90.0%	4.21	6.69	12685	19587	0.45		
TOWN CENTER 1 (3)	1.66	0.0%	0.0%	10.0%	90.0%	4.21	6.98	13245	20452	0.47		
TOWN CENTER 2 (3)	1.84	0.0%	0.0%	0.0%	100.0%	4.34	7.97	15523	24384	0.56		
	TOTAL @ AP1								219846	5.05		
TOWN CENTER 3 (4)	4.45	0.0%	5.0%	5.0%	90.0%	4.18	18.60	35372	54718	1.26		
M1	14.02	80.0%	0.0%	20.0%	0.0%	1.98	27.73	35726	35726	0.82		
					124.01	215420	310290	7.12				

- (1) Land treatment percentages from COA Hydro File # R16-D003A
 (2) Land treatment percentages from COA Hydro File # R16-D003A 2018 Update
 (3) Land treatment percentages per COA Hydro File #R16-DA3005
 (4) Land treatment percentages per COA Hydro File #R16-DA004

RETENTION POND INFORMATION:

BOTTOM ELEVATION = 5274.00 TOP ELEVATION = 5291.00 FREEBOARD = 2.00 FT V_STORAGE = 8.00 AC-FT 7.12 AC-FT V_100YR_10D = 124.01 CFS Q 100YR 6HR = WSE 100YR_10D = 5288.20 WSE_100YR_6HR = 5285.50

PIPE INFORMATION:

DIAMETER = 60"

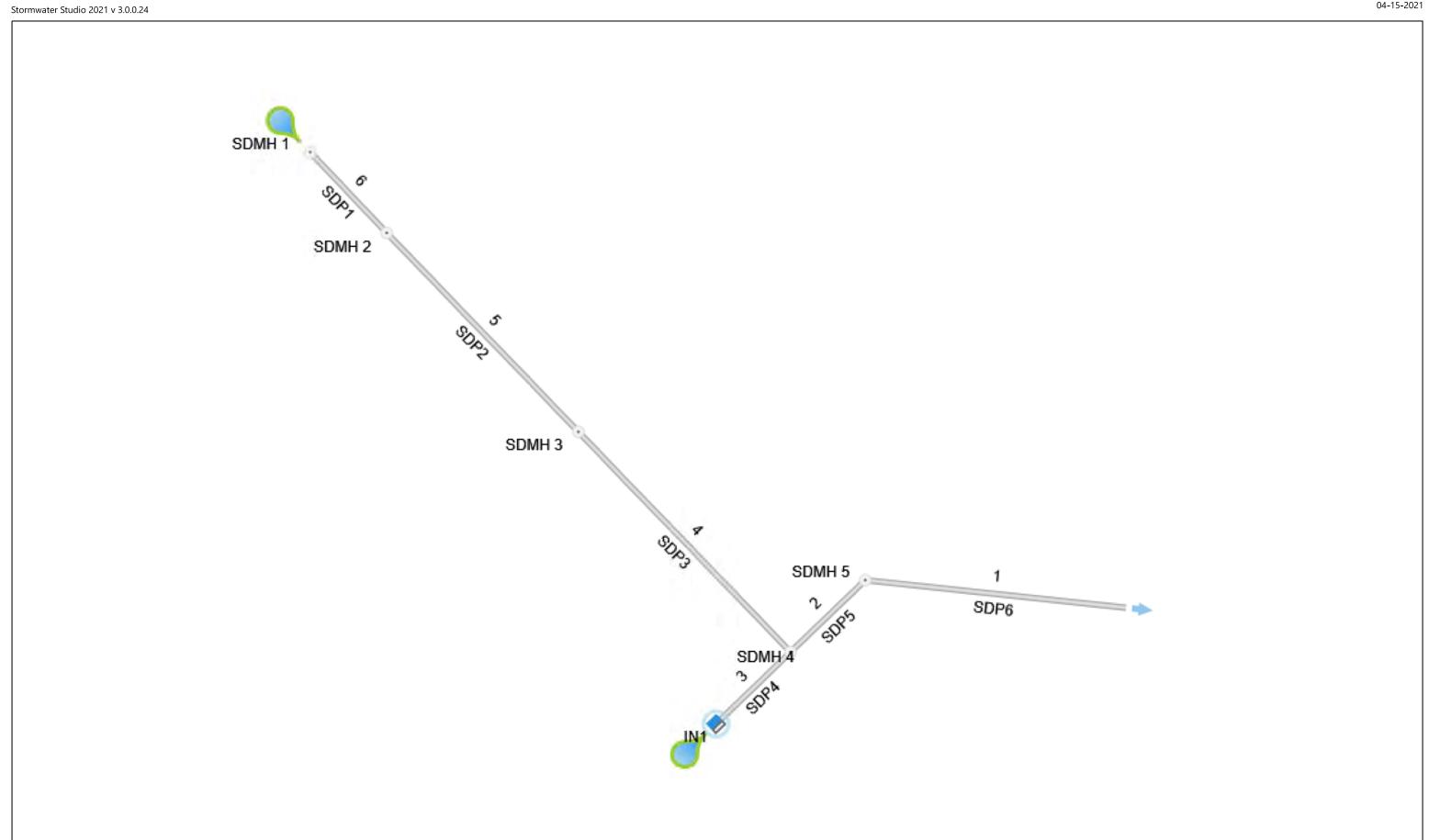
INVERT ELEVATION = 5275.50

Retention Pond Stage Storage
Project: MDS OS-7 SD Relocation

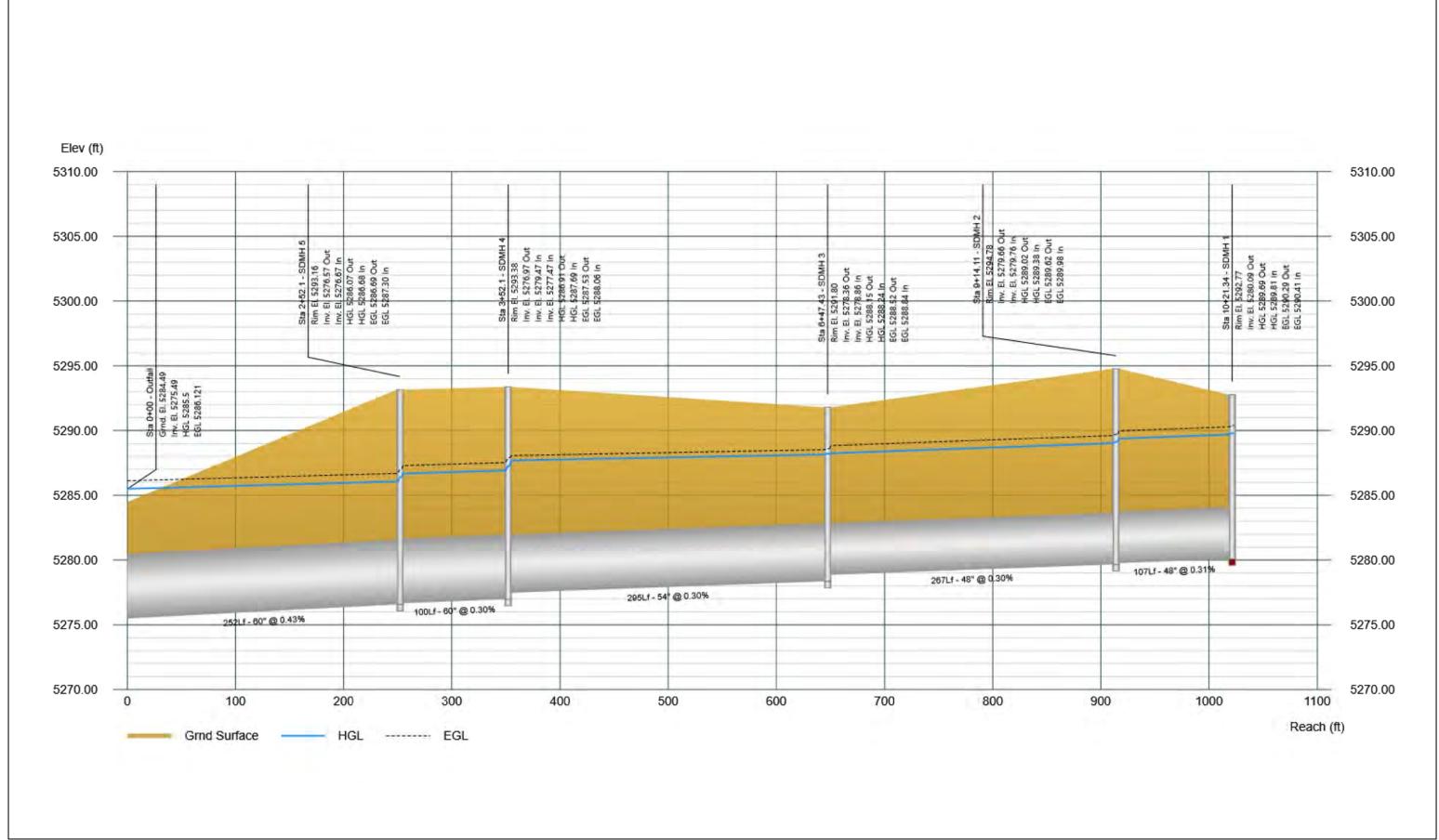
Project:
Basin Description: Retention Pond at MDS Blvd Couplet

Contour	Contour	Cumul	ative	Increme	ntal	Cumulat	ive			
Elevation	Area	Volum	e	Volume		Volume	١	olume/		
	(sq. ft)		Avg. I	End	Avg.	End	Conic		Conic	
			(cu	ft)	(cu.	ft)	(cu. ft)		(cu. ft)	
5,274.000	119.51	N/A	N/A		0.00		N/A		0.00	
5,275.000	8,981.65	4550.	-	4550.58			3379.08		3379.08	
5,276.000	11,565.66	10273	.65	14824.2	3		10246.46		13625.54	
5,277.000	13,509.42	12537	.54	27361.7	7		12524.96		26150.50	
5,278.000	15,509.28	14509	.35	41871.1	2		14497.85		40648.35	
5,279.000	17,565.24	16537	.26	58408.3	8		16526.60		57174.95	
5,280.000	19,677.30	18621	.27	77029.6	5		18611.28		75786.23	
5,281.000	21,866.47	20771	.88	97801.5	3		20762.26		96548.49	
5,282.000	24,208.27	23037	.37	120838.	90		23027.45		119575.94	
5,283.000	26,787.89	25498	.08	146336.	98		25487.20		145063.14	
5,284.000	29,226.45	28007	.17	174344.	15		27998.32		173061.46	
5,285.000	31,488.24	30357	.35	204701.	50		30350.32		203411.78 -	4.66 ac-ft
5,285.500 =	100YR-6HR WSE -	$V_{100YR_{24HR}} = 5.$	0 ac-ft							
5,286.000	33,800.77	32644	.51	237346.	00		32637.68		236049.46 -	5.42 ac-ft
5,287.000	36,189.17	34994	.97	272340.	98		34988.18		271037.63	
5,288.000	38,654.39	37421	.78	309762.	75		37415.01		308452.64 -	7.08 ac-ft
5,288.200 =	100YR-10D WSE -	$V_{100YR_{10D}} = 7.1$	2 ac-ft							
5,289.000	41,199.65	39927	.02	349689.	77		39920.26		348372.90 -	8.00 ac-ft
5,290.000	43,825.24	42512	.45	392202.	22		42505.69		390878.59	
5,291.000	65,129.91	54477	.57	446679.	79		54127.04		445005.63 -	Top of Pond

APPENDIX B - STORM DRAIN HGL ANALYSIS

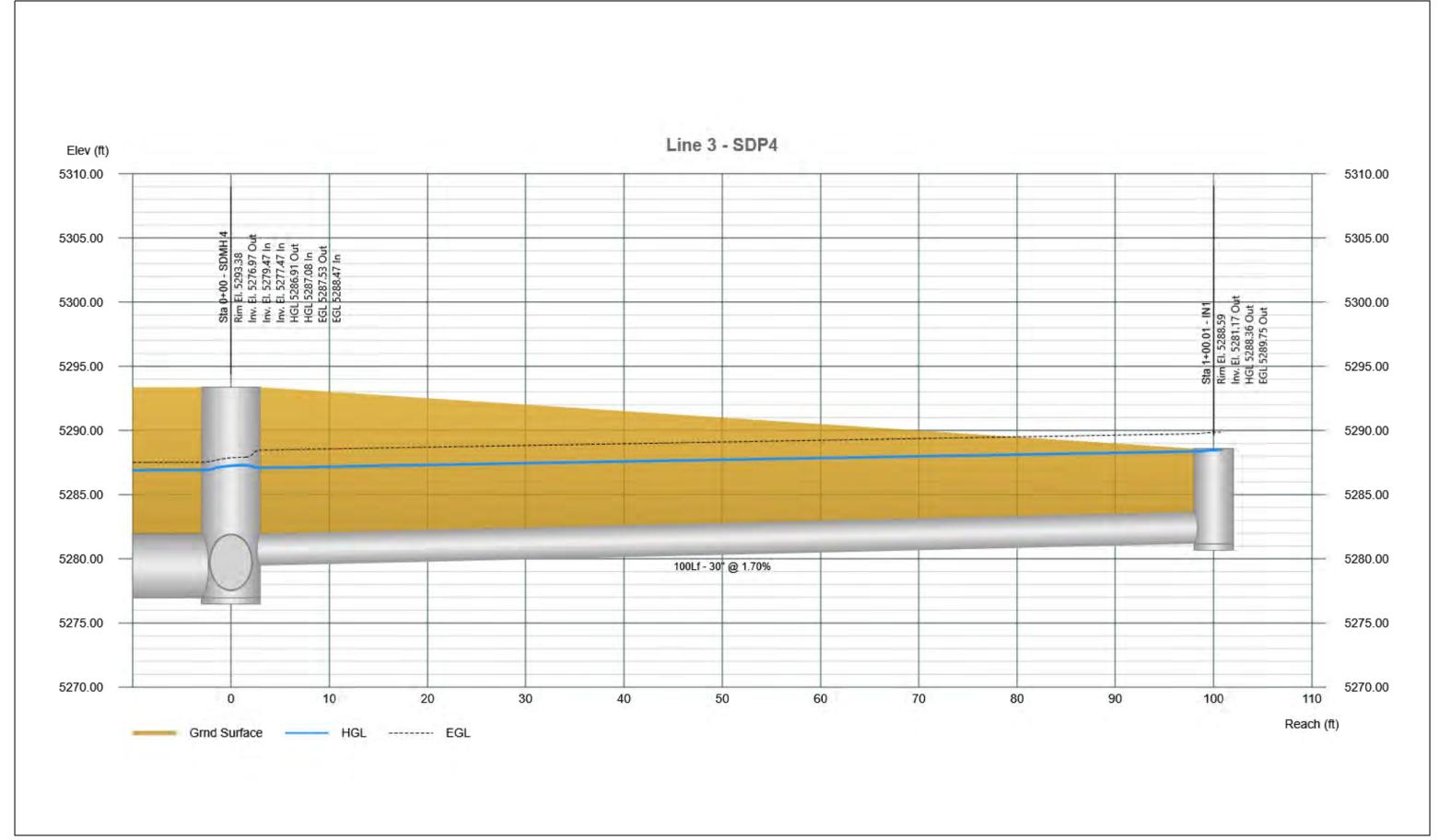


04-15-2021 Stormwater Studio 2021 v 3.0.0.24



Stormwater Studio 2021 v 3.0.0.24

04-15-2021





Energy Grade Line Calculations Stormwater Studio 2021 v 3.0.0.24

Notes: Return Period = 2-yrs.

pwater Studio 2021 v 3 0 0 24

Project Name: MDS OS-7 SD RELOCATION

04-15-2021

Line	Line Line		Downstream				Length	Upstream						Pi	pe	Junction						
No	Size	Q	Invert Elev	Depth	Area	HGL Elev	Vel	Vel Head	EGL Elev	Lei	Invert Elev	Depth	Area	HGL Elev	Vel	Vel Head	EGL Elev	n Value	Enrgy Loss	HGLa Elev	EGLa Elev	Enrgy Loss
	(in)	(cfs)	(ft)	(ft)	(sqft)	(ft)	(ft/s)	(ft)	(ft)	(ft)	(ft)	(ft)	(sqft)	(ft)	(ft/s)	(ft)	(ft)		(ft)	(ft)	(ft)	(ft)
1	60	124.01	5275.49	5.00	19.63	5285.50	6.32	0.62	5286.12	252.10	5276.57	5.00	19.63	5286.07	6.32	0.62	5286.69	0.013	0.571	5286.43	5287.05	0.36
2	60	124.01	5276.67	5.00	19.63	5286.68	6.32	0.62	5287.30	100.00	5276.97	5.00	19.63	5286.91	6.32	0.62	5287.53	0.013	0.227	5287.29	5287.92	0.39
3	30	46.33	5279.47	2.50	4.91	5287.08	9.44	1.39	5288.47	100.02	5281.17	2.50	4.91	5288.36	9.44	1.38	5289.75	0.013	1.277	5288.48	5289.86	0.12
4	54	77.68	5277.47	4.50	15.90	5287.69	4.89	0.37	5288.06	295.32	5278.36	4.50	15.90	5288.15	4.88	0.37	5288.52	0.013	0.461	5288.23	5288.60	0.07
5	48	77.68	5278.86	4.00	12.56	5288.24	6.18	0.59	5288.84	266.68	5279.66	4.00	12.57	5289.02	6.18	0.59	5289.62	0.013	0.780	5289.14	5289.74	0.12
6	48	77.68	5279.76	4.00	12.56	5289.38	6.18	0.59	5289.98	107.23	5280.09	4.00	12.57	5289.69	6.18	0.59	5290.29	0.013	0.314	5289.81	5290.41	0.12

Project File: 20210391STORM DRAINAGE.sws

<u>IN1</u>

Double D inle Open Area (for Length of We	or orifice calc		7.8628472 9.9166667			Calculation of open area:		
,						·	(in^2)	(ft^2)
	Head	Head	Weir Q	Orifice Q	Control Q	Total Grate Area	2000	13.888889
	(ft)	(in)				Cross Bar Area	-732	-5.0833333
	0.05	0.6	0.30	8.47	0.30	Supports (ends)	-231.25	-1.6059028
	0.1	1.2	0.84	11.97	0.84	Areas Counted Twice	<u>95.5</u>	0.6631944
	0.15	1.8	1.54	14.66	1.54		1132.25	7.8628472
	0.2	2.4	2.38	16.93	2.38			
	0.25	3	3.32	18.93	3.32			
	0.3	3.6	4.37	20.74	4.37	Calculation of Length of Weir:		
	0.35	4.2	5.50	22.40	5.50		(in)	(ft)
	0.4	4.8	6.72	23.94	6.72	Total Perimeter of Grate	130	10.833333
	0.45	5.4	8.02	25.40	8.02	Short Cross Bars	-7	-0.5833333
	0.5	6	9.40	26.77	9.40	End Supports	9	0.75
	0.55	6.6	10.84	28.08	10.84	Bearing Bars	-13	-1.0833333
	0.6	7.2	12.35	29.33	12.35	-	119	9.9166667
	0.65	7.8	13.93	30.52	13.93			
	0.7	8.4	15.56	31.68	15.56			
	0.75	9	17.26	32.79	17.26			
	8.0	9.6	19.02	33.86	19.02			
	0.85	10.2	20.83	34.90	20.83			
	0.9	10.8	22.69	35.92	22.69			
	0.95	11.4	24.61	36.90	24.61			
	1	12	26.58	37.86	26.58			
	1.5	18	48.82	46.37	46.37	Q_100 = 46.33 cfs		
	2	24	75.17	53.54	53.54			
	3	36	138.10	65.57	65.57			
	5	60	297.14	84.66	84.66	Q_Capacity = 84.00 cfs		

EXHIBITS

EXHIBIT A: IDO ZONE ATLAS MAP

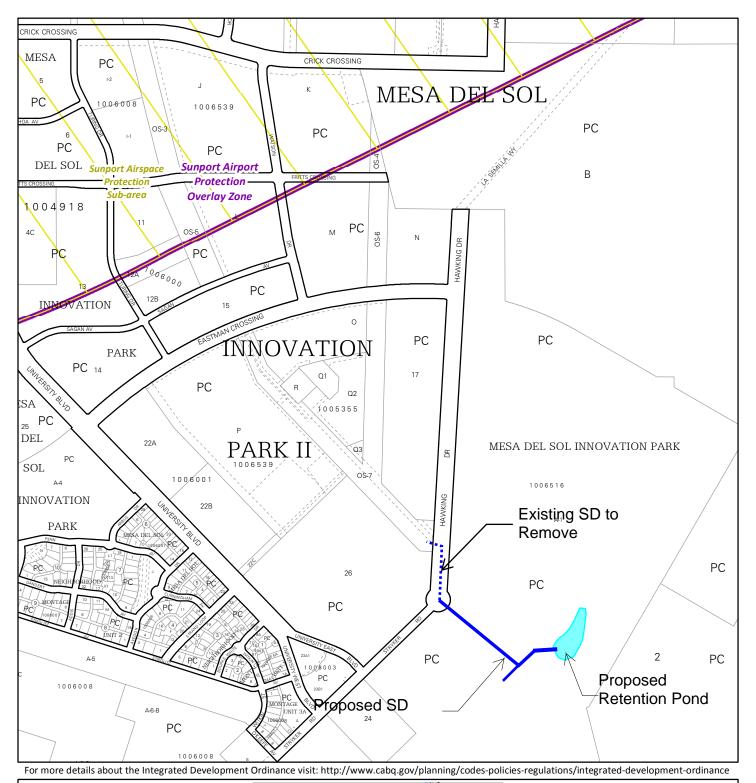
EXHIBIT B: INFRASTRUCTURE AS-BUILTS

EXHIBIT C: BASIN MAP EXHIBIT

EXHIBIT D: GRADING PLAN

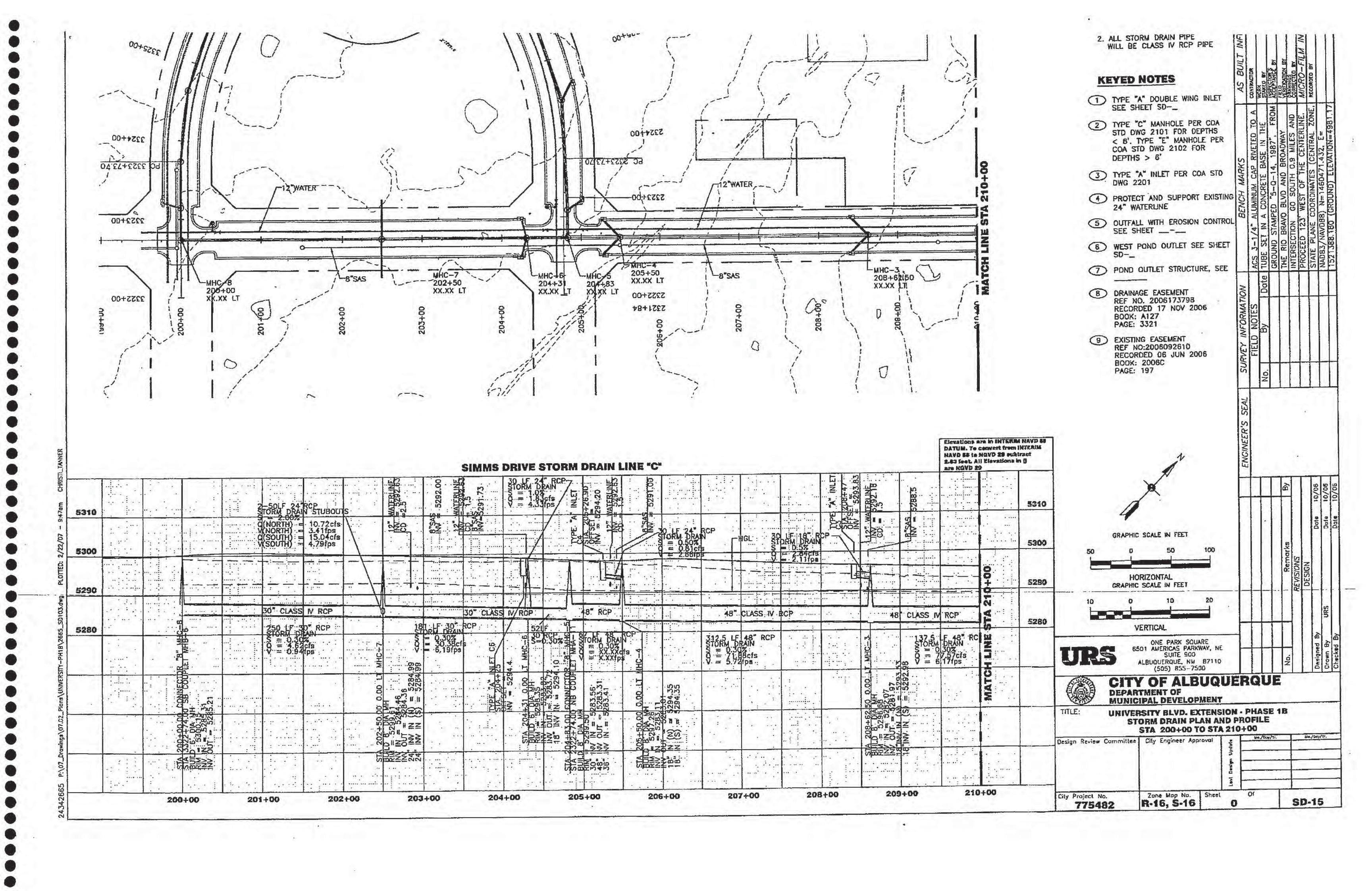
EXHIBIT E: FELLINI BLVD. PNP

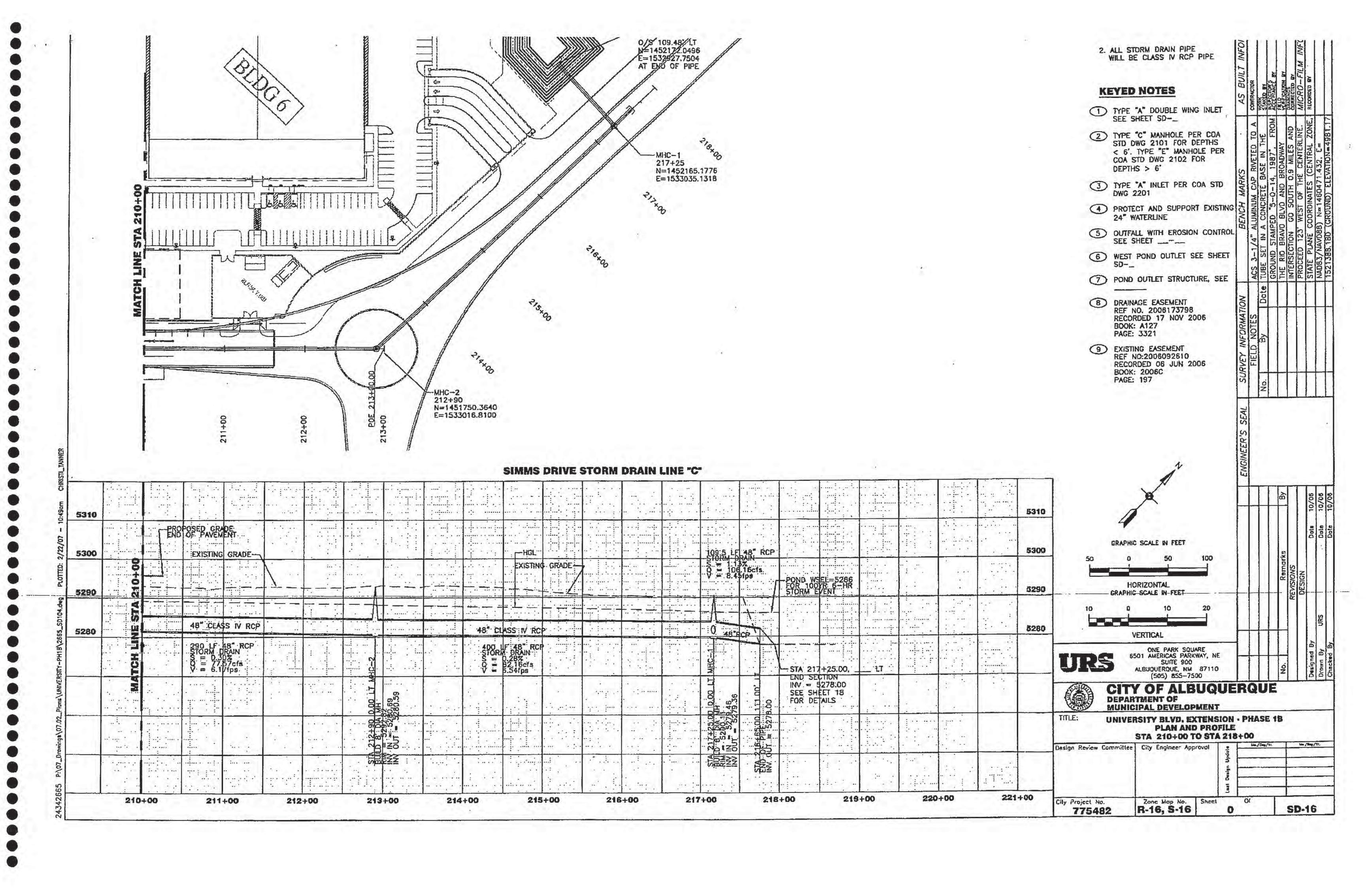
EXHIBIT A IDO ZONE ATLAS MAP

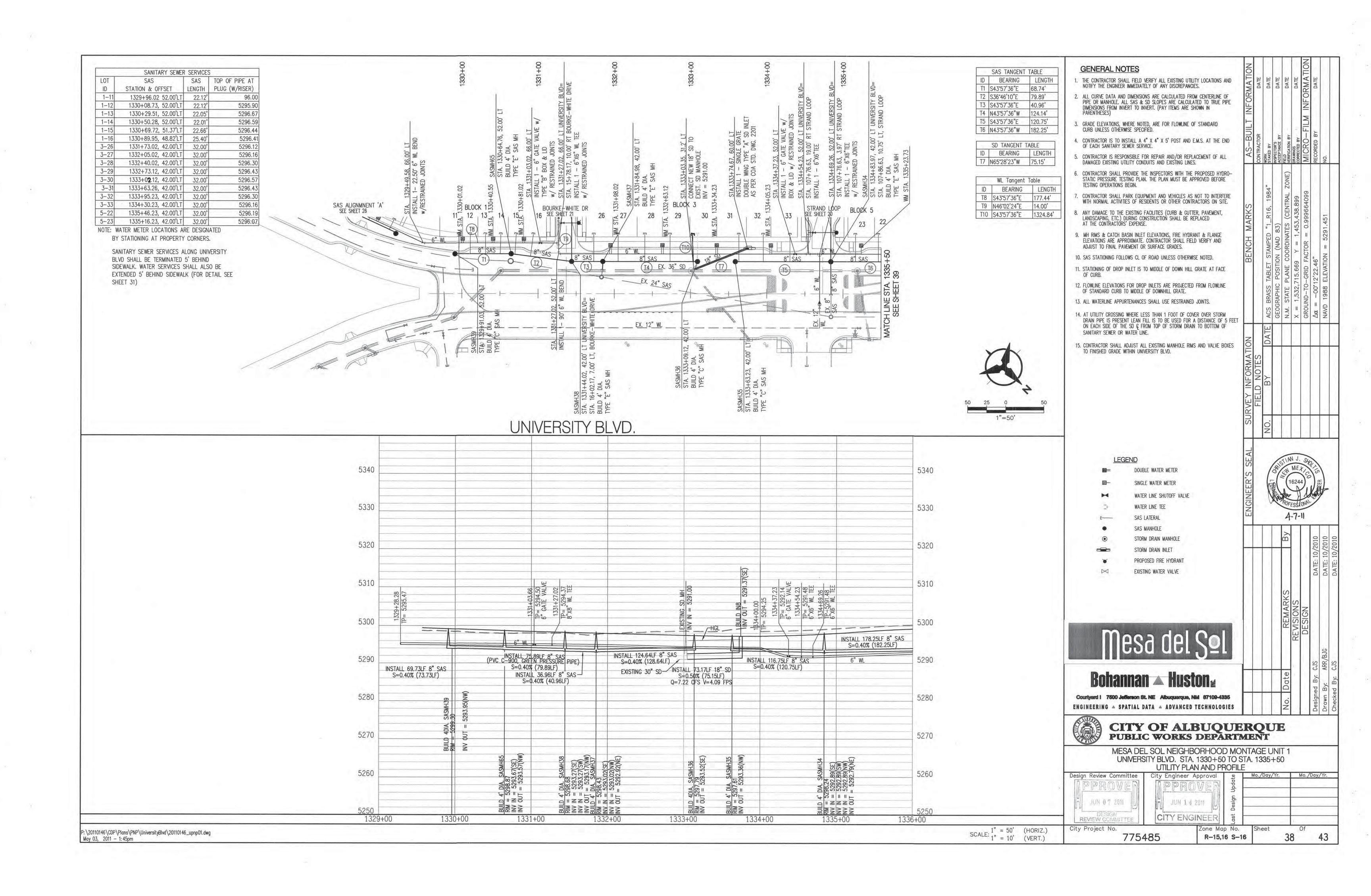


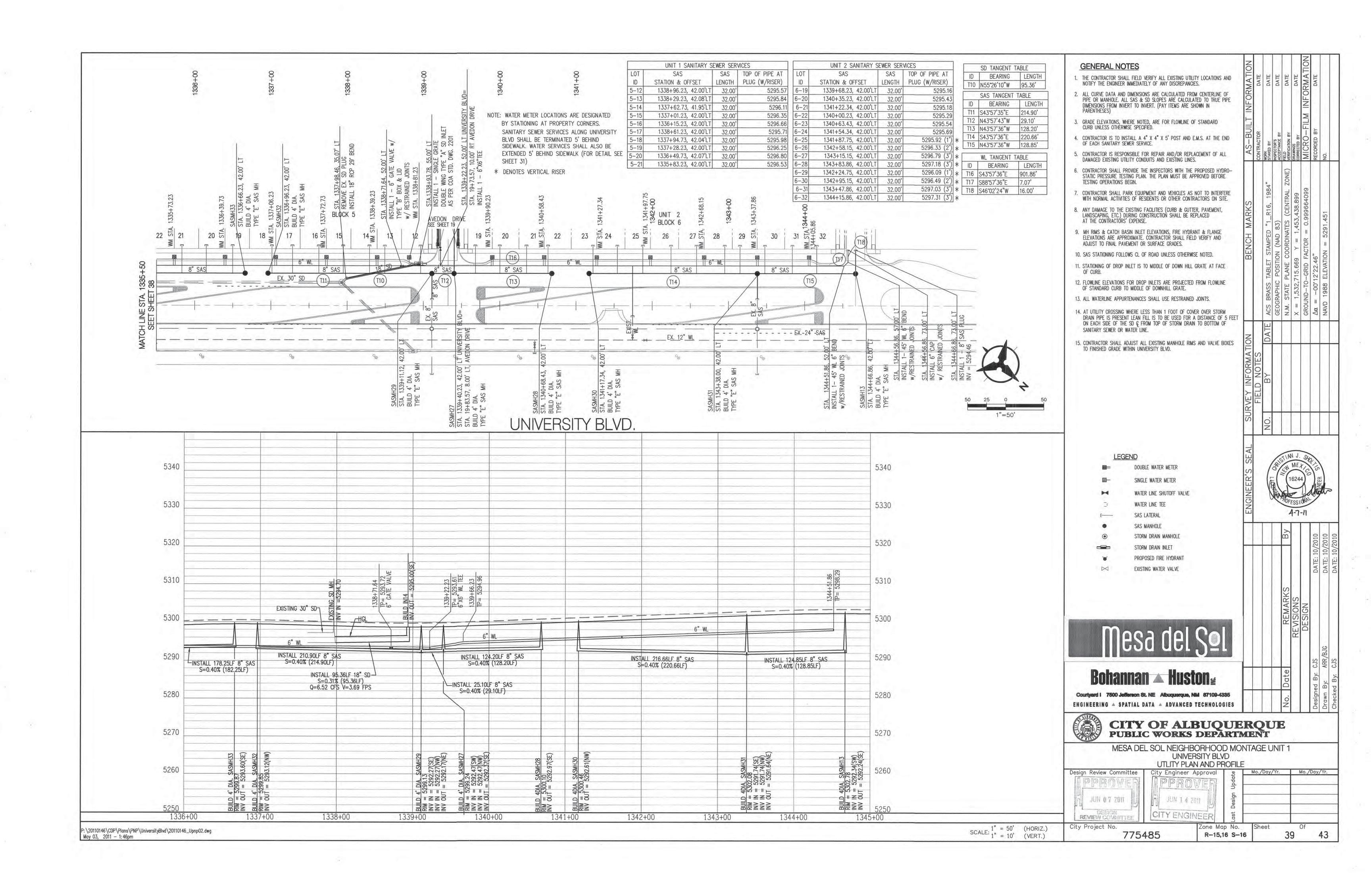
Zone Atlas Page: **IDO Zone Atlas** R-16-Z May 2018 Escarpment Easement Petroglyph National Monument Areas Outside of City Limits Airport Protection Overlay (APO) Zone Character Protection Overlay (CPO) Zone Historic Protection Overlay (HPO) Zone IDO Zoning information as of May 17, 2018 View Protection Overlay (VPO) Zone **Gray Shading** The Zone Districts and Overlay Zones Represents Area Outside are established by the **∃** Feet of the City Limits Integrated Development Ordinance (IDO). 1,000

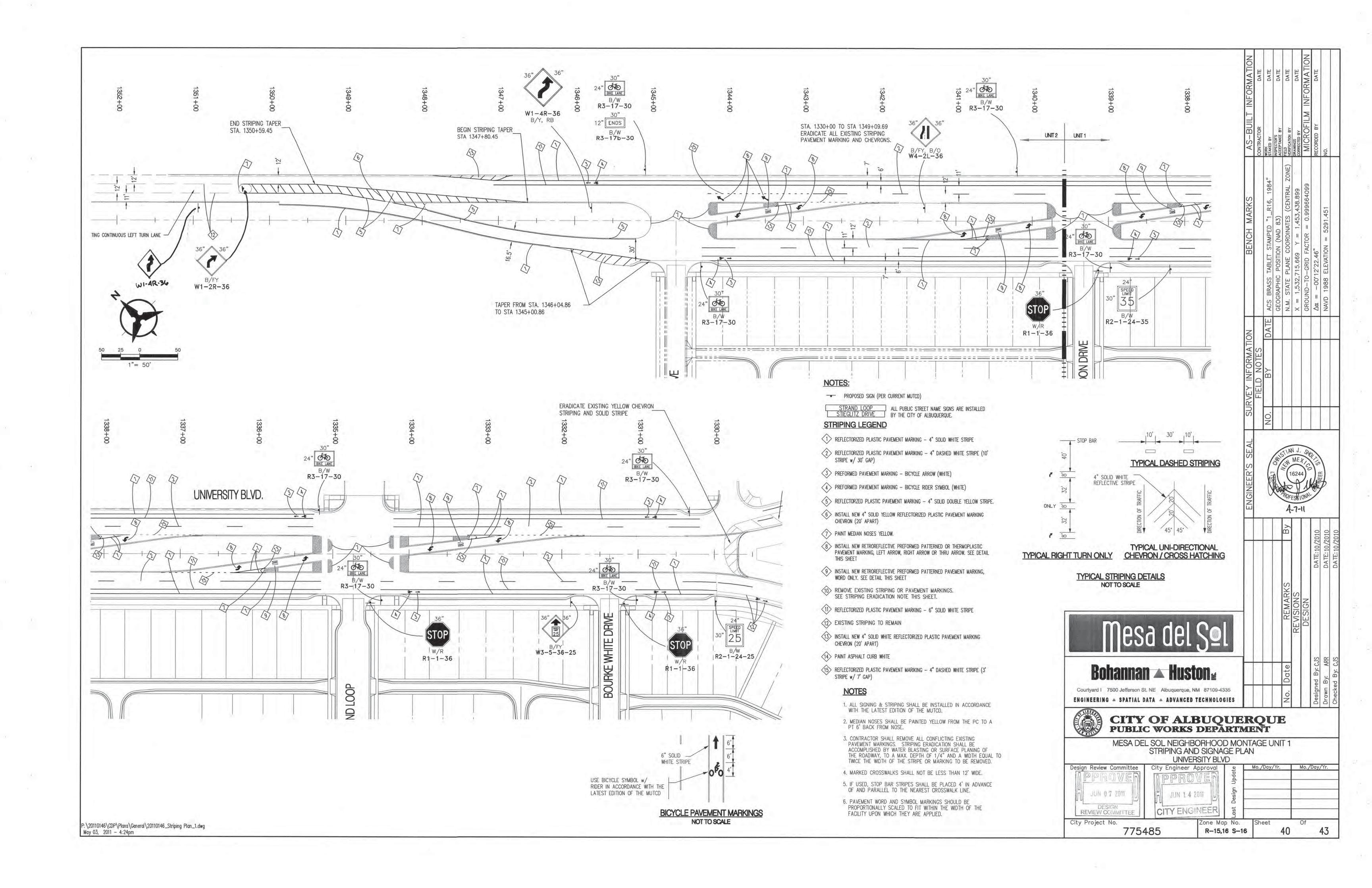
EXHIBIT B - INFRASTRUCTURE AS-BUILTS

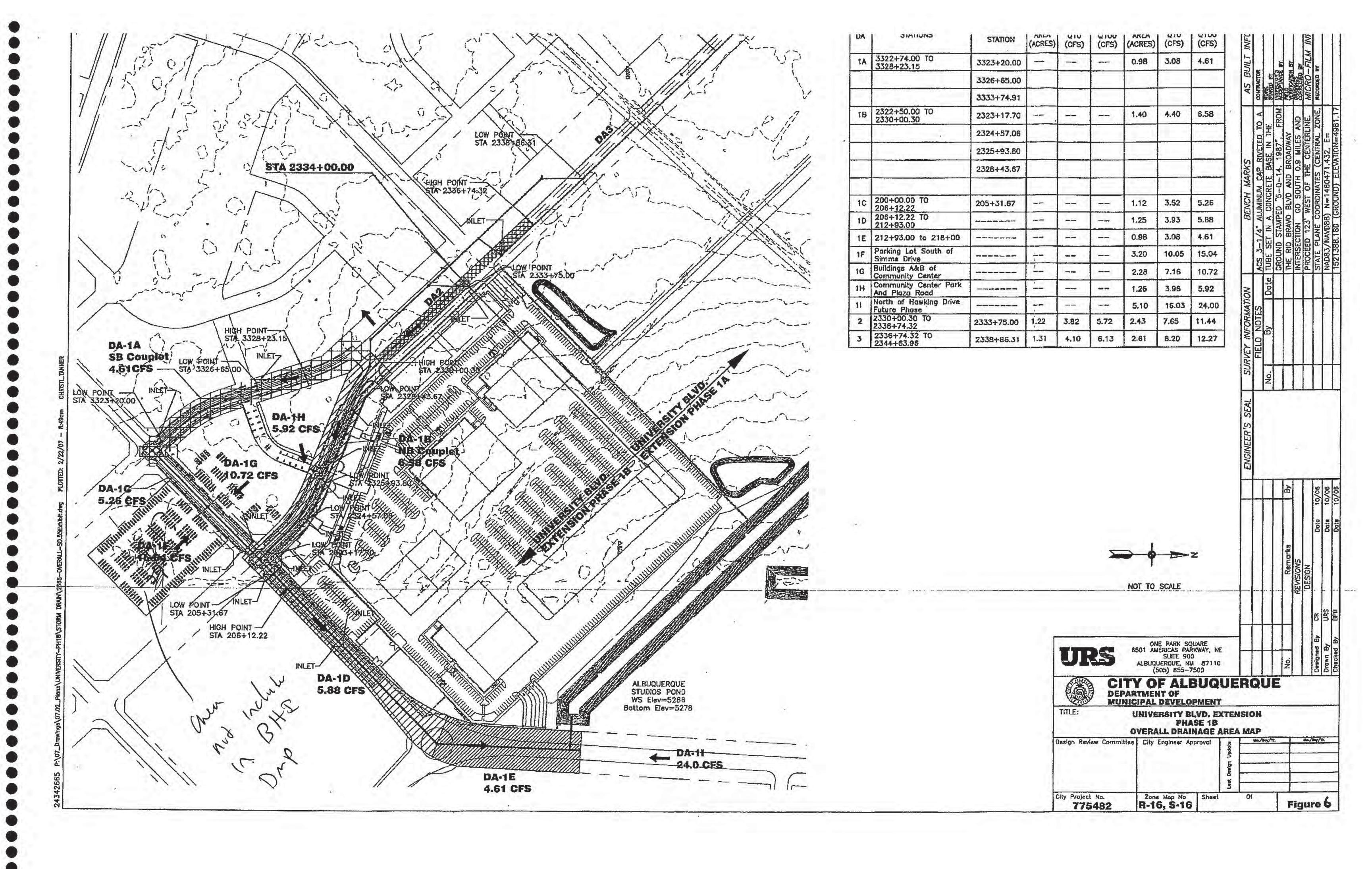


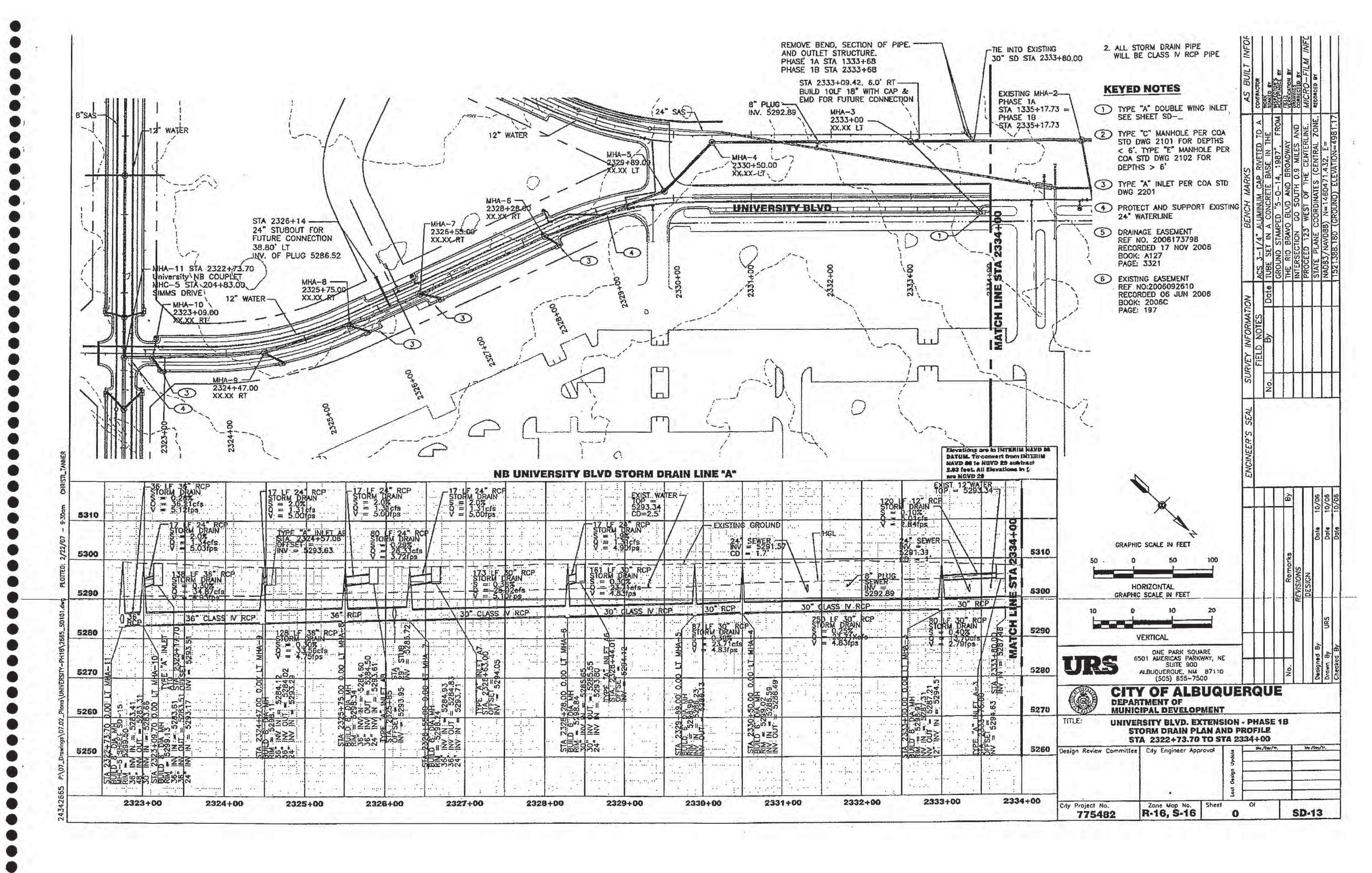


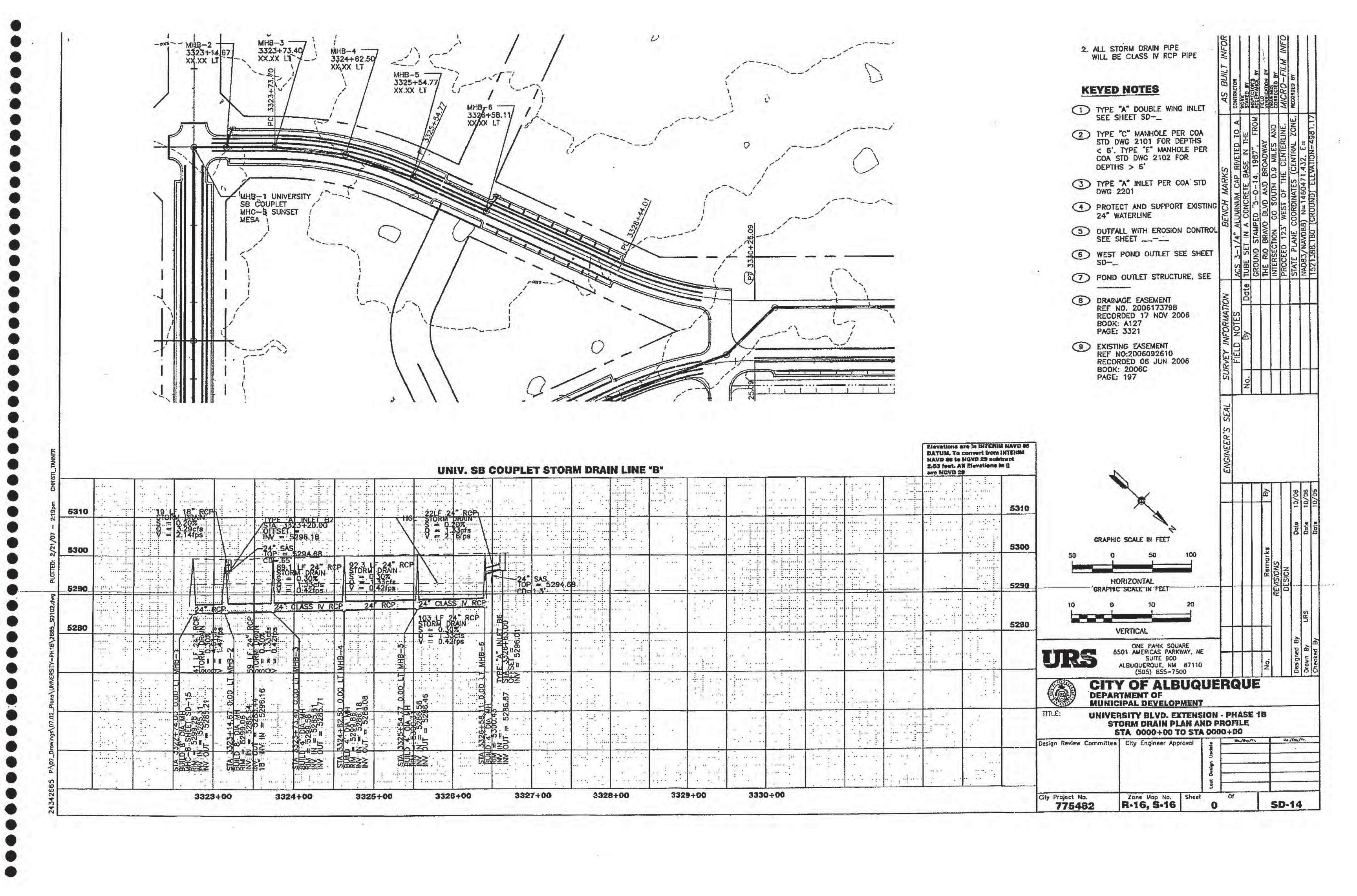












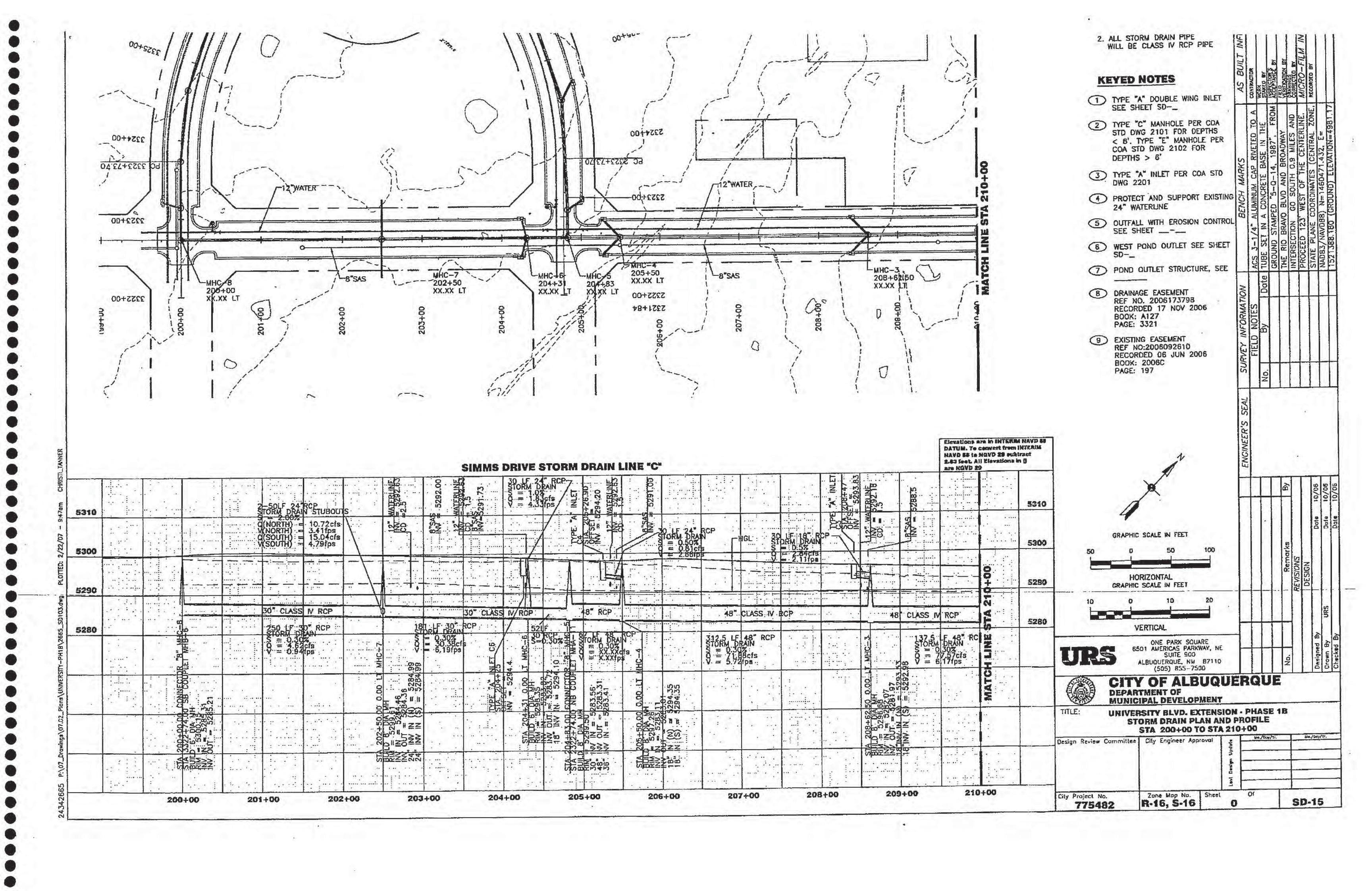
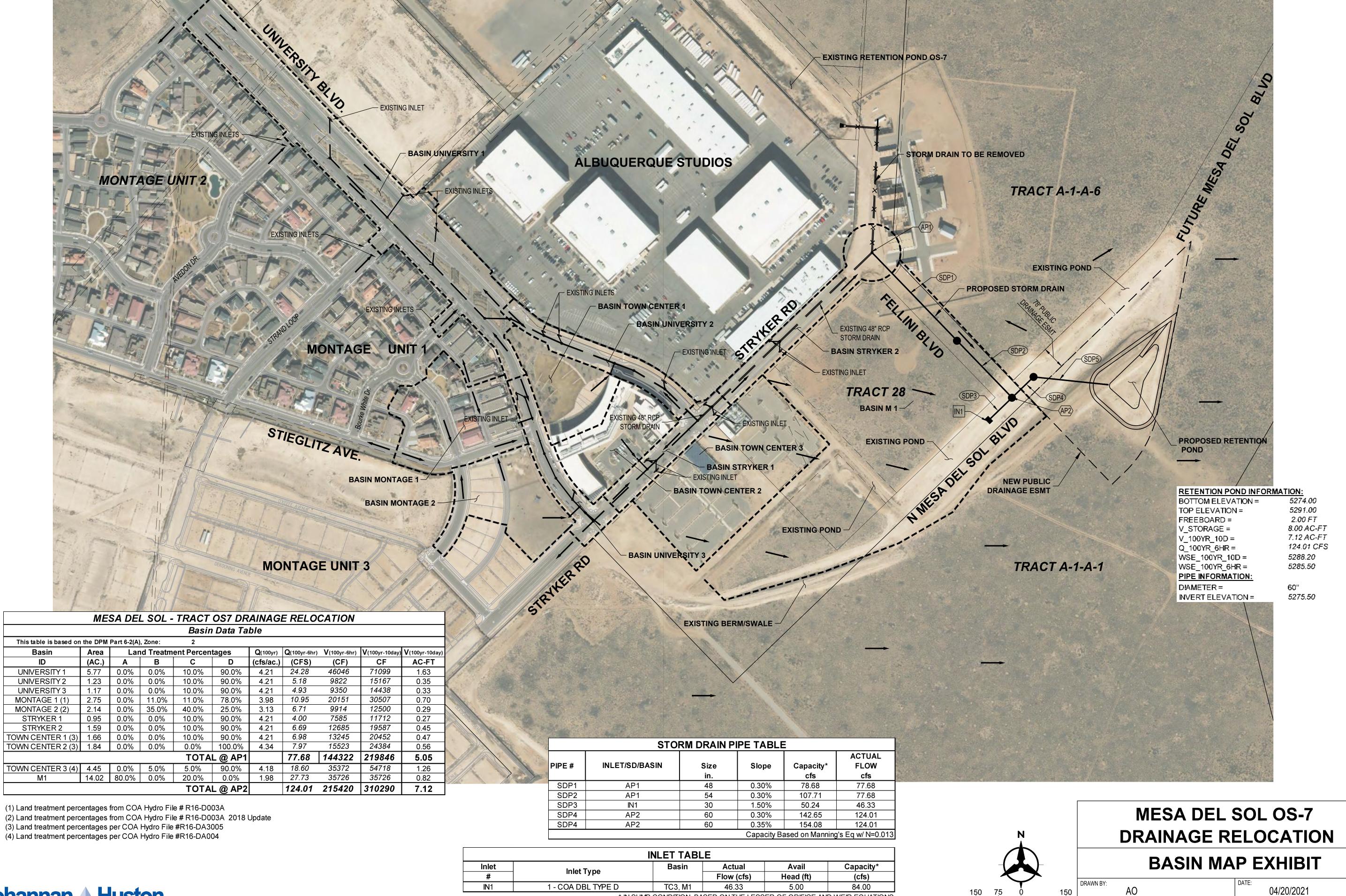


EXHIBIT C -BASIN MAP EXHIBIT



* IN SUMP CONDITION, BASED ON THE LESSER OF ORIFICE AND WEIR EQUATIONS

CHECKED BY:

MJB

1" = 150'

BHI PROJECT NO.

20210391

1 OF 1

Bohannan A Huston
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EXHIBIT D -GRADING PLAN

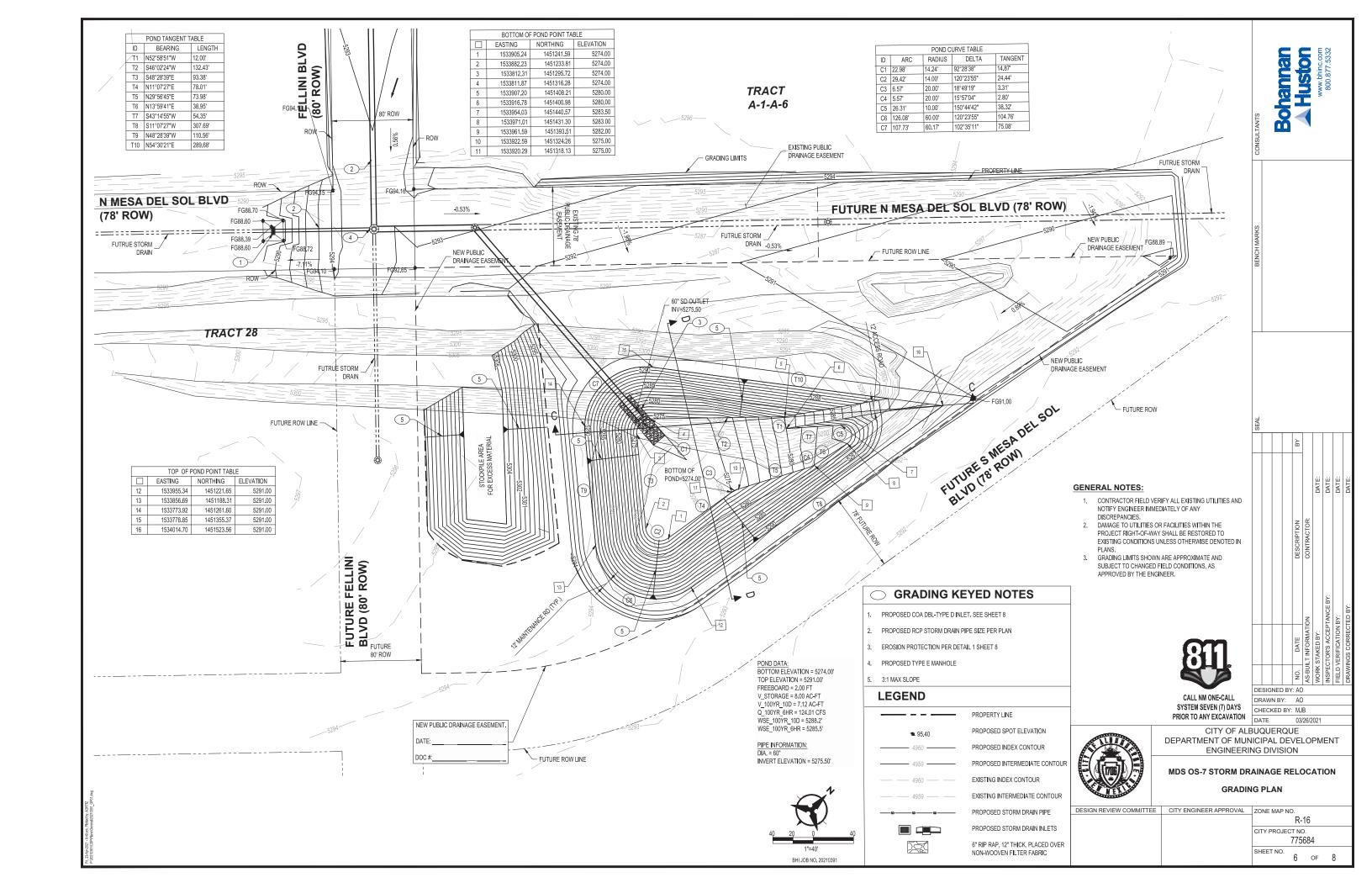
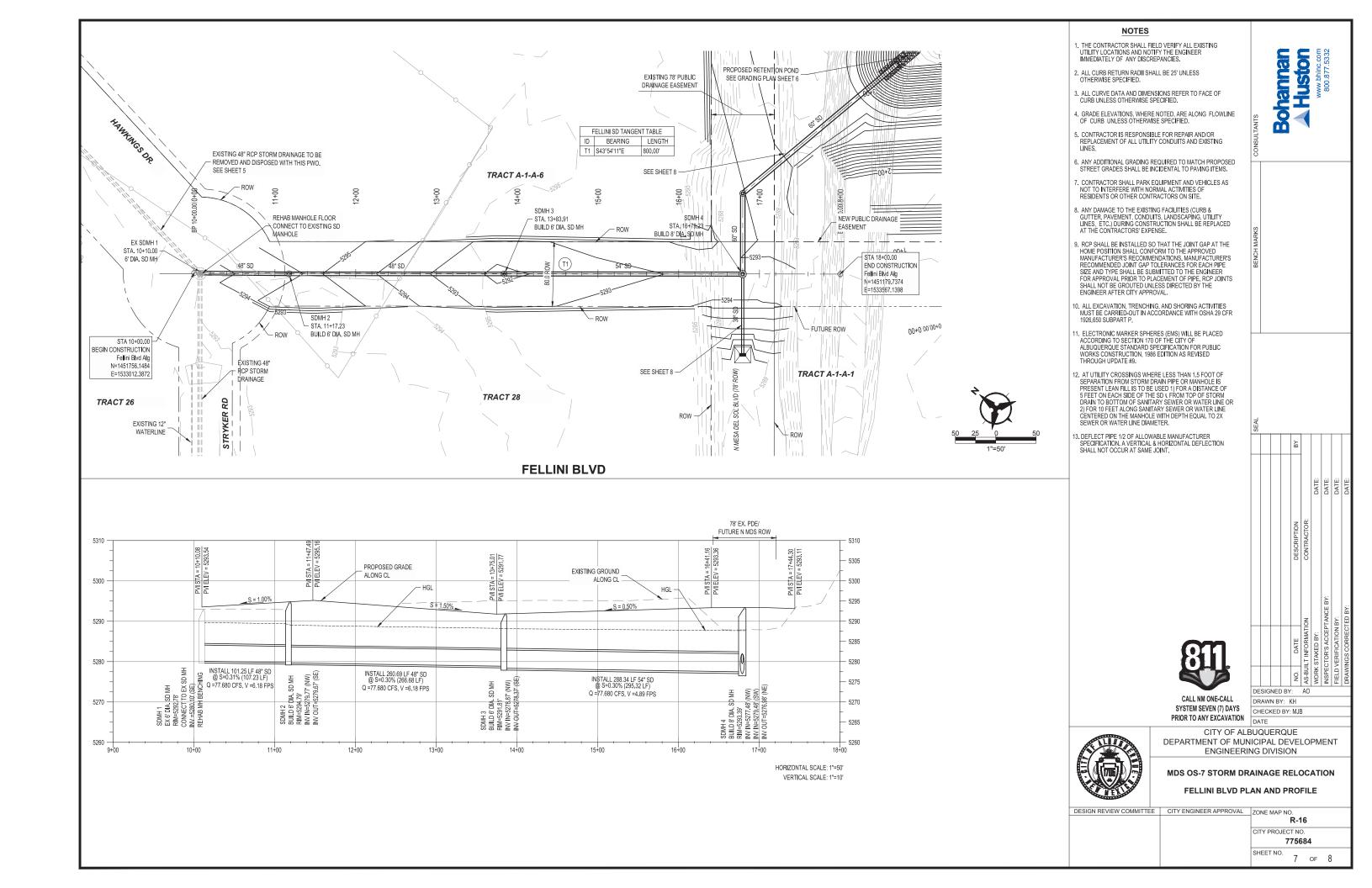
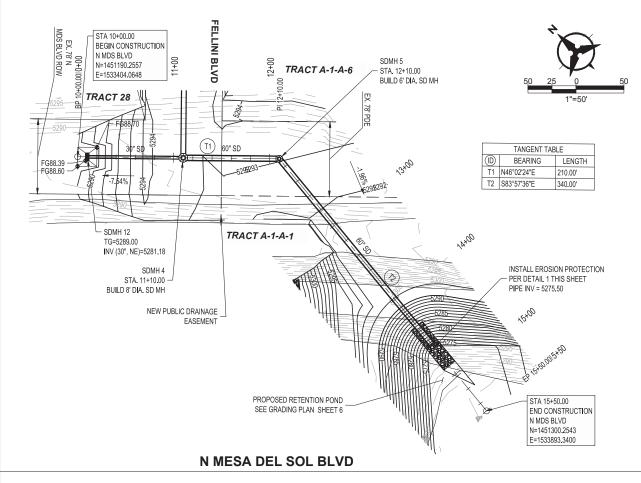
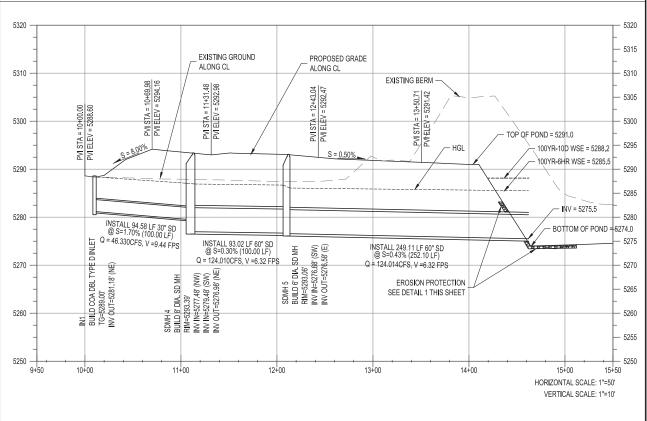
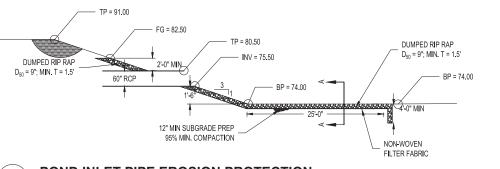


EXHIBIT E -FELLINI BLVD. PNP

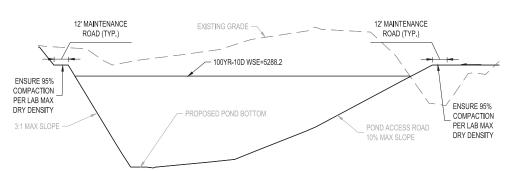




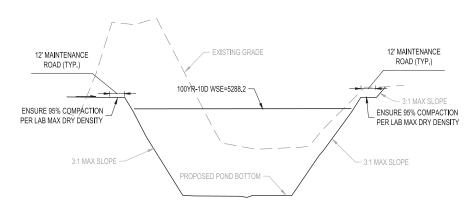




POND INLET PIPE EROSION PROTECTION 12" MIN SUBGRADE PREP DUMPED RIP RAP 95% MIN. COMPACTION NON-WOVEN MIN. THICKNESS = 1.5' **SECTION A-A**



SECTION C-C SOUTH TO NORTH POND



SECTION D-D **WEST TO EAST POND**

NOTES

- . THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING UTILITY LOCATIONS AND NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCIES
- 2. ALL CURB RETURN RADII SHALL BE 25' UNLESS
- 3. ALL CURVE DATA AND DIMENSIONS REFER TO FACE OF CURB UNLESS OTHERWISE SPECIFIED.
- 4. GRADE ELEVATIONS, WHERE NOTED, ARE ALONG FLOWLINE OF CURB UNLESS OTHERWISE SPECIFIED.
- 5. CONTRACTOR IS RESPONSIBLE FOR REPAIR AND/OR REPLACEMENT OF ALL UTILITY CONDUITS AND EXISTING
- 3. ANY ADDITIONAL GRADING REQUIRED TO MATCH PROPOSED STREET GRADES SHALL BE INCIDENTAL TO PAVING ITEMS.
- CONTRACTOR SHALL PARK EQUIPMENT AND VEHICLES AS NOT TO INTERFERE WITH NORMAL ACTIVITIES OF RESIDENTS OR OTHER CONTRACTORS ON SITE.
- . ANY DAMAGE TO THE EXISTING FACILITIES (CURB & ANT DAWNSET OF THE EXISTING PAULITIES COMPA GUTTER, PAVEMENT, CONDUITS, LANDSCAPING, UTILITY LINES, ETC.) DURING CONSTRUCTION SHALL BE REPLACED AT THE CONTRACTORS' EXPENSE.
- 9. RCP SHALL BE INSTALLED SO THAT THE JOINT GAP AT THE HOME POSITION SHALL CONFORM TO THE APPROVED MANUFACTURER'S RECOMMENDATIONS. MANUFACTURER'S RECOMMENDED JOINT GAP TOLERANCES FOR EACH PIPE SIZE AND TYPE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO PLACEMENT OF PIPE. RCP JOINTS SHALL NOT BE GROUTED UNLESS DIRECTED BY THE ENGINEER AFTER CITY APPROVAL.
- 0. ALL EXCAVATION, TRENCHING, AND SHORING ACTIVITIES MUST BE CARRIED-OUT IN ACCORDANCE WITH OSHA 29 CFR
- ELECTRONIC MARKER SPHERES (EMS) WILL BE PLACED ACCORDING TO SECTION 170 OF THE CITY OF ALBUQUERQUE STANDARD SPECIFICATION FOR PUBLIC WORKS CONSTRUCTION, 1986 EDITION AS REVISED THROUGH UPDATE #9.
- 2. AT UTILITY CROSSINGS WHERE LESS THAN 1.5 FOOT OF SEPARATION FROM STORM DRAIN PIPE OR MANHOLE IS PRESENT LEAN FILL IS TO BE USED 1) FOR A DISTANCE OF 5 FEET ON EACH SIDE OF THE SD & FROM TOP OF STORM DRAIN TO BOTTOM OF SANITARY SEWER OR WATER LINE OR 2) FOR 10 FEET ALONG SANITARY SEWER OR WATER LINE CENTERED ON THE MANHOLE WITH DEPTH EQUAL TO 2X SEWER OR WATER LINE DIAMETER
- 3. DEFLECT PIPE 1/2 OF ALLOWABLE MANUFACTURER SPECIFICATION. A VERTICAL & HORIZONTAL DEFLECTION SHALL NOT OCCUR AT SAME JOINT



DESIGNED BY: AO DRAWN BY: KH CHECKED BY: MJB

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CALL NM ONE-CALL SYSTEM SEVEN (7) DAYS PRIOR TO ANY EXCAVATION

CITY OF ALBUQUERQUE DEPARTMENT OF MUNICIPAL DEVELOPMENT **ENGINEERING DIVISION**



MDS OS-7 STORM DRAINAGE RELOCATION

POND INLET PIPE PNP

DESIGN REVIEW COMMITTEE CITY ENGINEER APPROVAL ZONE MAP NO. CITY PROJECT NO. 775684 SHEET NO.