



**Albuquerque Studios Master Plan
Development**

Traffic Impact Study

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ALBUQUERQUE STUDIOS MASTER PLAN DEVELOPMENT

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Executive Summary

This Traffic Impact Study (TIS) was prepared for the Albuquerque Studios Master Plan development (Phase II) and incorporates Phase I traffic at the Albuquerque Studios Site located at (5650 University Blvd) Mesa del Sol (MdS) in Southeast Albuquerque. The study area for the Studio Master Plan development (Phase I plus Phase II) encompasses University Boulevard, Mesa del Sol Boulevard (future), Eastman Avenue, and Crick Avenue adjacent to the site frontage and areas within the boundary. The study area is shown in **Figure 3**.

The Studio Master Plan development is anticipated for build-out, implementation and opening in Quarter four (4) of Year 2023 (two years from present).

Phase I (Northern Phase) consists of an expansion of the existing film studio production operation to the north of existing (approximately 27 acres) as follows:

• Vendor Village	2 Buildings	100 TSF
• Mill	2* Buildings	50 TSF
• Production Office	1 Buildings	145 TSF
• Total	5 Buildings	295 TSF

*A second Mill is proposed as a replacement for an existing Mill for a net-zero increase in traffic for this building replacement.

The total building square footage of the existing Albuquerque Studios site is approximately 331 TSF.

Phase II (Eastern Phase) consists of an expansion of the existing and north film studio production operations to the east portion of the site (approximately 82 acres) as follows:

• Production Office	1 Building	75 TSF
• Mills	2 Buildings	120 TSF
• Double Stages, production support with basecamps	5 Units	260 TSF
• Daycare	1 Building	15 TSF
• Total	9 Buildings/units	470 TSF

Phase II also consists of vacating a portion of Hawking Drive from Stryker Road to Eastman Avenue and Eastman Avenue from Gate D to Mesa del Sol Boulevard.



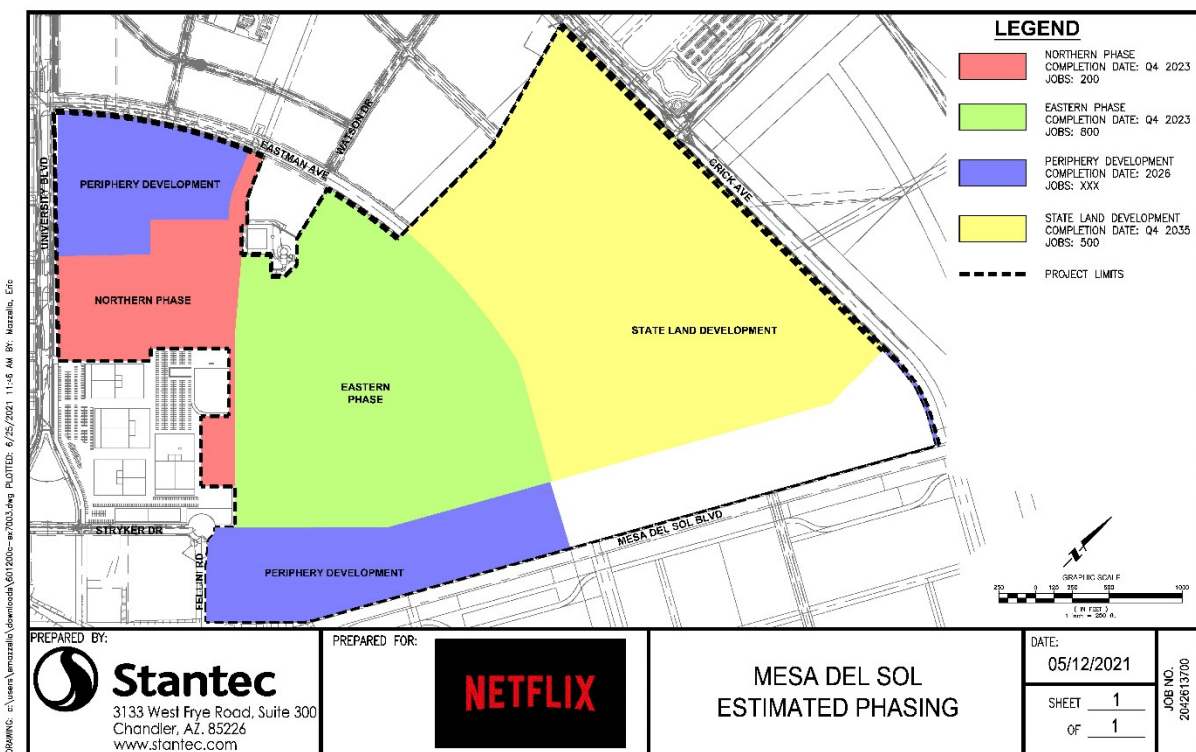
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Phase III consists of site development (site periphery use) on the north (approximately 22 acres) and south (approximately 59 acres) portions of the site. The following periphery development is anticipated to occur by the end Year 2026.

- Office Headquarters (North) 400 TSF
- Office (South) 400 TSF
- Retail (South) 150 TSF

Additionally, the Owner has leased approximately 114 acres from the State Land Office (SLO) east of the eastern phase for long-term use. This long-term use is excluded from this TIS and will be considered at a future time when plans have been more fully developed.

Figure 1. Studio Master Plan Phasing Exhibit



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The Average Daily Traffic (ADT) on University Blvd is 3,602 vehicles per day (April 2021) with nearly equal directional distribution (50%) in the northbound and southbound directions. The percentage of Heavy Commercial (%HC) was determined to be 5.15% during the study count period in April 2021.

The peak hour periods varied for the study area. On University Boulevard adjacent to the site, the corresponding AM Peak Hour, Noon Peak Hour, and PM Peak Hour occurred from 7:45 AM to 8:45 AM, 11:45 AM to 12:45 PM, and from 4:15 PM to 5:15 PM, respectively.

Six gates (Gates A-F) are proposed for the Studio Master Plan development area and are labeled on the site plan. Two additional future gates (Gates G and H) are preliminarily planned for the future studio development of the SLO area.

The projected trip generation for each gate is forecasted to be 43 in the AM Peak Hour and 43 for the PM Peak Hour.

The projected trip generation for the north periphery development is 260 in the AM Peak Hour, and 240 in the PM Peak Hour.

The projected trip generation for the south periphery development per driveway is: 78 in the AM Peak Hour and 146 in the PM Peak Hour.

Relative to the approved Level B Master Plan for MdS, this Albuquerque Studios Master Plan development including periphery is estimated at approximately 575 Thousand Square Feet (TSF) less than the current Level B Master Plan. In the AM peak hour, a decrease in overall trips by 739, including a decrease of 812 trips in the critical inbound direction (1071 vs. 1883), is forecasted. In the PM peak hour, a decrease of 202 trips in the critical outbound direction (1269 vs. 1471) is forecasted. During the PM peak hour in the inbound direction, the studio uses generate 112 trips compared to 103 trips for the current Level B Master Plan; however, with periphery development (retail and office), the PM inbound is a total of 775 trips due to retail use, resulting in an increase of 652 trips. The added retail use is complementary to the nearby large employment and residential areas and is not expected to have a regional impact. Future development of the SLO area with similar uses as proposed for the Studio Master Plan area would result in additional traffic reductions.

The traffic composition forecast on Hawking Drive and Eastman Avenue consists of through (background) traffic and site traffic. Upon vacation of Hawking Drive and Eastman Avenue, the traffic will be redistributed from Eastman Avenue and Hawking Drive to the adjacent roadways of University Boulevard, Mesa del Sol Boulevard, Crick Avenue and Watson Drive.



Figure 2 shows the forecasted ADT volumes for the Study Area based on the current Level B Master Plan with a comparison to the forecasted ADT volumes for the Study Area with the reduced Studio Master Plan trip generation (note: roadway vacations are not included to provide a direct comparison of trip generation effects only).



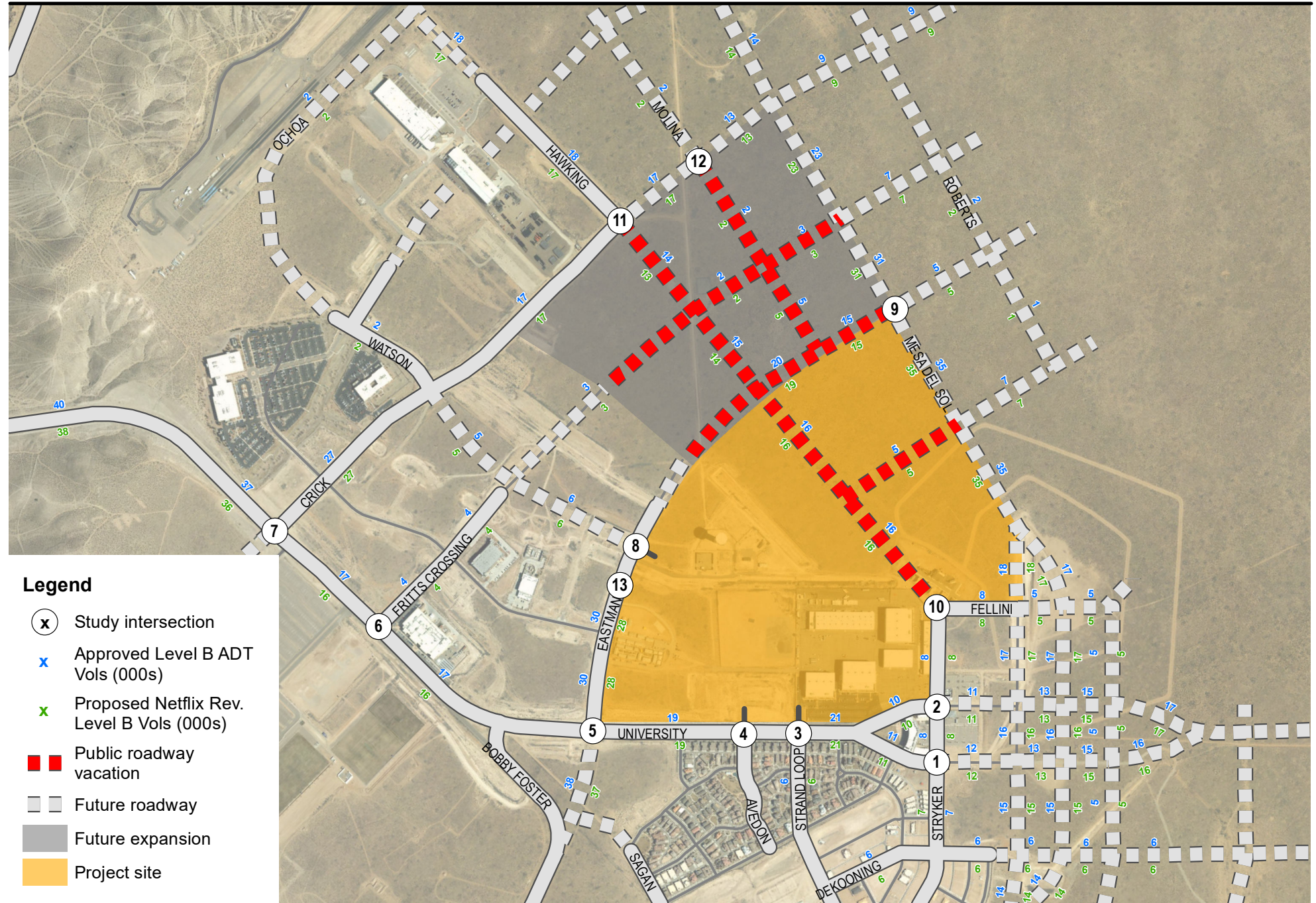


Figure 2

Proposed Level B ADT Volumes

Site Access Recommendations

Site access recommendations include all gates to be constructed with two exit lanes, two enter lanes, and stop-controlled traffic operation. Gates should be constructed similar to Gate A to allow for pedestrian access with ADA accommodations and required queue length of 160 feet (FT) on-site and 100 FT Left Turn bays on public streets where possible. This storage length is beyond the required length needed and analyzed. It is recommended that turn-around locations be provided in advance of the gate. Additionally, pull-off areas designated for taxi and ridesharing should be provided at each gate.

Adjacent Street Recommendations

- University Boulevard: Minor modifications to accommodate Gate A improvements and Gate B addition.
- Mesa del Sol Boulevard: Construction of ½ street improvements from Fellini Road to Crick Avenue for Master Plan traffic implementation year. Construct full build-out for horizon year traffic.
- Eastman Avenue: Construction of ½ street improvements from University Boulevard to Gate D for Master plan traffic implementation year. Construct build-out for horizon year traffic.
- Crick Avenue: Construction of ½ street improvements to connect Crick Avenue west to Mesa del Sol Boulevard for Master Plan traffic implementation year; Construct full build-out for Horizon Year traffic.
- Watson Drive: Watson Drive is an un-built street that would run N-S east of University. Watson Drive is currently proposed as a two-lane connector street in the Level B Master Plan for MdS. During the long-term horizon year, a four-lane facility should be considered to accommodate the future traffic volumes. It is recommended that a traffic analysis be revisited to assess the growth in MdS to validate this recommendation. As an alternative, a new two-lane connector could be considered along the west side of the SLO parcel (east of Watson Drive) that could accommodate the anticipated traffic flow, leaving Watson Drive as a two-lane street.



Intersection Recommendations

1. University Boulevard and Eastman Avenue: Implementation year recommendations include a signalized intersection in the near term when one or more of the following “triggers” occur:

- Mixed-use development growth to the west of the site,
- Athletic Facilities/Complex improvements by Bernalillo County,
- Bobby Foster re-alignment,
- north periphery build-out and/or other development northeast of University and Eastman Avenue that will contribute additional Horizon Year traffic flows at this intersection.

It is also recommended that width for dual lefts for Westbound to Southbound for horizon year traffic conditions be included that can be re-marked as necessary in the future.

2. Mesa del Sol and Crick Avenue: Continuous flow Northbound to Westbound and Eastbound to Southbound movements until such time as other MdS development occurs north and east creating the need for additional traffic control for conflicting movements at this intersection. Ultimate build-out for the horizon year is recommended as a signalized intersection. Exclusive lanes (left-turn, through, right-turn) are recommended for horizon year traffic flows.

3. Mesa del Sol Couplets: Recommended for construction and implementation consistent with the Level B Horizon Year time frame.

Periphery Driveway Recommendations

North Driveway: The north periphery driveway is recommended for the office build-out lanes use planned for this tract. Two inbound lanes, two outbound separated by a minimum four (4) FT raised median is recommended. Stop control is recommended for the outbound access lanes. The geometric designed to accommodate WB-50 design vehicle.

South Driveways: The south driveways are recommended for the forecasted office and retail use for these parcels. Driveways are recommended to be stop-controlled, 35 FT wide to provide two (2) outbound travel ways, and one inbound travel way. Geometry is to be designed per CABQ Curb Cut ordinance for either the WB-40 or WB 50 design vehicle.



Abbreviations

AADT	Annualized Average Daily Traffic
AAWDT	Annualized Average Weekday Traffic
ADA	Americans with Disabilities Act
ADT	Average Daily Traffic
AMPA	Albuquerque Metropolitan Planning Area
AWSC	All-way Stop Control
BC	Bernalillo County
CABQ	City of Albuquerque
COVID	Coronavirus
DPM	Development Process Manual
FAR	Floor Area Ratio
HC	Heavy Commercial
ITE	Institute of Transportation Engineers
ITS	Intelligent Transportation System
LOS	Level of Service
MdS	Mesa del Sol
MRCOG	Mid-Region Council of Governments
NMDOT	New Mexico Department of Transportation
PC	Planned Community
PHF	Peak Hour Factor
SLO	State Land Office



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SF	Square Feet
TAQA	Traffic Analysis and Querying Application
TIS	Traffic Impact Study
TMC	Turning Movement Count
TWSC	Two-Way Stop Control
SWA	Signal Warrant Analysis
UNM-TRU	University of New Mexico Traffic Research Unit
VPH	Vehicles per Hour



1.0 INTRODUCTION AND BACKGROUND

This Traffic Impact Study (TIS) was conducted to support the Albuquerque Studios Master Plan and MdS Level B Master Plan amendment approval (including Partial Street Vacation for Hawking Street and Eastman Drive) for the expansion of the Albuquerque Studios site located at 5650 University Blvd in Mesa del Sol Planned Community in the City of Albuquerque. This TIS scope encompasses/includes the North Development Phase I TIS submitted on June 2, 2021, additional development for Phase 2 (East Studio Development) and includes a traffic assessment for site periphery uses described herein.

1.1 STUDY PURPOSE

The study purpose is to assess the traffic impacts on the roadway network within the study area for the Master Plan expansion of the Albuquerque Studios site.

1.2 STUDY PROCEDURES

The TIS procedures follow the current edition of the Development Process Manual (DPM), City of Albuquerque, dated September 4, 2020.

1.2.1 Information Sources

Existing traffic data were collected during the week of April 19, 2021, for use in this TIS. This data was used to develop the trip generation estimates for the site development and for development of the baseline for the traffic analyses.

Crash data were obtained from UNM-TRU (a division of the University of New Mexico geospatial and Population Studies Department) for the study area [TRU Request Data | Geospatial and Population Studies \(unm.edu\)](#)

The MRCOG website [Traffic Flow Maps and Busiest Intersections | Mid-Region Council of Governments, NM \(mrcog-nm.gov\)](#) was referenced in determination of background traffic and growth of traffic expected on University Blvd. Additionally, the Traffic Analysis and Querying Application (TAQA) available from MRCOG was referenced for existing traffic data to support this TIS.

“Big Data” Platform using anonymous cell phone “pings” and other Global Positioning System (GPS) devices was used to calibrate the traffic data for impacts of COVID to reflect Pre-COVID (“Normal”) traffic patterns and volumes was obtained for use in this TIS.

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Existing conditions

1.2.2 Scope

The scope of the TIS includes the Albuquerque Studios proposed site, exterior adjacent streets, and interior future streets considered for vacation (Hawking Drive and Eastman Avenue). Exterior streets include University Boulevard, Eastman Avenue, Mesa del Sol Boulevard (Future), and Crick Avenue (future) adjacent to the site. A total of eight (8) proposed gated access locations are proposed. Gate A is an existing gate for the site. Gate B is proposed for Phase I, Gates C-F are proposed for Phase II, and Gates G and H are anticipated for future development of the SLO area. Up to eleven (11) proposed driveways are proposed, ten (10) onto future Mesa del Sol Boulevard/Fellini Road and one (1) onto Eastman Drive for future “Periphery” site uses contemplated for mixed-use of Office and Retail by year 2026.

1.2.3 LOS

The desired Level of Service (LOS) corresponds to LOS C-D (Table 7.5.88 pg. 7-164 DPM). LOS is a traffic analysis term that represents the delay traveling through intersections. Traffic LOS is designated “A” through “F” with LOS A representing free flow conditions and LOS F representing severe traffic congestion.

Table 1. City of Albuquerque (CABQ) Level of Service (LOS) Criteria

Functional Classification and Roadway Type	Employment Center
Collector	LOS C-D
Arterial at Employment Center (EC)	LOS D

2.0 EXISTING CONDITIONS

The roadway network and existing conditions are described in this section. Also described are current traffic volumes and roadway conditions used in the traffic analysis for this TIS.

2.1 GENERAL AREA CHARACTERISTICS

The project site is located in the planned community of Mesa Del Sol in southeast Albuquerque. Land use for the site is designated as Employment Center. Adjacent to the site and to the west is residential single-family housing. The Mesa del Sol Master

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Existing conditions

Plan includes a complete mixed-use land use. The site is currently zoned as a Planned Community (PC). A copy of the zone atlas page R-16-Z is provided in the Appendix. Other planned development at Mesa del Sol includes residential construction, schools, and planned construction/expansion of athletic facilities.

The study area is shown in **Figure 3**. The Site Plan is shown in **Figure 4**.



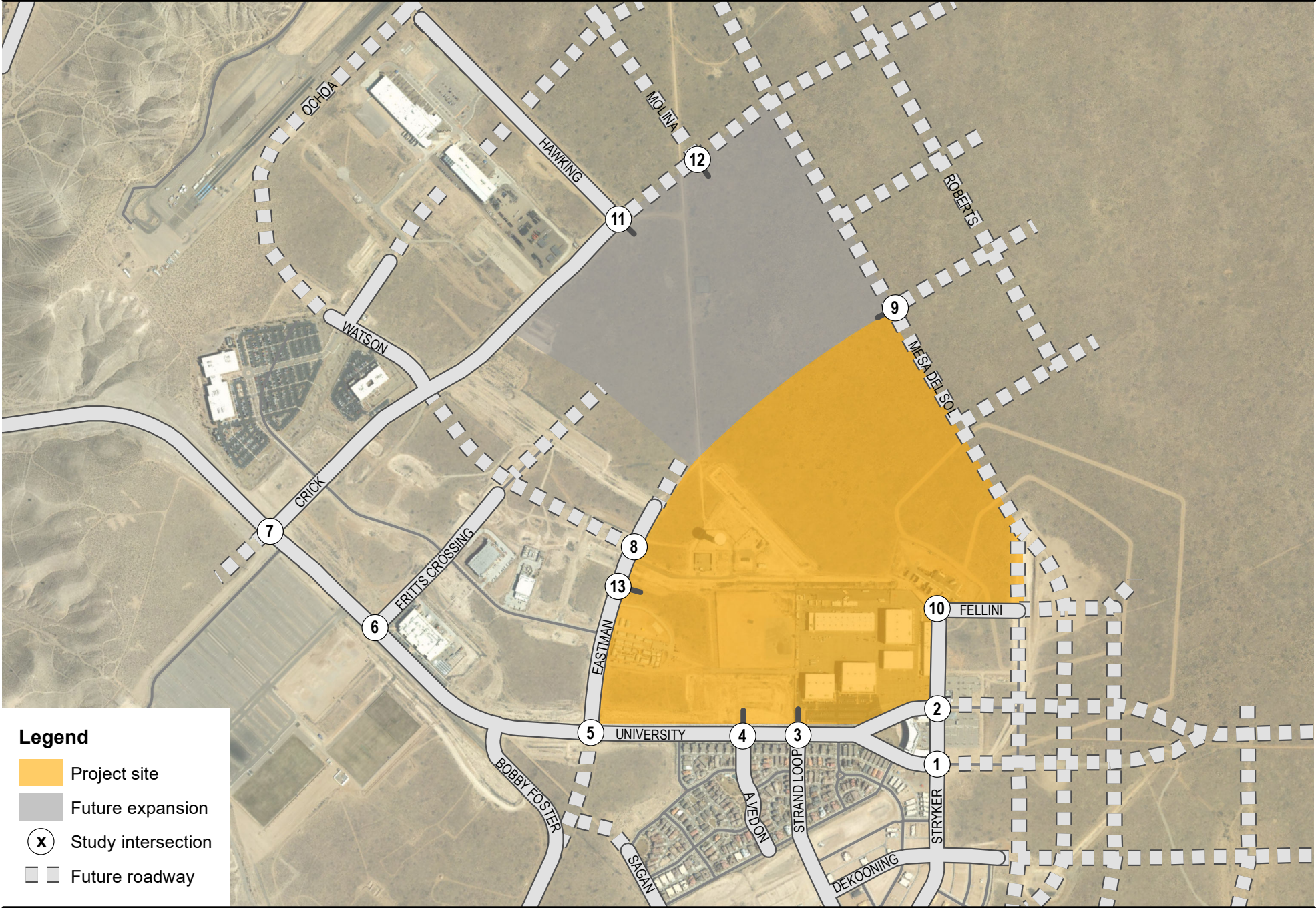
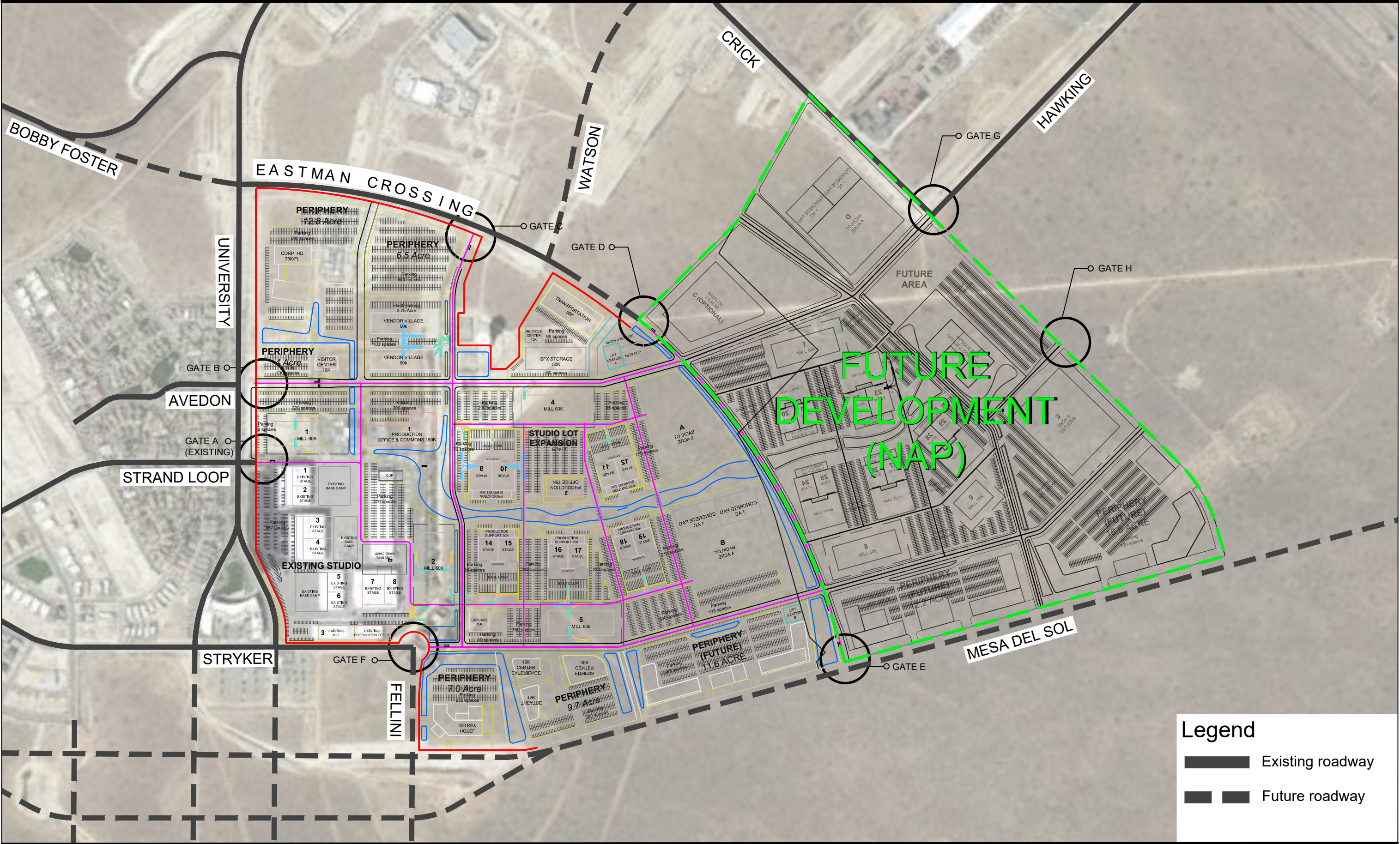


Figure 3
Study Area
4



Legend

- Existing roadway
- Future roadway

Figure 4
Project Site Plan
5

Existing conditions

2.2 AREA STREET NETWORK

The street network in the influence area includes University Boulevard adjacent to the site and the corresponding intersecting streets Strand Loop SE/Gate A and Avedon Avenue SE. Both intersecting streets provide access to single-family residential.

University Boulevard is classified as an Urban Major Collector Street (Source MRCOG, Roadway-Functional Classification in the Albuquerque Metropolitan Planning Area [AMPA]). University Boulevard is constructed with two standard width driving lanes in the northbound (NB) and southbound (SB) directions with parallel parking along the outside curb line. The existing speed limit is 35 mph for University Boulevard. Bike lanes (5 feet) are provided along University Boulevard on the outside of the right thru travel lane. The street is a paved typical urban typical asphalt concrete pavement section with roadway lighting along University.

University Boulevard has existing permanent “Wrong Way” and “Do Not Enter” signs to alert vehicles of the illegal movement of NB traffic traveling in the SB lanes.

Figure 5. Southbound University Boulevard at Site



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Existing conditions

University Boulevard is constructed with a raised landscaped median (50 feet wide). 110 feet long left-turn lanes exist at the median openings along University Boulevard.

2.3 EXISTING TRAFFIC VOLUMES

Traffic data were collected for the study along University Boulevard from Crick Avenue (North) to Stryker Road (South). Average Daily Traffic (ADT) and intersection turning movement counts (TMC) were collected for this (TIS). Traffic data were collected during the week of April 19th, 2021. Raw traffic data is provided in the appendix.

Traffic data were calibrated using the NMDOT Methodology issued in October 2020. An analysis was conducted pre-COVID (April 2019) and during COVID (April 2020). A corresponding factor was obtained and applied to the April 2021 TMC traffic data collected for this Study. Any outliers determined using the “Big Data” platform were limited to a 1.42 increase or a 0.42 decrease. This value was determined from the ADT decrease from April 2019 to April 2020.

Table 2. Summary of Intersection TMCs

Intersection	AM Peak Hour*	PM Peak Hour*
1. University (SB) and Stryker	69 (7:30 AM – 8:30 AM)	43 (3:45 PM – 4:45 PM)
2. University (NB) and Stryker	32 (8:45 AM – 9:45 AM)	44 (3:45 PM – 4:45 PM)
3. University and Gate A/Strand Loop	200 (7:45 AM – 8:45 AM)	286 (4:15 PM – 5:15 PM)
4. University and Avedon	320 (7:45 AM – 8:45 AM)	299 (4:15 PM – 5:15 PM)
5. University and Eastman Crossing	387 (7:45 AM – 8:45 AM)	308 (3:00 PM – 4:00 PM)
6. University and Bobby Foster	295 (7:45 AM – 8:45 AM)	322 (3:00 PM – 4:00 PM)
7. University and Fritts	504 (7:45 AM – 8:45 AM)	433 (3:00 PM – 4:00 PM)
8. University and Crick	597 (7:45 AM – 8:45 AM)	560 (3:00 PM – 4:00 PM)

*Data calibrated for COVID using NMDOT Methodology (Method 3, October 2020)



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Existing conditions

Table 3. Summary of ADT (April 2021)

Location	ADT – Direction 1	ADT – Direction 2
Stryker Rd.	233 (Eastbound)	250 (Westbound)
University Blvd*	1,602 (Northbound)	1,606 (Southbound)
*The percentage of Heavy Commercial (%HC) for University was 5.1%		

Table 4. Summary of Intersection TMCs

Intersection	AM Peak Hour*, **	PM Peak Hour*, **
1. Bobby Foster and Los Picaros	97 (7:45 AM – 8:45 AM)	82 (3:45 PM – 4:45 PM)
2. Bobby Foster and Broadway	882 (6:30 AM – 7:30 AM)	1118 (3:45 PM – 4:45 PM)
3. University and Rio Bravo	1,072 (7:45 AM – 8:45 AM)	976 (3:00 PM – 4:00 PM)
4. Rio Bravo and Broadway	3,769 (6:45 AM – 7:45 AM)	4,519 (3:00 PM – 4:00 PM)
5. 2 nd Street and Rio Bravo	3,750 (7:45 AM – 8:45 AM)	4,774 (3:00 PM – 4:00 PM)

*Data calibrated for COVID using NMDOT Methodology (Method 3, October 2020)

**Traffic Data Collected by others for use in MdS Mixed-use development project April 2021. Calibrated by Albuquerque Studios TIS team for inclusion and use in this study.

Traffic data calibration limits of 52% increase/decrease for Bobby Foster Road, 19% increase/decrease for Broadway, 39% increase/decrease for Rio Bravo Boulevard, and 47% increase/decrease for 2nd Street were developed and used in this study when calibrating the TMCs.

2.4 EXISTING LEVELS OF SERVICE (LOS)

Table 5. Summary of Existing LOS

Intersection	Traffic Control	AM Peak Hour Delay (sec), LOS	PM Peak Hour Delay (sec), LOS
1. University (SB) & Stryker	TWSC	0, A	9.3, A
2. University (NB) & Stryker	TWSC	8.3, A	0.0, A



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Existing conditions

3. University & Ex Gate/Strand Loop	TWSC	10.8, B	10.3, B
4. University & Future Gate B/Avedon	TWSC	10.1, B	9.8, A
5. University & Eastman Crossing	TWSC	8.8, A	9.3, A
6. University & Bobby Foster	TWSC	9.8, A	9.8, A
7. University & Fritts Crossing	TWSC	9.4, A	9.8, A
8. University & Crick	TWSC	9.2, A	9.7, A

The existing storage length for ingress traffic is 150 feet at the gate for the Southbound to Eastbound turning movement. Existing Gate A storage is for approximately twelve (12) vehicles on site and an additional four (4) vehicles on University Blvd. (SB to EB).

An existing queuing analysis was conducted on May 20, 2021. Arrivals to Gate A and processing of vehicles into the site were analyzed during the 11:30 AM to 1:00 PM noon peak period. The maximum queue length was two (2) vehicles during this period. Gate processing rates generally took less than five (5) seconds. Maximum gate processing times were approximately three (3) minutes. The maximum rate occurred on occasion but not routinely.

2.5 EXISTING TRANSIT SERVICE

At present time, public transit/transportation is not available to or from MdS. Rio Bravo Boulevard is the closest facility with transit service at the present time. Future transit service to MsD is currently being planned for. The nearest transit stop/service is from Rio Bravo Boulevard bus route 222.

2.6 BICYCLE AND PEDESTRIAN CONSIDERATIONS

Multi-modal transit plays an important role in the MdS community. Bike lanes exist along both sides of University Boulevard. Public sidewalks parallel University Boulevard and are constructed to meet Americans with Disabilities Act (ADA) requirements. Additional protection is provided to pedestrians with a landscaped buffer on University Boulevard.

Table 6. Existing Pedestrians and Bicycle Flow on University Boulevard

Intersection	Daily Pedestrian Flow	Daily Bicycle Flow
1. University (SB) and Stryker (9-hour)	26	2
2. University (NB) and Stryker (9-hour)	2	0
3. University and Gate A/Strand Loop (12-hour)	40	1
4. University and Avedon (9-hour)	33	0
5. University and Eastman Crossing (12-hour)	12	1
6. University and Bobby Foster (12-hour)	0	2



Existing conditions

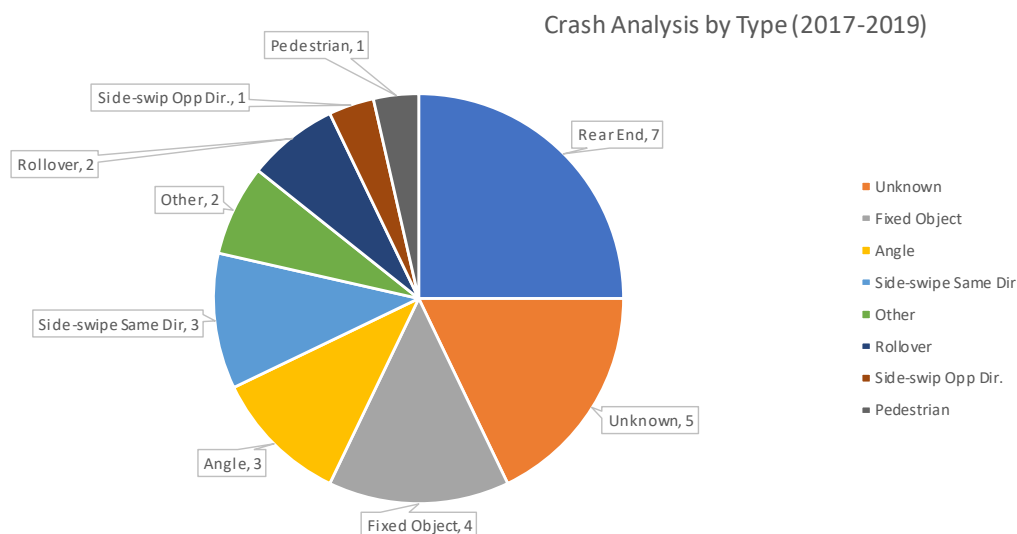
7. University and Fritts Crossing (9-hour)	4	2
8. University and Crick (9-hour)	1	0

2.7 SAFETY EVALUATION/CRASH DATA

Crash data along University Boulevard and in the study area were obtained from UNM-TRU for the three most recent years available (2017-2019). There was a total of twenty-eight (28) crashes during the three-year period. There were no reported fatal crashes. There was one (1) reported serious injury crash, two (2) non-serious injury crashes, three (3) possible injury crashes, and twenty-two (22) property damage only crashes. The average crash frequency for the area was just over nine (9) crashes per year during the study period.

The crash analysis revealed that the predominant types of crashes were rear end (7) and Fixed Object (4). There were five (5) crashes of unknown type. There were three (3) angle crashes and three (3) side-swipe same direction crashes. Angle crashes typically occur at intersections or access locations.

Figure 6. Crash Analysis (2017-2019)



future traffic conditions and analysis years

3.0 FUTURE TRAFFIC CONDITIONS AND ANALYSIS YEARS

3.1 PROJECT IMPLEMENTATION YEAR

The North Development (Phase I) and East Development (Phase II) are anticipated for implementation, build-out, and opening in Quarter 4 of 2023 (two years from present). The implementation year for the periphery development is year 2026.

3.2 SITE TRAFFIC

Site traffic is traffic attributable to the site development at time of implementation and opening (Q4, Year 2023).

The site traffic forecasted for the Studio development traffic (North and East Phases) during the AM and PM Peak hours for each gate are as follows:

- AM Peak Hour - Total of 43 Trip Ends
- PM Peak Hour - Total of 43 Trip Ends

The total traffic forecasted for the Periphery areas (North (1 driveway)) and South (up to 10 driveways) during the AM, and PM Peak hours for each driveway are as follows:

- North AM Peak Hour - Total of 260 Trip Ends
- North PM Peak Hour - Total of 216 Trip Ends
- South AM Peak Hour - Total of 88 Trip Ends
- South PM Peak Hour - Total of 146 Trip Ends

A Trip End is defined as either an arrival to the site or departure from the site.

The site traffic is further described and detailed in the Trip Generation Section of the report in Section 4.2 Trip Generation.

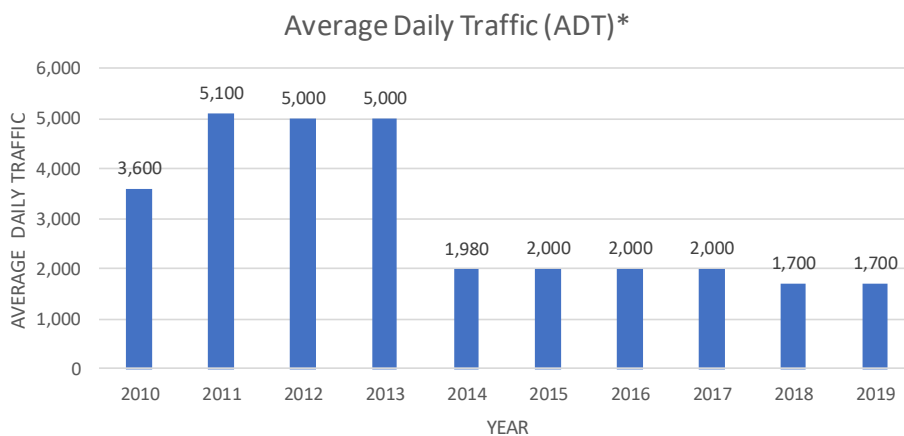
Internal Capture rates of 5% were used between residential and office and 10% for office to Retail. ITE Internal Capture data were reviewed. Upon review of the ITE data, engineering judgement was used to select the internal capture rates used in this analysis.

future traffic conditions and analysis years

3.3 GROWTH IN THROUGH TRAFFIC

A review of the MRCOG 10-year historical growth rate in traffic are shown to have declined from 3,600 ADT in 2010 to 1,700 in 2019. Based upon the decline, this study has elected to use a 0.5% annualized growth rate (Minimum required by DPM, reference pg. 7-167).

Figure 7. Historical Average Daily Traffic at Mesa del Sol



*Source MRCOG website.

The ADT forecasts for the current Level B Master Plan for MdS, amended for the Project and necessary roadway vacations are shown in **Figure 8**.

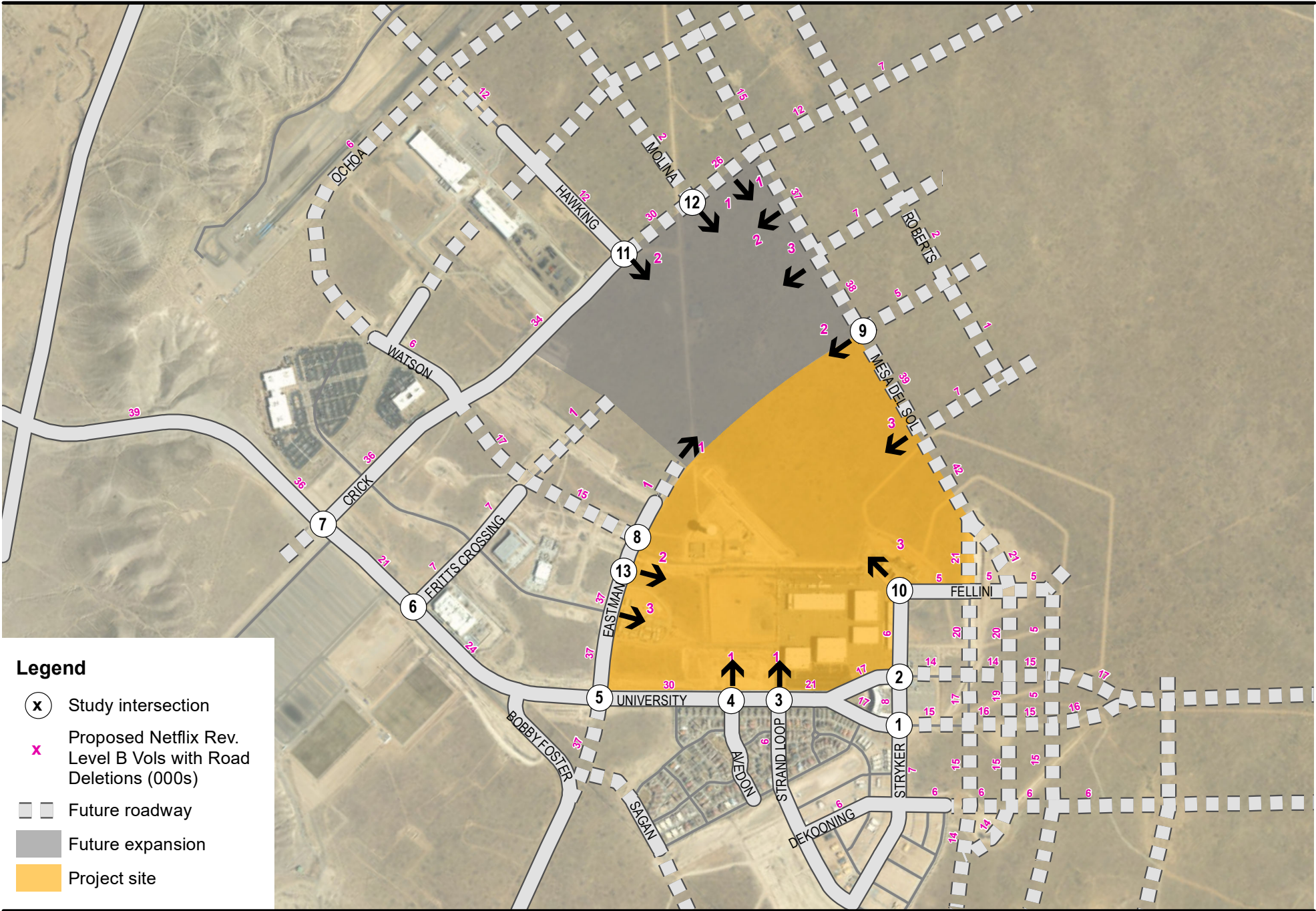


Figure 8

Amended Level B ADT Volumes with Roadway Deletions

future traffic conditions and analysis years

3.4 OTHER PLANNED DEVELOPMENT

There is ongoing development at MdS. The projects have been coordinating traffic data for existing conditions and proposed conditions for this study. This coordination is anticipated to continue with future master planning for this Albuquerque Studios Site. Other known planned development consists of residential, commercial, retail, and construction of a new school and athletic facilities.

Trip Generation provided to the Albuquerque Studios Project Development team for the MdS Mixed-use development is summarized as follows:

AM Peak Hour Enter	271
AM Peak Hour Exit	496
PM Peak Hour Enter	534
PM Peak Hour Exit	336

This site is located west of the Mesa del Sol residential area, south of Bobby Foster Road and west of University Boulevard. Based upon a review of the site, a trip distribution of 50% is forecasted to/from the west (Bobby Foster Road) and 50% is forecasted to/from University Boulevard. The implementation year for this TIS distributes the University Boulevard portion of the traffic 70% (University Boulevard & Bobby Foster Road/Eastman Avenue), 15% (University Boulevard & Avedon Avenue), and 15% (University Boulevard and Strand Loop SE):

Figures 9 and 10 show the AM and PM Peak Hour for 2026 without the Project based on the current MdS Level B Master Plan.

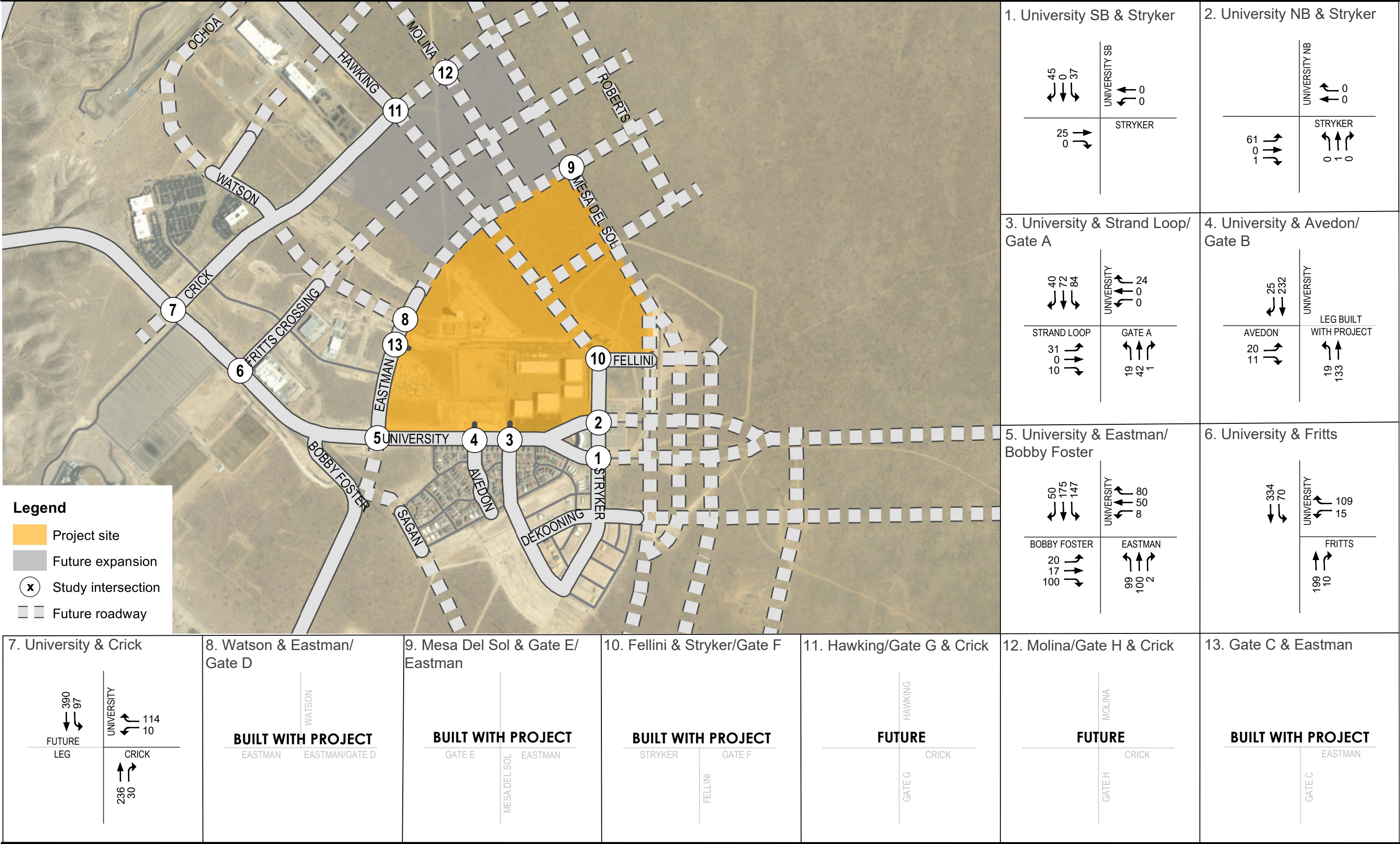


Figure 9

2026 No-Project AM Peak Hour Volumes

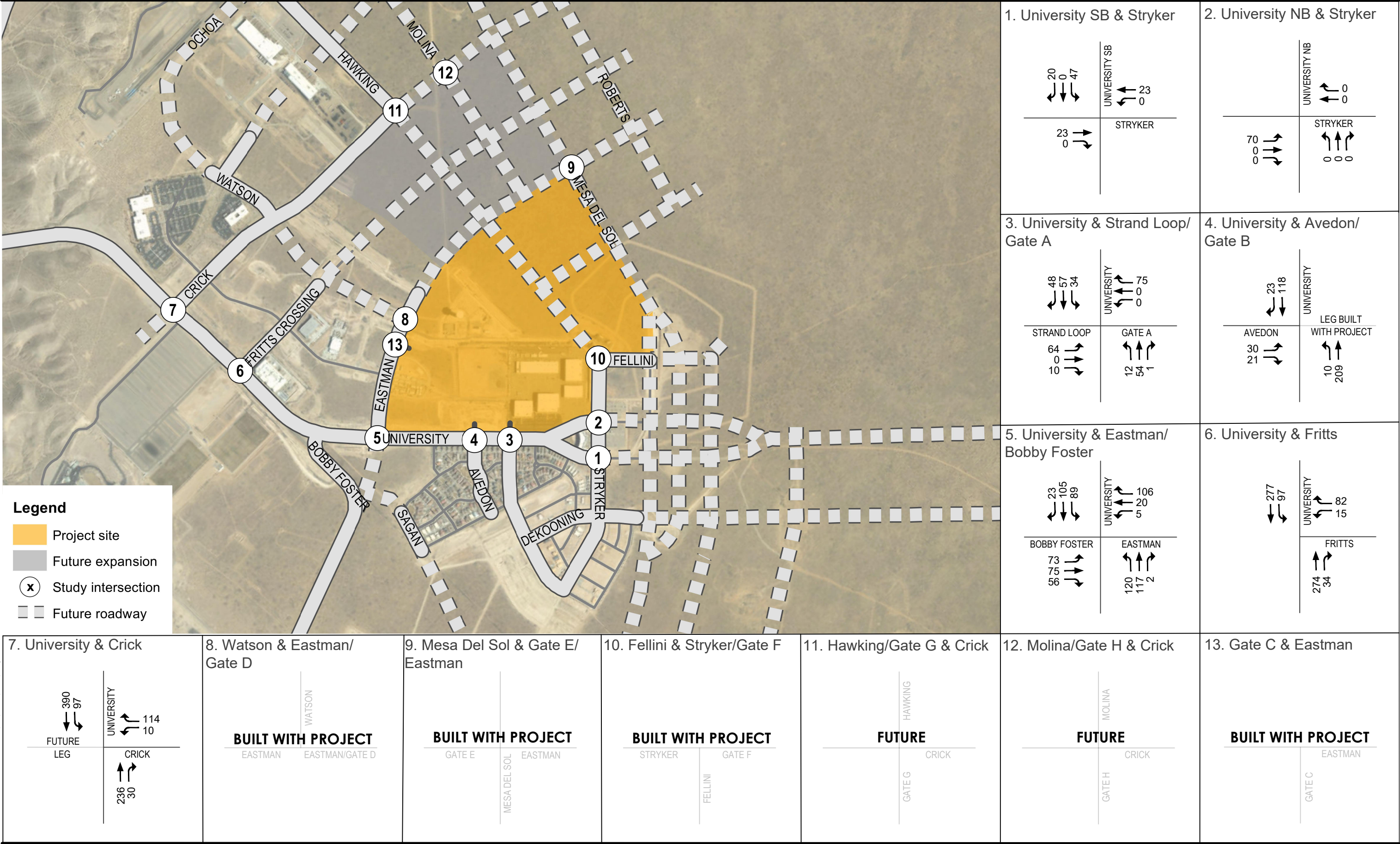


Figure 10

2026 No-Project PM Peak Hour Volumes

3.5 CONSIDERATION OF PROGRAMMED ROADWAY IMPROVEMENTS

Public stakeholders include the CABQ, BC, MRCOG, and NMDOT. Currently there are localized improvements planned north of the study area on University Boulevard. A widening project of the bridge over the Tijeras Arroyo is under construction. No other planned improvements are programmed for University Boulevard at this time.

Mesa del Sol Boulevard is a future proposed arterial roadway to Mesa del Sol. A timeline has not been established for the roadway at this time. Other improvements associated with Mesa del Sol Blvd include a study to plan and design of a new interchange at Mesa del Sol Boulevard & I-25. Also being studied are improvements at Bobby Foster/Los Picaros that include the possibility of a new Interchange with I-25. This study is expected to commence in Fall of 2021.

4.0 PROPOSED SITE TRAFFIC CHARACTERISTICS

4.1 SITE DEVELOPMENT CHARACTERISTICS

The North and East Development phases are proposed as an expansion of existing operations to reflect similar density and building facilities compared with the existing site. A summary of the existing site facilities and proposed facilities for the Northern Development are shown in Table 7 and Table 8, respectively.

Table 7. Existing Site Facilities

Development Summary	Quantity and Size
Building A, Stage 1, and 2	1 @ 50 TSF
Building B, Stage 3, and 4	1 @ 60 TSF
Stage 5 and 6	1 @ 36 TSF
Stage 7 and 8	1 @ 65 TSF
Mill	1 @ 80 TSF
Production Offices	1 @ 40 TSF
Total	6 @ 331 TSF
TSF – Thousand square feet	

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proposed site traffic characteristics

Table 8. Proposed Northern Development (Phase I)

Development Summary	Quantity and Size
Vendor Village	2 @ 50 TSF
Mill *	1 @ 50 TSF
Production Offices	1 @ 145 TSF
Total	4 @ 295 TSF
TSF – Thousand square feet	

*Note: Two (2) Mill Buildings are proposed for the North Development Phase, however one of the Mill Buildings is a replacement for an existing Mill Building on-site

Table 9. Proposed Eastern Development (Phase II)

Development Summary	Quantity and Size
Production Office	1 @ 75 TSF
Mills	2 @ 60 TSF
Double Stage Units with basecamp and support space	5 @ 52 TSF Average
Daycare	1 @ 15 TSF
Total	9 = 470 TSF
TSF – Thousand square feet	

Table 10. Proposed Phase III Periphery Development

Development Summary	Quantity and Size
Office (North)	4 @ 100 TSF
Office (South)	4 @ 100 TSF
Retail (South)	3 @ 50 TSF
Total	11 = 1,250 TSF
TSF – Thousand square feet	

State Land Office (Phase IV) future long-term development to year 2035 with similar supporting uses for the studio operations business is a future project. The SLO uses are excluded from the project evaluated by this TIS but the traffic generation is included in the horizon year background conditions.



proposed site traffic characteristics

4.2 TRIP GENERATION

Trip Generation for the Studios Expansion was estimated based upon existing conditions at Gate A. Existing Gate A traffic data was collected during April 2021 (April 20, 2021). April 2021 was indicated as a high-use period for business operations by the Owner. The April 2021 Gate A data were calibrated for COVID, following the NMDOT Guidelines (Method 3) issued in October 2020. The North Development Phase I is nearly equal to the size of the existing Albuquerque Studios Development (approximately 90% of the size). Corresponding trip generation is also projected at 90% of existing trip generation at Gate A. The Eastern Development is approximately 40% larger than the existing Albuquerque Studios site operations.

The periphery traffic was projected based upon the most conservative combinations of land-use contemplated for these areas, Office for the North Periphery, and Office and Retail for the South Periphery use. The ITE Trip Generation Manual was used for the periphery Traffic projections.

Table 11. Peak Hour Trip Generation Projection by Access Type for Albuquerque Studios Expansion and Periphery Development

Each Gate	Each Proposed Gate Total Proposed Gates = 8	North Proposed Periphery Driveway One Driveway Proposed	Each South Proposed Periphery Driveway Ten (10 Proposed Driveways
– Enter	Enter	Enter	Enter
– AM Peak Hour	33	247	56
– PM Peak Hour	13	6	64
– Exit	Exit	Exit	Exit
– AM Peak Hour	10	13	22
– PM Peak Hour	30	210	82

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proposed site traffic characteristics

The forecasted Trip Generation for the peak hour (Trips) by Building Type is summarized below:

Phase I (North)

	AM	PM
Mill (17%) of the Trips Generated:	17	17
Vendor Village (34%) of the Trips Generated	34	34
Production Offices (49%) of the Trips Generated	48	48

Phase II (East)

Mill (16%) of the Trips Generated	53	53
Production office (26%) of the Trips Generated	90	90
Stages (55%) of the Trips Generated	189	189
Day Care (3%) of the Trips Generated	10	10

Phase III Periphery

North Office (18%) of Periphery Trips Generated	260	217
South Office/Retail (82%) of the Periphery Trips Generated)	780	1,460

Figure 11 and **Figure 12** show the AM and PM Peak Hour 2026 with Project traffic volumes for the site. **Figure 13** and **Figure 14** show the Buildout year with Project AM and PM peak hour volumes.



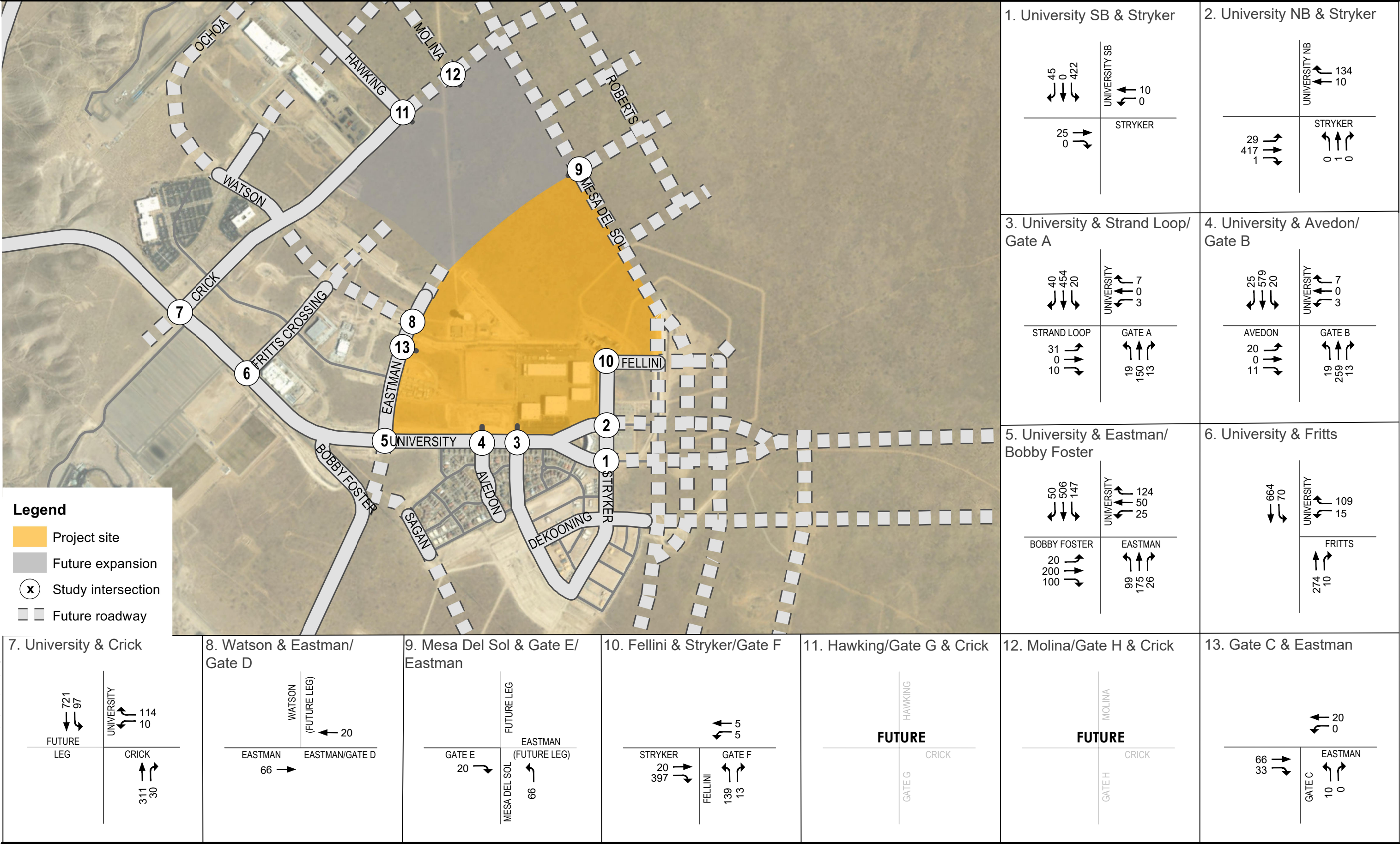


Figure 11
2026 With Project AM Peak Hour Volumes
21

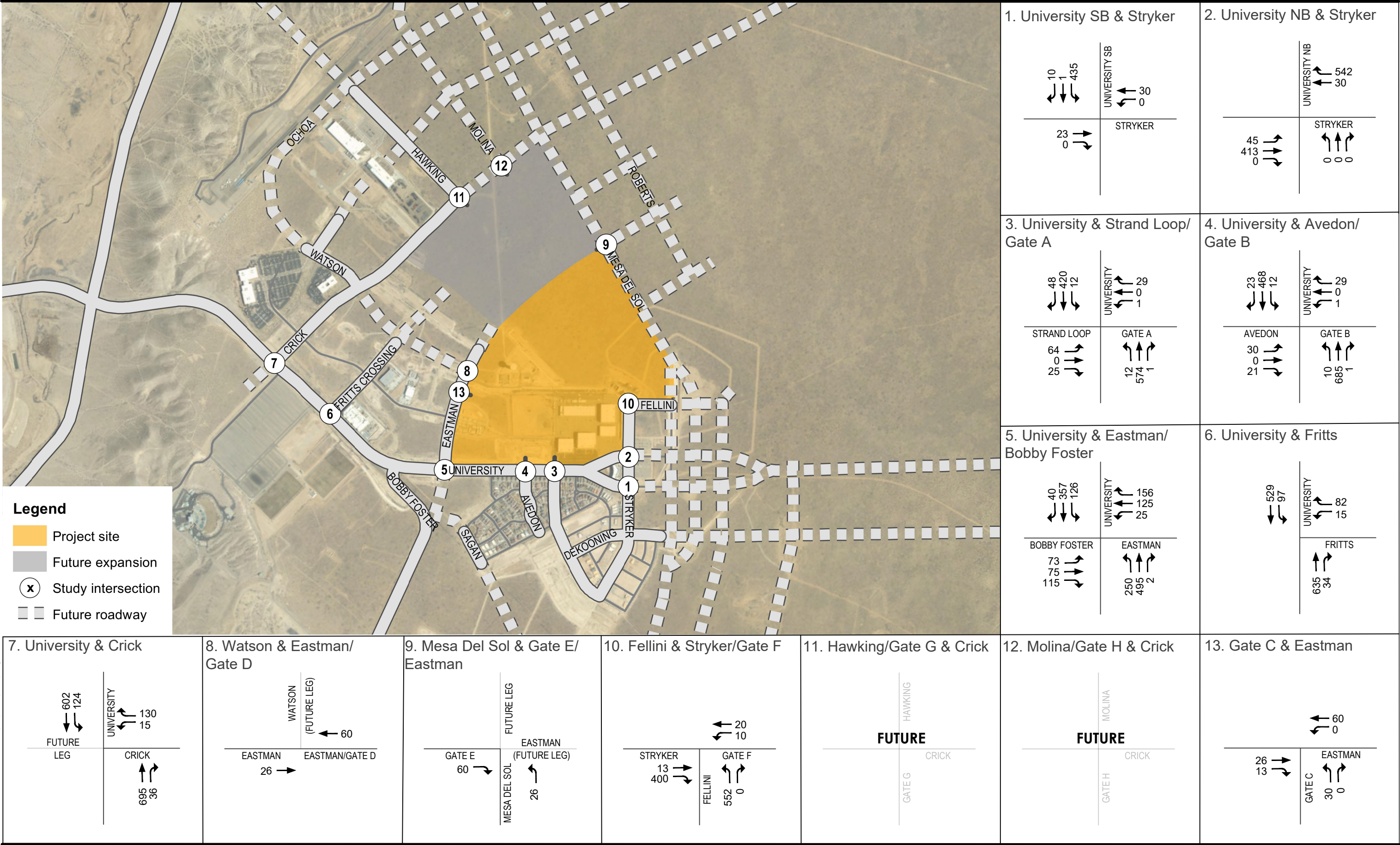


Figure 12
2026 With Project PM Peak Hour Volumes
22

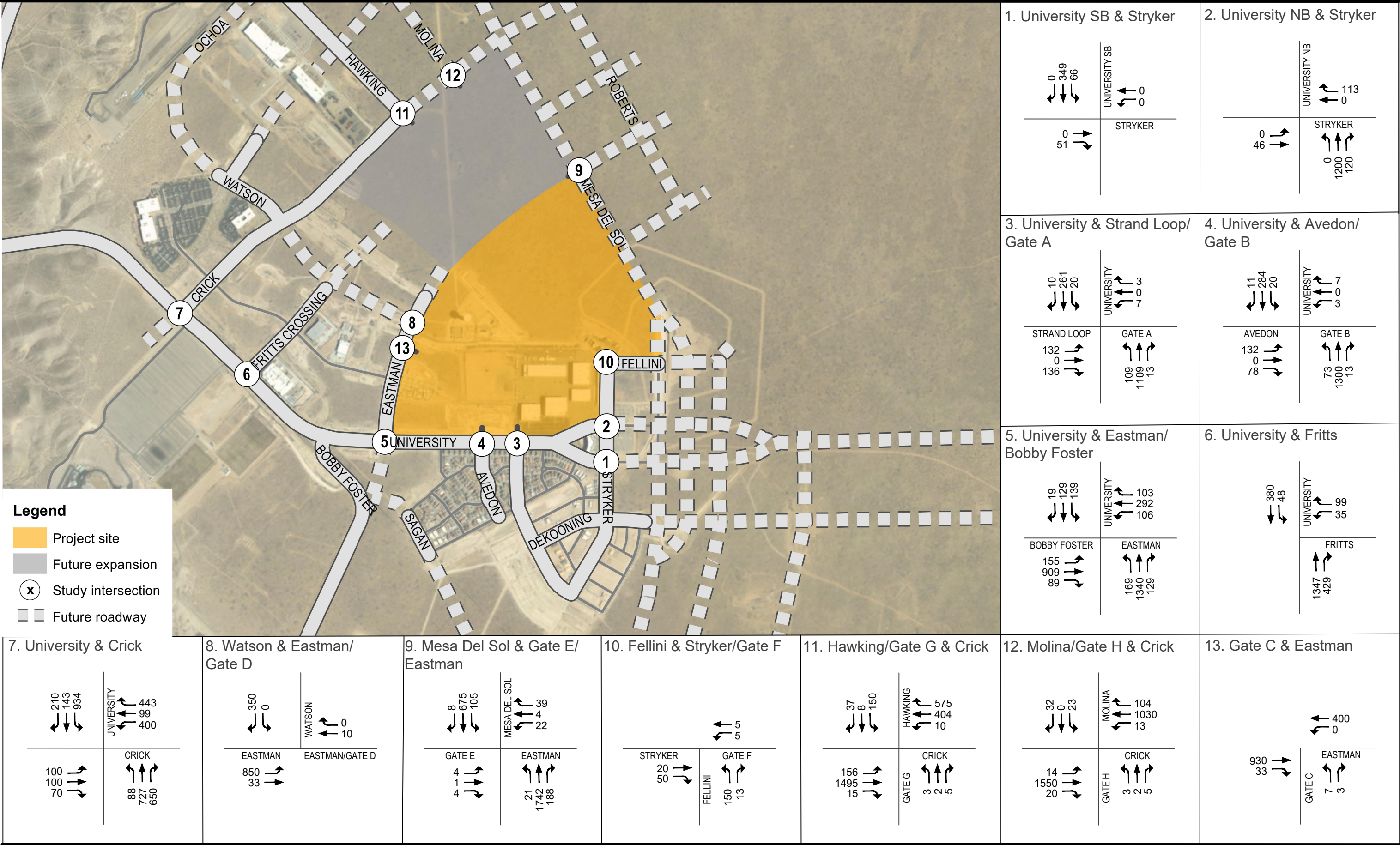


Figure 13
Buildout With Project AM Peak Hour Volumes
23

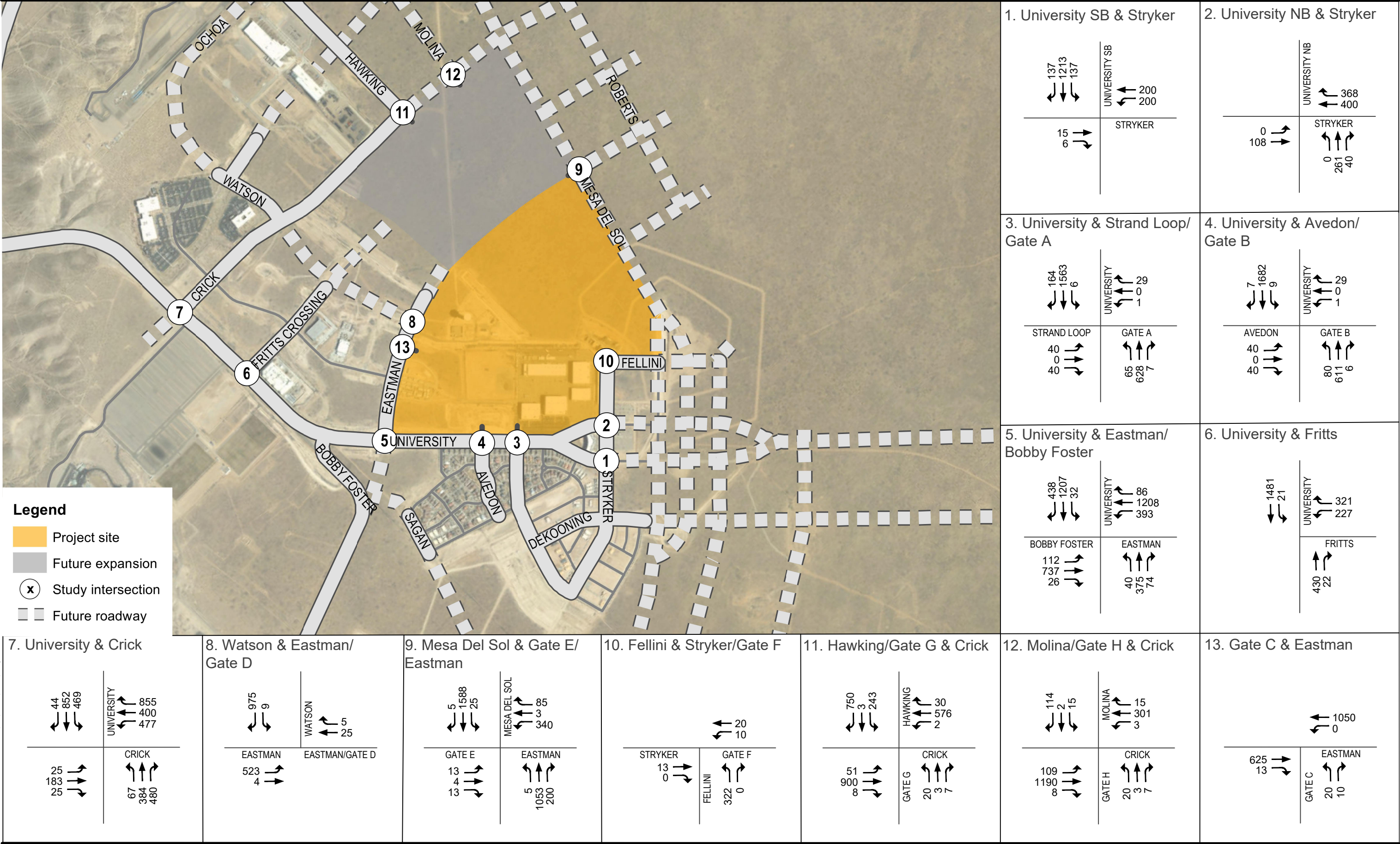


Figure 14
Buildout With Project PM Peak Hour Volumes
24

proposed site traffic characteristics

4.3 OTHER TRIP GENERATION CONSIDERATIONS

No other trip generation considerations were relevant to this analysis.

4.4 TRIP DISTRIBUTION

The primary distribution of Ingress/Egress traffic for this project is expected to use University Boulevard. This may change in the future, lessening the traffic on University and re-distributing to I-25 and MdS, and/or I-25 and Bobby Foster if future interchanges are constructed. Traffic was distributed taking account existing distribution and balancing of traffic for the future Implementation and horizon year study conditions.

Figures 13 and 14 show the forecasted traffic re-distributed to adjacent roadways upon vacation of Hawking Drive and Eastman Avenue and other connectors shown in the Level B circulation plan.



Hawking Vacation Redistribution

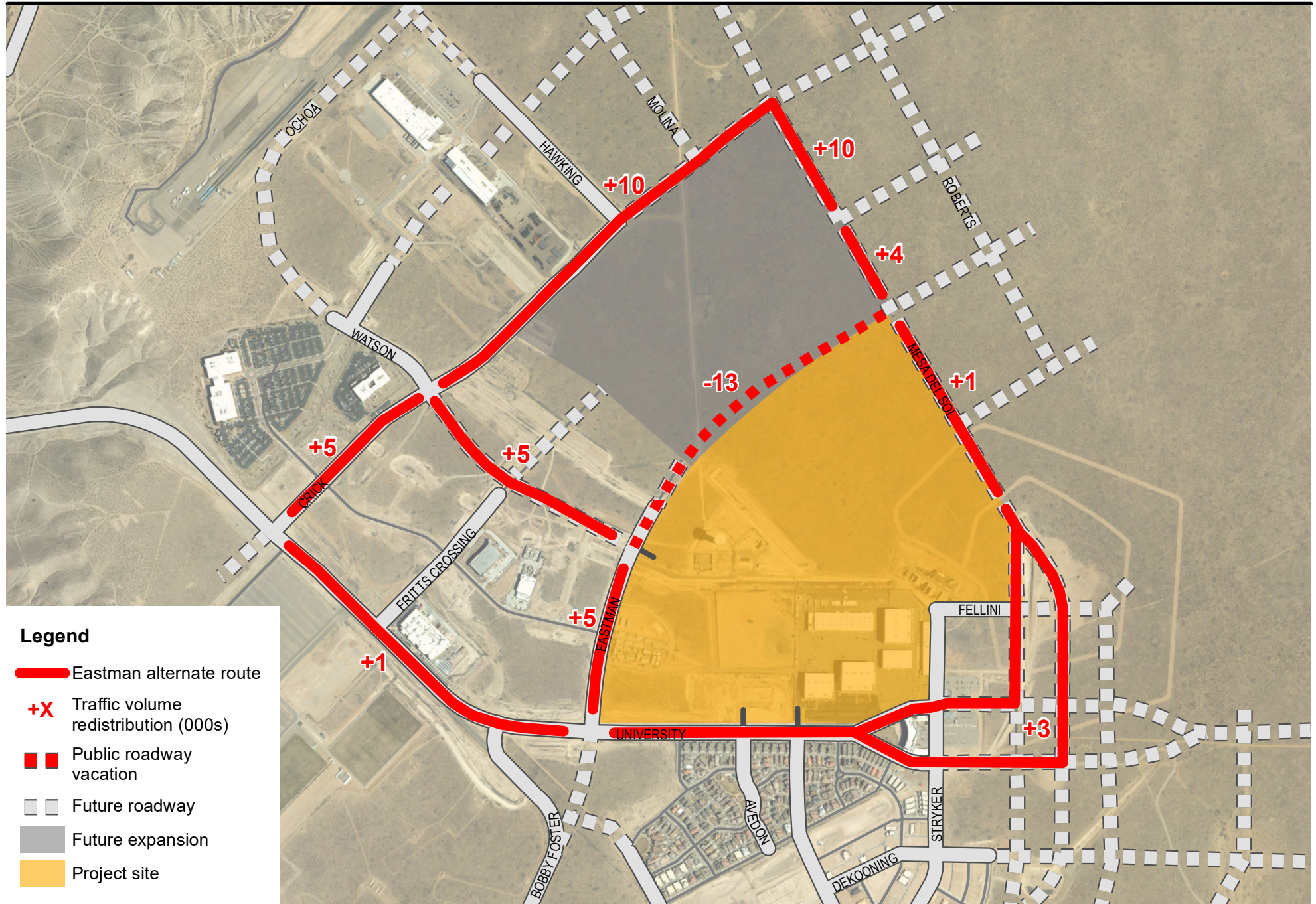


Figure 16

Eastman Vacation Redistribution

traffic analysis

4.5 TRAFFIC ASSIGNMENT

Forecasted traffic was balanced for the Master Plan among each of the access points. Each gate is anticipated to generate roughly the same traffic flows for the AM and PM peak hours.

5.0 TRAFFIC ANALYSIS

Synchro traffic engineering analysis software was used to conduct intersection and access operational analyses. Existing, build-out, and horizon year LOS were determined for each of the peak periods for the proposed gate accesses and study area intersections. Figure 9 and Figure 10 illustrate the forecasted traffic conditions for implementation year (2026) during the AM and PM Peak Hour analysis periods, respectively.

5.1 INTERSECTION AND ROADWAY ANALYSES

Below in Table 12, a summary of the Intersection LOS is provided for the implementation year scenario:

Table 12. Summary of Implementation Year (2026) LOS

Intersection	Traffic Control	2026 without Project				2026 with Project			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
		(sec/veh)		(sec/veh)		(sec/veh)		(sec/veh)	
1. University (SB) & Stryker	TWSC	9.9	A	9.9	A	24.5	C	22.1	C
2. University (NB) & Stryker	TWSC	0.0	A	0.0	A	9.2	A	-	-
3. University & Ex Gate/Strand Loop	TWSC	11.5	B	10.5	B	16.2	C	21.0	C
4. University & Future Gate/Avedon	TWSC	10.9	B	9.9	A	19.2	C	19.2	C
5. University & Bobby Foster/Eastman	Signal	12.3	B	14.1	B	18.1	B	18.9	B
6. University & Fritts Crossing	TWSC	11.9	B	12.0	B	15.3	C	20.7	C
7. University & Crick	TWSC	11.9	B	16.1	C	14.3	B	33.5	D
10. Fellini & Stryker/Gate F	AWSC	-	-	-	-	10.5	B	31.6	D
13. Gate C & Eastman	TWSC	-	-	-	-	9.2	A	9.2	A

Table 13 provides a summary of the Intersection LOS for the Level B long-range horizon year scenario:

Table 13. Summary of Horizon Year MdS Level B Master Plan LOS

Intersection	Traffic Control	Horizon Year with Project			
		AM Peak Hour		PM Peak Hour	
		Delay	LOS	Delay	LOS
		(sec/veh)		(sec/veh)	
1. University (SB) & Stryker	Signal	1.9	A	24.0	C
2. University (NB) & Stryker	Signal	7.2	A	13.9	B
3. University & Ex Gate/Strand Loop	Signal	13.6	B	11.4	B
4. University & Future Gate/Avedon	Signal	11.3	B	6.1	A
5. University & Bobby Foster/Eastman	Signal	53.2	D	47.6	D
6. University & Fritts Crossing	Signal	8.7	A	23.0	C
7. University & Crick	Signal	51.7	D	41.1	D
8. Eastman & Gate D/Watson	Signal	6.0	A	7.5	A
9. Mesa Del Sol & Gate E/Eastman	Signal	14.4	B	22.7	C
10. Fellini & Stryker/Gate F	AWSC	8.1	A	10.0	A
11. Gate G/Hawking & Crick	Signal	14.3	B	25.8	C
12. Gate H/Molina & Crick	Signal	6.7	A	6.2	A
13. Gate C & Eastman	TWSC	23.1	C	23.9	C

5.2 IDENTIFY ALTERNATIVE INTERSECTION AND ROADWAY DESIGNS

Ingress and egress are proposed through two travel lanes in each direction for each of the Gate accesses. A modification to the gate processing operation is being evaluated and proposed to allow Owner operation traffic to pass through the gate from the right lane using an electronic security detection system. The left lane is proposed for visitors and will require gate attendant interaction.

Two alternatives (Typical) were identified in this TIS, one alternative for Implementation year conditions, and one alternative for horizon year conditions. For the implementation year conditions, design and construction of adjacent ½ street improvements (side along site frontage) was identified. For the horizon year conditions, full Level B build-out improvements were identified with some minor modifications such as auxiliary lane configurations, left turning bays/lanes and potential expansion of Watson Drive if determined as need through future traffic analysis.

5.3 EVALUATE ALTERNATIVE INTERSECTION AND ROADWAY DESIGNS

The alternatives for Gate B-H are like the existing intersection configuration at Gate A. Minor modifications described in Chapter 6 are proposed at each gate to accommodate the site plus background traffic.

Alternative intersection and roadway designs were explored and evaluated for the Implementation year and horizon year conditions. Generally, ½ street improvements with auxiliary lanes to support implementation year traffic flows were evaluated and recommended for the implementation year conditions to attain the required LOS during the peak hours. Full-build-out of the streets and intersections are recommended for the Horizon Year conditions to attain the required LOS during the peak hours

5.4 PERFORM SIGNALIZATION AND STOP SIGN WARRANT ANALYSES

Traffic data for University Boulevard and Strand Loop/Gate A was reviewed and determined if more detailed signal warrant analyses should be conducted. The initial review indicated that the four primary warrants that are applicable include:

- Warrant 1. Eight-Hour Vehicular Volume
- Warrant 2. Four-Hour Vehicular Volume
- Warrant 3. Peak Hour
- Warrant 4. Pedestrian Volume

Based upon a review of the traffic data, existing and forecasted volumes are well below the flows the thresholds that would satisfy any of the applicable traffic signal warrants. Therefore, further investigation is not needed at this time.

Eastman Avenue and University Boulevard: The traffic forecasts for the North and East Phases (Implementation Year) are well below the thresholds to satisfy any of the applicable traffic signal warrants. There are several potential future triggers that may contribute traffic that may satisfy one or more of the signal warrants. A signal warrant review should be re-visited when one or more of the triggers occur:

1. Multi-use development at MdS west of the site and University in combination with a realignment of Bobby Foster into the Eastman Avenue/University Boulevard intersection.
2. North Periphery Implementation with office space land-use

site access requirements

3. Athletic Facilities/Complex Development northwest of the site in combination with a realignment of Bobby Foster into Eastman Avenue/University Boulevard intersection.
4. Other background traffic or development north of Eastman Avenue and east of University Boulevard, or in the vicinity of the intersection that will access through the intersection.

6.0 SITE ACCESS REQUIREMENTS

Gate Access

Access to the site is proposed through Gates A thru Gates F. Gate A is existing, Gates B-F are proposed site access gates. Two additional gates (Gates G and H) are preliminarily anticipated on future Crick Avenue for future development in the SLO area.

Minor roadway improvements are proposed to accommodate the Gate access design. Gate configuration and design are proposed to be similar to Gate A.

Installation of standard stop signs for exiting traffic are proposed for each Gate egress.

Any obstructions to limit sight distance to such as street lighting poles, landscaping, signing, etc., should be removed or relocated.

Access (Gate B) at the existing intersection of University Boulevard and Avedon Avenue is proposed for initial access to the North Development. The access is proposed as a stop-controlled access onto University Boulevard. Full access is proposed to allow all movements at the intersection.

Proposed Gate access configuration will include two inbound standard width driving lanes and one standard width exiting lane. A raised median approximately 12-15 feet is proposed to separate ingress and egress movements.

Based upon a preliminary analysis of queues, queue length for each of the gates are expected to be less than or similar to existing queues. Implementation of electronic gate processing for site employees and staffs should accelerate the rate of vehicles entering the site.

Periphery Access

summary of findings

Access to the North periphery uses is proposed thru a stop-controlled driveway onto Eastman Avenue, aligning with Turing Drive to the north.

Access for the South periphery uses are proposed thru up to ten (10) access points on the south perimeter of the site (Fellini Road and Mesa del Sol Boulevard). As periphery uses gets more defined, the access plan should be revisited to verify traffic operations and access recommendations.

A 35 FT wide driveway is recommended to provide two exiting movements and one lane for entering movements.

It is recommended that the curb cuts be constructed following the CABQ Curb-cut ordinance (6-5-4) with max design radii of (30 FT) for WB-40 and 35 FT for WB 50 design vehicle.

7.0 SUMMARY OF FINDINGS

A summary of the TIS findings are provided below:

Site Development: A decrease of 575 TSF of development from the Level B Master Plan

Site Development AM Peak Hour Traffic: A decrease of -739 Trips from the Level B Master Plan

Site Development PM Peak Hour Traffic: A decrease of -202 Trips from the Level B Master Plan in the critical outbound direction (due to retail uses in the periphery development, the PM inbound traffic volume increases by 652 Trips).

For the implementation year conditions, 1/2 street construction provides the required capacity and LOS for the site development and background traffic in the study area.

For the horizon year conditions, full buildout consistent with the Level B Master Plan with some modifications were found to achieve the acceptable LOS within the study area.

LOS for Implementation Year was found to be acceptable at all locations.

LOS for Horizon Year was found to be acceptable at all locations with implementation of recommended improvements.

recommendations and mitigation measures

The proposed storage length of 160 FT (for each lane) on-site at each gate is adequate for the peak conditions. It is recommended that additional left turn storage length of 100 ft be provided from the public street where possible and appropriate.

8.0 RECOMMENDATIONS AND MITIGATION MEASURES

The following recommendations and mitigation measures are proposed for the Albuquerque Studios Masterplan Expansion (North and East Development).

Access Gate Implementation Year Recommendations:

- Gate A Existing Gate on University Boulevard/Strand Loop Southeast
- Gate B Proposed Gate on University Boulevard/Avedon Avenue
- Gate C Proposed Gate on Eastman Avenue
- Gate D Proposed Gate on Eastman Avenue
- Gate E Proposed Gate on Mesa del Sol Boulevard
- Gate F Proposed Gate on Fellini Road/Stryker Road
- Recommendations specific to each gate are as follows: Gate A: Addition of an additional exiting lane is recommended to consist of two ingress lanes and two egress lanes.
- Gate B: Open (remove) existing temporary curb for the University Boulevard SB Left Turn Lanes to accommodate SB to EB left turning movements into the site.
- Gate B: Align Gate B with Avedon Avenue (West leg)
- Gate F: The Fellini/Striker Gate is recommended to be a three-way stop-controlled access.
- All Gates: Install typical standard stop sign for exiting gate traffic. All gates are recommended to be stop-controlled in the Implementation Year.
- All Gates: Remove or relocate any obstruction such as landscaping, signage, street light poles and other potential obstruction so that adequate sight distance is provided for traffic. Any landscaping or vegetation on University Boulevard limiting adequate sight distance should be removed, relocated, or pruned.
- All Gates, intersections, and streets: Provide ADA and bicycle related accommodations at all gate access locations.

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recommendations and mitigation measures

- All Gates: Proposed lane configuration to consist of two-ingress lanes and two egress lanes.
- All Gates: Just prior to the access gates, it is recommended that pull-out, drop off areas be provided for ridesharing operations.
- All Gates: It is recommended that all gates be provided with a turn-around area, similar to the configuration at Gate A.
- All Gates: Intersections and streets are recommended to have retro-reflective pavement markings as appropriate for each of the access gates, intersections, and street modifications.
- All Gates: Removals or relocation of existing infrastructure may be necessary for some of the gate accesses. These items include minor pavement marking removals, landscaping (NB University Boulevard), street lighting pole, signing, curb and gutter, and sidewalk, etc.
- All Gates: It is recommended that pedestrian access/accommodations be incorporated through sidewalks and ADA ramps. Bicycle lanes should remain unchanged on University Boulevard in the NB and SB directions. Future accommodations for transit/bus stops are contemplated as Mesa del Sol grows and develops.

Public Street Recommendations:

- University Boulevard: Minor modifications to accommodate Gate A improvements and Gate B addition for Implementation Year conditions.

Horizon Year: Build out University Boulevard at Eastman Avenue in conjunction with University Boulevard/Eastman Avenue/Bobby Foster Road realignment and signalization improvements (identified below in Public Intersection recommendations).

- Mesa del Sol Boulevard Construction of ½ street improvements from Fellini Road to Crick Avenue for Masterplan traffic implementation year. Construct full build-out for horizon year traffic.
- Eastman Avenue Construction of ½ street improvements from University to Gate D for Master plan traffic implementation year. Construct build-out for horizon year traffic.



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recommendations and mitigation measures

- Crick Avenue Construction of ½ street improvements to connect Crick Avenue west to Mesa del Sol Boulevard for Masterplan traffic implementation year; Construct Crick Avenue full build-out for Horizon Year traffic.
- Watson Drive: Future horizon year recommendation: Construct Watson Drive initially as a two-lane street for horizon year traffic conditions. A four-lane Street is recommended for consideration in the future. An alternate would be to construct a new two-lane roadway parallel to the west side of the SLO Parcel from Eastman Avenue to Crick Avenue. It is recommended to re-evaluate the growth and traffic in a future analysis to further justify that this additional capacity is needed.

Public Intersection recommendations:

1. University and Eastman Avenue; Implementation year recommendations include a signalized intersection in the near term when one or more of the following “triggers” occur:

- Mixed-use development growth to the west of the site
- Athletic Facilities/Complex improvements by Bernalillo County
- Bobby Foster Road re-alignment
- north periphery build-out and/or other development northeast of University Boulevard and Eastman Avenue that will contribute additional Horizon Year traffic flows at this intersection.

It is recommended that width for dual lefts for WB to SB for horizon year traffic conditions be included that can be re-marked as necessary in the future.

2. Mesa del Sol and Crick Avenue: Continuous flow northbound to eastbound and southbound to westbound until such time as other MdS development occurs north and east creating the need for additional traffic control for conflicting movements at this intersection. Ultimate build-out for the horizon year is recommended as a signalized intersection.

3. Mesa del Sol Couplets

Recommended as two-way adjacent to the site from Fellini Road, east to the location where they come together as one street at Mesa del Sol Boulevard. (Approximately 800-900 FT).



recommendations and mitigation measures

Recommended for construction per Level B Plan for Horizon Year conditions as a network roadway.

4. University Boulevard and Crick Avenue: Intersection as constructed to accommodate the implementation year traffic.

Recommend traffic signal for horizon year conditions as a key network intersection.

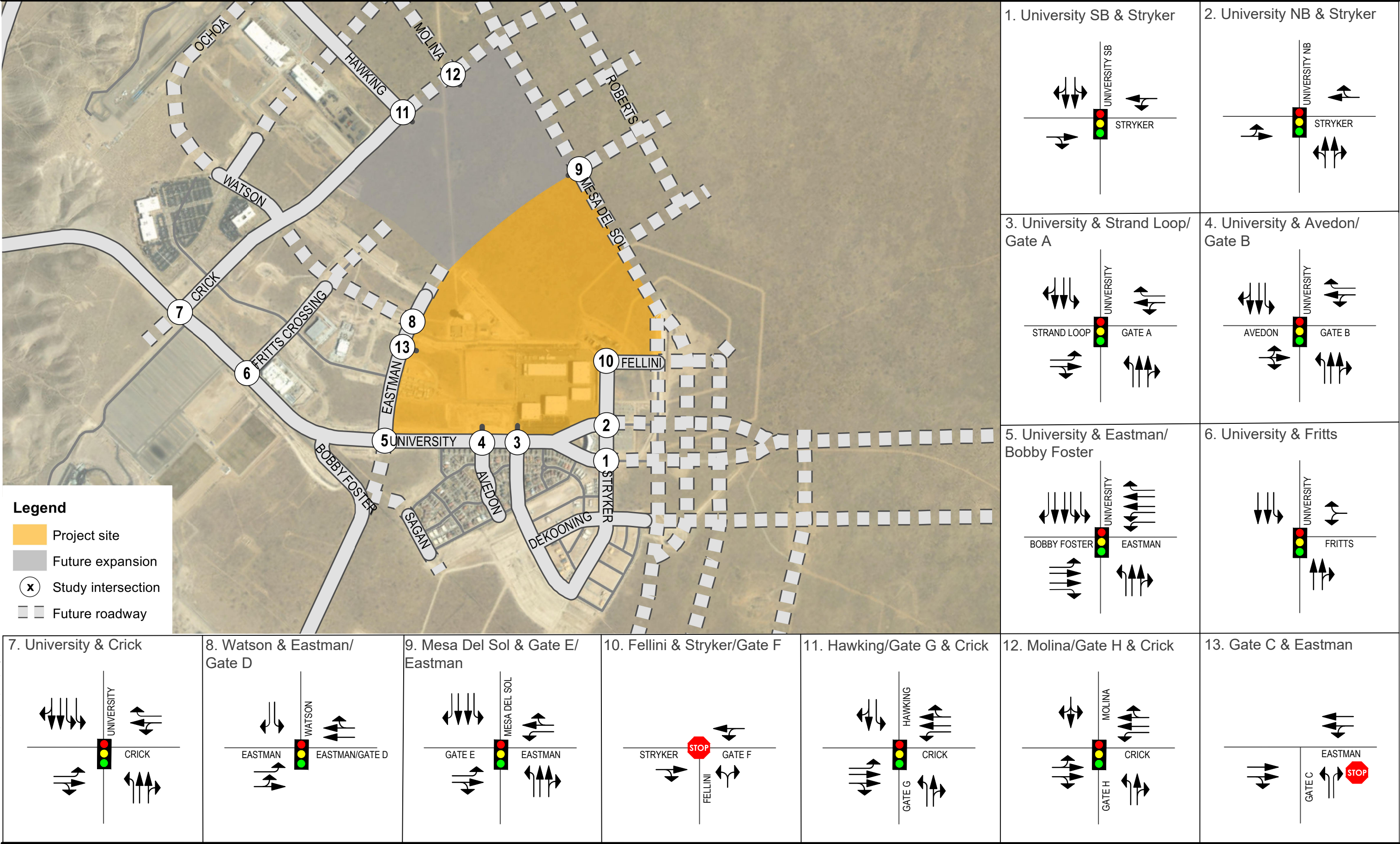
Periphery Driveway Recommendations (Implementation Year)

North Driveway: The north periphery driveway is recommended for the office build-out lanes use planned for this tract. Two inbound lanes, two outbound separated by a minimum four (4) FT. raised median is recommended. Stop control is recommended for the outbound access lanes. Access geometry to be designed to accommodate WB-50 design vehicle

South Driveways: The south driveways are recommended for the forecasted office and retail use for these parcels. Driveways are recommended to be stop-controlled, 35 FT wide to provide two (2) outbound travel ways, and one inbound travel way. Geometry is to be designed per CABQ Curb Cut ordinance for the appropriate design vehicle, either the WB-40 or WB 50 design vehicle.

Based upon the traffic analyses conducted herein, no adverse impacts associated with the development are foreseen. If the recommendations proposed in Section 8 are implemented, future traffic should be accommodated in a safe and efficient manner.

Figure 17 summarizes the recommendations for traffic control and lane configurations for the Implementation (Buildout) Year.



Legend

- Project site
- Future expansion
- Study intersection
- Future roadway

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Figure 17
Buildout Year Lane Configurations and Traffic Control
37

References

9.0 REFERENCES

Development Process Manual (DPM) City of Albuquerque September 4, 2020 7-158 to 7-172

MRCOG Website www.mrcog-nm.gov

[Geospatial and Population Studies | University of New Mexico \(unm.edu\)](http://Geospatial.and.Population.Studies|University.of.New.Mexico.(unm.edu)) Website

HOK Masterplan site planning documents

10.0 APPENDIX

I. Traffic Data – April 2021 TMCs and ADT and Trip Generation Spreadsheet

II. Crash Data – 2017-2019

III. NMDOT COVID Traffic Data Calibration Methodology

IV. CABQ Zone Atlas

V. Traffic Analysis Detail – Synchro Operational Analyses

- Existing Conditions AM Peak Hour
- Existing Conditions PM Peak Hour
- 2026 with Project AM Peak Hour
- 2026 with Project PM Peak Hour

TRAFFIC DATA

April 2021 TMCs and ADT

Albuquerque Studios Expansion Traffic Data Calibration Summary															
	AM Peak Hour					Noon Peak Hour					PM Peak Hour				
	2019	2020	Adjust	2021	2021	2019	2020	Adjust	2021	2021	2019	2020	Adjust	2021	2021
	Pre-COVID	COVID	Factor	Actual	Calibrated	Pre-COVID	COVID	Factor	Actual	Calibrated	Pre-COVID	COVID	Factor	Actual	Calibrated
Intersection	April	April		April	April	April	April		April	April	April	April		April	April
12 Hour TMC															
Univ & Ex Gate/Strand Loop				7:45 AM					11:45 AM					4:15 PM	
NB Left	0	0		0	0	0	0		5	5	0	0		2	2
NB Thru	13	19	0.68	24	16	38	19	1.42	39	55	33	10	1.42	37	53
NB Right	0	0		1	1	26	0		7	7	0	39	0.00	1	1
SB Left	256	129	1.42	59	84	90	73	1.23	81	100	65	19	1.42	24	34
SB Thru	46	81	0.57	59	34	72	50	1.42	55	78	32	25	1.28	33	42
SB Right	0	8	0.00	21	21	9	19	0.58	12	7	87	53	1.42	23	33
WB Left	0	0		1	0	0	0		3	3	0	0		1	1
WB Thru	0	0		0	0	17	0		0	0	0	0		0	0
WB Right	16	23	0.70	34	24	94	24	1.42	37	53	109	41	1.42	53	75
EB Left	16	46	0.58	36	21	7	6	1.17	21	25	28	15	1.42	30	43
EB Thru	0	0		0	0	0	0		0	0	0	0		0	0
EB Right	0	0		0	0	0	0		1	0	0	0		5	5
				235	200				261	327				209	286
12 Hour TMC															
Univ & Eastman Crossing				7:45 AM			*		11:30 AM					3:00 PM	
NB Left	0	0		0	0	0	0		0	0	0	0		0	0
NB Thru	74	131	0.56	120	68	134	63	1.42	114	162	63	0		114	114
NB Right	30	26	1.15	7	8	18	0		2	2	55	71	0.77	7	2
SB Left	222	37	1.42	70	99	23	0		5	5	118	0		36	36
SB Thru	144	117	1.23	139	171	98	70	1.40	171	239	85	78	1.09	94	102
SB Right	0	0		0	0	0	0		0	0	0	0		0	0
WB Left	70	0		5	0	33	0		3	3	66	0		5	5
WB Thru	0	0		0	0	0	0		0	0	0	0		0	0
WB Right	35	0		41	41	0	0		2	2	127	0		49	49
EB Left	0	0		0	0	0	0		0	0	0	0		0	0
EB Thru	0	0		0	0	0	0		0	0	0	0		0	0
EB Right	0	0		0	0	0	0		0	0	0	0		0	0
				382	387				297	413				305	308
12 Hour TMC															
Univ and Bobby Foster			3.5% HC	7:45 AM					11:30 AM					3:00 PM	
EB Left	34	6	1.42	14	20	10	8	1.25	5	6	0	7	0.00	8	8
EB Thru	0	0		0	0	0	0		0	0	0	0		0	0
EB Right	0	23	0.00	21	21	0	13	0.00	3	3	0	0		10	10
WB Left	0	0		0	0	0	0		0	0	0	0		0	0
WB Thru	0	0		0	0	0	0		0	0	0	0		0	0
WB Right	0	0		0	0	0	0		0	0	0	0		0	0
NB Left	27	21	1.29	19	24	24	58	0.58	5	3	76	10	1.42	27	38
NB Thru	107	119	0.90	138	124	108	0		113	113	116	61	1.42	134	190
NB Right	0	0		0	0	0	0		0	0	0	0		0	0
SB Left	0	0		0	0	0	0		0	0	0	0		0	0
SB Thru	32	125	0.58	182	106	9	71	0.58	174	101	49	87	0.58	119	69
SB Right	0	47	0.00	2	0	0	14	0.00	6	0	0	4	0.00	6	6
				376	295				306	226				304	322
Nine Hour TMC															
Univ & Crick Ave				7:45 AM					11:30 AM					3:00 PM	
NB Left	0	0	--	0	0	0	0		1	1	0	0		0	0
NB Thru	193	148	1.30	168	219	165	95	1.42	136	193	196	92	1.42	197	280
NB Right	47	0		5	5	19	0		6	0	0	0		3	3
EB Left	0	0		0	0	0	0		0	0	0	0		0	0
EB Thru	0	0		0	0	0	0		0	0	0	0		0	0
EB Right	37	0		0	0	0	0	--	0	0	0	0		0	0
SB Left	222	61	1.42	35	50	44	0		29	29	15	8	1.42	24	34
SB Thru	340	114	1.42	206	293	146	88	1.42	180	256	196	89	1.42	118	168
SB Right	0	0		0	0	0	0		0	0	0	0		0	0
WB Left	0	38	0.00	1	1	0	5	0.00	6	6	0	0		3	3
WB Thru	0	0		0	0	0	0		0	0	0	0		0	0
WB Right	37	0		30	30	36	54	0.67	17	11	461	51	1.42	51	72
				445	597				375	496				396	560
Nine Hour TMC															
Univ and Fritts Crossing				7:45 AM					11:15 AM					3:00 PM	
NB Left	0	0	--	0	0	0	0		0	0	0	0		0	0
NB Thru	146	123	1.19	146	173	92	77	1.19	115	137	132	60	1.42	150	213
NB Right	21	0		2	0	0	0	--	0	0	0	0		2	2
EB Left	0	0		0	0	0	0		0	0	0	0		0	0
EB Thru	0	0	--	0	0	0	0		0	0	0	0		0	0
EB Right	0	0		0	0	0	0		0	0	0	0		0	0
SB Left	103	10	1.42	17	24	17	28	0.61	7	4	44	0		8	8
SB Thru	278	146	1.42	198	281	98	73	1.34	177	238	145	84	1.42	129	183
SB Right	0	0		0	0	0	0		0	0	0	0		0	0
WB Left	61	0	--	0	0	0	0		2	2	50	0		2	2
WB Thru	0	0	--	0	0	0	0		0	0	0	0		0	0
WB Right	41	0	--	25	25	22	16	1.38	19	25	55	9	1.42	44	25
				388	504				320	406				335	433
Nine Hour TMC															
Univ & Avedon				7:45 AM					11:30 AM					4:15 PM	
NB Left	0	0		0	0	33	0		1	1	36	0		2	0
NB Thru	49	42	1.17	97	113	125	50	1.42	88	125	145	55	1.42	120	170
NB Right	0	0		0	0	0	0		0	0	0	0		0	0
SB Left	0	0		0	0	0	0		2	2	0	0		2	2
SB Thru	267	181	1.42	133	189	145	65	1.42	157	223	140	92	1.42	76	108
SB Right	8	7	1.14	6	7	1	0		4	0	13	14	0.93	8	7
WB Left	0	0		0	0	0	0		0	0	0	0		0	0
WB Thru	0	0		0	0	0	0		0	0	0	0		0	0
WB Right	0	0		0	0	0	0		0	0	0	0		0	0
EB Left	0	58	0.00	10	10	2	14	0.58	4	10	7	6	1.17	4	10
EB Thru	0	0		0	0	0	0		0	0	0	0		0	0
EB Right	0	0		1	1	16	21	0.76	0	1	10	0		1	1
				247	320				256	361				213	299
Nine Hour TMC															
Univ (SB) & Stryker Rd.				7:30 AM					11:45 AM					3:45 PM	
NB Left	0	0		0	0	0	0		0	0	0	0		0	0
NB Thru	0	0		0	0	0	0		0	0	0	0		0	0
NB Right	0	0		0	0	0	0		0	0	0	0		0	0
SB Left	0	36	0.00	25	36	109	34	1.42	43	61	24	33	0.73	23	17
SB Thru	0	0		0	0	0	0		1	1	0	0		1	1
SB Right	0	110	0.00	32	32	29	42	0.69	14	10	0	22	0.00	12	0
WB Left	70	0		0	0	0	0		0	0	0	0		0	0
WB Thru	0	9	0.00	1	1	0	0		3	3	0	0		3	3
WB Right	0	0		0	0	0	0		0	0	0	0		0	0
EB Left	0	0		0	0	0	0		1	1	0	0		0	0
EB Thru	0	0		12	12	0	12	0.00	21	21	0	33	0.00	22	22
EB Right	0	0		0	0	0	0		1	1	0	0		0	0
				70	69				84	98				61	43
Nine Hour TMC															
Univ (NB) & Stryker Rd.				8:45 AM					11:45 AM					3:45 PM	
NB Left	0	0		0	0	0	0		0	0	0	0		0	0
NB Thru	0	0		1	1	0	0		0	0	0	0		0	0
NB Right	0	0		0	0	0	0		0	0	0	0		0	0
SB Left	0	0		0	0	0	0		0	0	0	0		0	0
SB Thru	0	0		0	0	0	0		0	0	0	0		0	0
SB Right	0	0		0	0	0	0		0	0	0	0		0	0
WB Left	0	0		0	0										

Albuquerque Studios North
Development Trip Generation 5/11/2021

							AM Peak	AM Peak	Noon Peak	Noon Peak	PM Peak	PM Peak							
					Units	Existing Size X 1000 SQ. FT	Permanent Jobs/Employees	Trip Gen Entering	Trip Gen Exiting	Trip Gen Entering	Trip Gen Exiting	Trip Gen Entering	Trip Gen Exiting						
	Existing			Facility Existing															
				Bldg A, Stage 1 and 2	1	50													
				Bldg B. Stage 3 and 4	1	60													
				Stage5 & 6	1	36													
				Stage 7 & 8	1	65													
				Mill 2	1	80													
				Mill 3 (Ex) /Ex Production office (Stage 8?)	1	40													
	Already Built out			Total SF Building		331		85	25	107	56	34	76		Ingress/Egress thru Existing Gate A				
						Proposed Size X 1000 SQ. FT													
	Phase I			North															
	Buildout (Summer 2022)			Vendor Village (2@ 50K each)	2	100													
				Mill 1	1	50													
				Production Office and Commons 1&2	1	145													
				Mill 2 Demo and replacement	1	--													
				Total SF Building		295		76	22	95	50	30	68		Ingress/Egress thru proposed Gate B at Univ/Avedon				



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Location: 4 UNIVERSITY BLVD SE & EASTMAN CROSSING SE AM

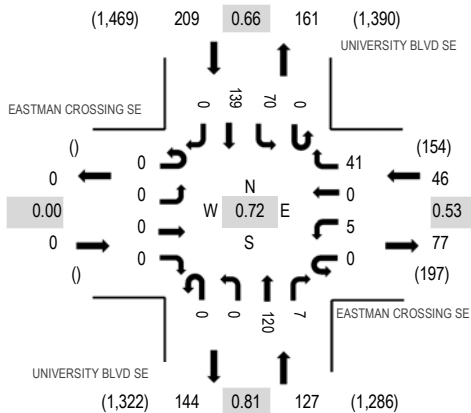
Date: Wednesday, April 21, 2021

Peak Hour: 07:45 AM - 08:45 AM

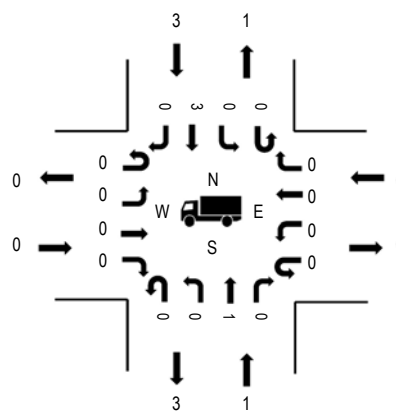
Peak 15-Minutes: 08:15 AM - 08:30 AM

Peak Hour

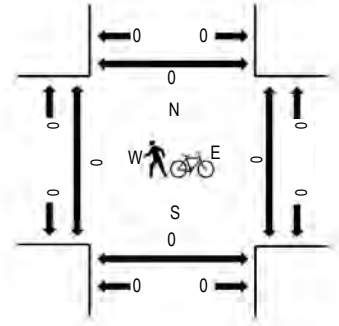
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



	HV%	PHF
EB	0.0%	0.00
WB	0.0%	0.53
NB	0.8%	0.81
SB	1.4%	0.66
All	1.0%	0.72

Traffic Counts - Motorized Vehicles

Interval Start Time	EASTMAN CROSSING SE Eastbound				EASTMAN CROSSING SE Westbound				UNIVERSITY BLVD SE Northbound				UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
7:00 AM	0	0	0	0	0	1	0	1	0	0	22	0	0	4	26	0	54	292
7:15 AM	0	0	0	0	0	0	0	1	0	0	35	1	0	24	28	0	89	317
7:30 AM	0	0	0	0	0	1	0	1	0	0	29	0	0	7	39	0	77	360
7:45 AM	0	0	0	0	0	0	0	0	0	0	25	0	0	6	41	0	72	382
8:00 AM	0	0	0	0	0	2	0	9	0	0	25	2	0	16	25	0	79	370
8:15 AM	0	0	0	0	0	2	0	17	0	0	27	5	0	41	40	0	132	348
8:30 AM	0	0	0	0	0	1	0	15	0	0	43	0	0	7	33	0	99	273
8:45 AM	0	0	0	0	0	0	0	1	0	0	22	0	0	1	36	0	60	217
9:00 AM	0	0	0	0	0	0	0	0	0	0	24	0	0	1	32	0	57	199
9:15 AM	0	0	0	0	0	0	0	1	0	0	28	0	0	0	28	0	57	199
9:30 AM	0	0	0	0	0	0	0	0	0	0	21	0	0	1	21	0	43	187
9:45 AM	0	0	0	0	0	1	0	0	0	0	25	0	0	0	16	0	42	182
10:00 AM	0	0	0	0	0	0	0	0	0	0	26	0	0	2	29	0	57	191
10:15 AM	0	0	0	0	0	0	0	2	0	0	25	1	0	0	17	0	45	182
10:30 AM	0	0	0	0	0	0	0	2	0	0	18	2	0	0	16	0	38	189
10:45 AM	0	0	0	0	0	0	0	2	0	0	26	0	0	1	22	0	51	221
11:00 AM	0	0	0	0	0	0	0	0	0	0	24	1	0	0	23	0	48	254
11:15 AM	0	0	0	0	0	0	0	1	0	0	18	1	0	1	31	0	52	282
11:30 AM	0	0	0	0	0	0	0	1	0	0	25	0	0	0	44	0	70	297
11:45 AM	0	0	0	0	0	2	0	0	0	0	32	2	0	1	47	0	84	289
12:00 PM	0	0	0	0	0	1	0	1	0	0	27	0	0	1	46	0	76	264
12:15 PM	0	0	0	0	0	0	0	0	0	0	30	0	0	3	34	0	67	243
12:30 PM	0	0	0	0	0	1	0	4	0	0	27	0	0	1	29	0	62	239
12:45 PM	0	0	0	0	0	0	0	1	0	0	25	0	0	0	33	0	59	218
1:00 PM	0	0	0	0	0	0	0	0	0	0	20	1	0	1	33	0	55	209
1:15 PM	0	0	0	0	0	0	0	0	0	0	32	0	0	1	30	0	63	200
1:30 PM	0	0	0	0	0	0	0	0	0	0	30	0	0	1	10	0	41	185

1:45 PM	0	0	0	0	0	0	0	1	0	0	29	0	0	1	19	0	50	197
2:00 PM	0	0	0	0	0	0	0	1	0	0	21	0	0	2	22	0	46	196
2:15 PM	0	0	0	0	0	0	0	1	0	0	26	0	0	1	20	0	48	227
2:30 PM	0	0	0	0	0	1	0	1	0	0	17	1	0	4	29	0	53	252
2:45 PM	0	0	0	0	0	0	0	0	0	0	25	3	0	1	20	0	49	283
3:00 PM	0	0	0	0	0	0	0	4	0	0	35	5	0	13	20	0	77	305
3:15 PM	0	0	0	0	0	0	0	4	0	0	27	1	0	16	25	0	73	292
3:30 PM	0	0	0	0	0	5	0	24	0	0	23	1	0	7	24	0	84	281
3:45 PM	0	0	0	0	0	0	0	17	0	0	29	0	0	0	25	0	71	258
4:00 PM	0	0	0	0	0	0	0	9	0	0	25	0	0	0	30	0	64	246
4:15 PM	0	0	0	0	0	1	0	5	0	0	32	0	0	1	23	0	62	250
4:30 PM	0	0	0	0	0	0	0	0	0	0	35	0	0	0	26	0	61	237
4:45 PM	0	0	0	0	0	0	0	1	0	0	29	0	0	0	29	0	59	218
5:00 PM	0	0	0	0	0	0	0	2	0	0	43	0	0	1	22	0	68	215
5:15 PM	0	0	0	0	0	0	0	1	0	0	20	0	0	0	28	0	49	193
5:30 PM	0	0	0	0	0	0	0	0	0	0	19	0	0	0	23	0	42	185
5:45 PM	0	0	0	0	0	1	0	1	0	0	28	0	0	0	26	0	56	184
6:00 PM	0	0	0	0	0	1	0	0	0	0	23	0	0	0	22	0	46	168
6:15 PM	0	0	0	0	0	0	0	0	0	0	19	1	0	0	21	0	41	
6:30 PM	0	0	0	0	0	0	0	0	0	0	21	0	0	0	20	0	41	
6:45 PM	0	0	0	0	0	0	0	1	0	0	20	1	0	0	18	0	40	
Count Total	0	0	0	0	0	21	0	133	0	0	1,257	29	0	168	1,301	0	2,909	
Peak Hour	0	0	0	0	0	5	0	41	0	0	120	7	0	70	139	0	382	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
7:00 AM	0	0	0	0	0	7:00 AM	0	0	1	0	1
7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	2	2	8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0	8:15 AM	0	0	0	0	0
8:30 AM	0	1	0	1	2	8:30 AM	0	0	0	0	0
8:45 AM	0	1	0	0	1	8:45 AM	0	0	0	0	0
9:00 AM	0	1	0	0	1	9:00 AM	0	0	0	0	0
9:15 AM	0	0	0	0	0	9:15 AM	0	0	0	0	0
9:30 AM	0	0	0	0	0	9:30 AM	0	0	0	0	0
9:45 AM	0	0	0	1	1	9:45 AM	0	0	0	0	0
10:00 AM	0	3	0	0	3	10:00 AM	0	0	0	0	0
10:15 AM	0	0	0	0	0	10:15 AM	0	0	0	0	0
10:30 AM	0	0	0	0	0	10:30 AM	0	0	0	0	0
10:45 AM	0	0	0	1	1	10:45 AM	0	0	0	0	0
11:00 AM	0	0	0	1	1	11:00 AM	0	0	0	0	0
11:15 AM	0	0	0	1	1	11:15 AM	0	0	1	0	1
11:30 AM	0	1	0	0	1	11:30 AM	0	0	0	0	0
11:45 AM	0	1	0	0	1	11:45 AM	0	0	0	0	0
12:00 PM	0	0	0	0	0	12:00 PM	0	0	0	0	0
12:15 PM	0	0	0	0	0	12:15 PM	0	0	0	0	0
12:30 PM	0	0	0	1	1	12:30 PM	0	0	0	0	0
12:45 PM	0	1	0	0	1	12:45 PM	0	0	0	0	0
1:00 PM	0	0	0	1	1	1:00 PM	0	0	0	0	0
1:15 PM	0	0	0	1	1	1:15 PM	0	0	0	0	0
1:30 PM	0	0	0	0	0	1:30 PM	0	0	0	0	0
1:45 PM	0	0	0	1	1	1:45 PM	0	0	1	0	1
2:00 PM	0	0	0	1	1	2:00 PM	0	0	0	0	0
2:15 PM	0	0	0	0	0	2:15 PM	0	0	0	0	0
2:30 PM	0	0	0	0	0	2:30 PM	0	0	0	0	0
2:45 PM	0	0	0	1	1	2:45 PM	0	0	0	0	0
3:00 PM	0	0	0	0	0	3:00 PM	0	0	0	0	0
3:15 PM	0	0	0	0	0	3:15 PM	0	0	0	0	0

3:30 PM	0	0	0	0	0	3:30 PM	0	0	0	0	0
3:45 PM	0	0	0	0	0	3:45 PM	0	0	0	0	0
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0	4:15 PM	0	0	1	0	1
4:30 PM	0	0	0	0	0	4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0	4:45 PM	0	0	2	0	2
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0	5:15 PM	0	0	1	0	1
5:30 PM	0	0	0	0	0	5:30 PM	0	0	4	0	4
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	0
6:00 PM	0	0	0	0	0	6:00 PM	0	0	0	0	0
6:15 PM	0	0	0	0	0	6:15 PM	0	0	2	0	2
6:30 PM	0	0	0	0	0	6:30 PM	0	0	0	0	0
6:45 PM	0	0	0	0	0	6:45 PM	0	0	0	0	0
Count Total	0	9	0	13	22	Count Total	0	0	13	0	13
Peak Hour	0	1	0	3	4	Peak Hour	0	0	0	0	0

Traffic Counts - Motorized Vehicles

1:45 PM	0	6	0	0	0	0	0	9	0	0	9	1	0	9	4	4	42	144
2:00 PM	0	3	0	0	0	0	0	9	1	0	6	0	0	7	4	1	31	143
2:15 PM	0	2	0	1	0	0	0	4	0	0	13	0	0	4	5	7	36	162
2:30 PM	0	4	0	0	0	0	0	2	0	0	9	0	0	13	7	0	35	175
2:45 PM	0	4	0	0	0	1	0	4	0	0	12	0	0	6	10	4	41	178
3:00 PM	0	12	0	0	0	1	0	4	0	0	13	0	1	8	10	1	50	186
3:15 PM	0	11	0	0	0	0	0	6	0	0	11	0	0	11	7	3	49	180
3:30 PM	0	7	0	0	0	0	0	5	0	0	5	0	0	6	7	8	38	186
3:45 PM	0	3	0	0	0	5	0	12	1	0	13	0	0	4	3	8	49	197
4:00 PM	0	3	0	1	0	2	0	12	1	0	4	0	0	4	8	9	44	202
4:15 PM	0	9	0	2	0	0	0	8	0	0	12	0	0	2	17	5	55	209
4:30 PM	0	7	0	2	0	0	0	13	1	0	9	0	1	6	7	3	49	188
4:45 PM	0	5	0	1	0	0	0	14	1	0	9	0	0	10	4	10	54	176
5:00 PM	0	9	0	0	0	1	0	18	0	0	7	1	1	4	5	5	51	171
5:15 PM	0	4	0	0	0	0	0	4	0	0	9	0	0	2	6	9	34	157
5:30 PM	0	2	0	0	0	0	0	8	0	0	7	0	0	2	8	10	37	155
5:45 PM	0	5	0	1	0	2	1	11	0	0	8	4	0	4	4	9	49	151
6:00 PM	0	5	0	0	0	0	0	11	0	0	5	0	0	5	5	6	37	135
6:15 PM	0	4	0	0	0	0	0	12	0	0	1	0	0	2	6	7	32	
6:30 PM	0	4	0	0	0	0	0	8	0	0	5	1	2	3	3	7	33	
6:45 PM	0	5	1	0	0	1	0	9	0	0	3	0	0	3	8	3	33	
Count Total	0	272	3	12	0	28	1	395	11	1	366	30	14	456	431	224	2,244	
Peak Hour	0	21	0	1	0	3	0	37	4	1	39	7	1	81	55	12	262	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
7:00 AM	0	0	0	0	0	7:00 AM	1	0	0	0	1
7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	1	1
7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0
8:00 AM	0	1	0	2	3	8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	1	1	8:15 AM	0	0	0	0	0
8:30 AM	0	1	0	1	2	8:30 AM	0	0	0	0	0
8:45 AM	0	0	1	0	1	8:45 AM	0	0	0	0	0
9:00 AM	0	0	1	0	1	9:00 AM	1	2	0	0	3
9:15 AM	0	0	0	0	0	9:15 AM	1	0	0	0	1
9:30 AM	0	0	0	0	0	9:30 AM	0	0	0	0	0
9:45 AM	0	1	0	1	2	9:45 AM	1	0	0	0	1
10:00 AM	0	1	2	0	3	10:00 AM	2	1	0	0	3
10:15 AM	0	0	0	0	0	10:15 AM	0	0	0	0	0
10:30 AM	0	0	0	0	0	10:30 AM	1	0	0	0	1
10:45 AM	0	0	0	1	1	10:45 AM	2	0	0	0	2
11:00 AM	0	0	0	0	0	11:00 AM	0	0	0	0	0
11:15 AM	0	0	0	1	1	11:15 AM	0	0	0	0	0
11:30 AM	0	0	1	0	1	11:30 AM	0	0	0	0	0
11:45 AM	0	0	0	0	0	11:45 AM	0	0	0	0	0
12:00 PM	0	0	0	0	0	12:00 PM	0	0	0	0	0
12:15 PM	0	0	0	0	0	12:15 PM	1	0	1	0	2
12:30 PM	0	1	0	1	2	12:30 PM	0	0	0	0	0
12:45 PM	0	0	1	0	1	12:45 PM	0	0	0	0	0
1:00 PM	0	0	0	0	0	1:00 PM	0	0	0	0	0
1:15 PM	0	0	0	0	0	1:15 PM	1	0	0	0	1
1:30 PM	0	0	0	0	0	1:30 PM	0	0	1	0	1
1:45 PM	0	0	0	1	1	1:45 PM	2	0	1	0	3
2:00 PM	0	0	0	1	1	2:00 PM	0	0	0	0	0
2:15 PM	0	0	0	0	0	2:15 PM	1	0	0	0	1
2:30 PM	0	0	0	0	0	2:30 PM	2	0	0	0	2
2:45 PM	0	0	0	0	0	2:45 PM	0	0	0	0	0
3:00 PM	0	0	0	0	0	3:00 PM	0	0	0	0	0
3:15 PM	0	0	0	0	0	3:15 PM	0	0	0	0	0

3:30 PM	0	0	0	0	0	3:30 PM	1	0	0	0	1
3:45 PM	0	0	0	0	0	3:45 PM	0	0	0	1	1
4:00 PM	0	0	0	0	0	4:00 PM	1	0	0	0	1
4:15 PM	0	0	0	0	0	4:15 PM	0	1	0	0	1
4:30 PM	0	0	0	0	0	4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0	4:45 PM	1	0	0	0	1
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0	5:15 PM	1	0	0	0	1
5:30 PM	0	0	0	0	0	5:30 PM	2	0	0	1	3
5:45 PM	0	0	0	0	0	5:45 PM	2	0	0	1	3
6:00 PM	0	0	0	0	0	6:00 PM	1	0	0	0	1
6:15 PM	0	0	0	0	0	6:15 PM	1	0	0	0	1
6:30 PM	0	0	0	0	0	6:30 PM	1	0	0	0	1
6:45 PM	0	0	0	0	0	6:45 PM	2	1	0	0	3
Count Total	0	5	6	10	21	Count Total	29	5	3	4	41
Peak Hour	0	1	0	1	2	Peak Hour	1	0	1	0	2

Interval Start Time	BOBBY FOSTER RD				BOBBY FOSTER RD				UNIVERSITY BLVD SE				UNIVERSITY BLVD SE				Total	Rolling Hour
	Eastbound				Westbound				Northbound				Southbound					
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
7:00 AM	0	0	0	3	0	0	0	0	0	6	18	0	0	0	29	0	56	286
7:15 AM	0	0	0	4	0	0	0	0	0	5	29	0	0	0	46	1	85	307
7:30 AM	0	0	0	1	0	0	0	0	0	0	27	0	0	0	44	0	72	350
7:45 AM	0	8	0	1	0	0	0	0	0	1	21	0	0	0	41	1	73	376
8:00 AM	0	1	0	5	0	0	0	0	0	3	32	0	0	0	36	0	77	367
8:15 AM	0	4	0	11	0	0	0	0	0	8	33	0	0	0	71	1	128	346
8:30 AM	0	1	0	4	0	0	0	0	0	7	52	0	0	0	34	0	98	275
8:45 AM	0	2	0	2	0	0	0	0	0	6	18	0	0	0	35	1	64	219
9:00 AM	0	0	0	3	0	0	0	0	0	1	22	0	0	0	30	0	56	200
9:15 AM	0	1	0	0	0	0	0	0	0	2	25	0	0	0	28	1	57	205
9:30 AM	0	1	0	1	0	0	0	0	0	2	17	0	0	0	20	1	42	193
9:45 AM	0	1	0	1	0	0	0	0	0	3	24	0	0	0	15	1	45	192
10:00 AM	0	1	0	2	0	0	0	0	0	5	19	0	0	0	33	1	61	206
10:15 AM	0	2	0	3	0	0	0	0	0	4	23	0	0	0	12	1	45	189
10:30 AM	0	2	0	0	0	0	0	0	0	2	14	0	0	0	20	3	41	195
10:45 AM	0	1	0	1	0	0	0	0	0	3	27	0	0	0	25	2	59	226
11:00 AM	0	2	0	2	0	0	0	0	0	0	20	0	0	0	20	0	44	251
11:15 AM	0	0	0	1	0	0	0	0	0	2	16	0	0	0	31	1	51	284
11:30 AM	0	2	0	0	0	0	0	0	0	1	24	0	0	0	44	1	72	306
11:45 AM	0	2	0	1	0	0	0	0	0	0	31	0	0	0	49	1	84	298
12:00 PM	0	0	0	1	0	0	0	0	0	2	27	0	0	0	47	0	77	276
12:15 PM	0	1	0	1	0	0	0	0	0	2	31	0	0	0	34	4	73	247
12:30 PM	0	4	0	6	0	0	0	0	0	1	29	0	0	0	24	0	64	246
12:45 PM	0	2	0	2	0	0	0	0	0	2	23	0	0	0	32	1	62	227
1:00 PM	0	3	0	5	0	0	0	0	0	0	20	0	0	0	20	0	48	215
1:15 PM	0	2	0	4	0	0	0	0	0	4	30	0	0	0	32	0	72	218
1:30 PM	0	3	0	1	0	0	0	0	0	1	27	0	0	0	11	2	45	194

1:45 PM	0	1	0	3	0	0	0	0	0	4	23	0	0	0	17	2	50	200
2:00 PM	0	4	0	3	0	0	0	0	0	2	20	0	0	0	21	1	51	208
2:15 PM	0	0	0	2	0	0	0	0	0	2	25	0	0	0	19	0	48	237
2:30 PM	0	0	0	2	0	0	0	0	0	0	16	0	0	0	31	2	51	266
2:45 PM	0	0	0	2	0	0	0	0	0	1	24	0	0	0	31	0	58	290
3:00 PM	0	3	0	5	0	0	0	0	0	5	35	0	0	0	30	2	80	304
3:15 PM	0	1	0	5	0	0	0	0	0	5	26	0	0	0	38	2	77	288
3:30 PM	0	2	0	0	0	0	0	0	0	13	33	0	0	0	26	1	75	272
3:45 PM	0	2	0	0	0	0	0	0	0	4	40	0	0	0	25	1	72	256
4:00 PM	0	0	0	2	0	0	0	0	0	2	32	0	0	0	28	0	64	246
4:15 PM	0	0	0	5	0	0	0	0	0	7	30	0	0	0	18	1	61	252
4:30 PM	0	0	0	5	0	0	0	0	0	1	28	0	0	0	21	4	59	241
4:45 PM	0	0	0	3	0	0	0	0	0	3	30	0	0	0	25	1	62	226
5:00 PM	0	2	0	2	0	0	0	0	0	5	40	0	0	0	21	0	70	214
5:15 PM	0	0	0	0	0	0	0	0	0	3	19	0	0	0	28	0	50	196
5:30 PM	0	0	0	2	0	0	0	0	0	2	18	0	0	0	22	0	44	185
5:45 PM	0	0	0	1	0	0	0	0	0	2	24	0	0	0	23	0	50	181
6:00 PM	0	0	0	2	0	0	0	0	0	2	23	0	0	0	20	5	52	173
6:15 PM	0	1	0	1	0	0	0	0	0	0	17	0	0	0	20	0	39	
6:30 PM	0	0	0	2	0	0	0	0	0	0	17	0	0	0	19	2	40	
6:45 PM	0	0	0	1	0	0	0	0	0	2	22	0	0	0	17	0	42	
Count Total	0	62	0	114	0	0	0	0	0	138	1,221	0	0	0	1,363	48	2,946	
Peak Hour	0	14	0	21	0	0	0	0	0	19	138	0	0	0	182	2	376	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
7:00 AM	0	0	0	0	0	7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	1	1	8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0	8:15 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0	8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0	8:45 AM	0	0	0	0	0
9:00 AM	0	0	0	0	0	9:00 AM	0	0	0	0	0
9:15 AM	0	0	0	0	0	9:15 AM	0	0	0	0	0
9:30 AM	0	0	0	0	0	9:30 AM	0	0	0	0	0
9:45 AM	0	0	0	1	1	9:45 AM	0	0	0	0	0
10:00 AM	0	1	0	0	1	10:00 AM	0	0	0	0	0
10:15 AM	0	0	0	0	0	10:15 AM	0	0	0	0	0
10:30 AM	0	0	0	0	0	10:30 AM	0	0	0	0	0
10:45 AM	0	0	0	1	1	10:45 AM	0	0	0	0	0
11:00 AM	0	0	0	0	0	11:00 AM	0	0	0	0	0
11:15 AM	0	0	0	1	1	11:15 AM	0	0	0	0	0
11:30 AM	0	0	0	0	0	11:30 AM	0	0	0	0	0
11:45 AM	0	0	0	0	0	11:45 AM	0	0	0	0	0
12:00 PM	0	0	0	0	0	12:00 PM	0	0	0	0	0
12:15 PM	0	0	0	0	0	12:15 PM	0	0	0	0	0
12:30 PM	1	0	0	0	1	12:30 PM	0	0	0	0	0
12:45 PM	0	0	0	1	1	12:45 PM	0	0	0	0	0
1:00 PM	0	0	0	0	0	1:00 PM	0	0	0	0	0
1:15 PM	0	0	0	0	0	1:15 PM	0	0	0	0	0
1:30 PM	0	0	0	0	0	1:30 PM	0	0	0	0	0
1:45 PM	0	0	0	1	1	1:45 PM	0	0	0	0	0
2:00 PM	0	0	0	1	1	2:00 PM	0	0	0	0	0
2:15 PM	0	0	0	0	0	2:15 PM	0	0	0	0	0
2:30 PM	0	0	0	0	0	2:30 PM	0	0	0	0	0
2:45 PM	0	0	0	0	0	2:45 PM	0	0	0	0	0
3:00 PM	0	0	0	0	0	3:00 PM	0	0	0	0	0
3:15 PM	0	0	0	0	0	3:15 PM	0	0	0	0	0

3:30 PM	0	0	0	0	0	3:30 PM	0	0	0	0	0
3:45 PM	0	0	0	0	0	3:45 PM	0	0	0	0	0
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	1	1	4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0	4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0	5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	0
6:00 PM	0	0	0	0	0	6:00 PM	0	0	0	2	2
6:15 PM	0	0	0	0	0	6:15 PM	0	0	0	0	0
6:30 PM	0	0	0	0	0	6:30 PM	0	0	0	0	0
6:45 PM	0	0	0	0	0	6:45 PM	0	0	0	0	0
Count Total	1	1	0	8	10	Count Total	0	0	0	2	2
Peak Hour	0	0	0	1	1	Peak Hour	0	0	0	0	0



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Location: 1 UNIVERSITY BLVD SE & AVENDON AVE AM

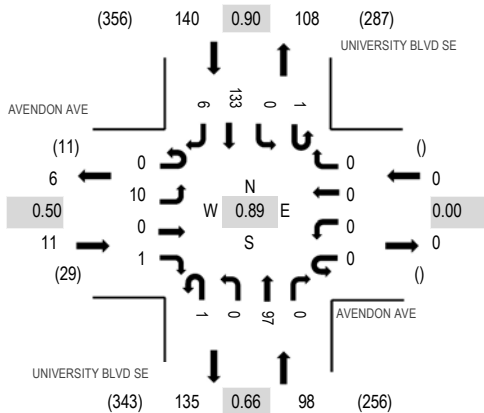
Date: Wednesday, April 21, 2021

Peak Hour: 07:45 AM - 08:45 AM

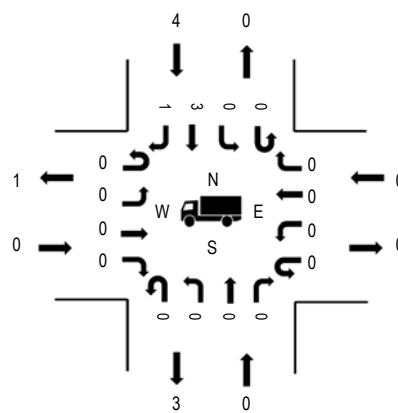
Peak 15-Minutes: 08:30 AM - 08:45 AM

Peak Hour

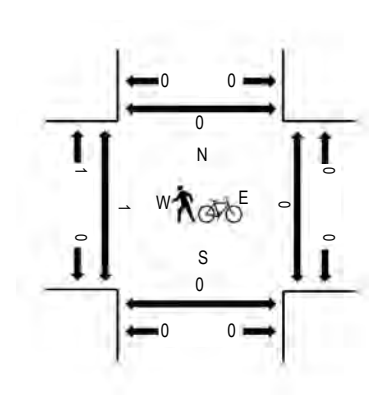
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.50
WB	0.0%	0.00
NB	0.0%	0.66
SB	2.9%	0.90
All	1.6%	0.89

Traffic Counts - Motorized Vehicles

Interval Start Time	AVENDON AVE Eastbound				AVENDON AVE Westbound				UNIVERSITY BLVD SE Northbound				UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
7:00 AM	0	1	0	0	0	0	0	0	0	0	17	0	0	0	24	2	44	220
7:15 AM	0	2	0	0	0	0	0	0	0	0	29	0	1	0	24	0	56	225
7:30 AM	0	3	0	0	0	0	0	0	0	0	15	0	2	0	38	0	58	237
7:45 AM	0	2	0	0	0	0	0	0	0	0	19	0	0	0	39	2	62	249
8:00 AM	0	2	0	0	0	0	0	0	1	0	21	0	0	0	25	0	49	239
8:15 AM	0	6	0	1	0	0	0	0	0	0	20	0	1	0	39	1	68	240
8:30 AM	0	0	0	0	0	0	0	0	0	0	37	0	0	0	30	3	70	220
8:45 AM	0	3	0	0	0	0	0	0	0	0	15	0	0	0	34	0	52	196
9:00 AM	0	2	0	0	0	0	0	0	0	0	20	0	0	0	27	1	50	182
9:15 AM	0	5	0	0	0	0	0	0	0	0	18	0	1	0	22	2	48	
9:30 AM	0	0	0	0	0	0	0	0	0	0	25	0	0	0	21	0	46	
9:45 AM	0	2	0	0	0	0	0	0	1	0	18	0	0	0	17	0	38	
Count Total	0	28	0	1	0	0	0	0	2	0	254	0	5	0	340	11	641	
Peak Hour	0	10	0	1	0	0	0	0	1	0	97	0	1	0	133	6	249	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
7:00 AM	0	0	0	0	0	7:00 AM	0	0	0	1	1
7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	1	1	7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	2	2	8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0	8:15 AM	1	0	0	0	1
8:30 AM	0	0	0	1	1	8:30 AM	0	0	0	0	0
8:45 AM	0	1	0	0	1	8:45 AM	0	0	0	0	0
9:00 AM	0	1	0	0	1	9:00 AM	4	0	0	0	4

9:15 AM	0	0	0	0	0	9:15 AM	0	0	0	0	0
9:30 AM	0	0	0	0	0	9:30 AM	0	0	0	0	0
9:45 AM	0	0	0	1	1	9:45 AM	0	0	0	0	0
Count Total	0	2	0	5	7	Count Total	5	0	0	1	6
Peak Hour	0	0	0	4	4	Peak Hour	1	0	0	0	1

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

9:15 AM	0	0	0	0	0	9:15 AM	0	0	0	0	0
9:30 AM	0	0	0	0	0	9:30 AM	0	0	0	0	0
9:45 AM	0	0	0	1	1	9:45 AM	1	0	1	1	3
Count Total	1	0	0	3	4	Count Total	2	3	1	2	8
Peak Hour	0	0	0	2	2	Peak Hour	1	0	0	0	1

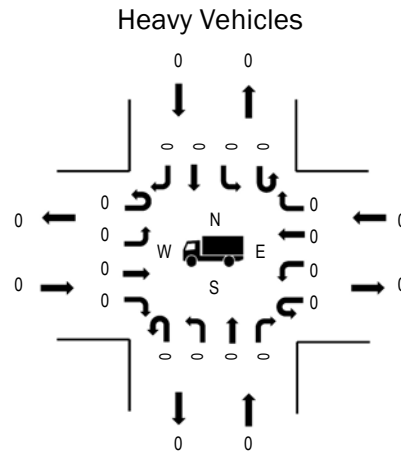
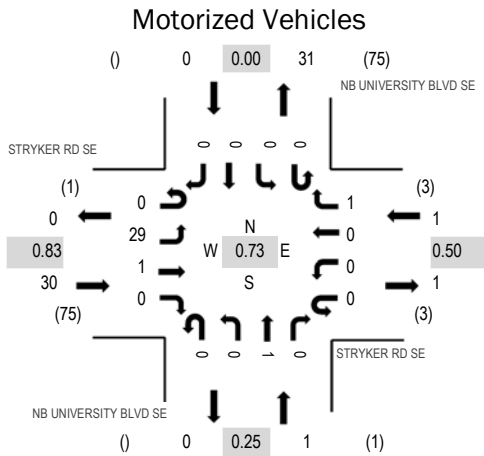
Location: 3 NB UNIVERSITY BLVD SE & STRYKER RD SE AM

Date: Wednesday, April 21, 2021

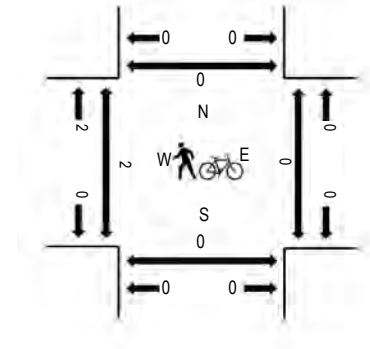
Peak Hour: 08:45 AM - 09:45 AM

Peak 15-Minutes: 09:30 AM - 09:45 AM

Peak Hour



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.83
WB	0.0%	0.50
NB	0.0%	0.25
SB	0.0%	0.00
All	0.0%	0.73

Traffic Counts - Motorized Vehicles

Interval Start Time	STRYKER RD SE Eastbound				STRYKER RD SE Westbound				NB UNIVERSITY BLVD SE Northbound				NB UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
7:00 AM	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	18
7:15 AM	0	5	2	0	0	0	0	1	0	0	0	0	0	0	0	0	8	22
7:30 AM	0	2	0	0	0	0	1	0	0	0	0	0	0	0	0	0	3	20
7:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	26
8:00 AM	0	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9	30
8:15 AM	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	28
8:30 AM	0	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9	30
8:45 AM	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	32
9:00 AM	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	31
9:15 AM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	
9:30 AM	0	8	1	0	0	0	0	1	0	0	1	0	0	0	0	0	11	
9:45 AM	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	
Count Total	0	72	3	0	0	0	1	2	0	0	1	0	0	0	0	0	79	
Peak Hour	0	29	1	0	0	0	0	1	0	0	1	0	0	0	0	0	32	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
7:00 AM	0	0	0	0	0	7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0	7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0
8:00 AM	1	0	0	0	1	8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0	8:15 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0	8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0	8:45 AM	2	0	0	0	2
9:00 AM	0	0	0	0	0	9:00 AM	0	0	0	0	0

9:15 AM	0	0	0	0	0	9:15 AM	0	0	0	0	0
9:30 AM	0	0	0	0	0	9:30 AM	0	0	0	0	0
9:45 AM	0	0	0	0	0	9:45 AM	0	0	0	0	0
Count Total	1	0	0	0	1	Count Total	2	0	0	0	2
Peak Hour	0	0	0	0	0	Peak Hour	2	0	0	0	2

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

9:15 AM	0	0	0	0	0	9:15 AM	0	0	0	0	0
9:30 AM	0	0	0	1	1	9:30 AM	0	0	0	0	0
9:45 AM	0	0	0	0	0	9:45 AM	0	0	0	0	0
Count Total	0	3	0	4	7	Count Total	0	0	0	0	0
Peak Hour	0	1	0	3	4	Peak Hour	0	0	0	0	0

Location: 9 UNIVERSITY BLVD SE & CRICK AVE SE AM

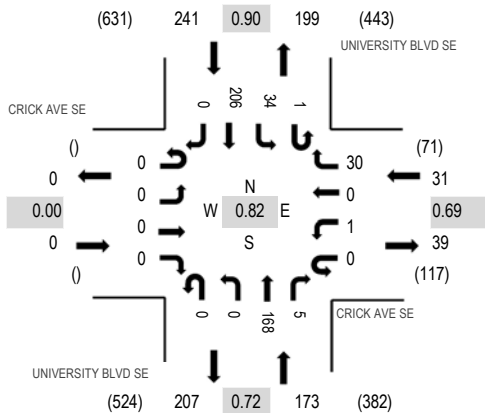
Date: Wednesday, April 21, 2021

Peak Hour: 07:45 AM - 08:45 AM

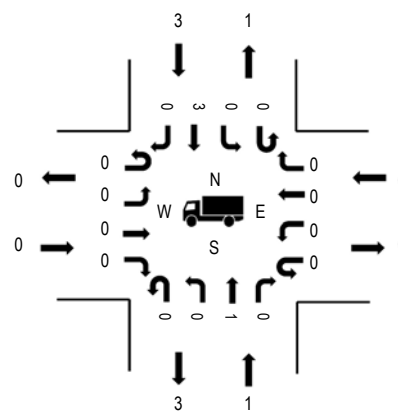
Peak 15-Minutes: 08:15 AM - 08:30 AM

Peak Hour

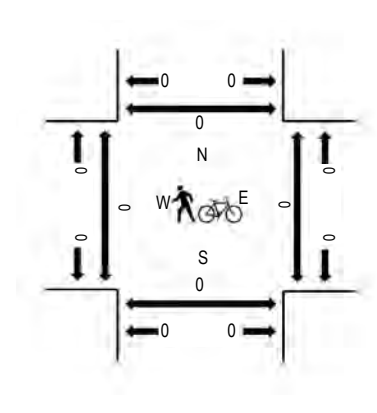
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.00
WB	0.0%	0.69
NB	0.6%	0.72
SB	1.2%	0.90
All	0.9%	0.82

Traffic Counts - Motorized Vehicles

Interval Start Time	CRICK AVE SE Eastbound				CRICK AVE SE Westbound				UNIVERSITY BLVD SE Northbound				UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
7:00 AM	0	0	0	0	0	1	0	7	0	0	20	0	0	13	29	0	70	359
7:15 AM	0	0	0	0	1	0	0	6	0	0	26	2	2	16	55	0	108	406
7:30 AM	0	0	0	0	0	0	0	1	0	0	34	0	0	11	54	0	100	433
7:45 AM	0	0	0	0	0	1	0	2	0	0	21	2	0	10	45	0	81	445
8:00 AM	0	0	0	0	0	0	0	8	0	0	39	1	0	14	55	0	117	445
8:15 AM	0	0	0	0	0	0	0	12	0	0	50	0	1	7	65	0	135	398
8:30 AM	0	0	0	0	0	0	0	8	0	0	58	2	0	3	41	0	112	345
8:45 AM	0	0	0	0	0	0	0	5	0	0	23	0	0	6	47	0	81	292
9:00 AM	0	0	0	0	0	0	0	2	0	0	23	1	0	9	35	0	70	280
9:15 AM	0	0	0	0	0	1	0	7	0	0	30	0	0	7	37	0	82	
9:30 AM	0	0	0	0	0	2	0	2	0	0	24	1	1	3	26	0	59	
9:45 AM	0	0	0	0	0	0	0	5	0	0	24	1	2	7	30	0	69	
Count Total	0	0	0	0	1	5	0	65	0	0	372	10	6	106	519	0	1,084	
Peak Hour	0	0	0	0	0	1	0	30	0	0	168	5	1	34	206	0	445	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
7:00 AM	0	0	0	0	0	7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	1	1	7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0	7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0	7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	2	2	8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0	8:15 AM	0	0	0	0	0
8:30 AM	0	1	0	1	2	8:30 AM	0	0	0	0	0
8:45 AM	0	1	0	0	1	8:45 AM	0	0	0	0	0
9:00 AM	0	1	0	1	2	9:00 AM	0	0	0	0	0

9:15 AM	0	0	0	0	0	9:15 AM	0	0	0	0	0
9:30 AM	0	0	0	0	0	9:30 AM	0	0	0	0	0
9:45 AM	0	0	1	1	2	9:45 AM	0	0	0	0	0
Count Total	0	3	1	6	10	Count Total	0	0	0	0	0
Peak Hour	0	1	0	3	4	Peak Hour	0	0	0	0	0

Location: 1 UNIVERSITY BLVD SE & AVENDON AVE Noon

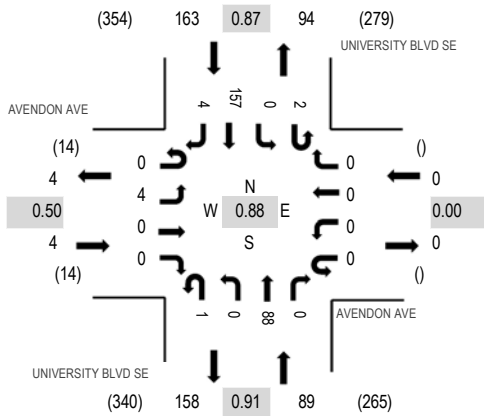
Date: Wednesday, April 21, 2021

Peak Hour: 11:30 AM - 12:30 PM

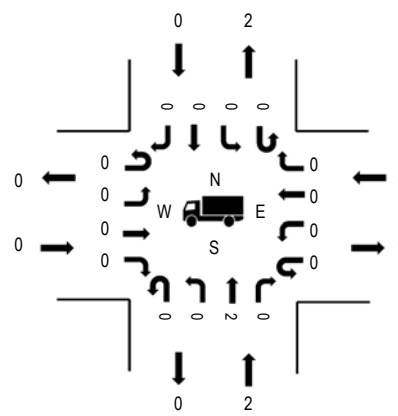
Peak 15-Minutes: 12:00 PM - 12:15 PM

Peak Hour

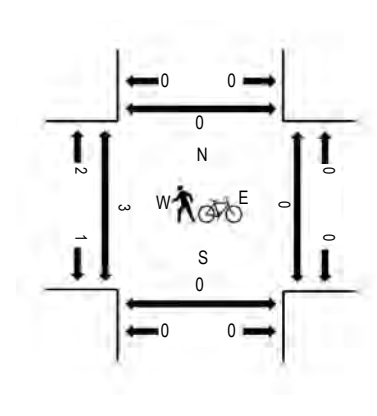
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.50
WB	0.0%	0.00
NB	2.2%	0.91
SB	0.0%	0.87
All	0.8%	0.88

Traffic Counts - Motorized Vehicles

Interval Start Time	AVENDON AVE Eastbound				AVENDON AVE Westbound				UNIVERSITY BLVD SE Northbound				UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
11:00 AM	0	1	0	1	0	0	0	0	0	0	19	0	0	0	19	3	43	220
11:15 AM	0	1	0	0	0	0	0	0	0	0	16	0	0	0	28	0	45	250
11:30 AM	0	2	0	0	0	0	0	0	1	0	17	0	2	0	38	0	60	256
11:45 AM	0	0	0	0	0	0	0	0	0	0	26	0	0	0	43	3	72	252
12:00 PM	0	1	0	0	0	0	0	0	0	0	25	0	0	0	46	1	73	230
12:15 PM	0	1	0	0	0	0	0	0	0	0	20	0	0	0	30	0	51	203
12:30 PM	0	0	0	0	0	0	0	0	0	0	27	0	0	0	29	0	56	207
12:45 PM	0	1	0	0	0	0	0	0	0	0	19	0	0	0	27	3	50	191
1:00 PM	0	2	0	1	0	0	0	0	0	0	18	0	0	0	22	3	46	183
1:15 PM	0	1	0	0	0	0	0	0	0	0	25	0	0	0	29	0	55	
1:30 PM	0	1	0	0	0	0	0	0	0	0	28	0	1	0	9	1	40	
1:45 PM	0	1	0	0	0	0	0	0	0	0	24	0	0	0	17	0	42	
Count Total	0	12	0	2	0	0	0	0	1	0	264	0	3	0	337	14	633	
Peak Hour	0	4	0	0	0	0	0	0	1	0	88	0	2	0	157	4	256	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
11:00 AM	0	0	0	0	0	11:00 AM	0	0	0	0	0
11:15 AM	0	0	0	1	1	11:15 AM	0	0	0	0	0
11:30 AM	0	2	0	0	2	11:30 AM	0	0	0	0	0
11:45 AM	0	0	0	0	0	11:45 AM	0	0	0	0	0
12:00 PM	0	0	0	0	0	12:00 PM	2	0	0	0	2
12:15 PM	0	0	0	0	0	12:15 PM	1	0	0	0	1
12:30 PM	0	0	0	1	1	12:30 PM	0	0	0	0	0
12:45 PM	0	1	0	0	1	12:45 PM	0	0	0	0	0
1:00 PM	0	0	0	0	0	1:00 PM	0	0	0	0	0

1:15 PM	0	0	0	0	0	1:15 PM	1	0	0	0	1
1:30 PM	0	0	0	0	0	1:30 PM	0	0	0	0	0
1:45 PM	0	0	0	1	1	1:45 PM	2	0	0	0	2
Count Total	0	3	0	3	6	Count Total	6	0	0	0	6
Peak Hour	0	2	0	0	2	Peak Hour	3	0	0	0	3

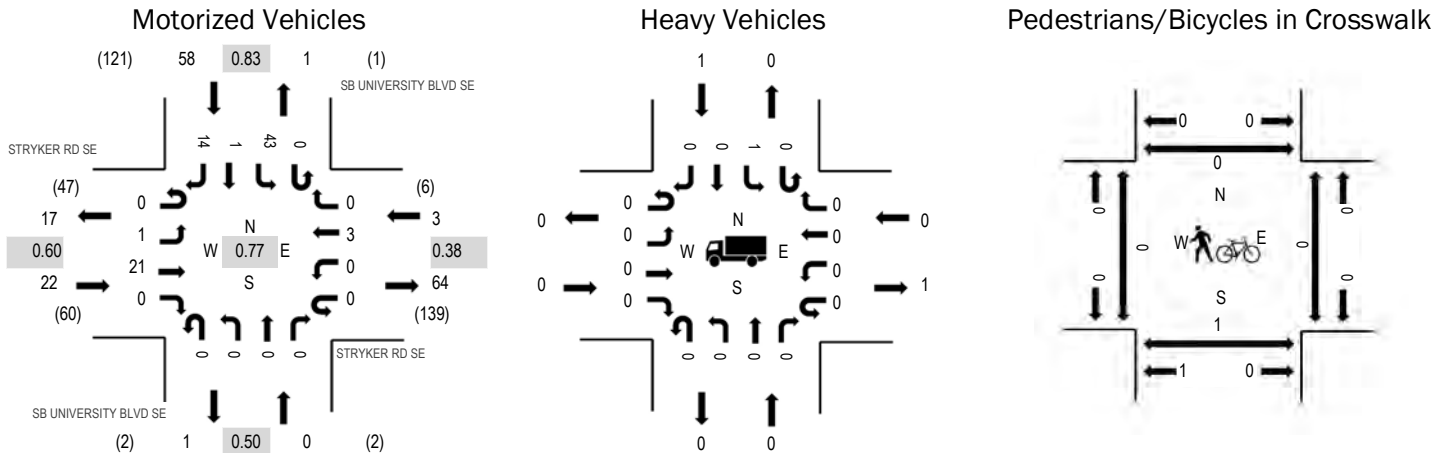
Location: 2 SB UNIVERSITY BLVD SE & STRYKER RD SE Noon

Date: Wednesday, April 21, 2021

Peak Hour: 11:45 AM - 12:45 PM

Peak 15-Minutes: 12:30 PM - 12:45 PM

Peak Hour



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.60
WB	0.0%	0.38
NB	0.0%	0.50
SB	1.7%	0.83
All	1.2%	0.77

Traffic Counts - Motorized Vehicles

Interval Start Time	STRYKER RD SE Eastbound				STRYKER RD SE Westbound				SB UNIVERSITY BLVD SE Northbound				SB UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
11:00 AM	0	0	4	0	0	0	1	0	0	0	0	0	0	3	0	4	12	59
11:15 AM	0	0	10	0	0	0	0	0	0	0	0	0	0	5	0	3	18	72
11:30 AM	0	0	5	0	0	0	0	0	0	0	0	0	0	5	0	3	13	69
11:45 AM	0	0	4	0	0	0	0	0	0	0	0	0	0	9	0	3	16	83
12:00 PM	0	0	5	0	0	0	2	0	0	0	0	0	0	14	0	4	25	82
12:15 PM	0	0	4	0	0	0	1	0	0	0	0	0	0	6	0	4	15	69
12:30 PM	0	1	8	0	0	0	0	0	0	0	0	0	0	14	1	3	27	69
12:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	7	0	7	15	53
1:00 PM	0	0	3	0	0	0	0	0	0	0	0	0	0	6	1	2	12	48
1:15 PM	0	0	2	0	0	0	1	0	0	0	0	1	0	5	0	6	15	
1:30 PM	0	0	7	0	0	0	0	0	0	0	0	1	0	2	0	1	11	
1:45 PM	0	0	6	0	0	0	1	0	0	0	0	0	0	2	0	1	10	
Count Total	0	1	59	0	0	0	6	0	0	0	0	2	0	78	2	41	189	
Peak Hour	0	1	21	0	0	0	3	0	0	0	0	0	0	43	1	14	83	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
11:00 AM	0	0	0	0	0	11:00 AM	0	0	0	1	1
11:15 AM	0	0	0	0	0	11:15 AM	0	0	0	0	0
11:30 AM	1	0	0	0	1	11:30 AM	0	0	0	0	0
11:45 AM	0	0	0	0	0	11:45 AM	0	1	0	0	1
12:00 PM	0	0	0	0	0	12:00 PM	0	0	0	0	0
12:15 PM	0	0	0	0	0	12:15 PM	0	0	0	0	0
12:30 PM	0	0	0	1	1	12:30 PM	0	0	0	0	0
12:45 PM	0	0	0	0	0	12:45 PM	0	0	0	0	0
1:00 PM	0	0	0	0	0	1:00 PM	0	0	0	0	0

1:15 PM	0	0	0	0	0	1:15 PM	0	0	0	1	1
1:30 PM	0	0	0	0	0	1:30 PM	0	2	2	0	4
1:45 PM	0	0	0	0	0	1:45 PM	0	0	0	2	2
Count Total	1	0	0	1	2	Count Total	0	3	2	4	9
Peak Hour	0	0	0	1	1	Peak Hour	0	1	0	0	1

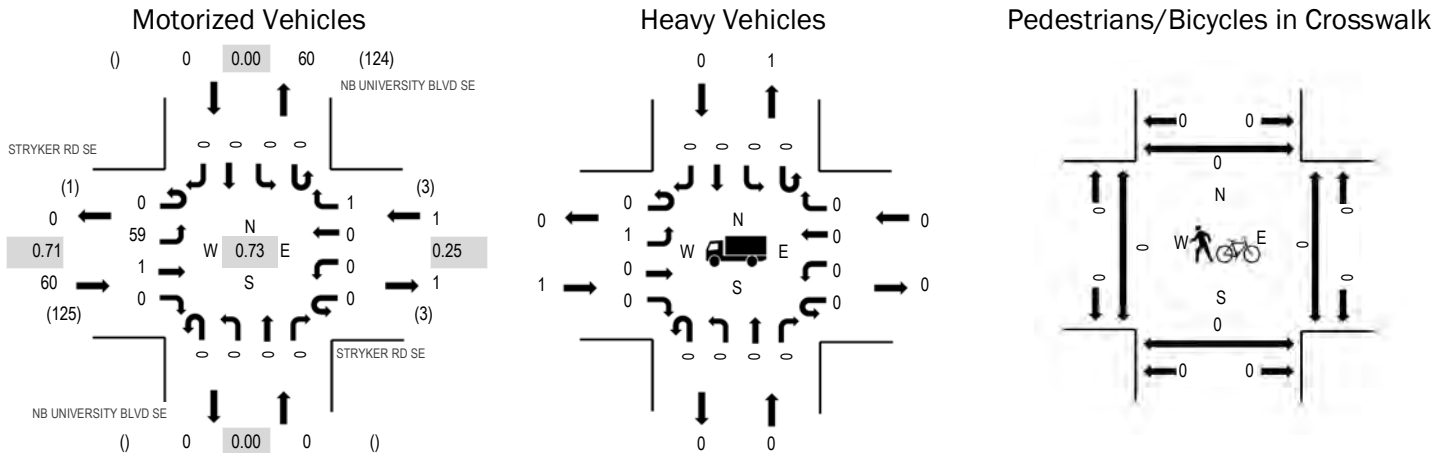
Location: 3 NB UNIVERSITY BLVD SE & STRYKER RD SE Noon

Date: Wednesday, April 21, 2021

Peak Hour: 11:45 AM - 12:45 PM

Peak 15-Minutes: 12:30 PM - 12:45 PM

Peak Hour



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	1.7%	0.71
WB	0.0%	0.25
NB	0.0%	0.00
SB	0.0%	0.00
All	1.6%	0.73

Traffic Counts - Motorized Vehicles

Interval Start Time	STRYKER RD SE Eastbound				STRYKER RD SE Westbound				NB UNIVERSITY BLVD SE Northbound				NB UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
11:00 AM	0	4	1	0	0	0	1	0	0	0	0	0	0	0	0	0	6	40
11:15 AM	0	12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	54
11:30 AM	0	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	50
11:45 AM	0	12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	61
12:00 PM	0	18	1	0	0	0	0	1	0	0	0	0	0	0	0	0	20	57
12:15 PM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	45
12:30 PM	0	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	21	45
12:45 PM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	32
1:00 PM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	31
1:15 PM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	
1:30 PM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	
1:45 PM	0	5	1	0	0	0	0	1	0	0	0	0	0	0	0	0	7	
Count Total	0	122	3	0	0	0	1	2	0	0	0	0	0	0	0	0	128	
Peak Hour	0	59	1	0	0	0	0	1	0	0	0	0	0	0	0	0	61	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
11:00 AM	0	0	0	0	0	11:00 AM	0	0	0	0	0
11:15 AM	0	0	0	0	0	11:15 AM	0	0	0	0	0
11:30 AM	1	0	0	0	1	11:30 AM	0	0	0	0	0
11:45 AM	0	0	0	0	0	11:45 AM	0	0	0	0	0
12:00 PM	0	0	0	0	0	12:00 PM	0	0	0	0	0
12:15 PM	0	0	0	0	0	12:15 PM	0	0	0	0	0
12:30 PM	1	0	0	0	1	12:30 PM	0	0	0	0	0
12:45 PM	0	0	0	0	0	12:45 PM	0	0	0	0	0
1:00 PM	0	0	0	0	0	1:00 PM	0	0	0	0	0

1:15 PM	0	0	0	0	0	1:15 PM	0	0	0	0	0
1:30 PM	0	0	0	0	0	1:30 PM	0	0	0	0	0
1:45 PM	0	0	0	0	0	1:45 PM	0	0	0	0	0
Count Total	2	0	0	0	2	Count Total	0	0	0	0	0
Peak Hour	1	0	0	0	1	Peak Hour	0	0	0	0	0

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

1:15 PM	0	0	0	0	0	1:15 PM	0	0	1	0	1
1:30 PM	0	0	0	0	0	1:30 PM	0	0	0	0	0
1:45 PM	0	0	0	1	1	1:45 PM	0	0	1	0	1
Count Total	0	2	3	4	9	Count Total	0	0	2	0	2
Peak Hour	0	2	2	2	6	Peak Hour	0	0	0	0	0

Location: 9 UNIVERSITY BLVD SE & CRICK AVE SE Noon

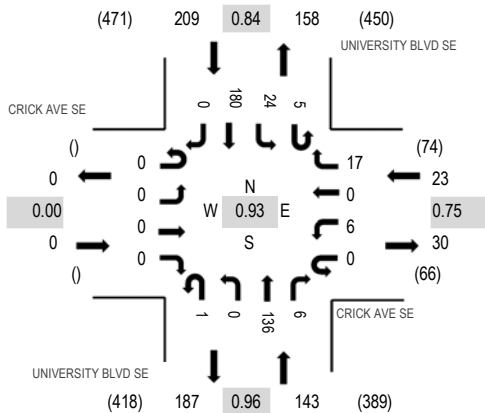
Date: Wednesday, April 21, 2021

Peak Hour: 11:30 AM - 12:30 PM

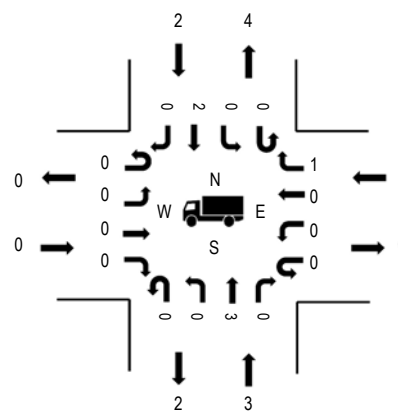
Peak 15-Minutes: 11:45 AM - 12:00 PM

Peak Hour

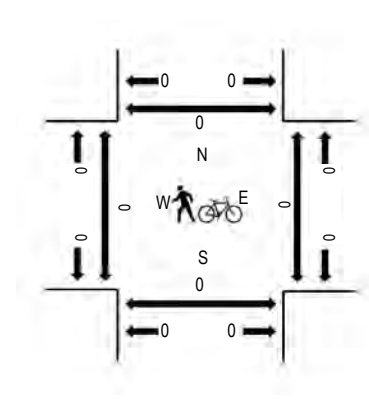
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.00
WB	4.3%	0.75
NB	2.1%	0.96
SB	1.0%	0.84
All	1.6%	0.93

Traffic Counts - Motorized Vehicles

Interval Start Time	CRICK AVE SE Eastbound				CRICK AVE SE Westbound				UNIVERSITY BLVD SE Northbound				UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
11:00 AM	0	0	0	0	0	1	0	8	0	0	29	1	0	4	27	0	70	319
11:15 AM	0	0	0	0	0	0	0	4	0	0	18	0	1	4	32	0	59	348
11:30 AM	0	0	0	0	0	2	0	2	0	0	35	0	1	3	46	0	89	375
11:45 AM	0	0	0	0	0	0	0	2	1	0	35	1	2	10	50	0	101	362
12:00 PM	0	0	0	0	0	4	0	7	0	0	33	1	1	7	46	0	99	338
12:15 PM	0	0	0	0	0	0	0	6	0	0	33	4	1	4	38	0	86	314
12:30 PM	0	0	0	0	0	0	0	7	0	0	38	0	0	4	27	0	76	303
12:45 PM	0	0	0	0	0	1	0	8	0	0	25	0	0	5	38	0	77	285
1:00 PM	0	0	0	0	0	0	0	5	1	0	29	0	2	8	30	0	75	277
1:15 PM	0	0	0	0	0	0	0	8	0	0	31	2	0	1	33	0	75	
1:30 PM	0	0	0	0	0	0	0	7	0	0	36	1	0	2	12	0	58	
1:45 PM	0	0	0	0	0	1	0	1	0	0	35	0	0	4	28	0	69	
Count Total	0	0	0	0	0	9	0	65	2	0	377	10	8	56	407	0	934	
Peak Hour	0	0	0	0	0	6	0	17	1	0	136	6	5	24	180	0	375	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
11:00 AM	0	0	0	1	1	11:00 AM	0	0	0	0	0
11:15 AM	0	1	0	0	1	11:15 AM	0	0	0	0	0
11:30 AM	0	1	0	0	1	11:30 AM	0	0	0	0	0
11:45 AM	0	1	1	2	4	11:45 AM	0	0	0	0	0
12:00 PM	0	0	0	0	0	12:00 PM	0	0	0	0	0
12:15 PM	0	1	0	0	1	12:15 PM	0	0	0	0	0
12:30 PM	0	0	0	1	1	12:30 PM	0	0	0	0	0
12:45 PM	0	0	1	0	1	12:45 PM	0	0	0	0	0
1:00 PM	0	0	0	0	0	1:00 PM	0	0	0	0	0

1:15 PM	0	0	0	1	1	1:15 PM	0	0	0	0	0
1:30 PM	0	0	2	1	3	1:30 PM	0	0	0	0	0
1:45 PM	0	0	0	1	1	1:45 PM	0	0	0	0	0
Count Total	0	4	4	7	15	Count Total	0	0	0	0	0
Peak Hour	0	3	1	2	6	Peak Hour	0	0	0	0	0

Location: 1 UNIVERSITY BLVD SE & AVENDON AVE PM

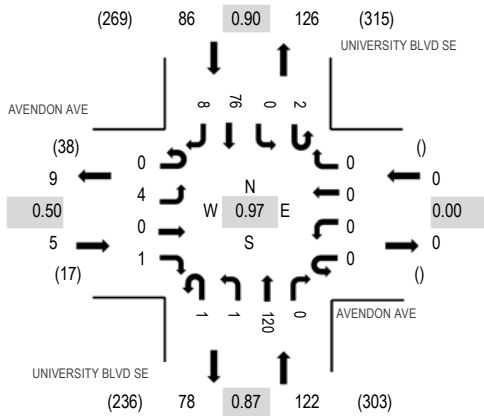
Date: Wednesday, April 21, 2021

Peak Hour: 04:15 PM - 05:15 PM

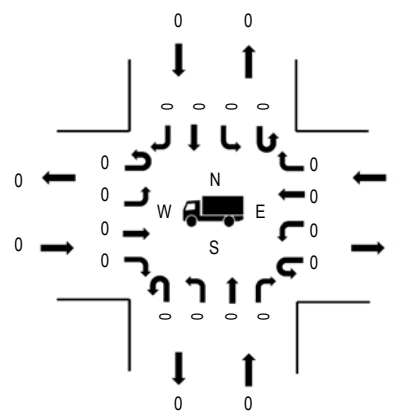
Peak 15-Minutes: 04:15 PM - 04:30 PM

Peak Hour

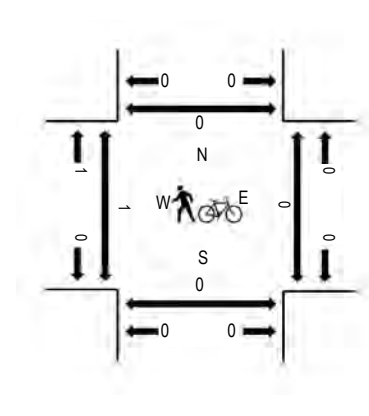
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.50
WB	0.0%	0.00
NB	0.0%	0.87
SB	0.0%	0.90
All	0.0%	0.97

Traffic Counts - Motorized Vehicles

Interval Start Time	AVENDON AVE Eastbound				AVENDON AVE Westbound				UNIVERSITY BLVD SE Northbound				UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
3:00 PM	0	3	0	2	0	0	0	0	0	0	29	0	0	0	18	2	54	196
3:15 PM	0	1	0	0	0	0	0	0	0	2	26	0	0	0	22	1	52	191
3:30 PM	0	1	0	0	0	0	0	0	0	0	18	0	0	0	20	3	42	194
3:45 PM	0	0	0	0	0	0	0	0	0	0	28	0	0	0	16	4	48	203
4:00 PM	0	3	0	0	0	0	0	0	0	0	20	0	0	0	21	5	49	209
4:15 PM	0	0	0	1	0	0	0	0	0	1	28	0	1	0	23	1	55	213
4:30 PM	0	4	0	0	0	0	0	0	1	0	29	0	0	0	15	2	51	200
4:45 PM	0	0	0	0	0	0	0	0	0	0	28	0	0	0	24	2	54	189
5:00 PM	0	0	0	0	0	0	0	0	0	0	35	0	1	0	14	3	53	184
5:15 PM	0	0	0	0	0	0	0	0	0	1	16	0	1	0	20	4	42	
5:30 PM	0	0	0	1	0	0	0	0	0	0	17	0	0	0	20	2	40	
5:45 PM	0	1	0	0	0	0	0	0	0	0	24	0	1	0	18	5	49	
Count Total	0	13	0	4	0	0	0	0	1	4	298	0	4	0	231	34	589	
Peak Hour	0	4	0	1	0	0	0	0	1	1	120	0	2	0	76	8	213	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
3:00 PM	0	0	0	0	0	3:00 PM	0	0	0	0	0
3:15 PM	0	0	0	1	1	3:15 PM	0	1	0	1	2
3:30 PM	0	0	0	0	0	3:30 PM	2	3	0	4	9
3:45 PM	0	0	0	0	0	3:45 PM	1	0	0	0	1
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0	4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0	4:45 PM	1	0	0	0	1
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0

5:15 PM	0	0	0	0	0	5:15 PM	1	0	0	0	1
5:30 PM	0	0	0	0	0	5:30 PM	3	0	0	3	6
5:45 PM	0	0	0	0	0	5:45 PM	1	0	0	0	1
Count Total	0	0	0	1	1	Count Total	9	4	0	8	21
Peak Hour	0	0	0	0	0	Peak Hour	1	0	0	0	1

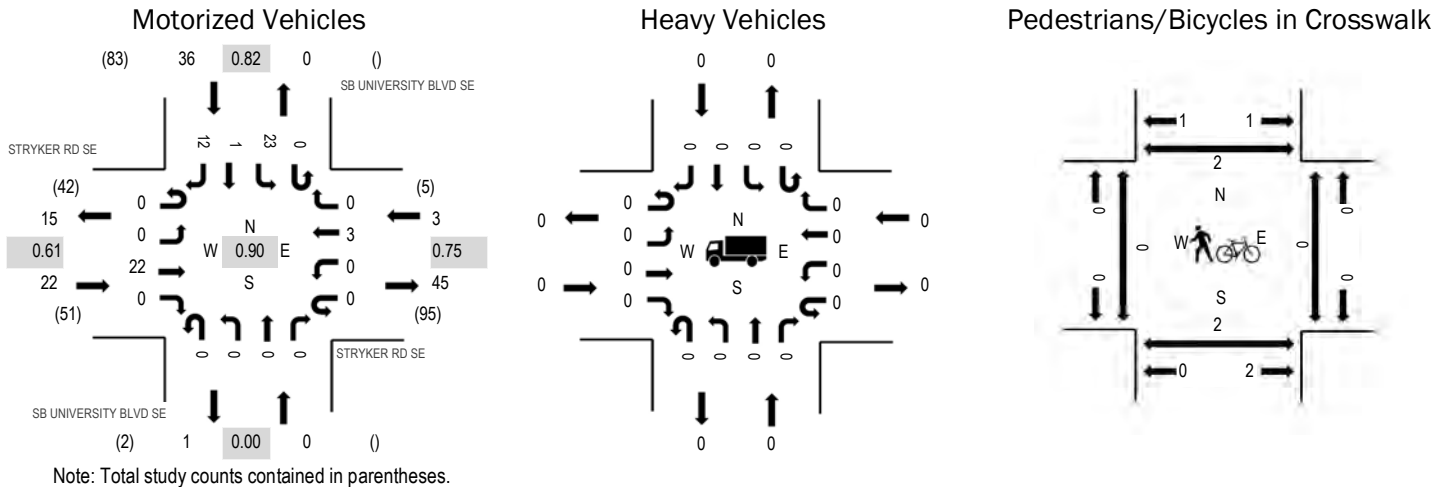
Location: 2 SB UNIVERSITY BLVD SE & STRYKER RD SE PM

Date: Wednesday, April 21, 2021

Peak Hour: 03:45 PM - 04:45 PM

Peak 15-Minutes: 03:45 PM - 04:00 PM

Peak Hour



	HV%	PHF
EB	0.0%	0.61
WB	0.0%	0.75
NB	0.0%	0.00
SB	0.0%	0.82
All	0.0%	0.90

Traffic Counts - Motorized Vehicles

Interval Start Time	STRYKER RD SE Eastbound				STRYKER RD SE Westbound				SB UNIVERSITY BLVD SE Northbound				SB UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
3:00 PM	0	0	4	0	0	0	0	0	0	0	0	0	0	5	0	8	17	51
3:15 PM	0	0	4	0	0	0	1	0	0	0	0	0	0	5	0	3	13	47
3:30 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	1	0	1	4	49
3:45 PM	0	0	9	0	0	0	1	0	0	0	0	0	0	6	0	1	17	61
4:00 PM	0	0	3	0	0	0	1	0	0	0	0	0	0	5	1	3	13	56
4:15 PM	0	0	5	0	0	0	1	0	0	0	0	0	0	4	0	5	15	52
4:30 PM	0	0	5	0	0	0	0	0	0	0	0	0	0	8	0	3	16	46
4:45 PM	0	0	6	0	0	0	0	0	0	0	0	0	0	2	0	4	12	44
5:00 PM	0	0	3	1	0	0	0	0	0	0	0	0	0	2	0	3	9	32
5:15 PM	0	0	5	0	0	0	0	0	0	0	0	0	0	4	0	0	9	
5:30 PM	0	0	4	0	0	0	1	0	0	0	0	0	0	3	0	6	14	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Count Total	0	0	50	1	0	0	5	0	0	0	0	0	0	45	1	37	139	
Peak Hour	0	0	22	0	0	0	3	0	0	0	0	0	0	23	1	12	61	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
3:00 PM	0	0	0	0	0	3:00 PM	0	0	0	0	0
3:15 PM	0	0	0	0	0	3:15 PM	0	0	0	0	0
3:30 PM	0	0	0	0	0	3:30 PM	0	0	0	0	0
3:45 PM	0	0	0	0	0	3:45 PM	0	0	0	0	0
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	2	2
4:15 PM	0	0	0	0	0	4:15 PM	0	1	0	0	1
4:30 PM	0	0	0	0	0	4:30 PM	0	1	0	0	1
4:45 PM	0	0	0	0	0	4:45 PM	1	0	0	0	1
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	2	2

5:15 PM	0	0	0	0	0	5:15 PM	0	0	0	2	2
5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	0
Count Total	0	0	0	0	0	Count Total	1	2	0	6	9
Peak Hour	0	0	0	0	0	Peak Hour	0	2	0	2	4



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Location: 3 NB UNIVERSITY BLVD SE & STRYKER RD SE PM

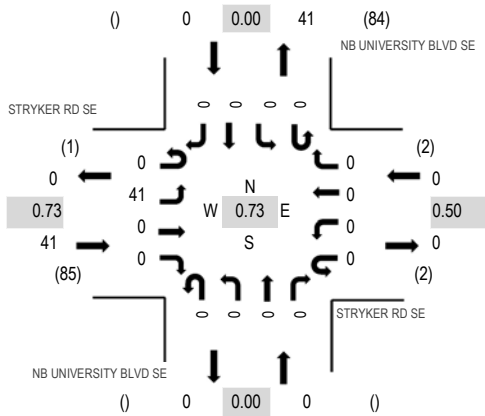
Date: Wednesday, April 21, 2021

Peak Hour: 03:45 PM - 04:45 PM

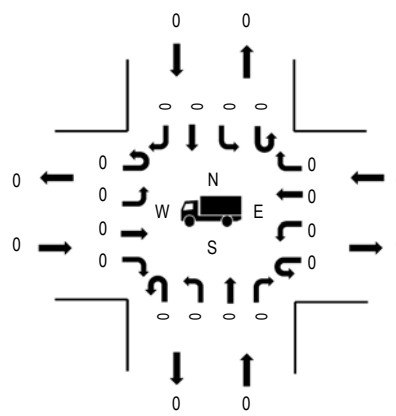
Peak 15-Minutes: 03:45 PM - 04:00 PM

Peak Hour

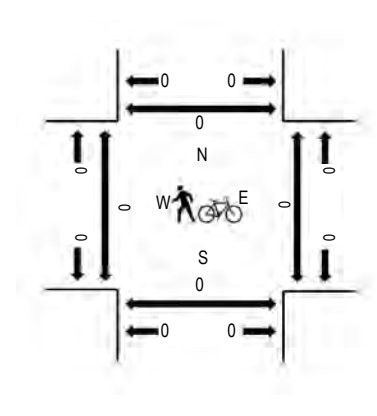
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.73
WB	0.0%	0.50
NB	0.0%	0.00
SB	0.0%	0.00
All	0.0%	0.73

Traffic Counts - Motorized Vehicles

Interval Start Time	STRYKER RD SE Eastbound				STRYKER RD SE Westbound				NB UNIVERSITY BLVD SE Northbound				NB UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
3:00 PM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	32
3:15 PM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	31
3:30 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	32
3:45 PM	0	14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	14	41
4:00 PM	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	35
4:15 PM	0	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9	33
4:30 PM	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11	32
4:45 PM	0	6	1	0	0	0	0	1	0	0	0	0	0	0	0	0	8	28
5:00 PM	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	20
5:15 PM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	
5:30 PM	0	5	1	0	0	0	1	0	0	0	0	0	0	0	0	0	7	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Count Total	0	83	2	0	0	0	1	1	0	0	0	0	0	0	0	0	87	
Peak Hour	0	41	0	0	0	0	0	0	0	0	0	0	0	0	0	0	41	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
3:00 PM	0	0	0	0	0	3:00 PM	0	0	0	0	0
3:15 PM	0	0	0	0	0	3:15 PM	0	0	0	0	0
3:30 PM	0	0	0	0	0	3:30 PM	0	0	0	0	0
3:45 PM	0	0	0	0	0	3:45 PM	0	0	0	0	0
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0	4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0	4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0	4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0

5:15 PM	0	0	0	0	0	5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	0
Count Total	0	0	0	0	0	Count Total	0	0	0	0	0
Peak Hour	0	0	0	0	0	Peak Hour	0	0	0	0	0

Location: 8 UNIVERSITY BLVD SE & FRITTS CROSSING RD SE PM

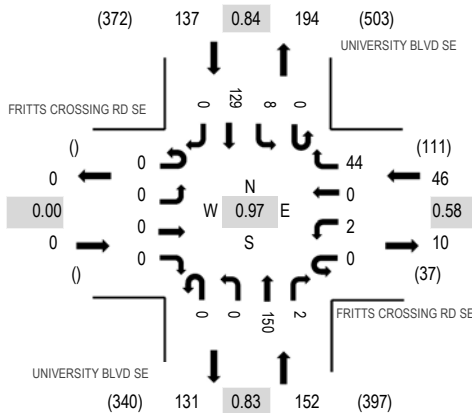
Date: Wednesday, April 21, 2021

Peak Hour: 03:00 PM - 04:00 PM

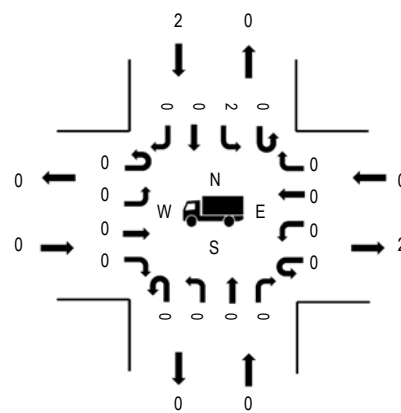
Peak 15-Minutes: 03:15 PM - 03:30 PM

Peak Hour

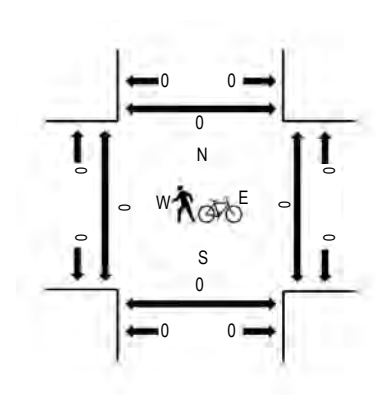
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.00
WB	0.0%	0.58
NB	0.0%	0.83
SB	1.5%	0.84
All	0.6%	0.97

Traffic Counts - Motorized Vehicles

Interval Start Time	FRITTS CROSSING RD SE Eastbound				FRITTS CROSSING RD SE Westbound				UNIVERSITY BLVD SE Northbound				UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
3:00 PM	0	0	0	0	0	0	0	6	0	0	36	1	0	1	35	0	79	335
3:15 PM	0	0	0	0	0	1	0	10	0	0	33	1	0	2	39	0	86	316
3:30 PM	0	0	0	0	0	0	0	20	0	0	35	0	0	1	30	0	86	287
3:45 PM	0	0	0	0	0	1	0	8	0	0	46	0	0	4	25	0	84	277
4:00 PM	0	0	0	0	0	0	0	4	0	0	29	0	0	0	27	0	60	267
4:15 PM	0	0	0	0	0	1	0	6	0	0	29	0	0	1	20	0	57	272
4:30 PM	0	0	0	0	0	0	0	11	0	0	40	0	0	1	24	0	76	274
4:45 PM	0	0	0	0	0	0	0	9	0	0	34	0	0	3	28	0	74	267
5:00 PM	0	0	0	0	0	0	0	6	0	0	38	0	0	1	20	0	65	278
5:15 PM	0	0	0	0	0	0	0	6	0	0	19	0	0	2	32	0	59	
5:30 PM	0	0	0	0	0	0	0	4	0	0	24	0	0	11	30	0	69	
5:45 PM	0	0	0	0	0	0	0	18	0	0	32	0	0	8	27	0	85	
Count Total	0	0	0	0	0	3	0	108	0	0	395	2	0	35	337	0	880	
Peak Hour	0	0	0	0	0	2	0	44	0	0	150	2	0	8	129	0	335	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
3:00 PM	0	0	0	0	0	3:00 PM	0	0	0	0	0
3:15 PM	0	0	0	0	0	3:15 PM	0	0	0	0	0
3:30 PM	0	0	0	1	1	3:30 PM	0	0	0	0	0
3:45 PM	0	0	0	1	1	3:45 PM	0	0	0	0	0
4:00 PM	0	0	0	0	0	4:00 PM	0	0	0	0	0
4:15 PM	0	0	1	1	2	4:15 PM	0	0	0	0	0
4:30 PM	0	0	1	1	2	4:30 PM	0	0	0	0	0
4:45 PM	0	0	1	0	1	4:45 PM	0	1	0	0	1
5:00 PM	0	0	1	0	1	5:00 PM	0	0	0	0	0

5:15 PM	0	0	0	0	0	5:15 PM	0	1	0	0	1
5:30 PM	0	0	0	0	0	5:30 PM	0	0	2	0	2
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	0	0
Count Total	0	0	4	4	8	Count Total	0	2	2	0	4
Peak Hour	0	0	0	2	2	Peak Hour	0	0	0	0	0

Location: 9 UNIVERSITY BLVD SE & CRICK AVE SE PM

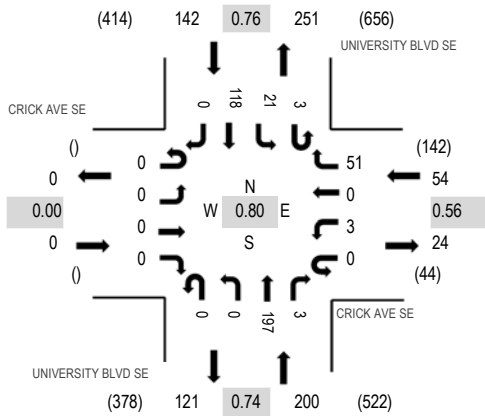
Date: Wednesday, April 21, 2021

Peak Hour: 03:00 PM - 04:00 PM

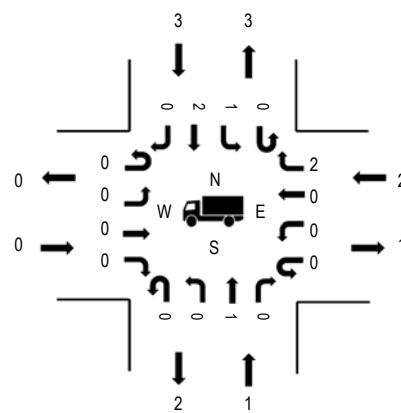
Peak 15-Minutes: 03:30 PM - 03:45 PM

Peak Hour

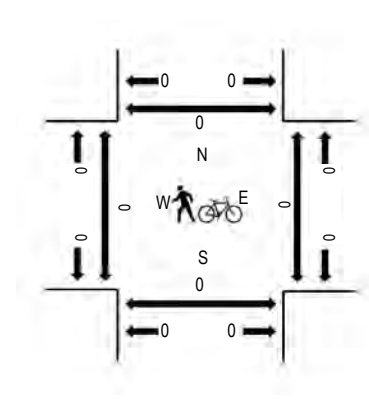
Motorized Vehicles



Heavy Vehicles



Pedestrians/Bicycles in Crosswalk



Note: Total study counts contained in parentheses.

	HV%	PHF
EB	0.0%	0.00
WB	3.7%	0.56
NB	0.5%	0.74
SB	2.1%	0.76
All	1.5%	0.80

Traffic Counts - Motorized Vehicles

Interval Start Time	CRICK AVE SE Eastbound				CRICK AVE SE Westbound				UNIVERSITY BLVD SE Northbound				UNIVERSITY BLVD SE Southbound				Total	Rolling Hour
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
3:00 PM	0	0	0	0	0	1	0	7	0	0	47	1	1	10	29	0	96	396
3:15 PM	0	0	0	0	0	1	0	7	0	0	27	1	1	3	35	0	75	370
3:30 PM	0	0	0	0	0	0	0	27	0	0	67	1	0	4	25	0	124	385
3:45 PM	0	0	0	0	0	1	0	10	0	0	56	0	1	4	29	0	101	364
4:00 PM	0	0	0	0	0	0	0	11	1	0	33	0	0	2	23	0	70	346
4:15 PM	0	0	0	0	0	1	0	11	0	0	42	0	2	6	28	0	90	366
4:30 PM	0	0	0	0	0	5	0	19	0	0	52	0	1	2	24	0	103	359
4:45 PM	0	0	0	0	0	2	0	2	0	0	47	0	0	3	29	0	83	321
5:00 PM	0	0	0	0	0	1	0	16	0	0	47	0	0	2	24	0	90	336
5:15 PM	0	0	0	0	0	0	0	15	0	0	29	0	1	3	35	0	83	
5:30 PM	0	0	0	0	0	0	0	2	0	0	26	0	0	1	36	0	65	
5:45 PM	0	0	0	0	0	1	0	2	0	0	45	0	2	1	47	0	98	
Count Total	0	0	0	0	0	13	0	129	1	0	518	3	9	41	364	0	1,078	
Peak Hour	0	0	0	0	0	3	0	51	0	0	197	3	3	21	118	0	396	

Traffic Counts - Heavy Vehicles and Pedestrians/Bicycles in Crosswalk

Interval Start Time	Heavy Vehicles					Interval Start Time	Pedestrians/Bicycles on Crosswalk				
	EB	NB	WB	SB	Total		EB	NB	WB	SB	Total
3:00 PM	0	0	1	0	1	3:00 PM	0	0	0	0	0
3:15 PM	0	1	0	0	1	3:15 PM	0	0	0	0	0
3:30 PM	0	0	1	1	2	3:30 PM	0	0	0	0	0
3:45 PM	0	0	0	2	2	3:45 PM	0	0	0	0	0
4:00 PM	0	0	0	1	1	4:00 PM	0	0	0	0	0
4:15 PM	0	0	1	1	2	4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0	4:30 PM	0	0	0	0	0
4:45 PM	0	1	0	0	1	4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0	5:00 PM	0	0	0	0	0

5:15 PM	0	1	0	0	1	5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0	5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0	5:45 PM	0	0	0	1	1
Count Total	0	3	3	5	11	Count Total	0	0	0	1	1
Peak Hour	0	1	2	3	6	Peak Hour	0	0	0	0	0

All Traffic Data Services
www.alltrafficdata.net

Date Start: 21-Apr-21
UNIVERSITY BLVD SE S.O. EASTMAN CROSSING
Site Code: 6
Station ID:

NB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Total
04/21/21	0	21	3	0	1	0	0	0	0	0	0	0	0	25
01:00	0	0	1	0	0	0	0	0	0	0	0	0	0	1
02:00	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:00	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:00	0	8	0	0	0	0	0	0	0	0	0	0	0	8
05:00	0	20	3	0	0	0	0	0	0	0	0	0	0	23
06:00	0	62	8	1	0	1	0	0	0	0	0	0	0	72
07:00	0	101	13	2	1	0	0	0	0	0	0	0	0	117
08:00	0	99	17	4	3	0	0	0	1	0	0	0	0	124
09:00	0	75	11	0	7	2	0	1	0	0	0	0	0	96
10:00	0	73	13	1	8	1	0	1	2	0	0	0	0	99
11:00	0	84	14	0	5	0	0	1	0	0	0	0	0	104
12 PM	0	86	16	0	4	0	0	2	1	0	0	0	0	109
13:00	1	87	14	1	5	1	0	1	1	0	0	0	0	111
14:00	0	84	7	1	4	0	0	1	0	0	0	0	0	97
15:00	0	97	16	0	4	0	0	3	1	0	0	0	0	121
16:00	0	99	17	2	0	0	0	0	0	0	0	0	0	118
17:00	0	102	10	1	1	1	0	1	0	0	0	0	0	116
18:00	0	72	7	0	3	0	0	0	0	0	0	0	0	82
19:00	0	46	9	0	1	0	0	1	0	0	0	0	0	57
20:00	0	29	2	0	0	0	0	0	1	0	0	0	0	32
21:00	0	15	3	0	0	0	0	0	0	0	0	0	0	18
22:00	0	11	0	0	0	0	0	0	0	0	0	0	0	11
23:00	0	65	3	0	0	0	0	0	0	0	0	0	0	68
Day Total	1	1338	187	13	47	6	0	12	7	0	0	0	0	1611
Percent	0.1%	83.1%	11.6%	0.8%	2.9%	0.4%	0.0%	0.7%	0.4%	0.0%	0.0%	0.0%	0.0%	
AM Peak		07:00	08:00	08:00	10:00	09:00		09:00	10:00					08:00
Vol.		101	17	4	8	2		1	2					124
PM Peak	13:00	17:00	16:00	16:00	13:00	13:00		15:00	12:00					15:00
Vol.	1	102	17	2	5	1		3	1					121

All Traffic Data Services
www.alltrafficdata.net

Date Start: 21-Apr-21
UNIVERSITY BLVD SE S.O. EASTMAN CROSSING
Site Code: 6
Station ID:

NB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Total
04/22/21	0	23	2	0	0	0	0	0	0	0	0	0	0	25
01:00	0	1	0	0	0	0	0	0	0	0	0	0	0	1
02:00	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:00	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:00	0	8	0	0	0	0	0	0	0	0	0	0	0	8
05:00	0	23	0	0	0	0	0	0	0	0	0	0	0	23
06:00	0	62	8	0	1	0	0	0	0	0	0	0	0	71
07:00	0	102	13	0	5	0	0	0	0	0	0	0	0	120
08:00	0	102	20	0	6	0	0	0	1	0	0	0	0	129
09:00	0	82	6	0	5	1	0	1	0	0	0	0	0	95
10:00	0	74	13	0	6	1	0	3	0	0	0	0	0	97
11:00	0	85	16	0	2	0	0	2	0	0	0	0	0	105
12 PM	2	90	16	0	2	0	0	0	0	0	0	0	0	110
13:00	1	95	11	0	3	1	0	0	0	0	0	0	0	111
14:00	0	84	9	0	3	0	0	0	0	0	0	0	0	96
15:00	0	104	11	0	3	0	0	1	0	0	0	0	0	119
16:00	0	113	3	0	2	0	0	0	0	0	0	0	0	118
17:00	3	98	10	0	2	1	0	1	0	0	0	0	0	115
18:00	0	73	1	0	1	0	0	0	0	0	0	0	0	75
19:00	1	39	4	0	1	0	0	0	0	0	0	0	0	45
20:00	0	30	1	0	0	0	0	0	0	0	0	0	0	31
21:00	0	15	3	0	0	0	0	0	0	0	0	0	0	18
22:00	0	11	0	0	0	0	0	0	0	0	0	0	0	11
23:00	1	67	0	0	0	0	0	0	0	0	0	0	0	68
Day Total	8	1383	147	0	42	4	0	8	1	0	0	0	0	1593
Percent	0.5%	86.8%	9.2%	0.0%	2.6%	0.3%	0.0%	0.5%	0.1%	0.0%	0.0%	0.0%	0.0%	
AM Peak		07:00	08:00		08:00	09:00		10:00	08:00					08:00
Vol.		102	20		6	1		3	1					129
PM Peak	17:00	16:00	12:00		13:00	13:00		15:00						15:00
Vol.	3	113	16		3	1		1						119
Grand Total	9	2721	334	13	89	10	0	20	8	0	0	0	0	3204
Percent	0.3%	84.9%	10.4%	0.4%	2.8%	0.3%	0.0%	0.6%	0.2%	0.0%	0.0%	0.0%	0.0%	

All Traffic Data Services
www.alltrafficdata.net

Date Start: 21-Apr-21
UNIVERSITY BLVD SE S.O. EASTMAN CROSSING
Site Code: 6
Station ID:

SB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Total
04/21/21	0	4	0	0	0	0	0	0	0	0	0	0	0	4
01:00	0	2	1	0	0	0	0	0	0	0	0	0	0	3
02:00	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	0	5	0	0	0	0	0	0	0	0	0	0	0	5
05:00	0	53	13	0	0	0	0	0	0	0	0	0	0	66
06:00	0	45	7	0	5	0	0	0	0	0	0	0	0	57
07:00	0	102	30	1	3	2	0	0	0	0	0	0	0	138
08:00	0	105	21	1	6	1	0	2	0	0	0	0	0	136
09:00	0	77	6	1	8	0	0	1	1	0	0	0	0	94
10:00	0	70	14	0	5	1	0	2	0	0	0	0	0	92
11:00	0	119	18	0	8	0	0	3	0	0	0	0	0	148
12 PM	0	115	19	0	8	0	0	1	0	0	0	0	0	143
13:00	0	75	11	1	4	0	0	3	0	0	0	0	0	94
14:00	1	73	9	3	7	0	0	1	1	0	0	0	0	95
15:00	0	84	10	4	3	0	0	0	0	0	0	0	0	101
16:00	0	87	10	1	6	2	0	0	0	0	0	0	0	106
17:00	0	84	9	3	3	0	0	0	0	0	0	0	0	99
18:00	0	71	10	1	3	0	0	0	0	0	0	0	0	85
19:00	0	45	7	0	1	0	0	0	0	0	0	0	0	53
20:00	0	33	5	0	1	0	0	0	0	0	0	0	0	39
21:00	0	21	4	0	1	0	0	0	0	0	0	0	0	26
22:00	0	9	2	0	0	0	0	0	0	0	0	0	0	11
23:00	0	4	0	0	0	0	0	0	0	0	0	0	0	4
Day Total	1	1284	206	16	72	6	0	13	2	0	0	0	0	1600
Percent	0.1%	80.3%	12.9%	1.0%	4.5%	0.4%	0.0%	0.8%	0.1%	0.0%	0.0%	0.0%	0.0%	
AM Peak		11:00	07:00	07:00	09:00	07:00		11:00	09:00					11:00
Vol.		119	30	1	8	2		3	1					148
PM Peak	14:00	12:00	12:00	15:00	12:00	16:00		13:00	14:00					12:00
Vol.	1	115	19	4	8	2		3	1					143

All Traffic Data Services
www.alltrafficdata.net

Date Start: 21-Apr-21
UNIVERSITY BLVD SE S.O. EASTMAN CROSSING
Site Code: 6
Station ID:

SB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Total
04/22/21	0	4	0	0	0	0	0	0	0	0	0	0	0	4
01:00	0	3	0	0	0	0	0	0	0	0	0	0	0	3
02:00	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	0	5	0	0	0	0	0	0	0	0	0	0	0	5
05:00	0	56	11	0	0	0	0	0	0	0	0	0	0	67
06:00	0	49	7	0	1	0	0	0	0	0	0	0	0	57
07:00	0	113	21	1	2	2	0	0	0	0	0	0	0	139
08:00	0	108	25	1	0	2	0	0	2	0	0	0	0	138
09:00	0	76	8	1	7	1	0	1	1	0	0	0	0	95
10:00	0	70	19	0	1	0	1	1	1	0	0	0	0	93
11:00	0	122	18	0	5	1	0	2	1	0	0	0	0	149
12 PM	0	115	24	0	3	0	0	1	0	0	0	0	0	143
13:00	0	81	9	1	2	1	0	0	1	0	0	0	0	95
14:00	1	76	11	1	5	1	0	0	1	0	0	0	0	96
15:00	0	86	9	1	4	1	0	0	0	0	0	0	0	101
16:00	0	90	9	0	5	2	0	0	0	0	0	0	0	106
17:00	0	82	13	1	4	0	0	1	0	0	0	0	0	101
18:00	0	71	11	1	2	0	0	0	0	0	0	0	0	85
19:00	0	42	10	0	1	0	0	0	0	0	0	0	0	53
20:00	0	34	4	0	1	0	0	0	0	0	0	0	0	39
21:00	0	23	3	0	0	0	0	0	0	0	0	0	0	26
22:00	0	11	0	0	0	0	0	0	0	0	0	0	0	11
23:00	0	4	0	0	0	0	0	0	0	0	0	0	0	4
Day Total	1	1322	212	8	43	11	1	6	7	0	0	0	0	1611
Percent	0.1%	82.1%	13.2%	0.5%	2.7%	0.7%	0.1%	0.4%	0.4%	0.0%	0.0%	0.0%	0.0%	
AM Peak		11:00	08:00	07:00	09:00	07:00	10:00	11:00	08:00					11:00
Vol.		122	25	1	7	2	1	2	2					149
PM Peak	14:00	12:00	12:00	13:00	14:00	16:00		12:00	13:00					12:00
Vol.	1	115	24	1	5	2		1	1					143
Grand Total	2	2606	418	24	115	17	1	19	9	0	0	0	0	3211
Percent	0.1%	81.2%	13.0%	0.7%	3.6%	0.5%	0.0%	0.6%	0.3%	0.0%	0.0%	0.0%	0.0%	

All Traffic Data Services
www.alltrafficdata.net

Date Start: 21-Apr-21
STRYKER RD SE W.O. SB UNIVERSITY BLVD SE
Site Code: 7
Station ID: 7

Start Time	21-Apr-21 Wed	EB	WB							Total
12:00 AM		0	1							1
01:00		0	0							0
02:00		0	1							1
03:00		0	0							0
04:00		0	2							2
05:00		2	2							4
06:00		12	12							24
07:00		15	37							52
08:00		19	27							46
09:00		18	11							29
10:00		16	19							35
11:00		26	14							40
12:00 PM		15	19							34
01:00		18	14							32
02:00		19	15							34
03:00		17	13							30
04:00		20	17							37
05:00		16	12							28
06:00		7	13							20
07:00		8	8							16
08:00		1	4							5
09:00		0	6							6
10:00		3	1							4
11:00		0	1							1
Total		232	249							481
Percent		48.2%	51.8%							
AM Peak	-	11:00	07:00	-	-	-	-	-	-	07:00
Vol.	-	26	37	-	-	-	-	-	-	52
PM Peak	-	16:00	12:00	-	-	-	-	-	-	16:00
Vol.	-	20	19	-	-	-	-	-	-	37

All Traffic Data Services
www.alltrafficdata.net

Date Start: 21-Apr-21
STRYKER RD SE W.O. SB UNIVERSITY BLVD SE
Site Code: 7
Station ID: 7

Start Time	22-Apr-21	EB	WB								Total
12:00 AM		1	0								1
01:00		1	1								2
02:00		0	0								0
03:00		0	1								1
04:00		0	3								3
05:00		2	2								4
06:00		10	14								24
07:00		16	32								48
08:00		16	28								44
09:00		19	14								33
10:00		18	21								39
11:00		22	14								36
12:00 PM		18	14								32
01:00		19	17								36
02:00		16	15								31
03:00		19	15								34
04:00		19	17								36
05:00		14	14								28
06:00		8	11								19
07:00		9	10								19
08:00		4	4								8
09:00		1	3								4
10:00		1	1								2
11:00		1	0								1
Total		234	251								485
Percent		48.2%	51.8%								
AM Peak	-	11:00	07:00	-	-	-	-	-	-	07:00	
Vol.	-	22	32	-	-	-	-	-	-	48	
PM Peak	-	13:00	13:00	-	-	-	-	-	-	13:00	
Vol.	-	19	17	-	-	-	-	-	-	36	
Grand Total		466	500								966
Percent		48.2%	51.8%								
ADT		ADT 483	AADT 483								

Albuquerque Studios Expansion Traffic Data Calibration Summary												
	AM Peak Hour							PM Peak Hour				
	2019	2020	Adjust	2021	2021			2019	2020	Adjust	2021	2021
	Pre-COVID	COVID	Factor	Actual	Calibrated			Pre-COVID	COVID	Factor	Actual	Calibrated
Intersection	April	April		April	April			April	April		April	April
9 Hour TMC												
<u>Bobby Foster & Los Picaros</u>				7:45 AM							3:30 PM	
NB Left	33	23	1.43	24	34			171	53	1.52	30	46
NB Thru	0	0		3	3			0	0		1	1
NB Right	0	0		0	0			0	0		0	0
SB Left	0	0		0	0			0	0		0	0
SB Thru	0	0		1	1			0	23	0.00	3	3
SB Right	6	15	0.48	18	9			61	45	1.36	40	54
WB Left	0	0		0	0			0	0		0	0
WB Thru	0	0		0	0			0	0		0	0
WB Right	0	0		0	0			0	0		0	0
EB Left	60	0		33	33			48	0		21	21
EB Thru	0	0		0	0			0	0		0	0
EB Right	183	36	1.52	34	52			35	0		3	3
				<u>113</u>	<u>97</u>						<u>98</u>	<u>82</u>
9 Hour TMC												
<u>Bobby Foster & Broadway</u>				6:30 AM							3:30 PM	
NB Left	0	0		2	2			0	0		0	0
NB Thru	757	515	1.47	371	545			330	345	0.96	334	319
NB Right	158	39	1.52	16	24			27	27	1.00	17	17
SB Left	94	87	1.08	36	39			45	29	1.52	41	62
SB Thru	331	333	0.99	249	248			732	589	1.24	467	580
SB Right	0	0		5	5			0	0		1	1
WB Left	0	4	0.00	9	9			86	20	1.52	44	67
WB Thru	0	0		0	0			0	0		0	0
WB Right	0	8	0.00	7	7			127	73	1.52	46	70
EB Left	0	0		3	3			0	0		1	1
EB Thru	0	0		0	0			0	0		0	0
EB Right	0	0		0	0			0	0		0	0
				<u>698</u>	<u>882</u>						<u>951</u>	<u>1118</u>
9 Hour TMC												
<u>University and Rio Bravo</u>				7:45 AM							3:00 PM	
EB Left	444	100	1.42	238	338			129	108	1.19	180	215
EB Thru	0	0		0	0			0	0		0	0
EB Right	337	141	1.42	206	293			110	72	1.42	92	131
WB Left	0	0		0	0			0	0		0	0
WB Thru	0	0		0	0			0	0		0	0
WB Right	0	0		0	0			0	0		0	0
NB Left	188	80	1.42	148	210			585	66	1.42	186	264
NB Thru	105	63	1.67	35	58			91	51	1.42	35	50
NB Right	0	0		0	0			0	0		0	0
SB Left	0	0		0	0			0	0		0	0
SB Thru	278	40	1.42	38	54			104	51	1.42	35	50
SB Right	55	9	1.42	84	119			337	93	1.42	188	267
				<u>749</u>	<u>1072</u>						<u>716</u>	<u>976</u>
9 Hour TMC												
<u>Rio Bravo and Broadway</u>				6:45 AM							3:00 PM	
NB Left	54	45	1.20	91	109			108	96	1.13	197	222
NB Thru	449	217	1.39	138	192			151	166	0.91	177	161
NB Right	382	238	1.39	228	317			413	271	1.39	338	470
EB Left	455	182	1.39	160	222			196	88	1.39	118	164
EB Thru	1425	866	1.39	1185	1647			556	457	1.22	674	820
EB Right	46	120	0.61	110	67			58	77	0.75	126	95
SB Left	20	30	0.67	54	36			59	20	1.39	99	138
SB Thru	201	153	1.31	101	133			311	298	1.04	282	294
SB Right	79	79	1.00	63	63			400	288	1.39	258	358
WB Left	260	254	1.02	287	294			402	284	1.39	286	398
WB Thru	430	313	1.37	476	654			955	650	1.39	995	1383
WB Right	39	38	1.03	34	35			31	47	0.66	25	16
				<u>2927</u>	<u>3769</u>						<u>3575</u>	<u>4519</u>
9 Hour TMC												
<u>2nd Street and Rio Bravo</u>				7:15 AM							4:00 PM	
NB Left	112	104	1.08	147	158			347	161	1.47	365	537
NB Thru	418	62	1.47	67	98			411	127	1.47	79	116
NB Right	72	19	1.47	47	69			758	68	1.47	63	93
EB Left	584	115	1.47	189	278			98	92	1.07	97	103
EB Thru	1483	988	1.47	1327	1951			541	436	1.24	780	968
EB Right	203	132	1.47	157	231			131	90	1.46	148	215
SB Left	138	23	1.47	70	103			105	74	1.42	114	162
SB Thru	24	65	0.53	43	23			184	64	1.47	87	128
SB Right	90	4	1.47	52	76			540	200	1.47	270	397
WB Left	62	60	1.03	76	79			32	66	0.53	62	33
WB Thru	420	324	1.30	486	630			1672	1141	1.47	1340	1964
WB Right	91	91	1.00	54	54			124	34	1.47	40	59
				<u>2715</u>	<u>3750</u>						<u>3445</u>	<u>4774</u>

Huitt-Zollars, Inc.

333 Rio Rancho Drive NW, Suite 101

Rio Rancho, NM 87124

ADVANCEDESIGN

Weather: Overcast
Serial Number: 3082
Collected By: DPitts
Other:

File Name : Bobby Foster - Los Picaros 04282021 DP
Site Code : 00000000
Start Date : 4/28/2021
Page No : 1

Groups Printed- Passenger Vehicles - Trucks

Start Time	Los Picaros From North			Los Picaros From South			Bobby Foster From West			Int. Total
	Right	Thru	Peds	Thru	Left	Peds	Right	Left	Peds	
06:30 AM	0	0	0	0	3	0	5	5	0	13
06:45 AM	4	0	0	0	8	0	6	9	0	27
Total	4	0	0	0	11	0	11	14	0	40
07:00 AM	2	0	0	0	5	0	3	14	0	24
07:15 AM	5	0	0	0	1	0	4	5	0	15
07:30 AM	1	0	0	0	1	0	4	9	0	15
07:45 AM	3	0	0	0	2	0	10	7	0	22
Total	11	0	0	0	9	0	21	35	0	76
08:00 AM	3	0	0	1	3	0	11	7	0	25
08:15 AM	3	1	0	2	10	0	8	11	0	35
08:30 AM	9	0	0	0	9	0	5	8	0	31
08:45 AM	6	0	0	0	5	0	6	3	0	20
Total	21	1	0	3	27	0	30	29	0	111
09:00 AM	3	0	0	0	1	0	4	3	0	11
09:15 AM	4	0	0	0	0	0	4	8	0	16
09:30 AM	0	0	0	0	1	0	0	1	0	2
*** BREAK ***										
Total	7	0	0	0	2	0	8	12	0	29
*** BREAK ***										
11:00 AM	6	1	0	0	3	0	4	2	0	16
11:15 AM	5	0	0	0	5	0	2	4	0	16
11:30 AM	13	0	0	0	2	0	2	7	0	24
11:45 AM	8	0	0	0	2	0	5	8	0	23
Total	32	1	0	0	12	0	13	21	0	79
12:00 PM	13	0	0	0	1	0	3	7	0	24
12:15 PM	9	0	0	2	2	0	8	7	0	28
12:30 PM	8	0	0	0	1	0	3	5	0	17
12:45 PM	7	0	0	1	2	0	0	8	0	18
Total	37	0	0	3	6	0	14	27	0	87
01:00 PM	3	0	0	0	1	0	3	14	0	21
01:15 PM	6	0	0	0	1	0	2	9	0	18
01:30 PM	8	0	0	0	1	0	2	6	0	17
01:45 PM	5	0	0	1	2	0	3	11	0	22
Total	22	0	0	1	5	0	10	40	0	78
02:00 PM	4	2	0	1	5	0	1	9	0	22
02:15 PM	2	0	0	0	2	0	0	4	0	8
*** BREAK ***										
Total	6	2	0	1	7	0	1	13	0	30
03:00 PM	6	0	0	0	3	0	4	10	0	23
03:15 PM	6	0	0	0	6	0	7	4	0	23

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File Name : Bobby Foster - Los Picaros 04282021 DP

Site Code : 00000000

Start Date : 4/28/2021

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Groups Printed- Passenger Vehicles - Trucks

	Los Picaros From North			Los Picaros From South			Bobby Foster From West			
Start Time	Right	Thru	Peds	Thru	Left	Peds	Right	Left	Peds	Int. Total
03:30 PM	11	3	0	0	20	0	1	5	0	40
03:45 PM	4	0	0	0	2	0	0	5	0	11
Total	27	3	0	0	31	0	12	24	0	97
04:00 PM	13	0	0	1	3	0	1	5	0	23
04:15 PM	12	0	0	0	5	0	1	6	0	24
04:30 PM	8	0	0	0	5	0	1	6	0	20
04:45 PM	8	0	0	0	4	0	1	3	0	16
Total	41	0	0	1	17	0	4	20	0	83
05:00 PM	11	0	0	0	6	0	2	2	0	21
05:15 PM	3	0	0	0	2	0	4	2	0	11
05:30 PM	9	0	0	1	0	0	1	5	0	16
05:45 PM	3	0	0	0	2	0	6	3	0	14
Total	26	0	0	1	10	0	13	12	0	62
Grand Total	234	7	0	10	137	0	137	247	0	772
Apprch %	97.1	2.9	0	6.8	93.2	0	35.7	64.3	0	
Total %	30.3	0.9	0	1.3	17.7	0	17.7	32	0	
Passenger Vehicles	173	6	0	8	127	0	131	191	0	636
% Passenger Vehicles	73.9	85.7	0	80	92.7	0	95.6	77.3	0	82.4
Trucks	61	1	0	2	10	0	6	56	0	136
% Trucks	26.1	14.3	0	20	7.3	0	4.4	22.7	0	17.6

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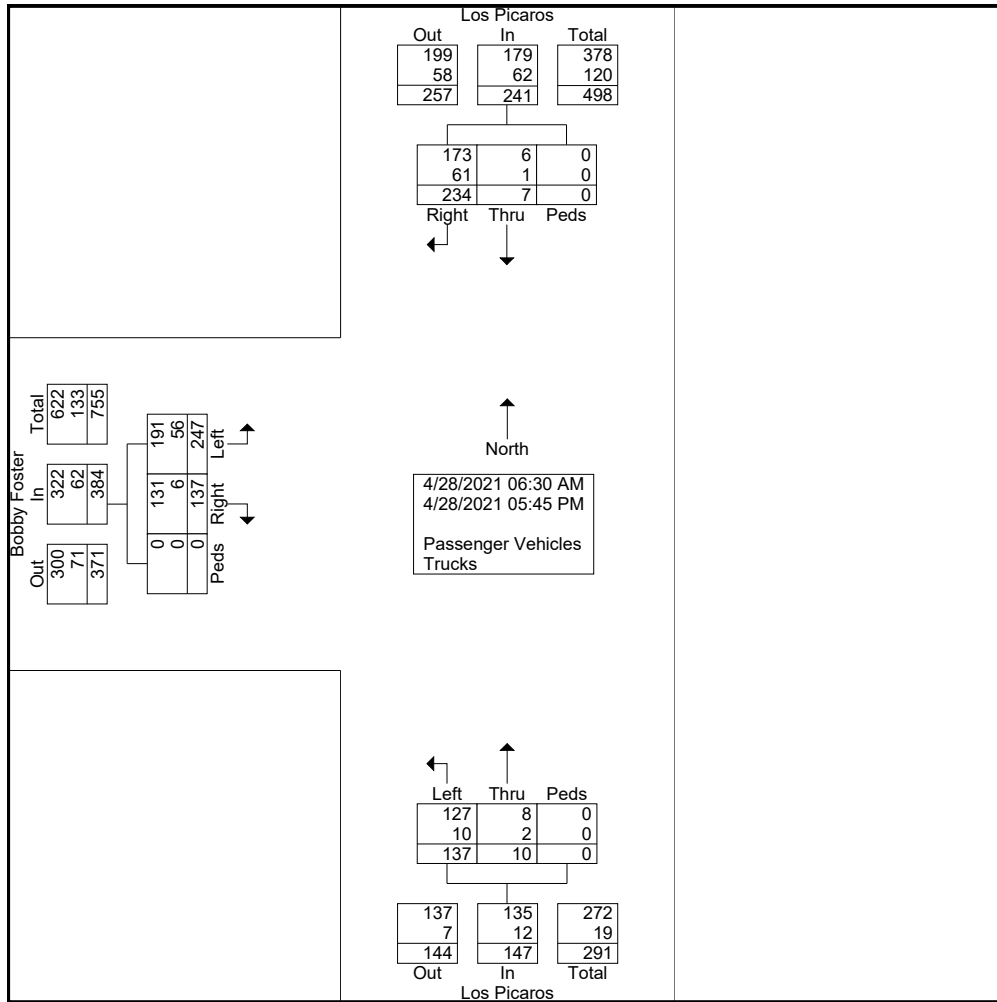
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Rio Rancho, NM 87124
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Site Code : 00000000

Start Date : 4/28/2021

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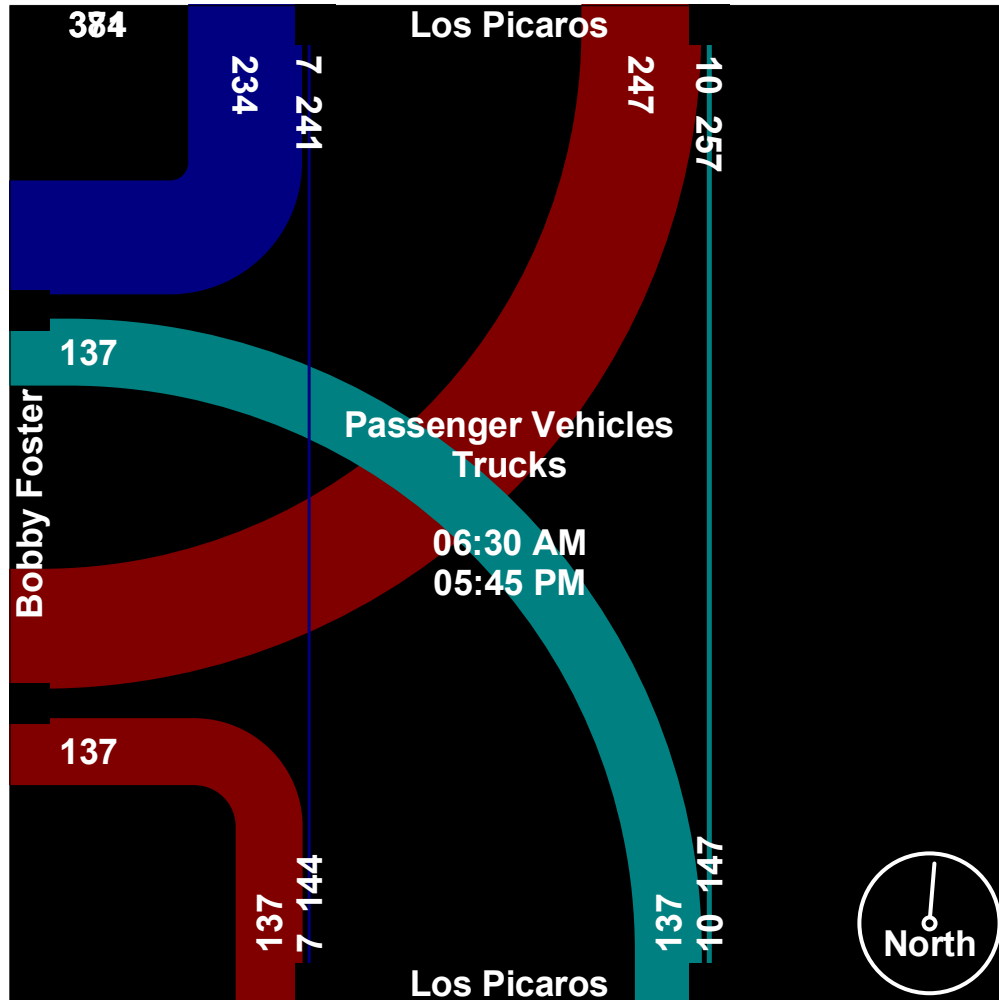
333 Rio Rancho Drive NW, Suite 101
Rio Rancho, NM 87124
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File Name : Bobby Foster - Los Picaros 04282021 DP

Site Code : 00000000

Start Date : 4/28/2021

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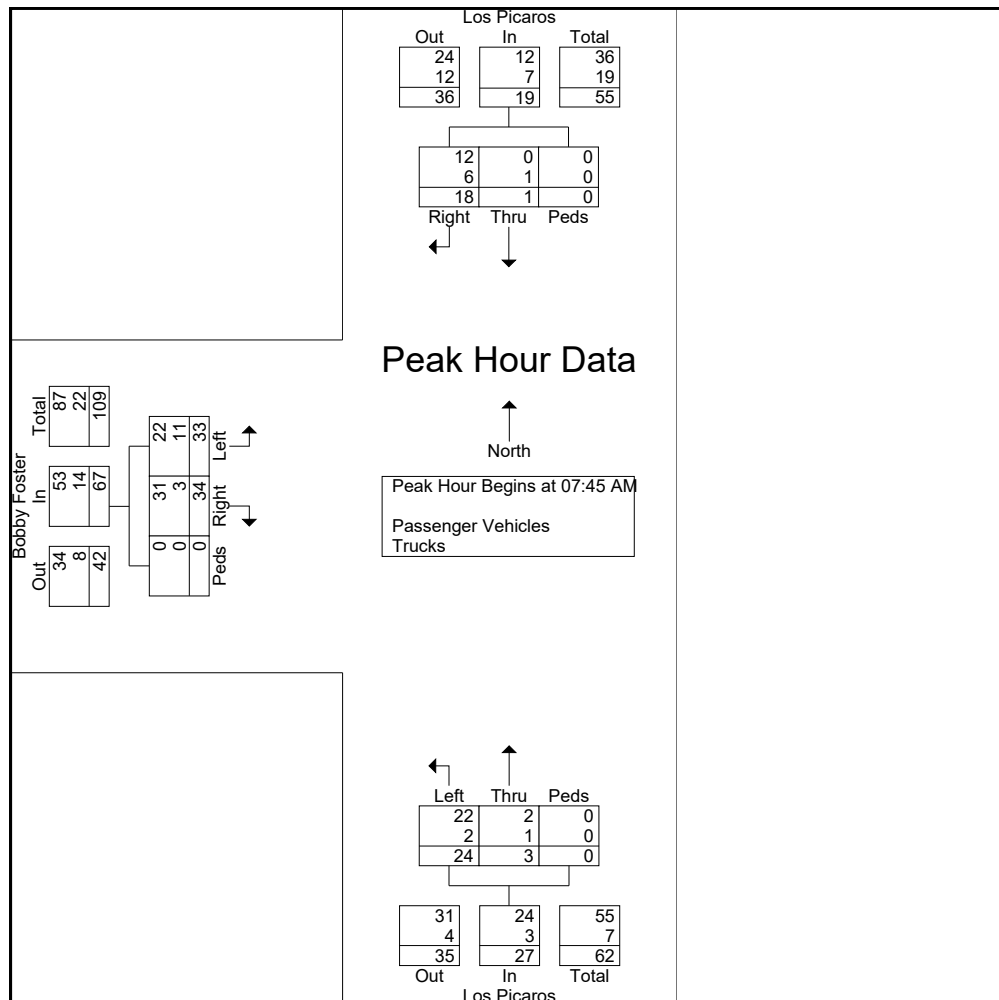


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File Name : Bobby Foster - Los Picaros 04282021 DP
Site Code : 00000000
Start Date : 4/28/2021
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	Los Picaros From North				Los Picaros From South				Bobby Foster From West				
Start Time	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 06:30 AM to 09:45 AM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 07:45 AM													
07:45 AM	3	0	0	3	0	2	0	2	10	7	0	17	22
08:00 AM	3	0	0	3	1	3	0	4	11	7	0	18	25
08:15 AM	3	1	0	4	2	10	0	12	8	11	0	19	35
08:30 AM	9	0	0	9	0	9	0	9	5	8	0	13	31
Total Volume	18	1	0	19	3	24	0	27	34	33	0	67	113
% App. Total	94.7	5.3	0		11.1	88.9	0		50.7	49.3	0		
PHF	.500	.250	.000	.528	.375	.600	.000	.563	.773	.750	.000	.882	.807
Passenger Vehicles	12	0	0	12	2	22	0	24	31	22	0	53	89
% Passenger Vehicles	66.7	0	0	63.2	66.7	91.7	0	88.9	91.2	66.7	0	79.1	78.8
Trucks	6	1	0	7	1	2	0	3	3	11	0	14	24
% Trucks	33.3	100	0	36.8	33.3	8.3	0	11.1	8.8	33.3	0	20.9	21.2



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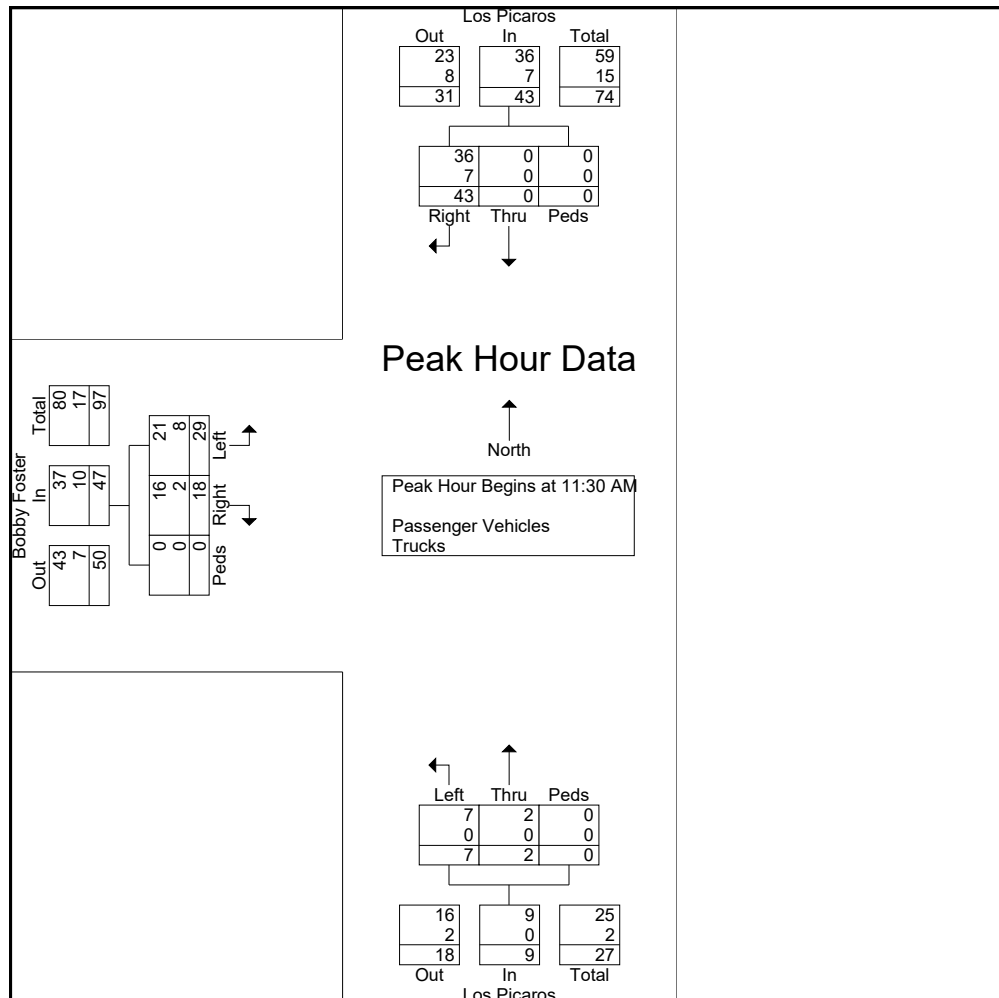
File Name : Bobby Foster - Los Picaros 04282021 DP

Site Code : 00000000

Start Date : 4/28/2021

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	Los Picaros From North				Los Picaros From South				Bobby Foster From West				
Start Time	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 10:00 AM to 01:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 11:30 AM													
11:30 AM	13	0	0	13	0	2	0	2	2	7	0	9	24
11:45 AM	8	0	0	8	0	2	0	2	5	8	0	13	23
12:00 PM	13	0	0	13	0	1	0	1	3	7	0	10	24
12:15 PM	9	0	0	9	2	2	0	4	8	7	0	15	28
Total Volume	43	0	0	43	2	7	0	9	18	29	0	47	99
% App. Total	100	0	0		22.2	77.8	0		38.3	61.7	0		
PHF	.827	.000	.000	.827	.250	.875	.000	.563	.563	.906	.000	.783	.884
Passenger Vehicles	36	0	0	36	2	7	0	9	16	21	0	37	82
% Passenger Vehicles	83.7	0	0	83.7	100	100	0	100	88.9	72.4	0	78.7	82.8
Trucks	7	0	0	7	0	0	0	0	2	8	0	10	17
% Trucks	16.3	0	0	16.3	0	0	0	0	11.1	27.6	0	21.3	17.2



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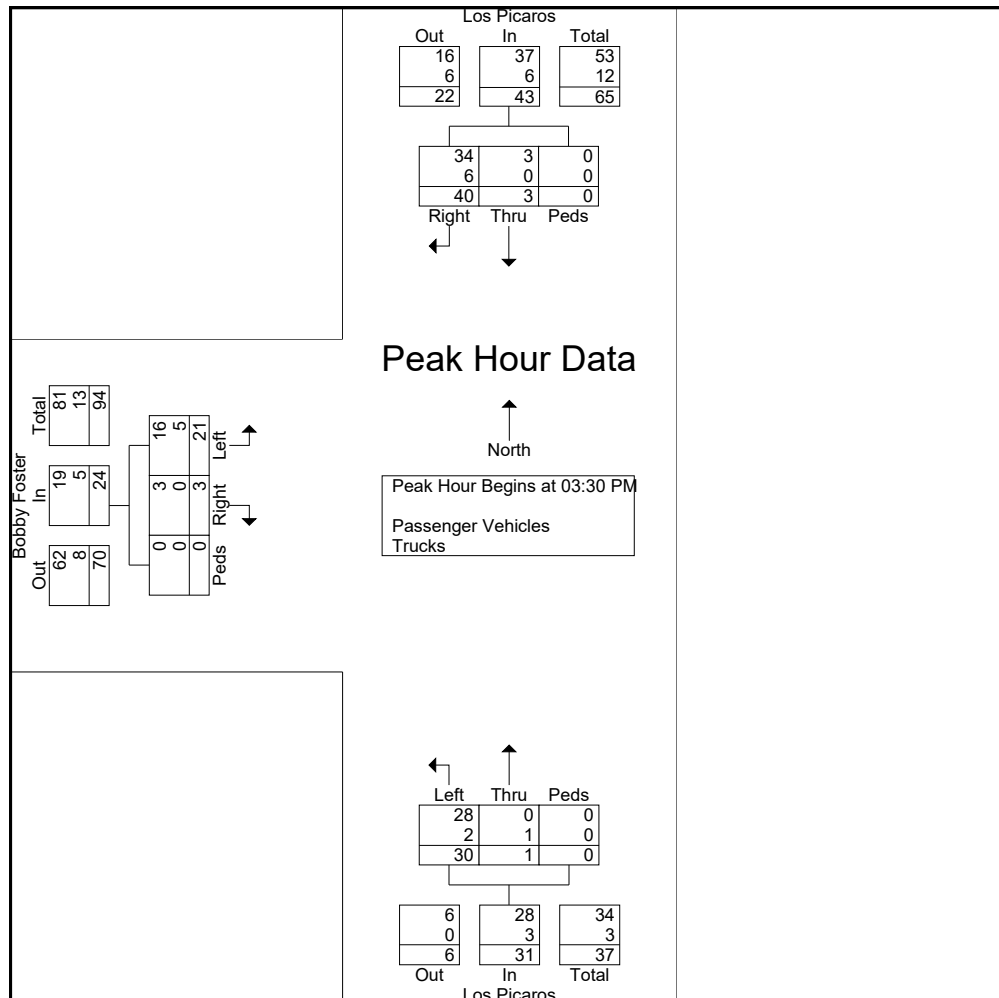
File Name : Bobby Foster - Los Picaros 04282021 DP

Site Code : 00000000

Start Date : 4/28/2021

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	Los Picaros From North				Los Picaros From South				Bobby Foster From West				
Start Time	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 02:00 PM to 06:00 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 03:30 PM													
03:30 PM	11	3	0	14	0	20	0	20	1	5	0	6	40
03:45 PM	4	0	0	4	0	2	0	2	0	5	0	5	11
04:00 PM	13	0	0	13	1	3	0	4	1	5	0	6	23
04:15 PM	12	0	0	12	0	5	0	5	1	6	0	7	24
Total Volume	40	3	0	43	1	30	0	31	3	21	0	24	98
% App. Total	93	7	0		3.2	96.8	0		12.5	87.5	0		
PHF	.769	.250	.000	.768	.250	.375	.000	.388	.750	.875	.000	.857	.613
Passenger Vehicles	34	3	0	37	0	28	0	28	3	16	0	19	84
% Passenger Vehicles	85.0	100	0	86.0	0	93.3	0	90.3	100	76.2	0	79.2	85.7
Trucks	6	0	0	6	1	2	0	3	0	5	0	5	14
% Trucks	15.0	0	0	14.0	100	6.7	0	9.7	0	23.8	0	20.8	14.3



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333 Rio Rancho Drive NW, Suite 101
Rio Rancho, NM 87124

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File Name : Bobby Foster - Los Picaros 04282021 DP

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333 Rio Rancho Drive NW, Suite 101
Rio Rancho, NM 87124
ADVANCEDESIGN

Weather: Overcast
Serial Number: 3082
Collected By: DPitts
Other:

File Name : Broadway - Bobby Foster 04282021 DP
Site Code : 00000000
Start Date : 4/29/2021
Page No : 1

Groups Printed- Passenger Vehicles - Trucks

	Broadway From North				Bobby Foster From East				Broadway From South				Driveway From West				Int. Total
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	
06:30 AM	5	72	8	0	1	0	3	0	2	97	0	0	0	0	2	0	190
06:45 AM	0	70	7	0	1	0	1	0	4	117	2	0	0	0	1	0	203
Total	5	142	15	0	2	0	4	0	6	214	2	0	0	0	3	0	393
07:00 AM	0	51	14	0	3	0	3	0	5	67	0	0	0	0	0	0	143
07:15 AM	0	56	7	0	2	0	2	0	5	90	0	0	0	0	0	0	162
07:30 AM	1	61	7	0	3	0	0	0	6	110	0	0	0	0	0	0	188
07:45 AM	0	62	5	0	1	0	3	0	8	75	0	0	0	0	0	0	154
Total	1	230	33	0	9	0	8	0	24	342	0	0	0	0	0	0	647
08:00 AM	0	55	16	0	5	0	4	0	8	70	0	0	0	0	2	0	160
08:15 AM	0	59	16	0	5	0	7	0	9	80	0	0	0	0	0	0	176
08:30 AM	0	47	3	0	11	0	6	0	3	71	0	0	0	0	0	0	141
08:45 AM	0	58	8	0	3	0	3	0	3	67	0	0	0	0	0	0	142
Total	0	219	43	0	24	0	20	0	23	288	0	0	0	0	2	0	619
09:00 AM	0	69	5	0	3	0	5	0	1	62	0	0	0	0	0	0	145
09:15 AM	0	63	7	0	5	0	0	0	7	57	0	0	0	0	0	0	139
09:30 AM	0	18	9	0	2	0	1	0	0	26	0	0	0	0	0	0	56
*** BREAK ***																	
Total	0	150	21	0	10	0	6	0	8	145	0	0	0	0	0	0	340
*** BREAK ***																	
11:00 AM	0	66	9	0	6	1	2	1	4	53	1	0	0	0	0	0	143
11:15 AM	0	61	6	0	3	0	6	0	3	41	0	0	0	0	0	0	120
11:30 AM	0	72	9	0	10	0	2	0	4	81	0	0	0	0	0	0	178
11:45 AM	1	79	10	0	13	0	3	0	5	78	0	0	0	0	1	0	190
Total	1	278	34	0	32	1	13	1	16	253	1	0	0	0	1	0	631
12:00 PM	0	90	13	0	10	0	0	0	3	71	0	0	0	0	0	0	187
12:15 PM	0	92	11	0	3	0	2	0	3	64	0	0	0	0	1	0	176
12:30 PM	0	84	15	0	7	0	5	0	6	70	0	0	0	0	0	0	187
12:45 PM	0	75	9	0	10	0	9	0	0	68	0	0	0	1	0	0	172
Total	0	341	48	0	30	0	16	0	12	273	0	0	0	0	1	1	722
01:00 PM	0	91	15	0	9	0	4	0	5	79	0	0	0	0	0	0	203
01:15 PM	0	82	27	0	13	0	2	0	3	67	0	0	0	0	1	0	195
01:30 PM	0	77	9	0	11	0	5	0	2	53	0	0	0	0	1	0	158
01:45 PM	1	70	13	0	11	0	6	0	4	69	0	0	0	0	0	0	174
Total	1	320	64	0	44	0	17	0	14	268	0	0	0	0	2	0	730
02:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
*** BREAK ***																	
02:45 PM	0	17	2	0	0	0	2	0	2	11	0	0	0	0	0	0	34
Total	0	17	2	0	0	0	2	0	2	12	0	0	0	0	0	0	35
03:00 PM	2	55	9	0	14	0	2	1	3	54	0	0	0	0	0	0	140
03:15 PM	1	61	5	0	3	0	3	0	4	77	0	0	0	0	0	0	154

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ADVANCEDESIGN

File Name : Broadway - Bobby Foster 04282021 DP

Site Code : 00000000

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Groups Printed- Passenger Vehicles - Trucks

	Broadway From North				Bobby Foster From East				Broadway From South				Driveway From West				Int. Total
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	
03:30 PM	0	120	15	0	10	0	15	1	7	82	0	1	0	0	0	0	251
03:45 PM	0	105	13	0	10	0	7	0	2	99	0	0	0	0	0	0	236
Total	3	341	42	0	37	0	27	2	16	312	0	1	0	0	0	0	781
04:00 PM	1	120	8	0	19	0	8	0	5	77	0	0	0	0	0	0	238
04:15 PM	0	122	5	0	7	0	14	0	3	76	0	0	0	0	1	0	228
04:30 PM	0	119	6	0	18	0	18	1	3	77	1	0	0	0	0	0	243
04:45 PM	0	106	5	0	8	0	7	0	2	83	0	0	0	0	0	0	211
Total	1	467	24	0	52	0	47	1	13	313	1	0	0	0	1	0	920
05:00 PM	0	117	4	0	11	0	9	0	2	83	0	0	0	0	0	0	226
05:15 PM	0	111	5	0	10	0	1	0	4	50	0	0	0	0	0	0	181
05:30 PM	0	72	5	0	19	0	3	0	2	48	0	0	0	0	0	0	149
05:45 PM	0	83	3	0	5	0	3	0	2	39	0	0	0	0	0	0	135
Total	0	383	17	0	45	0	16	0	10	220	0	0	0	0	0	0	691
Grand Total	12	2888	343	0	285	1	176	4	144	2640	4	1	0	1	10	0	6509
Apprch %	0.4	89.1	10.6	0	61.2	0.2	37.8	0.9	5.2	94.7	0.1	0	0	9.1	90.9	0	
Total %	0.2	44.4	5.3	0	4.4	0	2.7	0.1	2.2	40.6	0.1	0	0	0	0.2	0	
Passenger Vehicles	11	2288	298	0	258	1	155	2	126	2126	3	1	0	1	9	0	5279
% Passenger Vehicles	91.7	79.2	86.9	0	90.5	100	88.1	50	87.5	80.5	75	100	0	100	90	0	81.1
Trucks	1	600	45	0	27	0	21	2	18	514	1	0	0	0	1	0	1230
% Trucks	8.3	20.8	13.1	0	9.5	0	11.9	50	12.5	19.5	25	0	0	0	10	0	18.9

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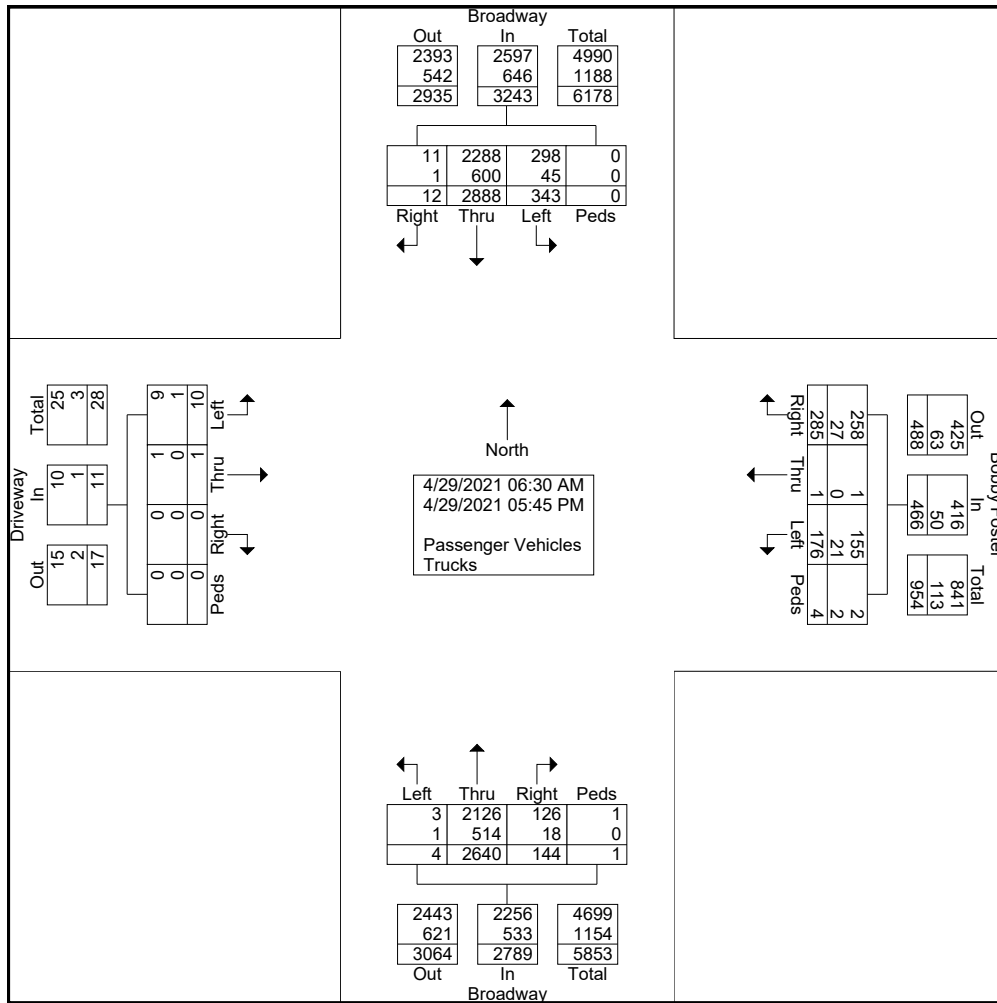
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File Name : Broadway - Bobby Foster 04282021 DP

Site Code : 00000000

Start Date : 4/29/2021

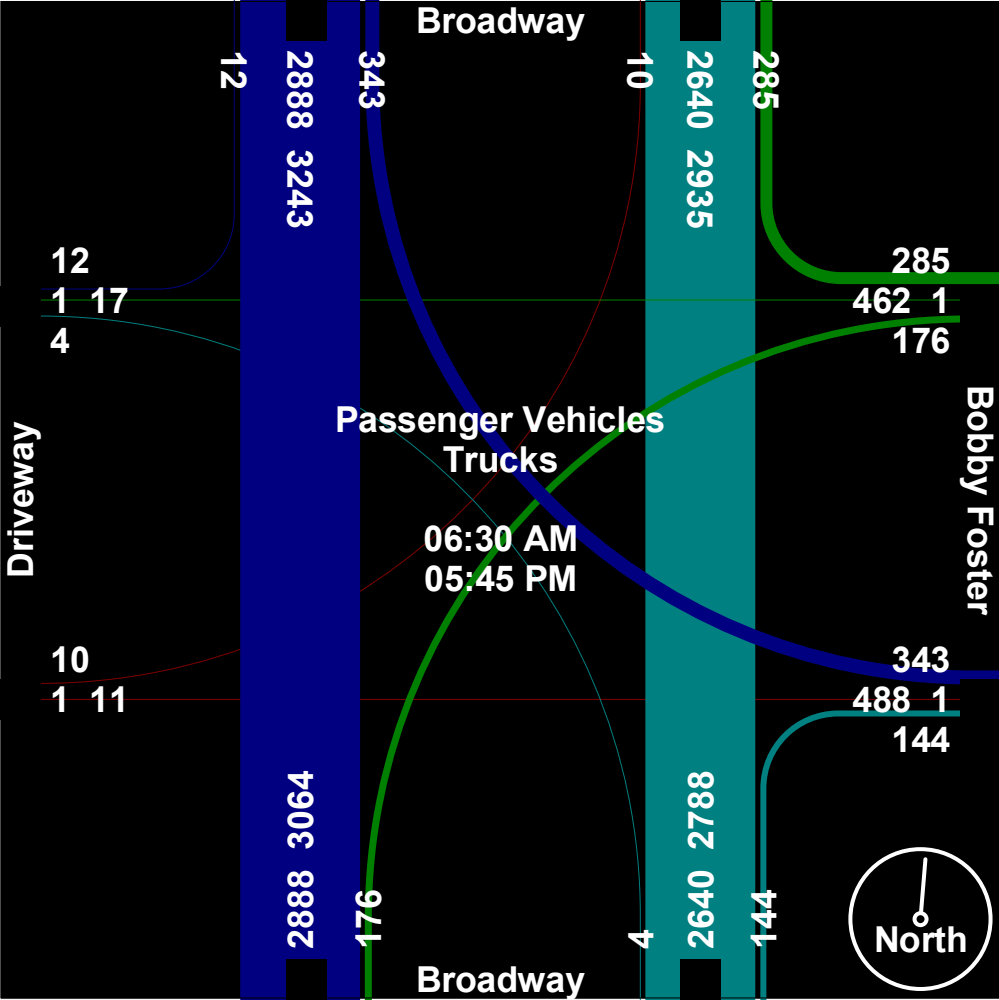
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Rio Rancho, NM 87124
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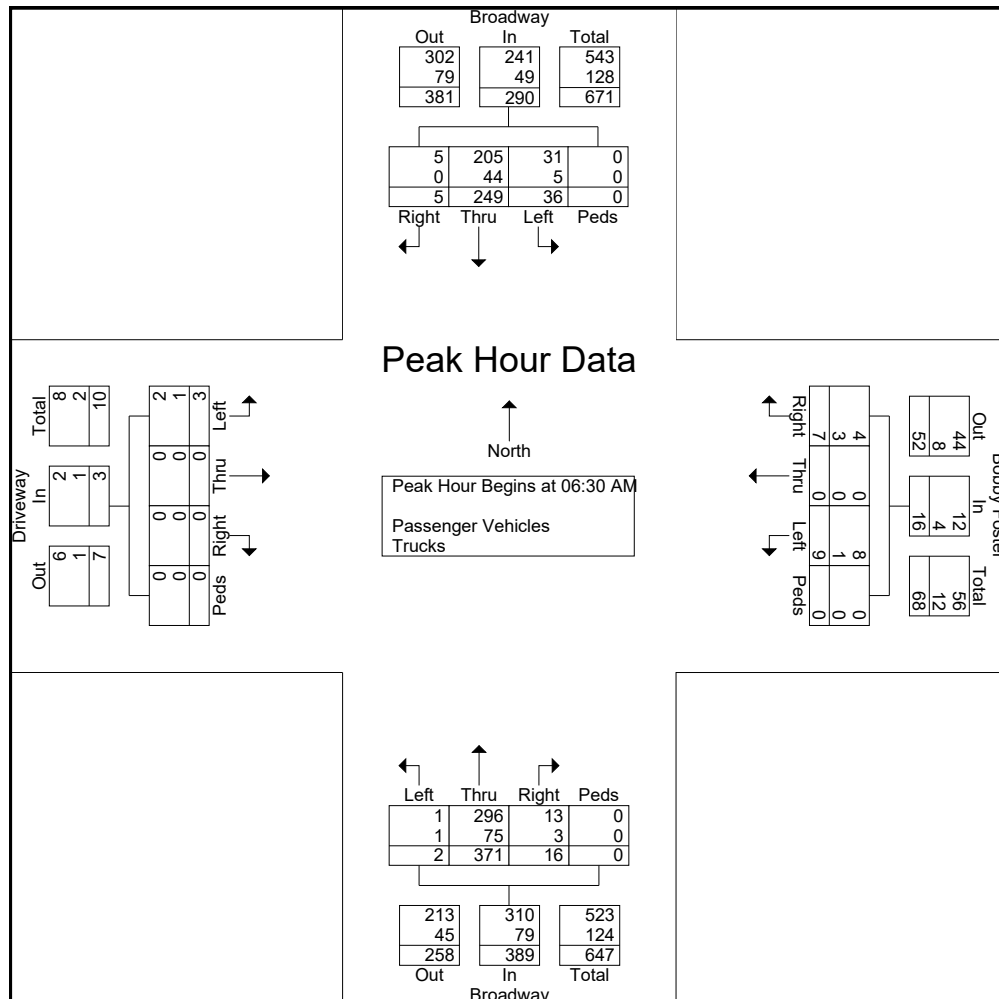
File Name : Broadway - Bobby Foster 04282021 DP

Site Code : 00000000

Start Date : 4/29/2021

Page No : 5

	Broadway From North					Bobby Foster From East					Broadway From South					Driveway From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 06:30 AM to 09:45 AM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 06:30 AM																					
06:30 AM	5	72	8	0	85	1	0	3	0	4	2	97	0	0	99	0	0	2	0	2	190
06:45 AM	0	70	7	0	77	1	0	1	0	2	4	117	2	0	123	0	0	1	0	1	203
07:00 AM	0	51	14	0	65	3	0	3	0	6	5	67	0	0	72	0	0	0	0	0	143
07:15 AM	0	56	7	0	63	2	0	2	0	4	5	90	0	0	95	0	0	0	0	0	162
Total Volume	5	249	36	0	290	7	0	9	0	16	16	371	2	0	389	0	0	3	0	3	698
% App. Total	1.7	85.9	12.4	0		43.8	0	56.2	0		4.1	95.4	0.5	0		0	0	100	0		
PHF	.250	.865	.643	.000	.853	.583	.000	.750	.000	.667	.800	.793	.250	.000	.791	.000	.000	.375	.000	.375	.860
Passenger Vehicles	5	205	31	0	241	4	0	8	0	12	13	296	1	0	310	0	0	2	0	2	565
% Passenger Vehicles	100	82.3	86.1	0	83.1	57.1	0	88.9	0	75.0	81.3	79.8	50.0	0	79.7	0	0	66.7	0	66.7	80.9
Trucks	0	44	5	0	49	3	0	1	0	4	3	75	1	0	79	0	0	1	0	1	133
% Trucks	0	17.7	13.9	0	16.9	42.9	0	11.1	0	25.0	18.8	20.2	50.0	0	20.3	0	0	33.3	0	33.3	19.1



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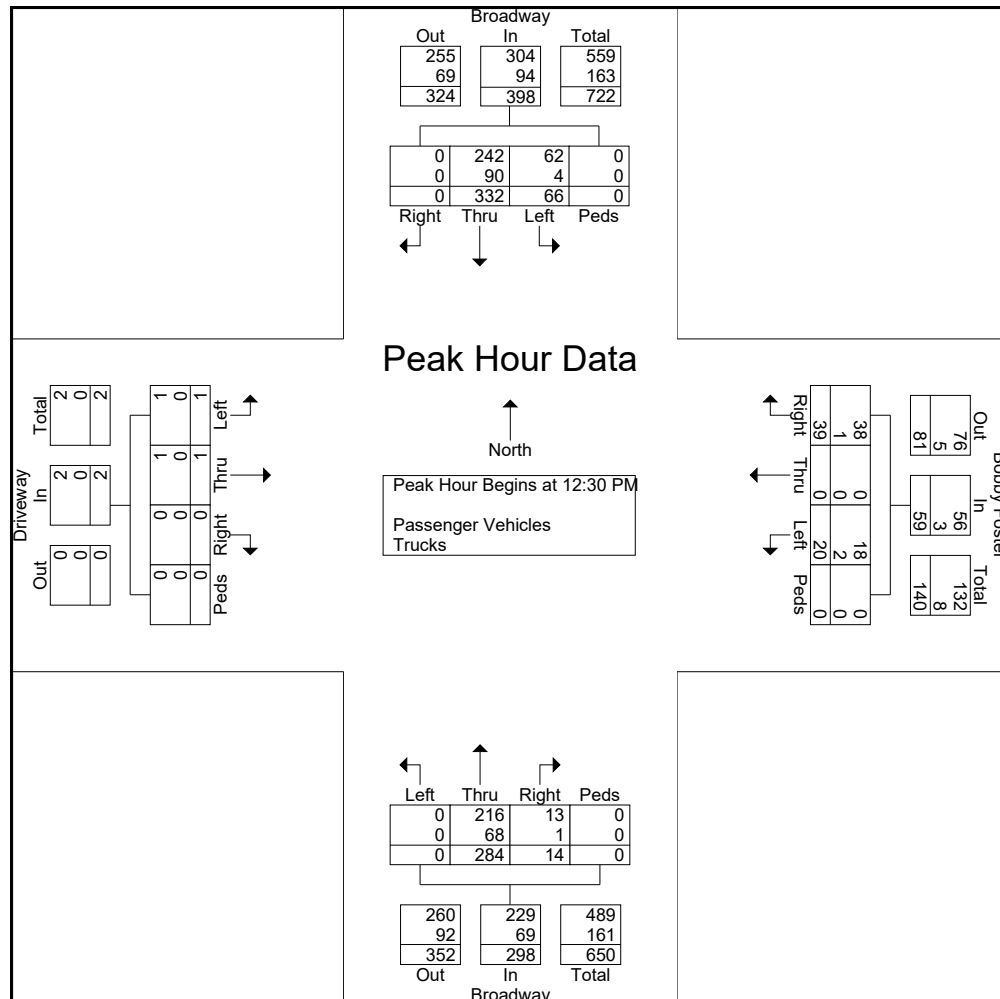
File Name : Broadway - Bobby Foster 04282021 DP

Site Code : 00000000

Start Date : 4/29/2021

Page No : 6

	Broadway From North					Bobby Foster From East					Broadway From South					Driveway From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 10:00 AM to 01:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 12:30 PM																					
12:30 PM	0	84	15	0	99	7	0	5	0	12	6	70	0	0	76	0	0	0	0	0	187
12:45 PM	0	75	9	0	84	10	0	9	0	19	0	68	0	0	68	0	1	0	0	1	172
01:00 PM	0	91	15	0	106	9	0	4	0	13	5	79	0	0	84	0	0	0	0	0	203
01:15 PM	0	82	27	0	109	13	0	2	0	15	3	67	0	0	70	0	0	1	0	1	195
Total Volume	0	332	66	0	398	39	0	20	0	59	14	284	0	0	298	0	1	1	0	2	757
% App. Total	0	83.4	16.6	0		66.1	0	33.9	0		4.7	95.3	0	0		0	50	50	0		
PHF	.000	.912	.611	.000	.913	.750	.000	.556	.000	.776	.583	.899	.000	.000	.887	.000	.250	.250	.000	.500	.932
Passenger Vehicles	0	242	62	0	304	38	0	18	0	56	13	216	0	0	229	0	1	1	0	2	591
% Passenger Vehicles	0	72.9	93.9	0	76.4	97.4	0	90.0	0	94.9	92.9	76.1	0	0	76.8	0	100	100	0	100	78.1
Trucks	0	90	4	0	94	1	0	2	0	3	1	68	0	0	69	0	0	0	0	0	166
% Trucks	0	27.1	6.1	0	23.6	2.6	0	10.0	0	5.1	7.1	23.9	0	0	23.2	0	0	0	0	0	21.9



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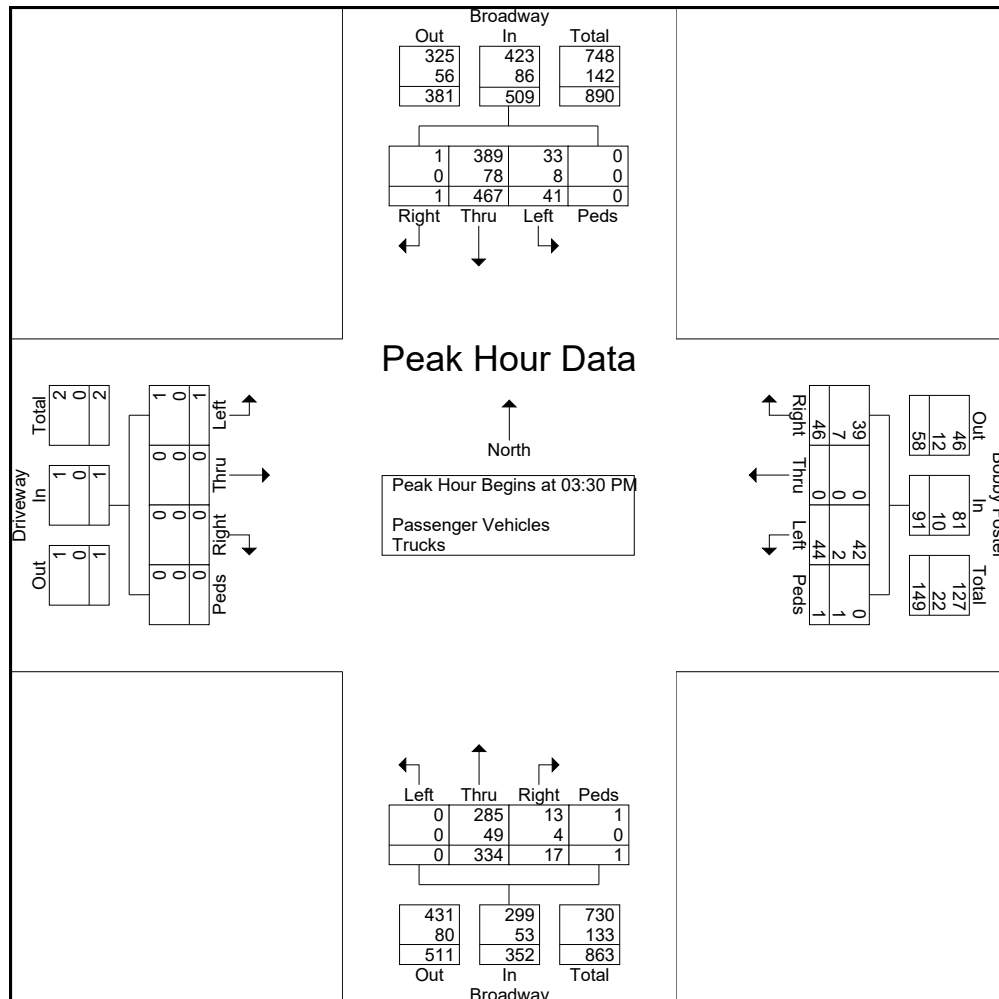
File Name : Broadway - Bobby Foster 04282021 DP

Site Code : 00000000

Start Date : 4/29/2021

Page No : 7

	Broadway From North					Bobby Foster From East					Broadway From South					Driveway From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 02:00 PM to 06:00 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 03:30 PM																					
03:30 PM	0	120	15	0	135	10	0	15	1	26	7	82	0	1	90	0	0	0	0	0	251
03:45 PM	0	105	13	0	118	10	0	7	0	17	2	99	0	0	101	0	0	0	0	0	236
04:00 PM	1	120	8	0	129	19	0	8	0	27	5	77	0	0	82	0	0	0	0	0	238
04:15 PM	0	122	5	0	127	7	0	14	0	21	3	76	0	0	79	0	0	1	0	1	228
Total Volume	1	467	41	0	509	46	0	44	1	91	17	334	0	1	352	0	0	1	0	1	953
% App. Total	0.2	91.7	8.1	0		50.5	0	48.4	1.1		4.8	94.9	0	0.3		0	0	100	0		
PHF	.250	.957	.683	.000	.943	.605	.000	.733	.250	.843	.607	.843	.000	.250	.871	.000	.000	.250	.000	.250	.949
Passenger Vehicles	1	389	33	0	423	39	0	42	0	81	13	285	0	1	299	0	0	1	0	1	804
% Passenger Vehicles	100	83.3	80.5	0	83.1	84.8	0	95.5	0	89.0	76.5	85.3	0	100	84.9	0	0	100	0	100	84.4
Trucks	0	78	8	0	86	7	0	2	1	10	4	49	0	0	53	0	0	0	0	0	149
% Trucks	0	16.7	19.5	0	16.9	15.2	0	4.5	100	11.0	23.5	14.7	0	0	15.1	0	0	0	0	0	15.6



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File Name : Broadway - Bobby Foster 04282021 DP

Site Code : 00000000

Start Date : 4/29/2021

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Rio Rancho, NM 87124

ADVANCEDESIGN

Weather: Overcast
Serial Number: 3083
Collected By: BTrejo
Other:

File Name : UNIVERSITY-RIO BRAVO_05042021 BT
Site Code : 00000000
Start Date : 4/28/2021
Page No : 1

Groups Printed- Passenger Vehicles - Trucks

Start Time	UNIVERSITY From North			UNIVERSITY From South			RIO BRAVO From West			Int. Total
	Right	Thru	Peds	Thru	Left	Peds	Right	Left	Peds	
06:30 AM	16	4	0	3	9	0	35	64	1	132
06:45 AM	24	8	0	3	35	0	45	61	0	176
Total	40	12	0	6	44	0	80	125	1	308
07:00 AM	16	5	0	5	25	0	35	50	0	136
07:15 AM	17	6	0	4	26	0	53	57	0	163
07:30 AM	15	4	0	7	28	0	46	73	0	173
07:45 AM	19	10	0	8	28	0	46	79	0	190
Total	67	25	0	24	107	0	180	259	0	662
08:00 AM	18	9	0	8	22	0	59	58	0	174
08:15 AM	28	10	0	5	40	0	65	52	0	200
08:30 AM	19	9	0	14	58	0	36	49	0	185
08:45 AM	15	8	0	9	19	0	41	59	0	151
Total	80	36	0	36	139	0	201	218	0	710
09:00 AM	19	10	0	9	30	0	26	43	0	137
09:15 AM	13	8	0	5	18	0	28	31	0	103
*** BREAK ***										
Total	32	18	0	14	48	0	54	74	0	240
*** BREAK ***										
11:00 AM	35	7	0	7	19	0	24	38	0	130
11:15 AM	26	1	0	5	33	0	19	39	0	123
11:30 AM	24	10	0	4	25	0	23	40	0	126
11:45 AM	21	4	0	6	28	0	25	35	0	119
Total	106	22	0	22	105	0	91	152	0	498
12:00 PM	30	4	0	7	34	0	26	33	0	134
12:15 PM	24	12	0	7	18	0	20	45	0	126
12:30 PM	24	5	0	10	25	0	22	41	0	127
12:45 PM	32	7	0	7	29	0	31	36	0	142
Total	110	28	0	31	106	0	99	155	0	529
01:00 PM	26	8	0	6	15	0	27	41	0	123
01:15 PM	23	4	0	5	20	0	33	50	0	135
01:30 PM	32	5	0	6	39	0	25	40	0	147
01:45 PM	25	5	0	9	26	0	26	44	0	135
Total	106	22	0	26	100	0	111	175	0	540
*** BREAK ***										
03:00 PM	50	7	0	4	44	0	29	50	0	184
03:15 PM	43	7	0	6	18	0	25	47	0	146
03:30 PM	52	16	0	14	57	0	19	50	0	208
03:45 PM	43	5	0	11	67	0	19	33	0	178
Total	188	35	0	35	186	0	92	180	0	716

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File Name : UNIVERSITY-RIO BRAVO_05042021 BT

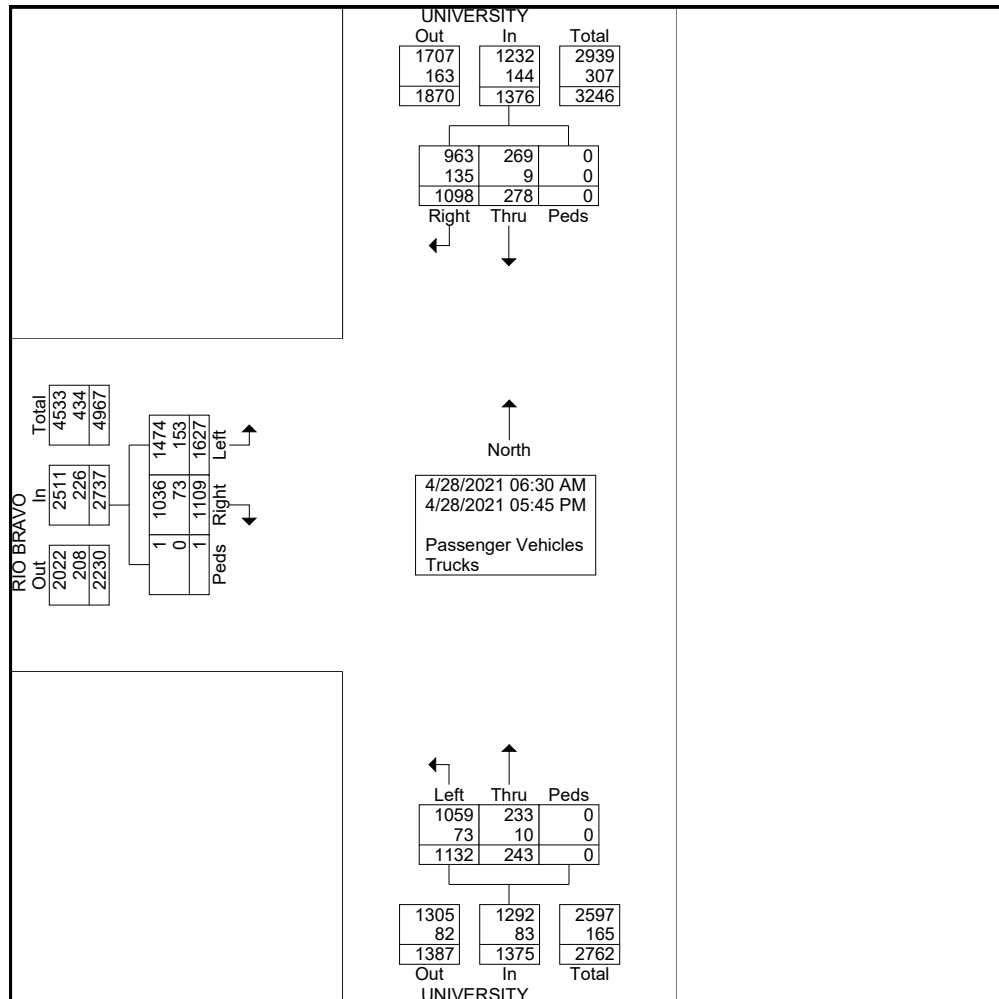
Site Code : 00000000

Start Date : 4/28/2021

Page No : 2

Groups Printed- Passenger Vehicles - Trucks

	UNIVERSITY From North			UNIVERSITY From South			RIO BRAVO From West			
Start Time	Right	Thru	Peds	Thru	Left	Peds	Right	Left	Peds	Int. Total
04:00 PM	33	7	0	3	38	0	17	42	0	140
04:15 PM	40	14	0	9	39	0	22	31	0	155
04:30 PM	63	9	0	6	49	0	25	24	0	176
04:45 PM	55	4	0	3	32	0	27	45	0	166
Total	191	34	0	21	158	0	91	142	0	637
05:00 PM	50	16	0	10	54	0	20	41	0	191
05:15 PM	44	10	0	5	32	0	25	33	0	149
05:30 PM	41	12	0	6	26	0	31	29	0	145
05:45 PM	43	8	0	7	27	0	34	44	0	163
Total	178	46	0	28	139	0	110	147	0	648
Grand Total	1098	278	0	243	1132	0	1109	1627	1	5488
Apprch %	79.8	20.2	0	17.7	82.3	0	40.5	59.4	0	
Total %	20	5.1	0	4.4	20.6	0	20.2	29.6	0	
Passenger Vehicles	963	269	0	233	1059	0	1036	1474	1	5035
% Passenger Vehicles	87.7	96.8	0	95.9	93.6	0	93.4	90.6	100	91.7
Trucks	135	9	0	10	73	0	73	153	0	453
% Trucks	12.3	3.2	0	4.1	6.4	0	6.6	9.4	0	8.3



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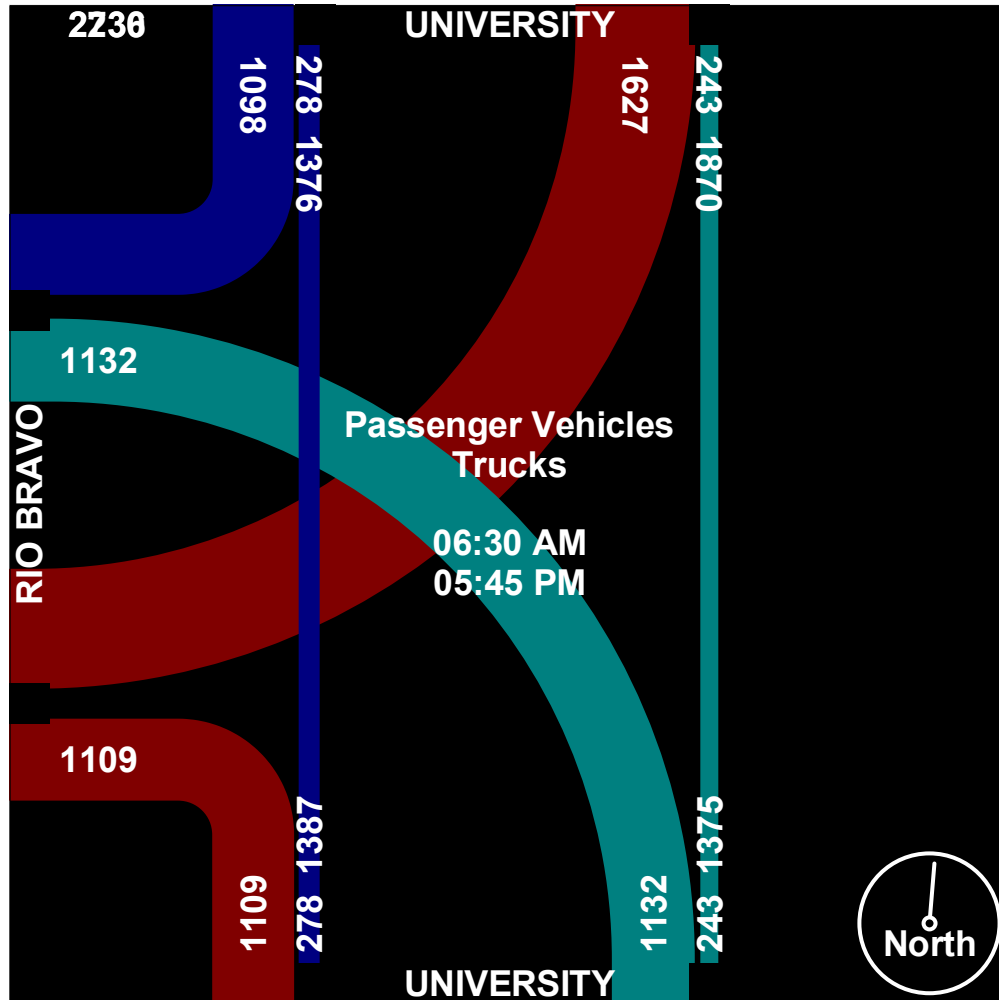
333 Rio Rancho Drive NW, Suite 101
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File Name : UNIVERSITY-RIO BRAVO_05042021 BT

Site Code : 00000000

Start Date : 4/28/2021

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Rio Rancho, NM 87124

ADVANCEDESIGN

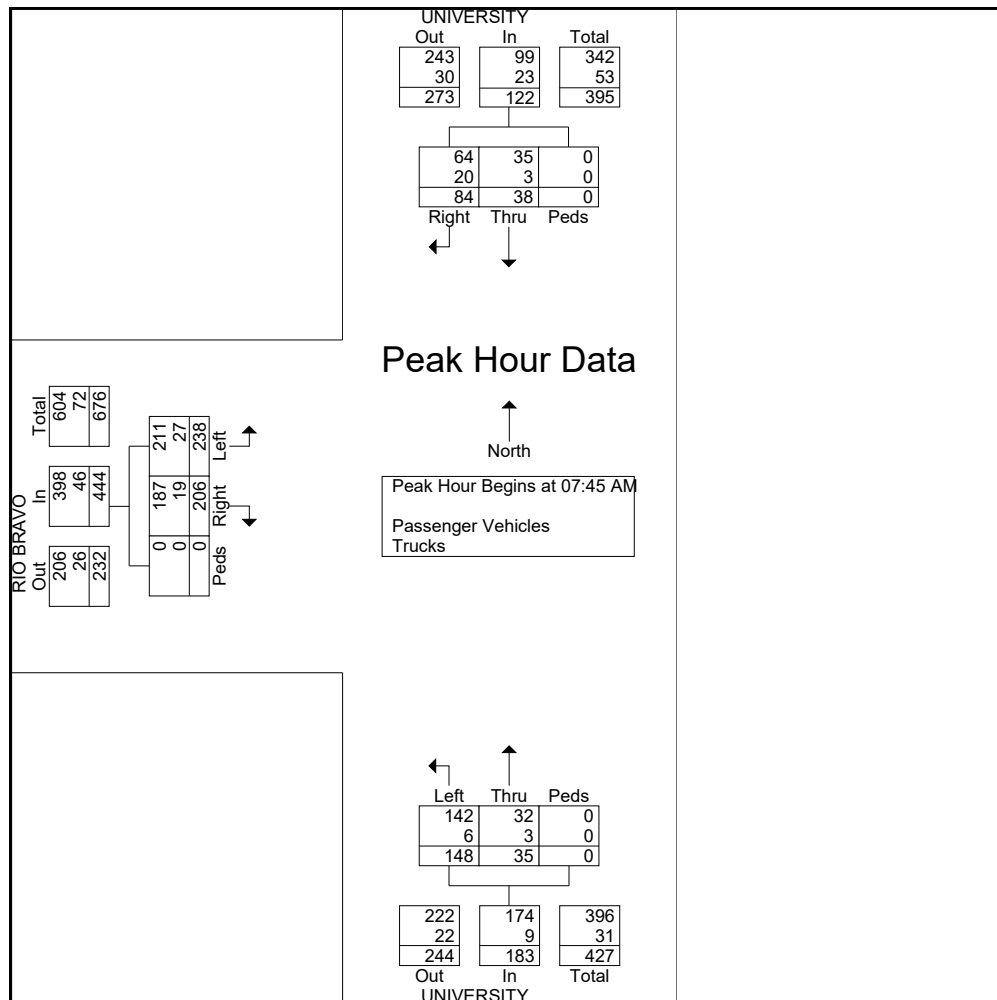
File Name : UNIVERSITY-RIO BRAVO_05042021 BT

Site Code : 00000000

Start Date : 4/28/2021

Page No : 4

	UNIVERSITY From North				UNIVERSITY From South				RIO BRAVO From West				
Start Time	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 06:30 AM to 09:45 AM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 07:45 AM													
07:45 AM	19	10	0	29	8	28	0	36	46	79	0	125	190
08:00 AM	18	9	0	27	8	22	0	30	59	58	0	117	174
08:15 AM	28	10	0	38	5	40	0	45	65	52	0	117	200
08:30 AM	19	9	0	28	14	58	0	72	36	49	0	85	185
Total Volume	84	38	0	122	35	148	0	183	206	238	0	444	749
% App. Total	68.9	31.1	0		19.1	80.9	0		46.4	53.6	0		
PHF	.750	.950	.000	.803	.625	.638	.000	.635	.792	.753	.000	.888	.936
Passenger Vehicles	64	35	0	99	32	142	0	174	187	211	0	398	671
% Passenger Vehicles	76.2	92.1	0	81.1	91.4	95.9	0	95.1	90.8	88.7	0	89.6	89.6
Trucks	20	3	0	23	3	6	0	9	19	27	0	46	78
% Trucks	23.8	7.9	0	18.9	8.6	4.1	0	4.9	9.2	11.3	0	10.4	10.4



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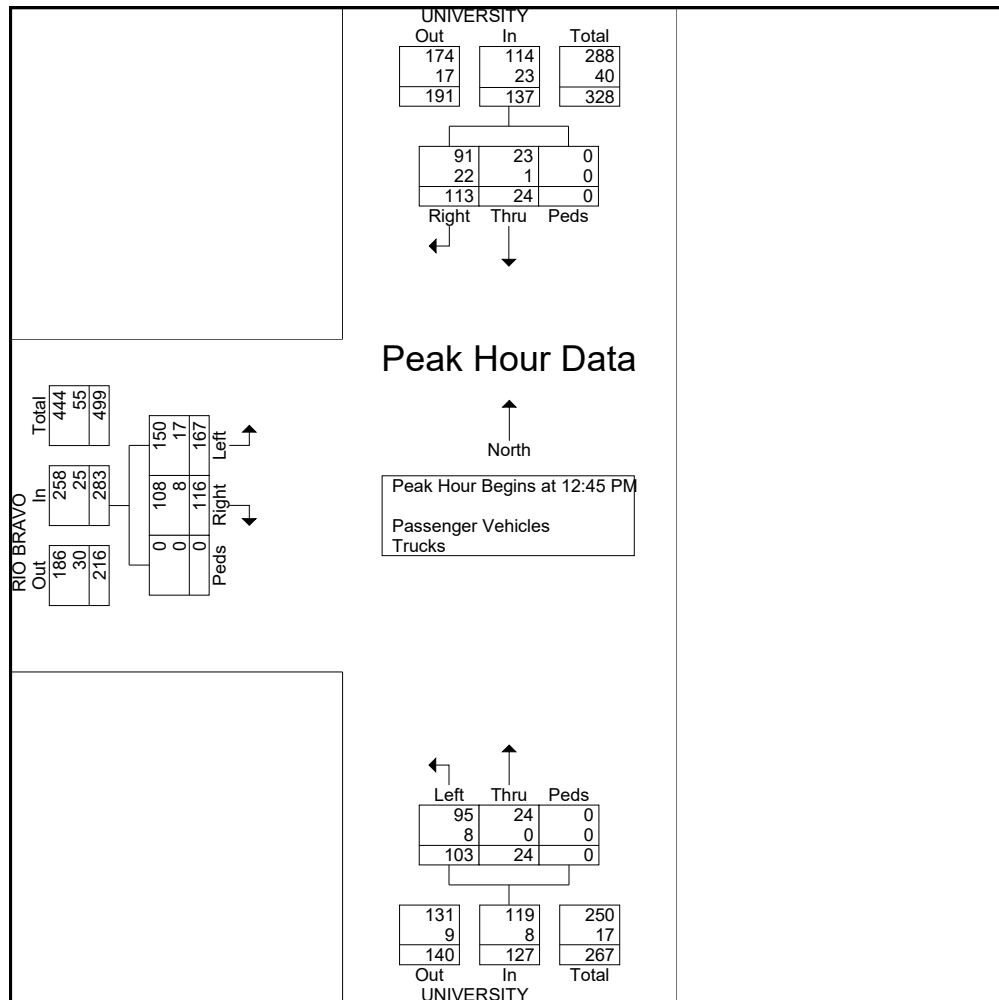
File Name : UNIVERSITY-RIO BRAVO_05042021 BT

Site Code : 00000000

Start Date : 4/28/2021

Page No : 5

	UNIVERSITY From North				UNIVERSITY From South				RIO BRAVO From West				
Start Time	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 10:00 AM to 01:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 12:45 PM													
12:45 PM	32	7	0	39	7	29	0	36	31	36	0	67	142
01:00 PM	26	8	0	34	6	15	0	21	27	41	0	68	123
01:15 PM	23	4	0	27	5	20	0	25	33	50	0	83	135
01:30 PM	32	5	0	37	6	39	0	45	25	40	0	65	147
Total Volume	113	24	0	137	24	103	0	127	116	167	0	283	547
% App. Total	82.5	17.5	0		18.9	81.1	0		41	59	0		
PHF	.883	.750	.000	.878	.857	.660	.000	.706	.879	.835	.000	.852	.930
Passenger Vehicles	91	23	0	114	24	95	0	119	108	150	0	258	491
% Passenger Vehicles	80.5	95.8	0	83.2	100	92.2	0	93.7	93.1	89.8	0	91.2	89.8
Trucks	22	1	0	23	0	8	0	8	8	17	0	25	56
% Trucks	19.5	4.2	0	16.8	0	7.8	0	6.3	6.9	10.2	0	8.8	10.2



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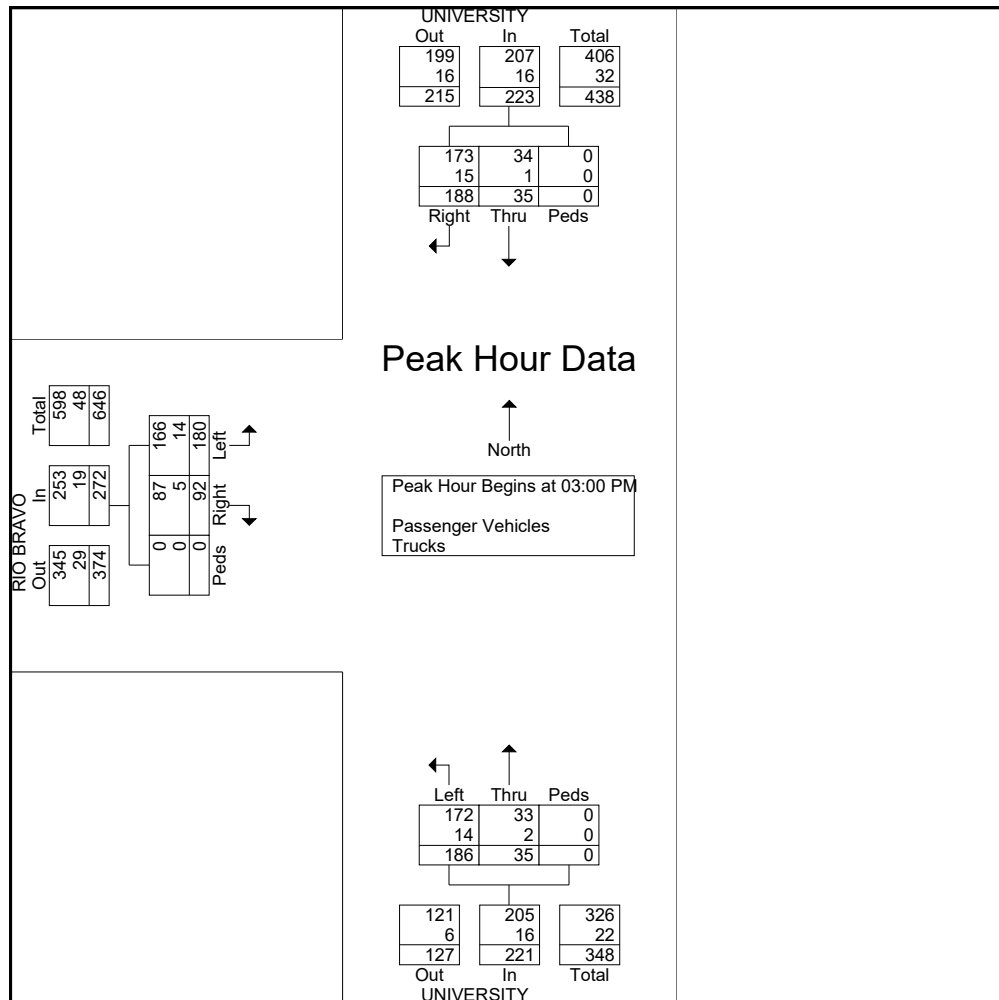
File Name : UNIVERSITY-RIO BRAVO_05042021 BT

Site Code : 00000000

Start Date : 4/28/2021

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	UNIVERSITY From North				UNIVERSITY From South				RIO BRAVO From West				
Start Time	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 02:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 03:00 PM													
03:00 PM	50	7	0	57	4	44	0	48	29	50	0	79	184
03:15 PM	43	7	0	50	6	18	0	24	25	47	0	72	146
03:30 PM	52	16	0	68	14	57	0	71	19	50	0	69	208
03:45 PM	43	5	0	48	11	67	0	78	19	33	0	52	178
Total Volume	188	35	0	223	35	186	0	221	92	180	0	272	716
% App. Total	84.3	15.7	0		15.8	84.2	0		33.8	66.2	0		
PHF	.904	.547	.000	.820	.625	.694	.000	.708	.793	.900	.000	.861	.861
Passenger Vehicles	173	34	0	207	33	172	0	205	87	166	0	253	665
% Passenger Vehicles	92.0	97.1	0	92.8	94.3	92.5	0	92.8	94.6	92.2	0	93.0	92.9
Trucks	15	1	0	16	2	14	0	16	5	14	0	19	51
% Trucks	8.0	2.9	0	7.2	5.7	7.5	0	7.2	5.4	7.8	0	7.0	7.1



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File Name : UNIVERSITY-RIO BRAVO_05042021 BT

Site Code : 00000000

Start Date : 4/28/2021

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333 Rio Rancho Drive NW, Suite 101

Rio Rancho, NM 87124

ADVANCEDESIGN

Weather: Overcast

Serial Number: 3083/3080

Collected By: BT/JS

Other:

File Name : roadway-rio bravo_05042021 combined

Site Code : 00000000

Start Date : 5/4/2021

Page No : 1

Groups Printed- Passenger Vehicles - Trucks

	BROADWAY From North				RIO BRAVO From East				BROADWAY From South				RIO BRAVO From West				
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
06:30 AM	5	38	8	0	12	93	74	0	50	32	11	0	31	279	33	0	666
06:45 AM	13	21	10	0	12	127	108	0	54	34	29	0	47	286	31	0	772
Total	18	59	18	0	24	220	182	0	104	66	40	0	78	565	64	0	1438
07:00 AM	10	30	4	0	9	121	66	0	49	21	27	0	23	251	33	0	644
07:15 AM	10	21	14	0	8	105	73	1	73	25	21	0	21	320	44	0	736
07:30 AM	30	29	26	0	5	123	40	0	52	58	14	3	19	328	52	0	779
07:45 AM	18	20	18	0	5	114	49	1	71	41	22	2	29	311	32	0	733
Total	68	100	62	0	27	463	228	2	245	145	84	5	92	1210	161	0	2892
08:00 AM	19	32	13	0	3	117	53	0	46	26	25	1	35	241	27	0	638
08:15 AM	24	28	12	0	2	129	47	0	38	31	15	0	24	233	24	0	607
08:30 AM	18	33	12	0	10	125	39	1	54	40	25	0	32	204	21	0	614
08:45 AM	37	41	12	0	1	136	49	0	55	34	17	1	32	187	21	0	623
Total	98	134	49	0	16	507	188	1	193	131	82	2	123	865	93	0	2482
09:00 AM	24	28	16	0	7	83	43	0	63	29	20	0	30	160	19	0	522
09:15 AM	14	27	7	0	5	113	42	2	42	22	36	0	26	157	18	0	511
*** BREAK ***																	
Total	38	55	23	0	12	196	85	2	105	51	56	0	56	317	37	0	1033
*** BREAK ***																	
11:00 AM	28	44	11	0	8	90	48	0	58	46	40	1	24	115	17	0	530
11:15 AM	25	28	8	0	4	128	63	0	48	25	36	0	32	177	25	0	599
11:30 AM	23	49	11	0	5	121	56	0	62	19	41	0	34	157	17	0	595
11:45 AM	31	38	14	0	2	134	53	0	59	38	57	0	35	128	33	0	622
Total	107	159	44	0	19	473	220	0	227	128	174	1	125	577	92	0	2346
12:00 PM	26	53	14	0	6	158	50	0	46	29	49	0	33	146	16	0	626
12:15 PM	39	52	14	0	3	179	74	0	57	32	58	1	42	155	36	0	742
12:30 PM	26	45	12	0	8	134	58	0	62	31	45	0	29	185	28	0	663
12:45 PM	32	43	15	0	3	147	60	0	66	35	53	0	43	153	26	0	676
Total	123	193	55	0	20	618	242	0	231	127	205	1	147	639	106	0	2707
01:00 PM	30	47	12	0	3	145	59	0	38	35	35	0	27	144	25	0	600
01:15 PM	35	46	9	0	4	121	61	0	61	34	36	0	40	135	22	0	604
01:30 PM	35	44	13	0	3	167	49	0	56	36	43	0	28	157	33	0	664
01:45 PM	24	56	10	0	5	171	59	0	48	35	32	0	38	161	25	0	664
Total	124	193	44	0	15	604	228	0	203	140	146	0	133	597	105	0	2532
*** BREAK ***																	
03:00 PM	41	51	11	0	8	221	78	0	58	33	16	0	23	169	20	0	729
03:15 PM	58	64	17	0	5	233	65	0	69	38	48	3	31	161	39	0	831
03:30 PM	62	71	23	0	4	216	57	0	86	40	50	2	33	179	28	0	851
03:45 PM	68	80	12	0	9	272	95	1	91	47	53	1	26	160	28	0	943
Total	229	266	63	0	26	942	295	1	304	158	167	6	113	669	115	0	3354

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333 Rio Rancho Drive NW, Suite 101

Rio Rancho, NM 87124

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File Name : roadway-rio bravo_05042021 combined

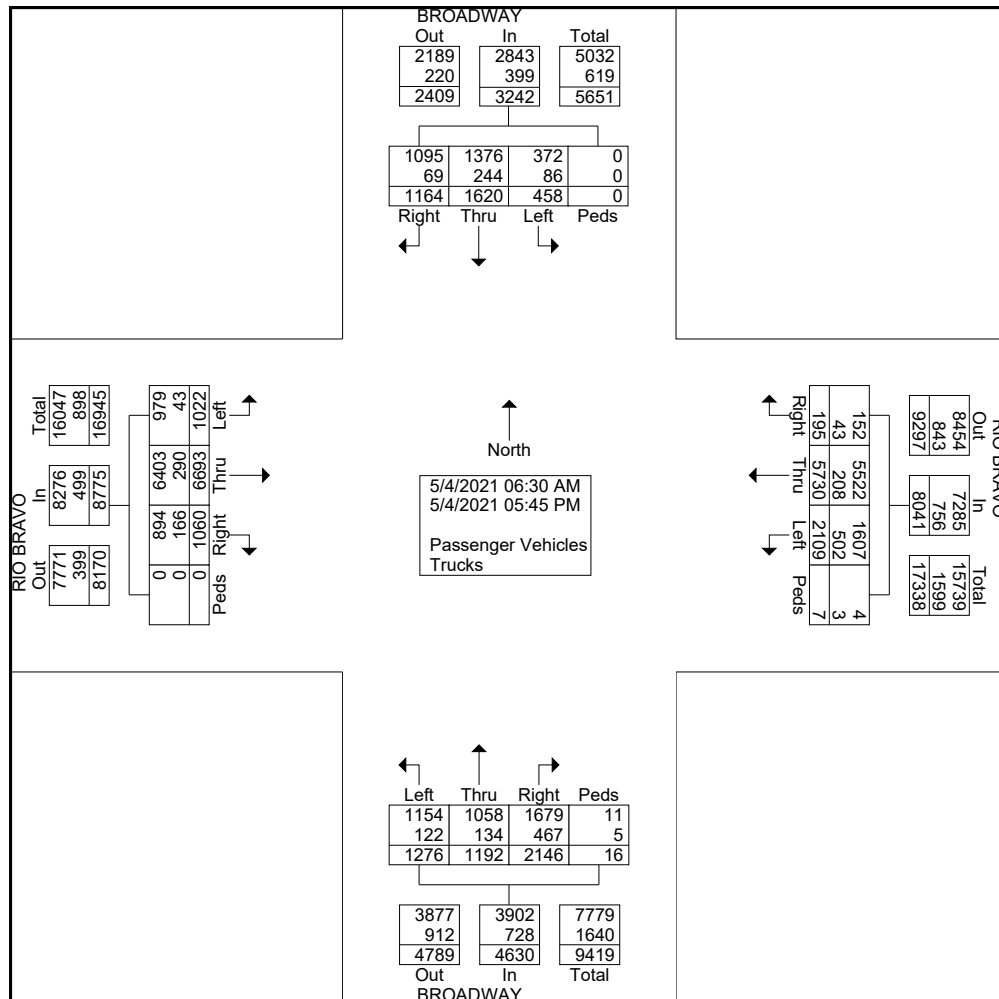
Site Code : 00000000

Start Date : 5/4/2021

Page No : 2

Groups Printed- Passenger Vehicles - Trucks

	BROADWAY From North				RIO BRAVO From East				BROADWAY From South				RIO BRAVO From West				Int. Total
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	
04:00 PM	67	66	15	0	5	222	60	0	80	46	50	0	37	176	28	0	852
04:15 PM	61	65	16	0	7	285	74	0	81	44	44	0	30	159	34	0	900
04:30 PM	53	59	7	0	3	228	61	0	76	28	41	0	35	158	47	0	796
04:45 PM	33	61	16	0	7	233	91	1	81	39	51	0	20	131	23	0	787
Total	214	251	54	0	22	968	286	1	318	157	186	0	122	624	132	0	3335
05:00 PM	25	61	6	0	3	261	67	0	94	39	53	1	26	170	22	0	828
05:15 PM	47	64	19	0	7	249	50	0	78	36	51	0	14	141	36	0	792
05:30 PM	29	45	9	0	4	229	38	0	44	14	32	0	13	176	32	0	665
05:45 PM	44	40	12	0	0	0	0	0	0	0	0	0	18	143	27	0	284
Total	145	210	46	0	14	739	155	0	216	89	136	1	71	630	117	0	2569
Grand Total	1164	1620	458	0	195	5730	2109	7	2146	1192	1276	16	1060	6693	1022	0	24688
Apprch %	35.9	50	14.1	0	2.4	71.3	26.2	0.1	46.3	25.7	27.6	0.3	12.1	76.3	11.6	0	
Total %	4.7	6.6	1.9	0	0.8	23.2	8.5	0	8.7	4.8	5.2	0.1	4.3	27.1	4.1	0	
Passenger Vehicles	1095	1376	372	0	152	5522	1607	4	1679	1058	1154	11	894	6403	979	0	22306
% Passenger Vehicles	94.1	84.9	81.2	0	77.9	96.4	76.2	57.1	78.2	88.8	90.4	68.8	84.3	95.7	95.8	0	90.4
Trucks	69	244	86	0	43	208	502	3	467	134	122	5	166	290	43	0	2382
% Trucks	5.9	15.1	18.8	0	22.1	3.6	23.8	42.9	21.8	11.2	9.6	31.2	15.7	4.3	4.2	0	9.6



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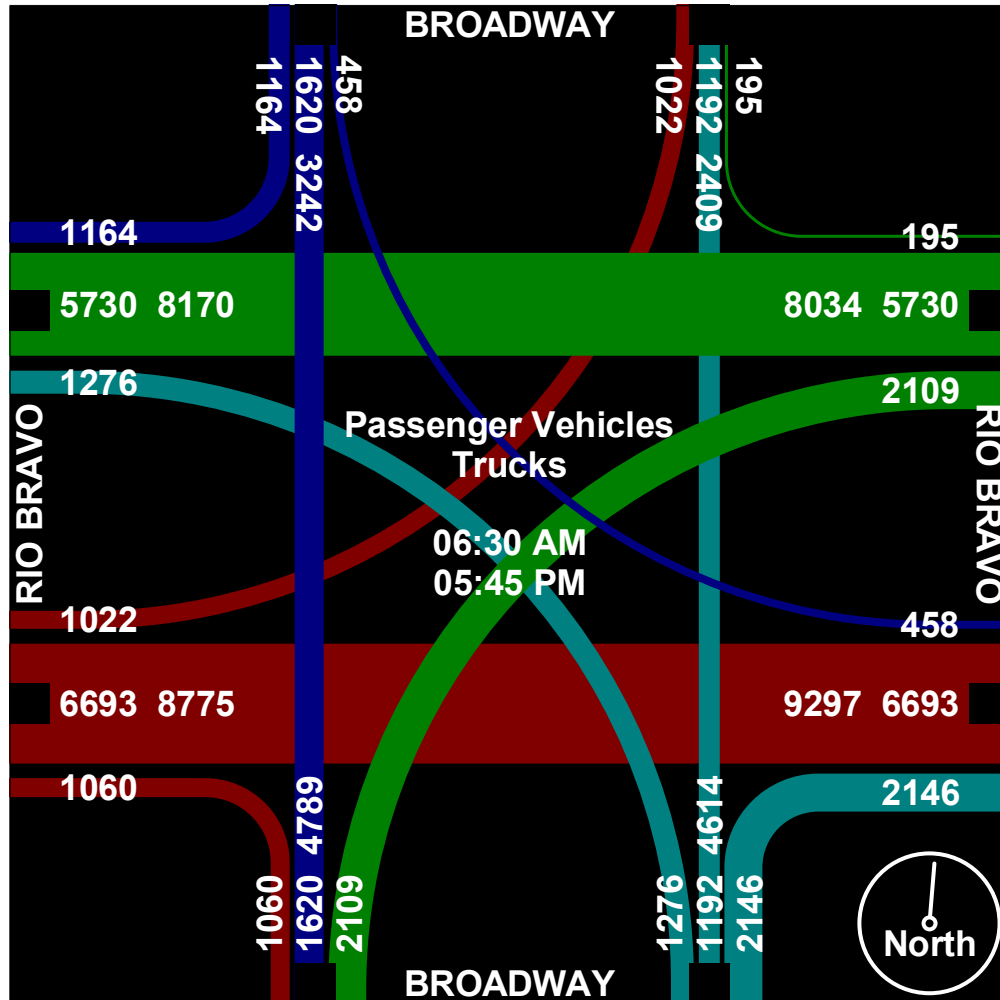
ADVANCEDDESIGN

File Name : roadway-rio bravo_05042021 combined

Site Code : 00000000

Start Date : 5/4/2021

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333 Rio Rancho Drive NW, Suite 101
Rio Rancho, NM 87124
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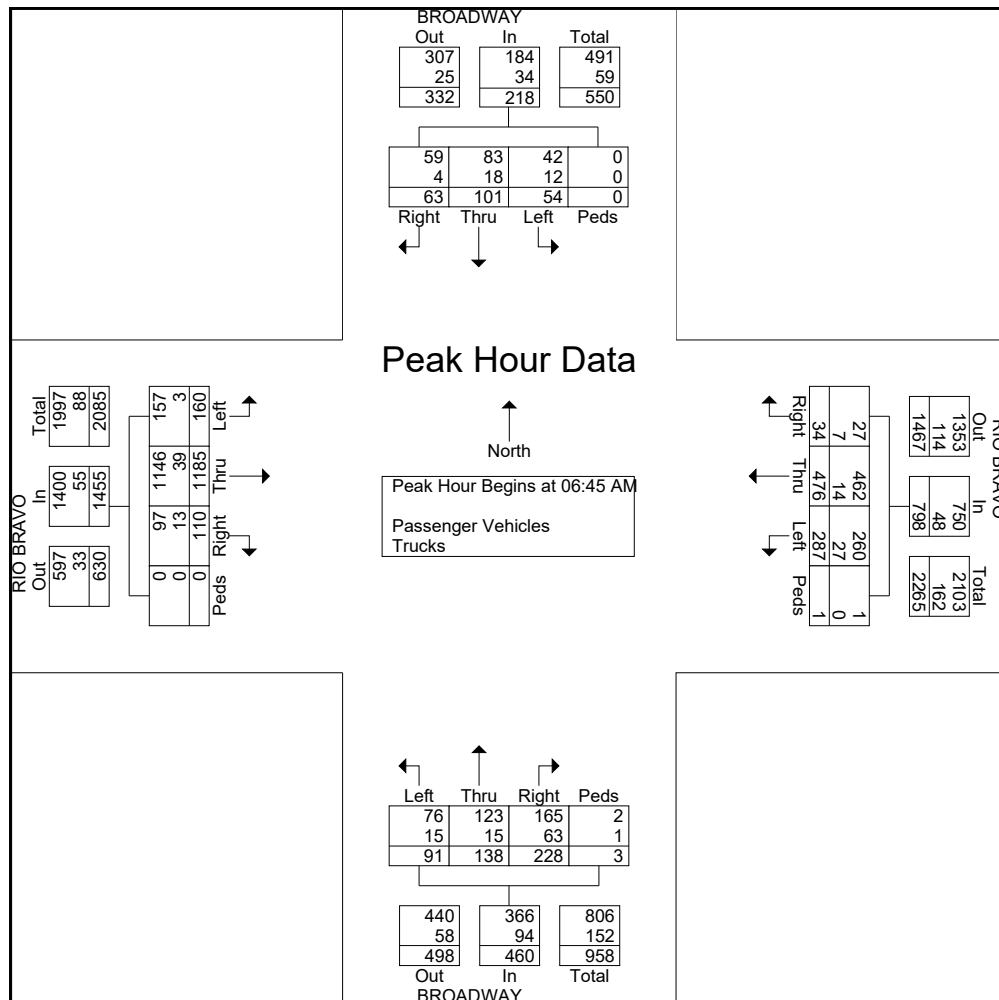
File Name : roadway-rio bravo_05042021 combined

Site Code : 00000000

Start Date : 5/4/2021

Page No : 4

	BROADWAY From North					RIO BRAVO From East					BROADWAY From South					RIO BRAVO From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 06:30 AM to 09:45 AM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 06:45 AM																					
06:45 AM	13	21	10	0	44	12	127	108	0	247	54	34	29	0	117	47	286	31	0	364	772
07:00 AM	10	30	4	0	44	9	121	66	0	196	49	21	27	0	97	23	251	33	0	307	644
07:15 AM	10	21	14	0	45	8	105	73	1	187	73	25	21	0	119	21	320	44	0	385	736
07:30 AM	30	29	26	0	85	5	123	40	0	168	52	58	14	3	127	19	328	52	0	399	779
Total Volume	63	101	54	0	218	34	476	287	1	798	228	138	91	3	460	110	1185	160	0	1455	2931
% App. Total	28.9	46.3	24.8	0		4.3	59.6	36	0.1		49.6	30	19.8	0.7		7.6	81.4	11	0		
PHF	.525	.842	.519	.000	.641	.708	.937	.664	.250	.808	.781	.595	.784	.250	.906	.585	.903	.769	.000	.912	.941
Passenger Vehicles	59	83	42	0	184	27	462	260	1	750	165	123	76	2	366	97	1146	157	0	1400	2700
% Passenger Vehicles	93.7	82.2	77.8	0	84.4	79.4	97.1	90.6	100	94.0	72.4	89.1	83.5	66.7	79.6	88.2	96.7	98.1	0	96.2	92.1
Trucks	4	18	12	0	34	7	14	27	0	48	63	15	15	1	94	13	39	3	0	55	231
% Trucks	6.3	17.8	22.2	0	15.6	20.6	2.9	9.4	0	6.0	27.6	10.9	16.5	33.3	20.4	11.8	3.3	1.9	0	3.8	7.9



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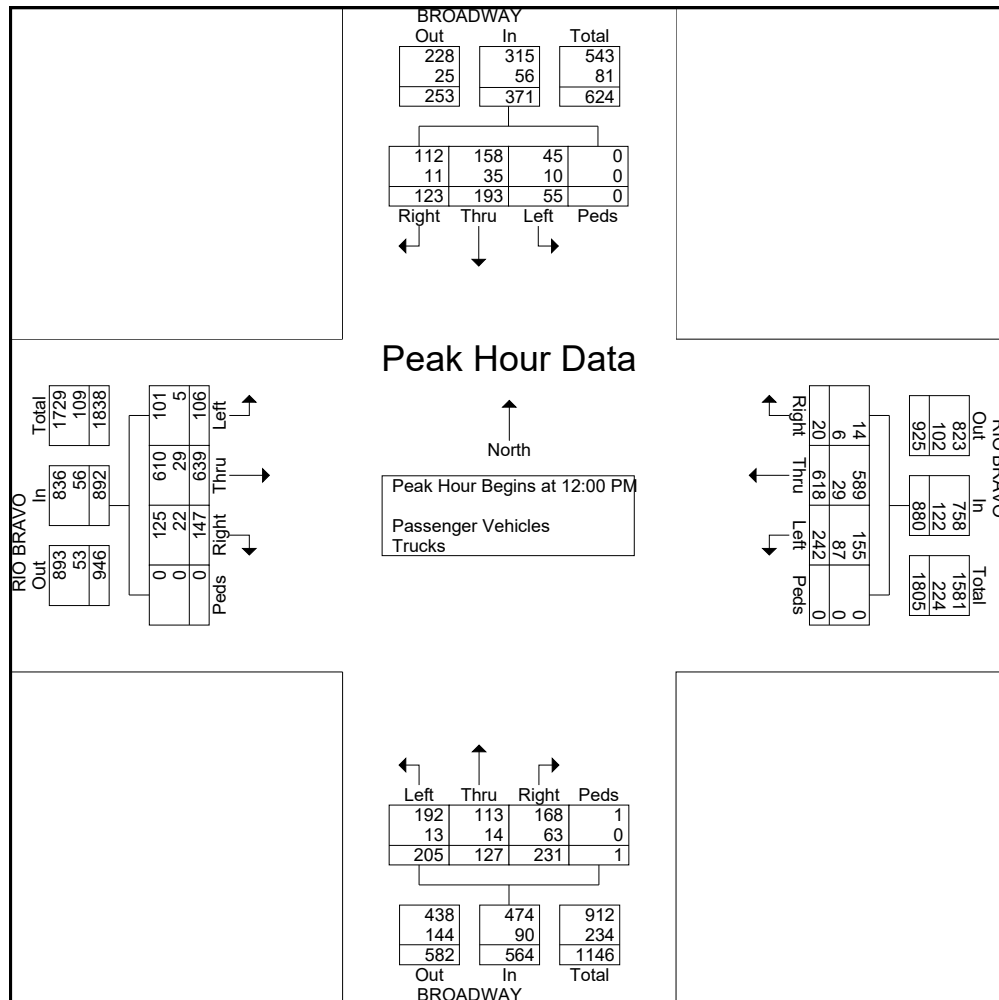
File Name : roadway-rio bravo_05042021 combined

Site Code : 00000000

Start Date : 5/4/2021

Page No : 5

	BROADWAY From North					RIO BRAVO From East					BROADWAY From South					RIO BRAVO From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 10:00 AM to 01:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 12:00 PM																					
12:00 PM	26	53	14	0	93	6	158	50	0	214	46	29	49	0	124	33	146	16	0	195	626
12:15 PM	39	52	14	0	105	3	179	74	0	256	57	32	58	1	148	42	155	36	0	233	742
12:30 PM	26	45	12	0	83	8	134	58	0	200	62	31	45	0	138	29	185	28	0	242	663
12:45 PM	32	43	15	0	90	3	147	60	0	210	66	35	53	0	154	43	153	26	0	222	676
Total Volume	123	193	55	0	371	20	618	242	0	880	231	127	205	1	564	147	639	106	0	892	2707
% App. Total	33.2	52	14.8	0		2.3	70.2	27.5	0		41	22.5	36.3	0.2		16.5	71.6	11.9	0		
PHF	.788	.910	.917	.000	.883	.625	.863	.818	.000	.859	.875	.907	.884	.250	.916	.855	.864	.736	.000	.921	.912
Passenger Vehicles	112	158	45	0	315	14	589	155	0	758	168	113	192	1	474	125	610	101	0	836	2383
% Passenger Vehicles	91.1	81.9	81.8	0	84.9	70.0	95.3	64.0	0	86.1	72.7	89.0	93.7	100	84.0	85.0	95.5	95.3	0	93.7	88.0
Trucks	11	35	10	0	56	6	29	87	0	122	63	14	13	0	90	22	29	5	0	56	324
% Trucks	8.9	18.1	18.2	0	15.1	30.0	4.7	36.0	0	13.9	27.3	11.0	6.3	0	16.0	15.0	4.5	4.7	0	6.3	12.0



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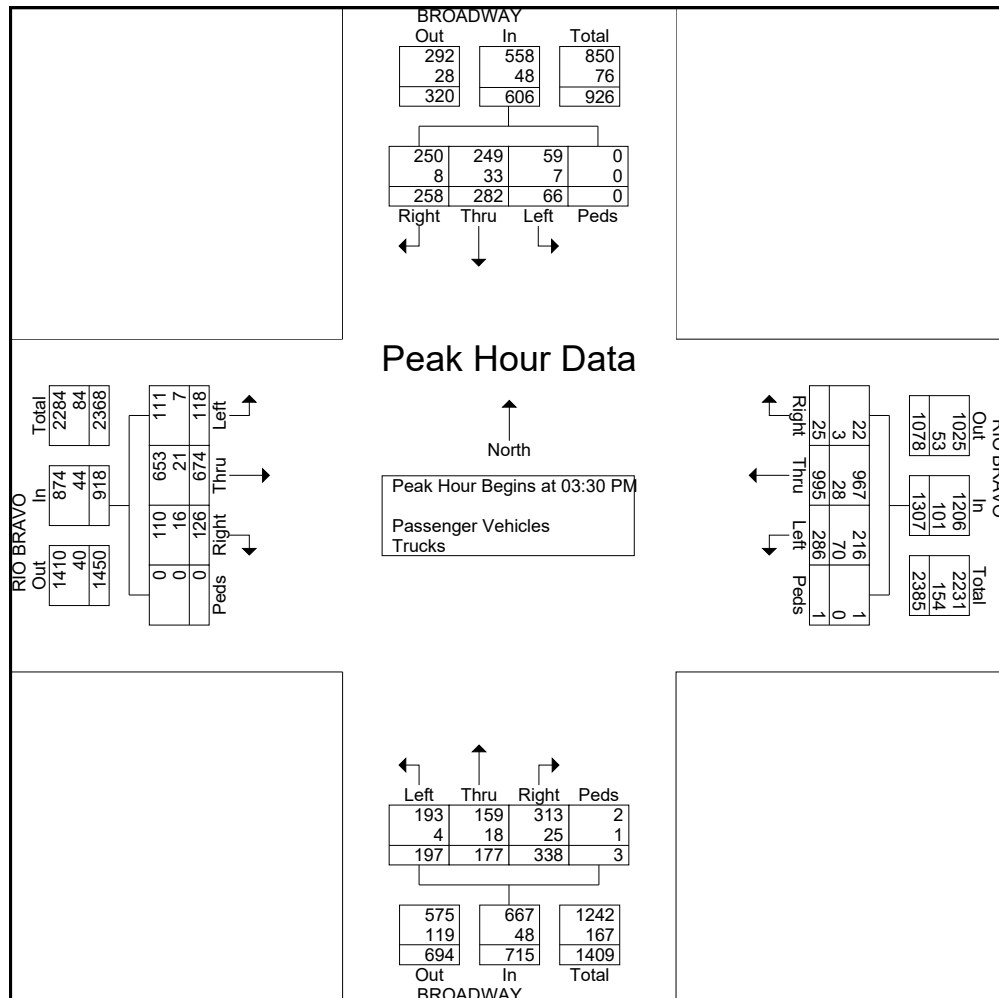
File Name : roadway-rio bravo_05042021 combined

Site Code : 00000000

Start Date : 5/4/2021

Page No : 6

	BROADWAY From North					RIO BRAVO From East					BROADWAY From South					RIO BRAVO From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 02:00 PM to 05:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 03:30 PM																					
03:30 PM	62	71	23	0	156	4	216	57	0	277	86	40	50	2	178	33	179	28	0	240	851
03:45 PM	68	80	12	0	160	9	272	95	1	377	91	47	53	1	192	26	160	28	0	214	943
04:00 PM	67	66	15	0	148	5	222	60	0	287	80	46	50	0	176	37	176	28	0	241	852
04:15 PM	61	65	16	0	142	7	285	74	0	366	81	44	44	0	169	30	159	34	0	223	900
Total Volume	258	282	66	0	606	25	995	286	1	1307	338	177	197	3	715	126	674	118	0	918	3546
% App. Total	42.6	46.5	10.9	0		1.9	76.1	21.9	0.1		47.3	24.8	27.6	0.4		13.7	73.4	12.9	0		
PHF	.949	.881	.717	.000	.947	.694	.873	.753	.250	.867	.929	.941	.929	.375	.931	.851	.941	.868	.000	.952	.940
Passenger Vehicles	250	249	59	0	558	22	967	216	1	1206	313	159	193	2	667	110	653	111	0	874	3305
% Passenger Vehicles	96.9	88.3	89.4	0	92.1	88.0	97.2	75.5	100	92.3	92.6	89.8	98.0	66.7	93.3	87.3	96.9	94.1	0	95.2	93.2
Trucks	8	33	7	0	48	3	28	70	0	101	25	18	4	1	48	16	21	7	0	44	241
% Trucks	3.1	11.7	10.6	0	7.9	12.0	2.8	24.5	0	7.7	7.4	10.2	2.0	33.3	6.7	12.7	3.1	5.9	0	4.8	6.8



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File Name : broadway-rio bravo_05042021 combined

Site Code : 00000000

Start Date : 5/4/2021

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333 Rio Rancho Drive NW, Suite 101

Rio Rancho, NM 87124

ADVANCEDESIGN

Weather: Overcast
Serial Number: 3083/3080
Collected By: BT/JS
Other:

File Name : 2ND-RIO BRAVO_05042021 COMBINED
Site Code : 00000000
Start Date : 4/29/2021
Page No : 1

Groups Printed- Passenger Vehicles - Trucks

	2ND From North				RIO BRAVO From East				2ND From South				RIO BRAVO From West				Int. Total
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	
06:30 AM	12	15	8	0	7	61	14	0	7	10	13	0	45	288	46	0	526
06:45 AM	17	23	12	0	22	99	23	0	7	8	31	0	98	323	58	0	721
Total	29	38	20	0	29	160	37	0	14	18	44	0	143	611	104	0	1247
07:00 AM	8	14	17	0	8	83	19	0	11	11	31	0	48	246	48	0	544
07:15 AM	10	4	12	0	11	109	14	0	13	5	28	1	41	331	50	0	629
07:30 AM	14	13	19	0	10	108	18	0	10	31	42	0	28	307	51	0	651
07:45 AM	10	16	17	0	14	141	22	0	18	20	43	0	42	401	46	1	791
Total	42	47	65	0	43	441	73	0	52	67	144	1	159	1285	195	1	2615
08:00 AM	18	10	22	0	19	128	22	0	6	11	34	0	46	288	42	0	646
08:15 AM	20	17	11	0	8	120	14	0	7	10	26	0	26	239	54	0	552
08:30 AM	26	16	20	0	22	134	11	0	11	7	16	0	35	282	39	0	619
08:45 AM	30	17	18	0	9	135	5	0	8	12	29	3	18	226	35	0	545
Total	94	60	71	0	58	517	52	0	32	40	105	3	125	1035	170	0	2362
09:00 AM	16	22	14	0	12	136	14	0	8	10	25	0	29	193	26	0	505
09:15 AM	30	15	20	0	12	89	14	7	8	16	38	0	31	156	20	0	456
*** BREAK ***																	
Total	46	37	34	0	24	225	28	7	16	26	63	0	60	349	46	0	961
*** BREAK ***																	
11:00 AM	18	11	20	0	16	142	5	0	13	4	56	0	32	154	36	0	507
11:15 AM	23	18	16	0	19	140	9	0	13	5	38	0	26	169	24	0	500
11:30 AM	29	18	23	0	16	178	11	1	16	9	51	0	34	166	28	0	580
11:45 AM	26	16	20	0	15	207	16	0	12	17	42	0	30	181	15	0	597
Total	96	63	79	0	66	667	41	1	54	35	187	0	122	670	103	0	2184
12:00 PM	32	13	21	0	10	171	19	0	7	10	51	0	36	186	43	0	599
12:15 PM	39	16	16	0	10	185	7	0	10	14	57	0	39	203	24	0	620
12:30 PM	21	18	24	0	20	196	14	0	10	17	53	0	31	182	26	0	612
12:45 PM	30	19	17	0	16	220	13	0	7	8	32	0	33	173	32	0	600
Total	122	66	78	0	56	772	53	0	34	49	193	0	139	744	125	0	2431
01:00 PM	26	18	18	0	17	227	16	0	7	14	51	1	35	195	28	0	653
01:15 PM	41	18	21	0	20	215	25	0	10	12	46	0	47	190	28	0	673
01:30 PM	31	20	18	0	14	187	14	0	17	12	52	0	43	200	26	0	634
01:45 PM	42	24	29	0	10	208	15	0	12	13	53	0	34	194	31	0	665
Total	140	80	86	0	61	837	70	0	46	51	202	1	159	779	113	0	2625
*** BREAK ***																	
03:00 PM	64	16	23	0	31	228	17	0	20	24	48	0	42	202	28	0	743
03:15 PM	58	22	20	0	8	263	12	0	16	12	83	0	35	202	31	0	762
03:30 PM	67	20	25	0	19	269	22	0	18	15	87	0	40	181	36	0	799
03:45 PM	90	21	22	0	22	283	14	0	15	11	65	0	33	189	27	0	792
Total	279	79	90	0	80	1043	65	0	69	62	283	0	150	774	122	0	3096

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File Name : 2ND-RIO BRAVO_05042021 COMBINED

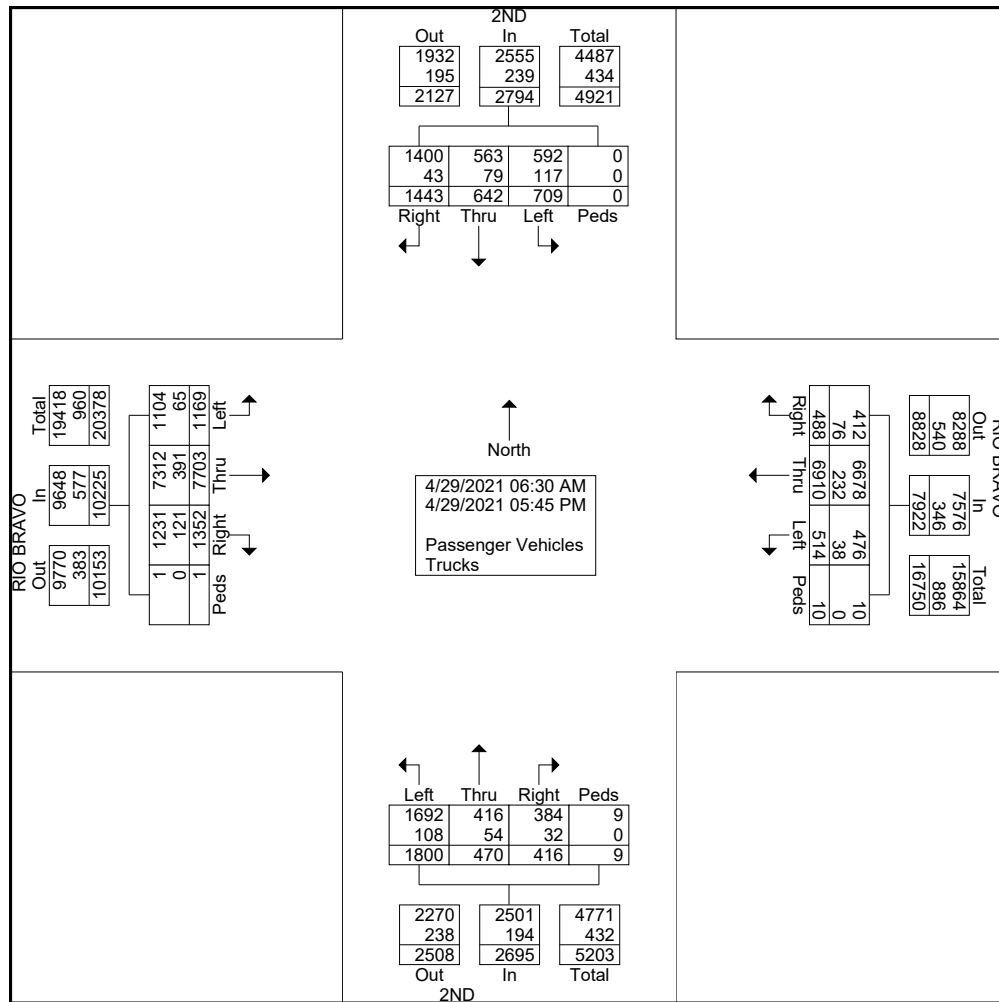
Site Code : 00000000

Start Date : 4/29/2021

Page No : 2

Groups Printed- Passenger Vehicles - Trucks

	2ND From North				RIO BRAVO From East				2ND From South				RIO BRAVO From West				Int. Total
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	
04:00 PM	66	24	28	0	7	296	10	0	21	17	99	2	37	195	21	0	823
04:15 PM	50	17	28	0	9	379	13	0	13	22	104	0	34	218	24	0	911
04:30 PM	89	28	35	0	15	308	28	1	12	12	83	0	37	187	24	0	859
04:45 PM	65	18	23	0	9	357	11	0	17	28	79	2	40	180	28	0	857
Total	270	87	114	0	40	1340	62	1	63	79	365	4	148	780	97	0	3450
05:00 PM	91	28	16	0	10	286	9	1	20	15	87	0	31	165	22	0	781
05:15 PM	88	24	19	0	7	369	11	0	6	17	71	0	47	187	17	0	863
05:30 PM	83	17	15	0	14	253	13	0	10	11	56	0	35	168	32	0	707
05:45 PM	63	16	22	0	0	0	0	0	0	0	0	0	34	156	23	0	314
Total	325	85	72	0	31	908	33	1	36	43	214	0	147	676	94	0	2665
Grand Total	1443	642	709	0	488	6910	514	10	416	470	1800	9	1352	7703	1169	1	23636
Apprch %	51.6	23	25.4	0	6.2	87.2	6.5	0.1	15.4	17.4	66.8	0.3	13.2	75.3	11.4	0	
Total %	6.1	2.7	3	0	2.1	29.2	2.2	0	1.8	2	7.6	0	5.7	32.6	4.9	0	
Passenger Vehicles	1400	563	592	0	412	6678	476	10	384	416	1692	9	1231	7312	1104	1	22280
% Passenger Vehicles	97	87.7	83.5	0	84.4	96.6	92.6	100	92.3	88.5	94	100	91.1	94.9	94.4	100	94.3
Trucks	43	79	117	0	76	232	38	0	32	54	108	0	121	391	65	0	1356
% Trucks	3	12.3	16.5	0	15.6	3.4	7.4	0	7.7	11.5	6	0	8.9	5.1	5.6	0	5.7



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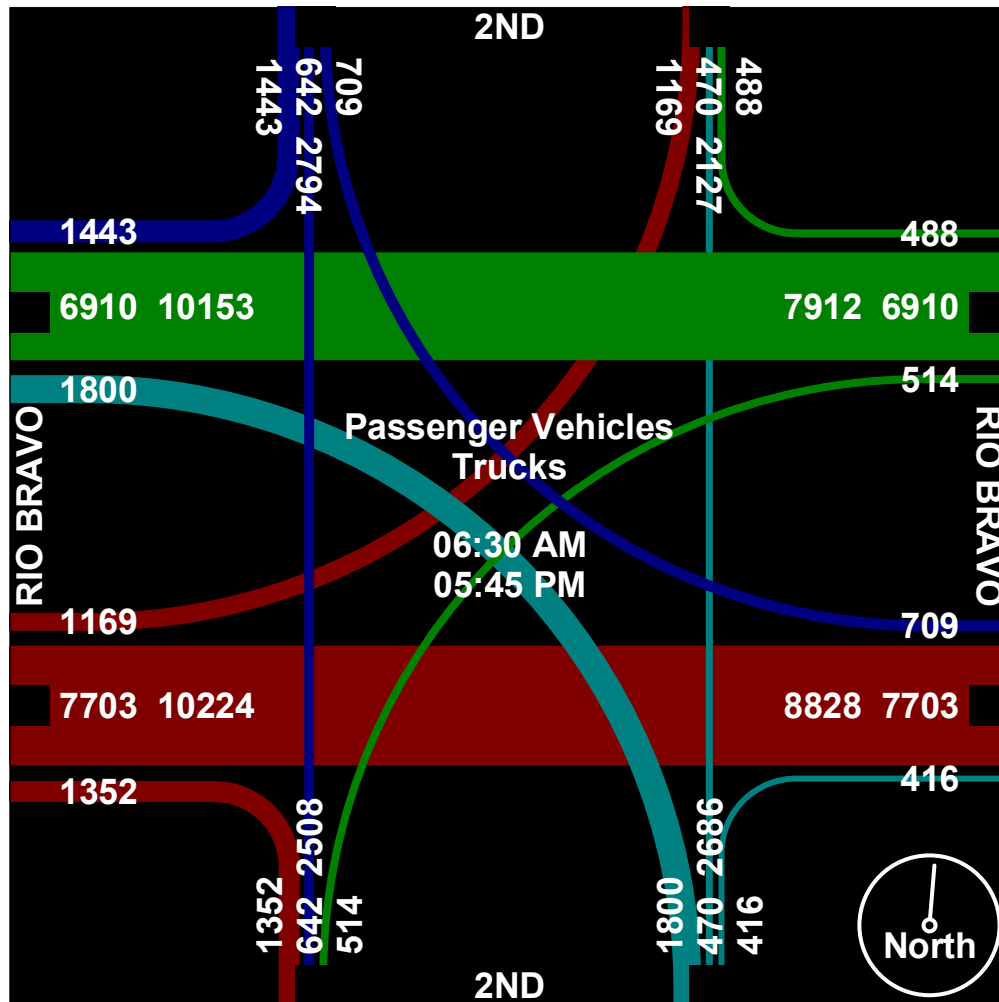
ADVANCED DESIGN

File Name : 2ND-RIO BRAVO_05042021 COMBINED

Site Code : 00000000

Start Date : 4/29/2021

Page No : 3



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Rio Rancho, NM 87124
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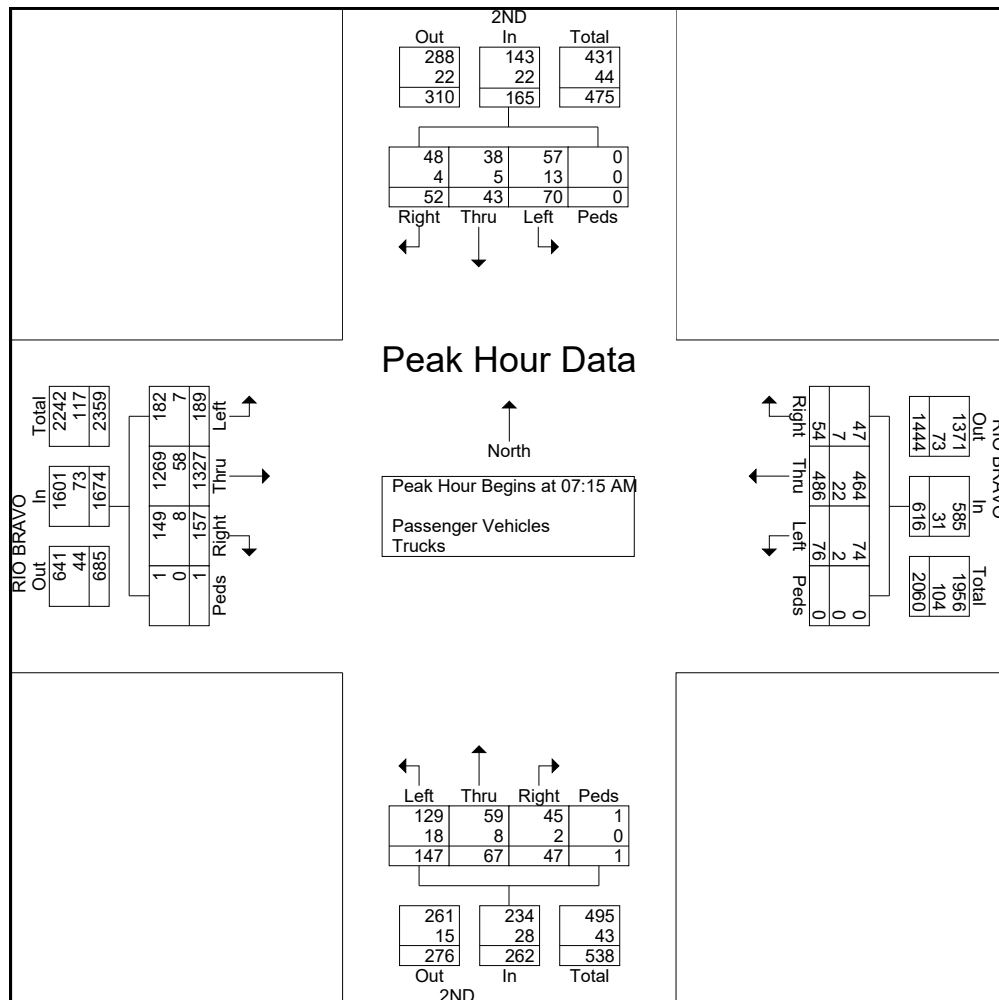
File Name : 2ND-RIO BRAVO_05042021 COMBINED

Site Code : 00000000

Start Date : 4/29/2021

Page No : 4

	2ND From North					RIO BRAVO From East					2ND From South					RIO BRAVO From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 06:30 AM to 09:45 AM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:15 AM																					
07:15 AM	10	4	12	0	26	11	109	14	0	134	13	5	28	1	47	41	331	50	0	422	629
07:30 AM	14	13	19	0	46	10	108	18	0	136	10	31	42	0	83	28	307	51	0	386	651
07:45 AM	10	16	17	0	43	14	141	22	0	177	18	20	43	0	81	42	401	46	1	490	791
08:00 AM	18	10	22	0	50	19	128	22	0	169	6	11	34	0	51	46	288	42	0	376	646
Total Volume	52	43	70	0	165	54	486	76	0	616	47	67	147	1	262	157	1327	189	1	1674	2717
% App. Total	31.5	26.1	42.4	0		8.8	78.9	12.3	0		17.9	25.6	56.1	0.4		9.4	79.3	11.3	0.1		
PHF	.722	.672	.795	.000	.825	.711	.862	.864	.000	.870	.653	.540	.855	.250	.789	.853	.827	.926	.250	.854	.859
Passenger Vehicles	48	38	57	0	143	47	464	74	0	585	45	59	129	1	234	149	1269	182	1	1601	2563
% Passenger Vehicles	92.3	88.4	81.4	0	86.7	87.0	95.5	97.4	0	95.0	95.7	88.1	87.8	100	89.3	94.9	95.6	96.3	100	95.6	94.3
Trucks	4	5	13	0	22	7	22	2	0	31	2	8	18	0	28	8	58	7	0	73	154
% Trucks	7.7	11.6	18.6	0	13.3	13.0	4.5	2.6	0	5.0	4.3	11.9	12.2	0	10.7	5.1	4.4	3.7	0	4.4	5.7



Huitt-Zollars, Inc.

333 Rio Rancho Drive NW, Suite 101

Rio Rancho, NM 87124

ADVANCEDESIGN

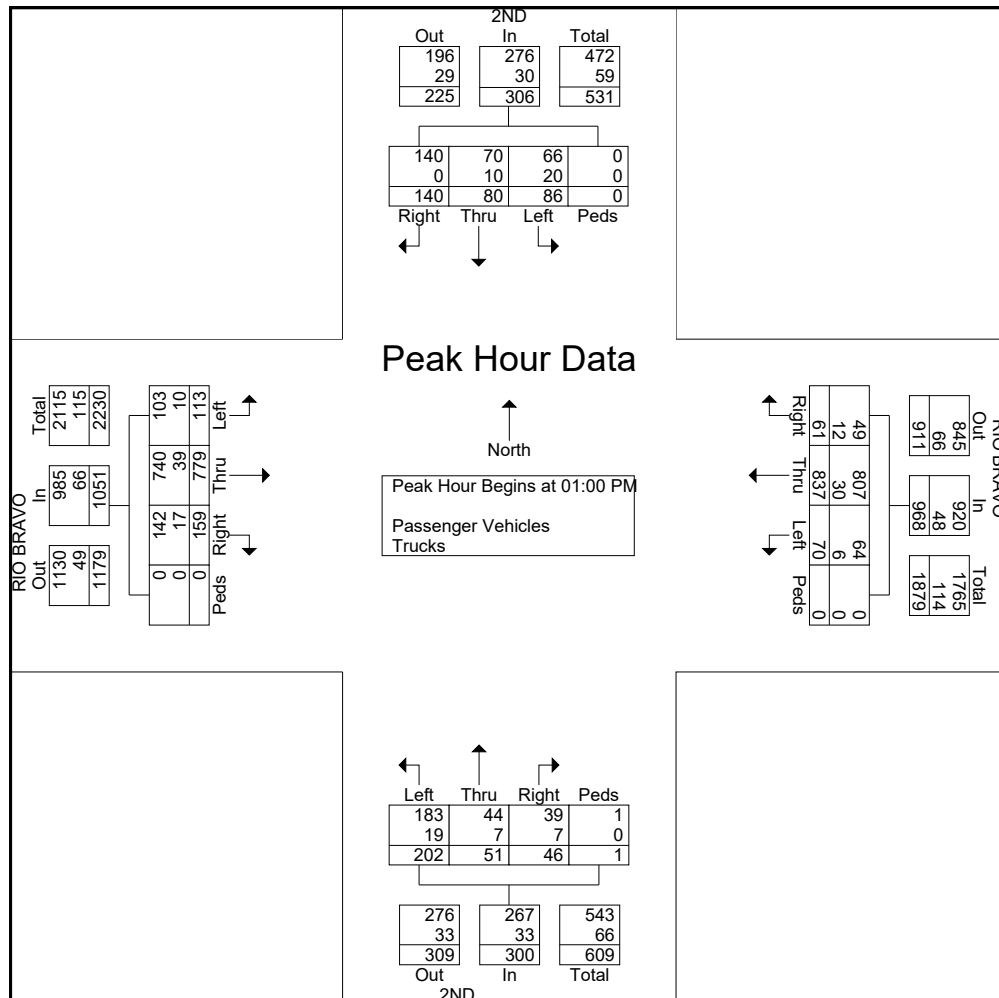
File Name : 2ND-RIO BRAVO_05042021 COMBINED

Site Code : 00000000

Start Date : 4/29/2021

Page No : 5

	2ND From North					RIO BRAVO From East					2ND From South					RIO BRAVO From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 10:00 AM to 01:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 01:00 PM																					
01:00 PM	26	18	18	0	62	17	227	16	0	260	7	14	51	1	73	35	195	28	0	258	653
01:15 PM	41	18	21	0	80	20	215	25	0	260	10	12	46	0	68	47	190	28	0	265	673
01:30 PM	31	20	18	0	69	14	187	14	0	215	17	12	52	0	81	43	200	26	0	269	634
01:45 PM	42	24	29	0	95	10	208	15	0	233	12	13	53	0	78	34	194	31	0	259	665
Total Volume	140	80	86	0	306	61	837	70	0	968	46	51	202	1	300	159	779	113	0	1051	2625
% App. Total	45.8	26.1	28.1	0		6.3	86.5	7.2	0		15.3	17	67.3	0.3		15.1	74.1	10.8	0		
PHF	.833	.833	.741	.000	.805	.763	.922	.700	.000	.931	.676	.911	.953	.250	.926	.846	.974	.911	.000	.977	.975
Passenger Vehicles	140	70	66	0	276	49	807	64	0	920	39	44	183	1	267	142	740	103	0	985	2448
% Passenger Vehicles	100	87.5	76.7	0	90.2	80.3	96.4	91.4	0	95.0	84.8	86.3	90.6	100	89.0	89.3	95.0	91.2	0	93.7	93.3
Trucks	0	10	20	0	30	12	30	6	0	48	7	7	19	0	33	17	39	10	0	66	177
% Trucks	0	12.5	23.3	0	9.8	19.7	3.6	8.6	0	5.0	15.2	13.7	9.4	0	11.0	10.7	5.0	8.8	0	6.3	6.7



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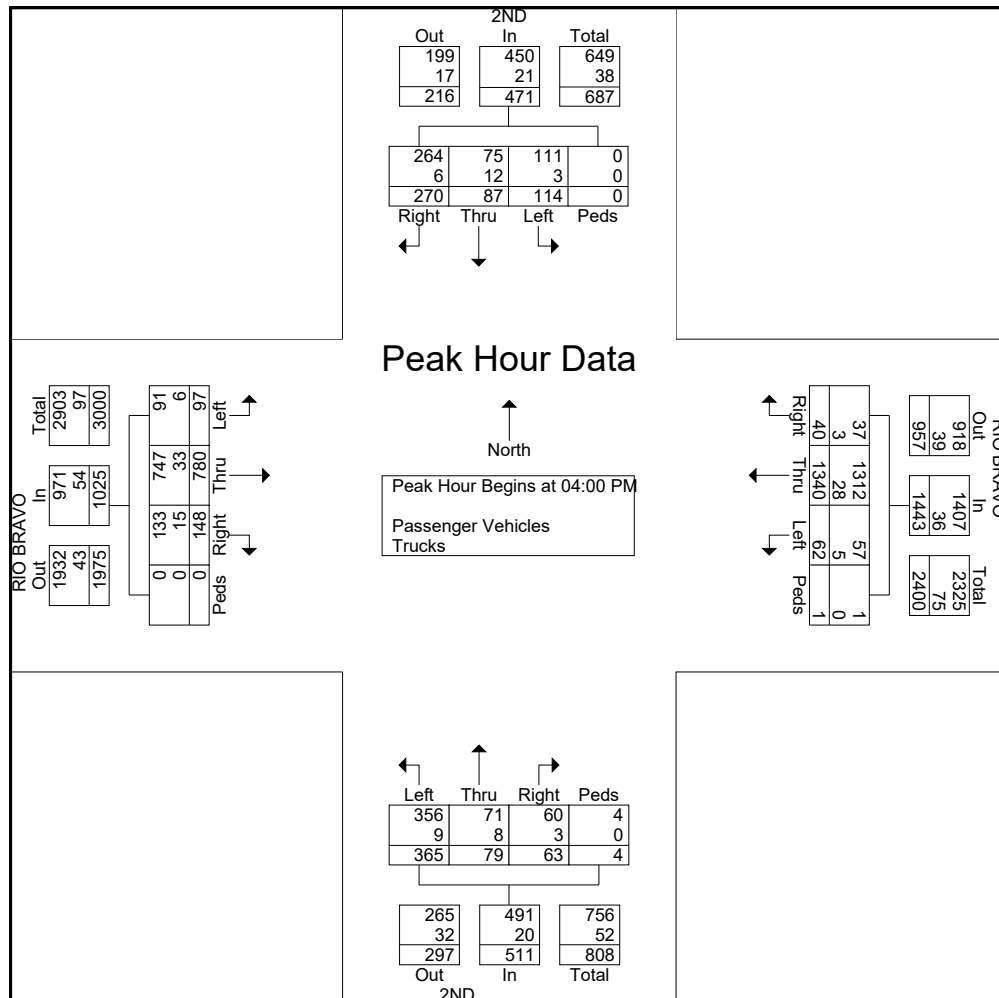
File Name : 2ND-RIO BRAVO_05042021 COMBINED

Site Code : 00000000

Start Date : 4/29/2021

Page No : 6

	2ND From North					RIO BRAVO From East					2ND From South					RIO BRAVO From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From 02:00 PM to 05:45 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:00 PM																					
04:00 PM	66	24	28	0	118	7	296	10	0	313	21	17	99	2	139	37	195	21	0	253	823
04:15 PM	50	17	28	0	95	9	379	13	0	401	13	22	104	0	139	34	218	24	0	276	911
04:30 PM	89	28	35	0	152	15	308	28	1	352	12	12	83	0	107	37	187	24	0	248	859
04:45 PM	65	18	23	0	106	9	357	11	0	377	17	28	79	2	126	40	180	28	0	248	857
Total Volume	270	87	114	0	471	40	1340	62	1	1443	63	79	365	4	511	148	780	97	0	1025	3450
% App. Total	57.3	18.5	24.2	0		2.8	92.9	4.3	0.1		12.3	15.5	71.4	0.8		14.4	76.1	9.5	0		
PHF	.758	.777	.814	.000	.775	.667	.884	.554	.250	.900	.750	.705	.877	.500	.919	.925	.894	.866	.000	.928	.947
Passenger Vehicles	264	75	111	0	450	37	1312	57	1	1407	60	71	356	4	491	133	747	91	0	971	3319
% Passenger Vehicles	97.8	86.2	97.4	0	95.5	92.5	97.9	91.9	100	97.5	95.2	89.9	97.5	100	96.1	89.9	95.8	93.8	0	94.7	96.2
Trucks	6	12	3	0	21	3	28	5	0	36	3	8	9	0	20	15	33	6	0	54	131
% Trucks	2.2	13.8	2.6	0	4.5	7.5	2.1	8.1	0	2.5	4.8	10.1	2.5	0	3.9	10.1	4.2	6.2	0	5.3	3.8



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CRASH DATA

2017-2019



NMDOT COVID TRAFFIC DATA CALIBRATION METHODOLOGY

October 2020





SUBJECT: Alternative methods for Traffic Counts

DATE: October 5, 2020

To: David Quintana, Chief Engineer

From: Afshin Jian, State Traffic Engineer *Afshin Jian*
New Mexico Department of Transportation

**Michelle Lujan
Grisham**
Governor

Michael R. Sandoval
Cabinet Secretary

Alternative Means to Develop Base Turning Movements Volumes for Traffic Impact Studies During COVID-19 Times:

Since February 2020 Governmental policies and social attitudes due to the COVID-19 crisis have impacted traffic volumes and traffic patterns during the AM, Noon, and PM Peak Hour periods. Therefore, traffic counts during this period are not representative of “normal” vehicular traffic volume or patterns. A memo was distributed for guidance on 5/1/2020. To provide more guidance to develop traffic counts and continue development within the State of New Mexico, alternative methods of generating base Turning Movements Volumes (or turning movement counts (TMC)) for Traffic Impact Studies have been developed using recent data and data generated from Big Data models. The “Big Data” models generate traffic counts from anonymized location record from smart phones and other GPS devices. Following are three alternative methods of developing base turning movements volumes based on the levels of data that might be available for any given intersection.

Method 1 – Use Recent Turning Movement Data

Recent pre-COVID19 traffic counts are the preferred data source since in most cases the data is still representative of normal traffic conditions and it provides turning movement volumes, not just approach volumes. The New Mexico Department of Transportation has allowed turning movements volumes up to four years old to be utilized as base Turning Movements Volumes for Traffic Impact Studies. Valid data collected is between September 2016 and February, 2020.

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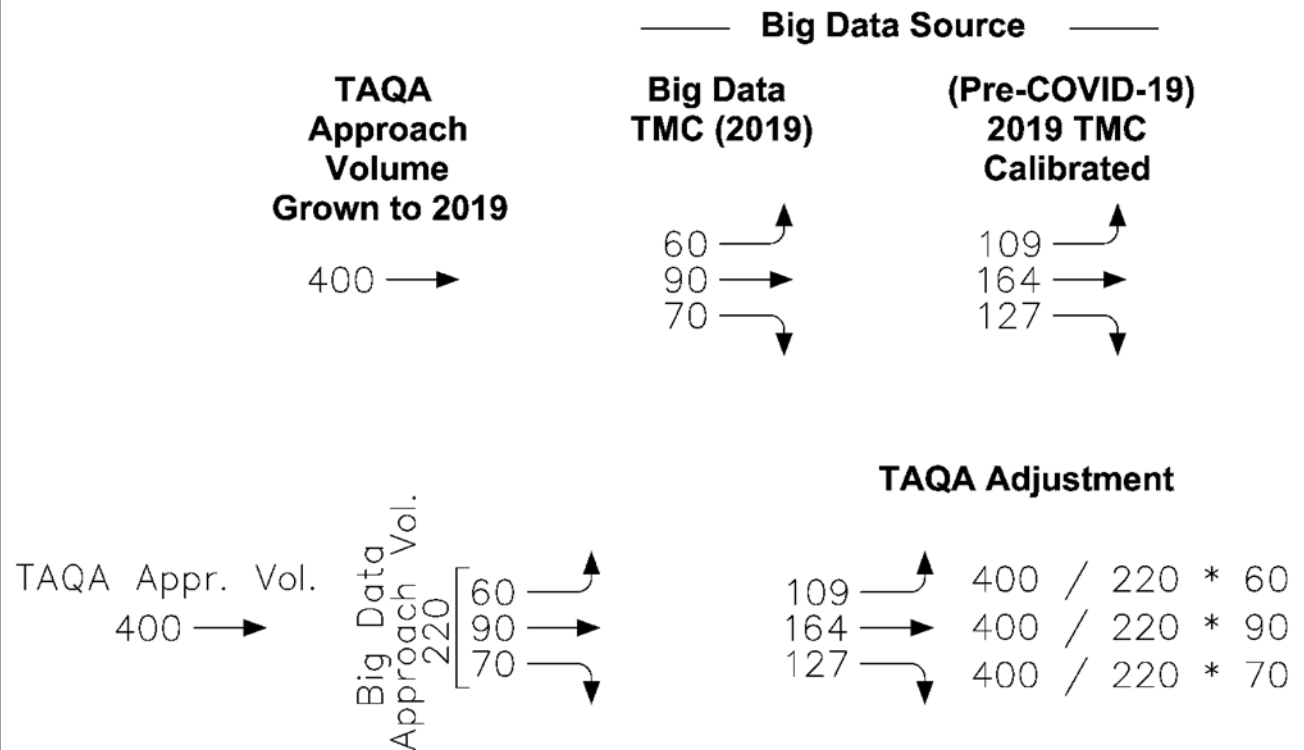
Charles Lundstrom
Commissioner, Secretary
District 6

Method 2 – Use Current Big Data Calibrated with Tube Count Data

For Intersections where there is no recent turning movements volumes data, recent tube count data may be available at most or all legs. This is the case for most of the Mid-Region Council of Governments area on major streets intersection from Mid-Region Council of Governments' (MRCOG) Transportation Analysis and Querying Application (TAQA) website. This case might not be available in rest of the state that is not under MRCOG. Tube counts provide approach volumes and departure volumes but do not provide turning movement volumes. The Big Data can be utilized to approximate raw turning movements volumes at these intersections which can be calibrated with recent TAQA data. This method calibrates the turning movements volumes at the intersection to comply with TAQA approach volumes, but does not account for the changes that may occur in traffic patterns (i.e., proportions of left, thru, and right turns) as a result of the temporarily changed traffic conditions. To adjust turning movements volumes at the intersection to account for changed traffic patterns, it is proposed to use Big Data to develop a comparative scenario to establish a turning movements volumes ratio approximating that of pre-COVID-19 turning movements volumes. The pre-COVID-19 ratio of the turning movements volumes for each approach to an intersection can be utilized to re-allocate the left / thru / right volumes at each approach of an intersection to correlate with pre-COVID-19 traffic patterns. The following page demonstrates a generic calculation for a single approach to an intersection which demonstrates the proposed methodology:

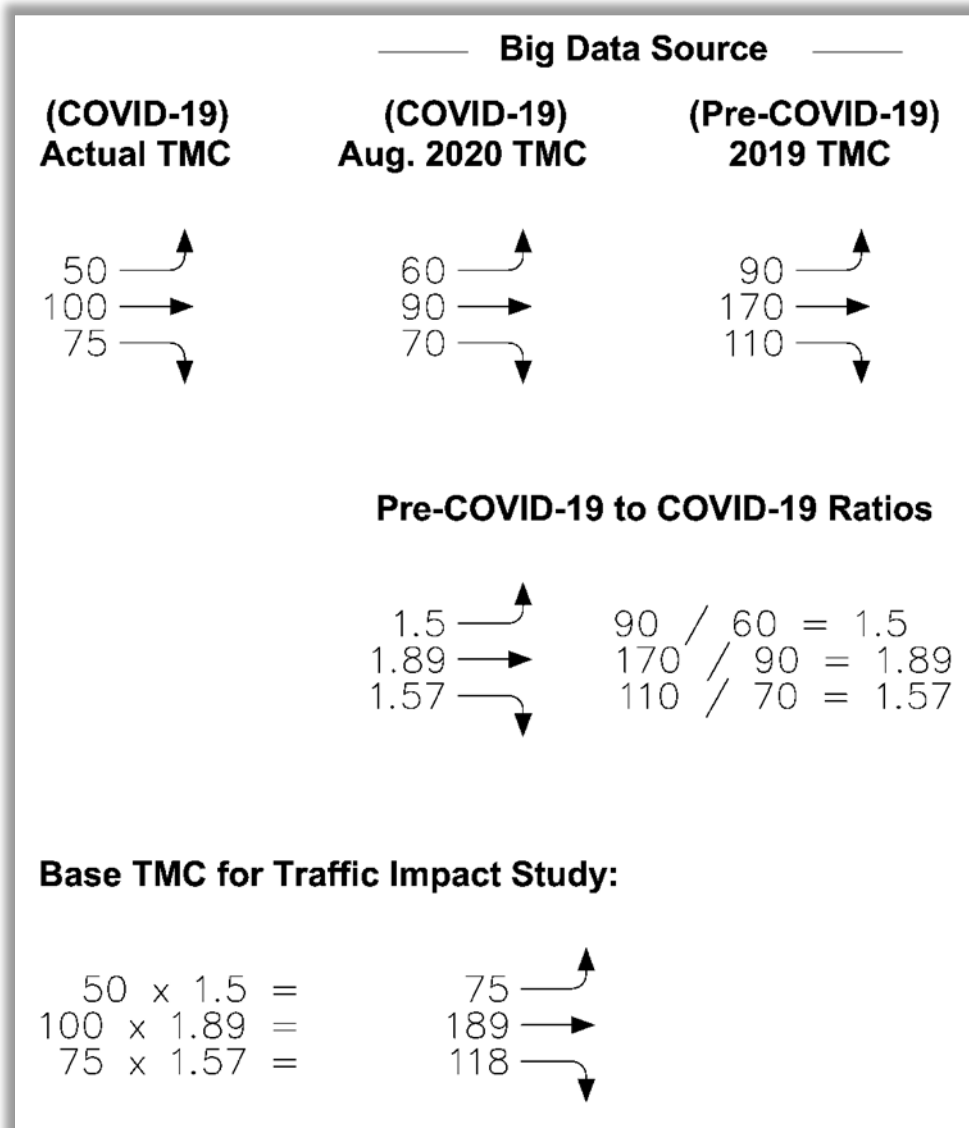
Method 2: User Pre-COVID Big Data Calibrated with Tube Count Data:

Method 2: Use Big Data Calibrated with Tube Count Data



Method 3 – Use Big Data Only and Current COVID-19 TMC Volumes

This method is used for intersections where there is no recent traffic data at all. These intersections are mostly in smaller communities in New Mexico where there is no formal data collection program such as the Mid-Region Council of Governments. In such cases, it is proposed to conduct a current turning movements volumes AM / Noon / PM count as needed to acquire current actual volumes (COVID-19 volumes). Subsequently, acquire two sets of turning movements volumes from an approved Big Data source. First, acquire COVID-19 turning movements volumes for the same month as the current actual traffic count was conducted. Next, acquire pre-COVID-19 turning movements volumes for the same intersection. Subsequently, the ratio of pre-COVID-19 to COVID-19 turning movements volumes (from Big Data) can be applied to adjust the current actual volumes to achieve base turning movements volumes for the Traffic Impact Study. The following page demonstrates a generic calculation for a single approach to an intersection which demonstrates the proposed methodology:



Method 3: Use Big Data Only and Current COVID-19 TMC Volumes:

The major concerns regarding Big Data turning movements volumes are:

- 1) The data is not reported in 15-minute increments. At least one company is working on developing the ability to acquire 15-minute volumes.
- 2) The sampling rate for Big Data is approximately 40%.
- 3) The data from Big Data sources is not considered to be demand volumes.

The proposed methodology addresses those issues as described below:

- 1) The existing current proposed field count will provide 15-minute increment volumes that will be proportioned to approximate pre-COVID-19 conditions.
- 2) The sampling rate becomes a non-issue because by dividing the pre-COVID-19 TMC's from Big Data by the COVID-19 TMC's from Big Data sources, the sampling rate is cancelled because it is the same for both pre-COVID and COVID conditions.
- 3) In cases where TAQA data is available, the TAQA adjustment should allow demand volumes to be achieved for the base turning movements volumes. In smaller communities where TAQA type of data is not available, it has been my experience that the adjustments made for demand volumes are not significant (i.e, less than 1% or 2% generally). It seems that adjustments for demand volumes is not as critical at intersections in smaller communities.

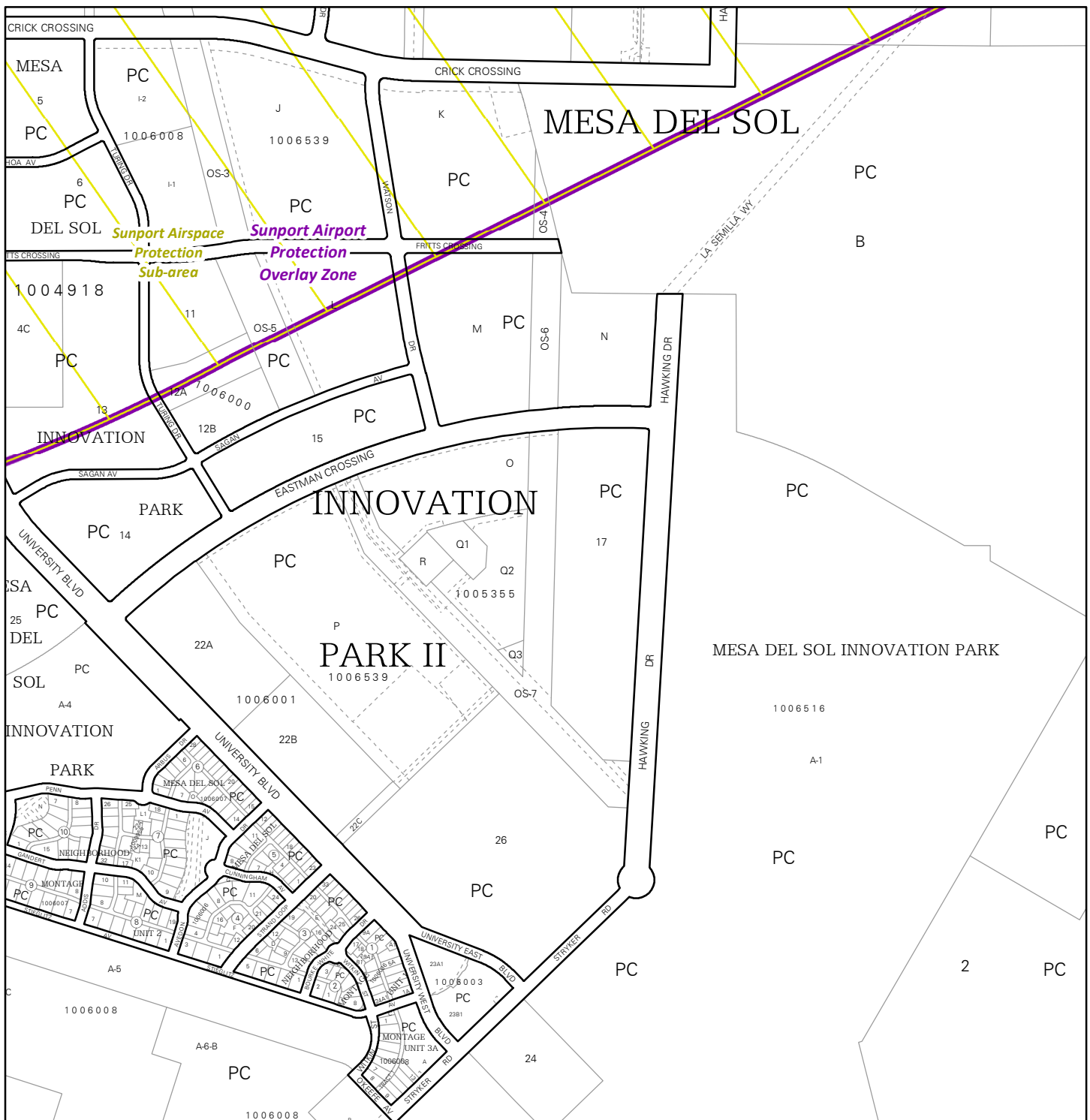
To compensate for any uncertainties in this methodology, it might be prudent to include a safety factor to be added to all of the base turning movements volumes as a general rule. That safety factor would be set and established by the New Mexico Department of Transportation.

In a recent meeting with **Streetlightdata.com** staff, I was informed that there is a four to six week period of time before data would be available on the Streetlightdata.com website application. Therefore, this method, if approved, would still incur a four to six-week delay while awaiting updating / vetting of the Streetlightdata.com data before posting to their website for use by the user.

CABQ ZONE ATLAS

Excerpt



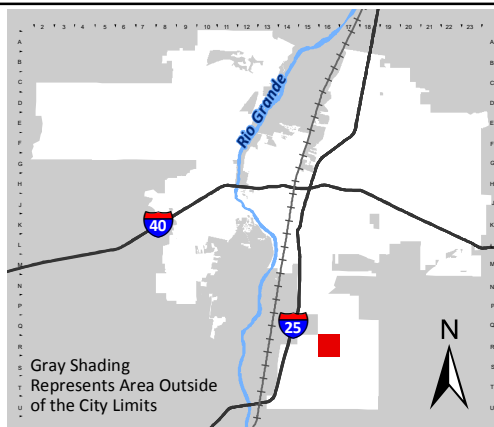


For more details about the Integrated Development Ordinance visit: <http://www.cabq.gov/planning/codes-policies-regulations/integrated-development-ordinance>

IDO Zone Atlas May 2018



IDO Zoning information as of May 17, 2018
The Zone Districts and Overlay Zones
are established by the
Integrated Development Ordinance (IDO).



Zone Atlas Page:
R-16-Z

- Easement
 - Escarpment
 - Petroglyph National Monument
 - Areas Outside of City Limits
 - Airport Protection Overlay (APO) Zone
 - Character Protection Overlay (CPO) Zone
 - Historic Protection Overlay (HPO) Zone
 - View Protection Overlay (VPO) Zone
- Gray Shading Represents Area Outside of the City Limits
- 0 250 500 1,000 Feet

TRAFFIC ANALYSIS DETAIL

Synchro Operational Analyses



2026 No-Project Conditions

AM Peak Hour

HCM 6th TWSC
1: University Blvd SE & Stryker Rd

2026 No-Project
AM Peak Hour

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↱			↰						↰↱	
Traffic Vol, veh/h	0	25	0	0	0	0	0	0	0	37	0	45
Future Vol, veh/h	0	25	0	0	0	0	0	0	0	37	0	45
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	0	31	0	0	0	0	0	0	0	46	0	56
Major/Minor	Minor2			Minor1			Major2					
Conflicting Flow All	-	120	28	108	148	-	0			0	0	0
Stage 1	-	120	-	0	0	-	-			-	-	-
Stage 2	-	0	-	108	148	-	-			-	-	-
Critical Hdwy	-	6.6	7	7.6	6.6	-	4.2			-	-	-
Critical Hdwy Stg 1	-	5.6	-	-	-	-	-			-	-	-
Critical Hdwy Stg 2	-	-	-	6.6	5.6	-	-			-	-	-
Follow-up Hdwy	-	4.05	3.35	3.55	4.05	-	2.25			-	-	-
Pot Cap-1 Maneuver	0	763	1031	851	736	0	-			-	-	-
Stage 1	0	788	-	-	-	0	-			-	-	-
Stage 2	0	-	-	877	767	0	-			-	-	-
Platoon blocked, %										-	-	-
Mov Cap-1 Maneuver	-	763	1031	825	736	-	-			-	-	-
Mov Cap-2 Maneuver	-	763	-	825	736	-	-			-	-	-
Stage 1	-	788	-	-	-	-	-			-	-	-
Stage 2	-	-	-	843	767	-	-			-	-	-
Approach	EB			WB			SB					
HCM Control Delay, s	9.9			0								
HCM LOS	A			A								
Minor Lane/Major Mvmt	EBLn1WBLn1			SBL	SBT	SBR						
Capacity (veh/h)	763			-	-	-						
HCM Lane V/C Ratio	0.04			-	-	-						
HCM Control Delay (s)	9.9			0	-	-						
HCM Lane LOS	A			A	-	-						
HCM 95th %tile Q(veh)	0.1			-	-	-						

HCM 6th TWSC
2: University Blvd SE & Stryker Rd

2026 No-Project
AM Peak Hour

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↰			↱			↰↱				
Traffic Vol, veh/h	61	0	1	0	0	0	0	1	0	0	0	0
Future Vol, veh/h	61	0	1	0	0	0	0	1	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	16965	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	73	73	73	73	73	73
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	84	0	1	0	0	0	0	1	0	0	0	0
Major/Minor	Minor2			Minor1			Major1					
Conflicting Flow All	1	1	0	-	1	1	0	0	0			
Stage 1	0	0	-	-	1	-	-	-	-			
Stage 2	1	1	-	-	0	-	-	-	-			
Critical Hdwy	7.15	6.55	6.25	-	6.55	6.25	4.15	-	-			
Critical Hdwy Stg 1	-	-	-	-	5.55	-	-	-	-			
Critical Hdwy Stg 2	6.15	5.55	-	-	-	-	-	-	-			
Follow-up Hdwy	3.545	4.045	3.345	-	4.045	3.345	2.245	-	-			
Pot Cap-1 Maneuver	1014	889	-	0	889	1075	-	-	-			
Stage 1	-	-	-	0	889	-	-	-	-			
Stage 2	1014	889	-	0	-	-	-	-	-			
Platoon blocked, %												
Mov Cap-1 Maneuver	1014	889	-	-	889	1075	-	-	-			
Mov Cap-2 Maneuver	1014	889	-	-	889	-	-	-	-			
Stage 1	-	-	-	-	889	-	-	-	-			
Stage 2	1014	889	-	-	-	-	-	-	-			
Approach	EB			WB			NB					
HCM Control Delay, s				0			0					
HCM LOS	-			A								
Minor Lane/Major Mvmt	NBL			NBT			NBR			EBLn1WBLn1		
Capacity (veh/h)	-			-			-			-		
HCM Lane V/C Ratio	-			-			-			-		
HCM Control Delay (s)	0			-			-			0		
HCM Lane LOS	A			-			-			A		
HCM 95th %tile Q(veh)	-			-			-			-		

HCM 6th TWSC
3: University Blvd SE & Strand Loop SE/Gate A

2026 No-Project
AM Peak Hour

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	31	0	10	0	0	24	19	42	1	84	72	40
Future Vol, veh/h	31	0	10	0	0	24	19	42	1	84	72	40
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	-	-	0	130	-	-	115	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	35	0	11	0	0	27	21	47	1	94	81	45

Major/Minor	Minor2		Minor1		Major1		Major2	
Conflicting Flow All	358	382	63	319	404	24	126	0
Stage 1	292	292	-	90	90	-	-	-
Stage 2	66	90	-	229	314	-	-	-
Critical Hdwy	7.6	6.6	7	7.6	6.6	7	4.2	-
Critical Hdwy Stg 1	6.6	5.6	-	6.6	5.6	-	-	-
Critical Hdwy Stg 2	6.6	5.6	-	6.6	5.6	-	-	-
Follow-up Hdwy	3.55	4.05	3.35	3.55	4.05	3.35	2.25	-
Pot Cap-1 Maneuver	565	543	979	603	527	1037	1436	-
Stage 1	683	662	-	899	813	-	-	-
Stage 2	928	813	-	744	647	-	-	-
Platoon blocked, %								-
Mov Cap-1 Maneuver	519	502	979	562	487	1037	1436	-
Mov Cap-2 Maneuver	519	502	-	562	487	-	-	-
Stage 1	673	622	-	886	801	-	-	-
Stage 2	891	801	-	690	608	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	11.5	8.6	2.3	3.2
HCM LOS	B	A		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1436	-	-	519	979	-	1037	1536	-	-
HCM Lane V/C Ratio	0.015	-	-	0.067	0.011	-	0.026	0.061	-	-
HCM Control Delay (s)	7.5	-	-	12.4	8.7	0	8.6	7.5	-	-
HCM Lane LOS	A	-	-	B	A	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.2	0	-	0.1	0.2	-	-

HCM 6th TWSC
4: University Blvd SE & Avedon Ave SE

2026 No-Project
AM Peak Hour

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	20	11	19	133	232	25
Future Vol, veh/h	20	11	19	133	232	25
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	125	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	89	89	89	89	89	89
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	22	12	21	149	261	28
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	392	145	289	0	-	0
Stage 1	275	-	-	-	-	-
Stage 2	117	-	-	-	-	-
Critical Hdwy	6.9	7	4.2	-	-	-
Critical Hdwy Stg 1	5.9	-	-	-	-	-
Critical Hdwy Stg 2	5.9	-	-	-	-	-
Follow-up Hdwy	3.55	3.35	2.25	-	-	-
Pot Cap-1 Maneuver	577	867	1248	-	-	-
Stage 1	738	-	-	-	-	-
Stage 2	886	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	567	867	1248	-	-	-
Mov Cap-2 Maneuver	567	-	-	-	-	-
Stage 1	725	-	-	-	-	-
Stage 2	886	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	10.9	1		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1248	-	646	-	-	
HCM Lane V/C Ratio	0.017	-	0.054	-	-	
HCM Control Delay (s)	7.9	-	10.9	-	-	
HCM Lane LOS	A	-	B	-	-	
HCM 95th %tile Q(veh)	0.1	-	0.2	-	-	

Lanes, Volumes, Timings
5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

2026 No-Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	17	100	8	50	80	99	100	2	147	175	50
Future Volume (vph)	20	17	100	8	50	80	99	100	2	147	175	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	200		200	325		0	325		325
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	1770	3529	0	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	1770	3529	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			176			176			3			176
Link Speed (mph)		40			35			35			35	
Link Distance (ft)		1786			774			1252			1357	
Travel Time (s)		30.4			15.1			24.4			26.4	
Peak Hour Factor	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	28	24	139	11	69	111	138	142	0	204	243	69
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8						6
Total Split (s)	9.5	22.5	22.5	9.5	22.5	22.5	10.4	22.6		10.4	22.6	22.6
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	5.2	6.5	6.5	5.2	6.5	6.5	6.2	13.4		6.2	9.3	9.3
Actuated g/C Ratio	0.15	0.19	0.19	0.15	0.19	0.19	0.18	0.39		0.18	0.27	0.27
v/c Ratio	0.11	0.04	0.32	0.04	0.10	0.25	0.44	0.10		0.33	0.26	0.12
Control Delay	17.4	14.5	4.7	17.0	14.5	3.0	23.4	12.1		16.7	12.8	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	17.4	14.5	4.7	17.0	14.5	3.0	23.4	12.1		16.7	12.8	0.5
LOS	B	B	A	B	B	A	C	B		B	B	A
Approach Delay		7.8			8.0			17.7			12.7	
Approach LOS		A			A			B			B	

Intersection Summary

Area Type: Other

Cycle Length: 65

Actuated Cycle Length: 34.7

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.44

Intersection Signal Delay: 12.3

Intersection LOS: B

Intersection Capacity Utilization 29.3%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

			
Ø1	Ø2	Ø3	Ø4
10.4 s	22.6 s	9.5 s	22.5 s
			
Ø5	Ø6	Ø7	Ø8
10.4 s	22.6 s	9.5 s	22.5 s

Synchro 10 Report

HCM 6th TWSC
6: University Blvd SE & Fritts Crossing SE

2026 No-Project
AM Peak Hour

Intersection						
Int Delay, s/veh	2.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	15	109	199	10	70	334
Future Vol, veh/h	15	109	199	10	70	334
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	120	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	18	131	240	12	84	402
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	816	246	0	0	252	0
Stage 1	246	-	-	-	-	-
Stage 2	570	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	342	785	-	-	1296	-
Stage 1	788	-	-	-	-	-
Stage 2	560	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	320	785	-	-	1296	-
Mov Cap-2 Maneuver	320	-	-	-	-	-
Stage 1	788	-	-	-	-	-
Stage 2	524	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.9		0		1.4	
HCM LOS	B					
Minor Lane/Major Mvmt		NBT	NBRWBLn1		SBL	SBT
Capacity (veh/h)		-	-	668	1296	-
HCM Lane V/C Ratio		-	-	0.224	0.065	-
HCM Control Delay (s)		-	-	11.9	8	-
HCM Lane LOS		-	-	B	A	-
HCM 95th %tile Q(veh)		-	-	0.9	0.2	-

HCM 6th TWSC
7: University Blvd SE & Crick Ave SE

2026 No-Project
AM Peak Hour

Intersection						
Int Delay, s/veh	2.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	10	114	236	30	97	390
Future Vol, veh/h	10	114	236	30	97	390
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	180	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	12	139	288	37	118	476
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	1019	307	0	0	325	0
Stage 1	307	-	-	-	-	-
Stage 2	712	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	259	726	-	-	1218	-
Stage 1	739	-	-	-	-	-
Stage 2	481	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	234	726	-	-	1218	-
Mov Cap-2 Maneuver	234	-	-	-	-	-
Stage 1	739	-	-	-	-	-
Stage 2	434	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	11.9	0	1.6			
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 234 726 1218	-	-		
HCM Lane V/C Ratio	-	- 0.052 0.191 0.097	-	-		
HCM Control Delay (s)	-	- 21.2 11.1 8.3	-	-		
HCM Lane LOS	-	- C B A	-	-		
HCM 95th %tile Q(veh)	-	- 0.2 0.7 0.3	-	-		

2026 No-Project Conditions

PM Peak Hour

HCM 6th TWSC
1: University Blvd SE & Stryker Rd

2026 No-Project
PM Peak Hour

Intersection													
Int Delay, s/veh	4												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↱			↰					↰↱			
Traffic Vol, veh/h	0	23	0	0	23	0	0	0	0	47	1	20	
Future Vol, veh/h	0	23	0	0	23	0	0	0	0	47	1	20	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5	
Mvmt Flow	0	26	0	0	26	0	0	0	0	52	1	22	
Major/Minor	Minor2		Minor1				Major2						
Conflicting Flow All	-	116	12	118	127	-				0	0	0	
Stage 1	-	116	-	0	0	-				-	-	-	
Stage 2	-	0	-	118	127	-				-	-	-	
Critical Hdwy	-	6.6	7	7.6	6.6	-				4.2	-	-	
Critical Hdwy Stg 1	-	5.6	-	-	-	-				-	-	-	
Critical Hdwy Stg 2	-	-	-	6.6	5.6	-				-	-	-	
Follow-up Hdwy	-	4.05	3.35	3.55	4.05	-				2.25	-	-	
Pot Cap-1 Maneuver	0	767	1056	837	756	0				-	-	-	
Stage 1	0	792	-	-	-	0				-	-	-	
Stage 2	0	-	-	865	783	0				-	-	-	
Platoon blocked, %											-	-	
Mov Cap-1 Maneuver	-	767	1056	816	756	-				-	-	-	
Mov Cap-2 Maneuver	-	767	-	816	756	-				-	-	-	
Stage 1	-	792	-	-	-	-				-	-	-	
Stage 2	-	-	-	837	783	-				-	-	-	
Approach	EB		WB				SB						
HCM Control Delay, s	9.9		9.9										
HCM LOS	A		A										
Minor Lane/Major Mvmt	EBLn1WBLn1		SBL	SBT	SBR								
Capacity (veh/h)	767		756	-	-	-							
HCM Lane V/C Ratio	0.033		0.034	-	-	-							
HCM Control Delay (s)	9.9		9.9	-	-	-							
HCM Lane LOS	A		A	-	-	-							
HCM 95th %tile Q(veh)	0.1		0.1	-	-	-							

HCM 6th TWSC
2: University Blvd SE & Stryker Rd

2026 No-Project
PM Peak Hour

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕				
Traffic Vol, veh/h	70	0	0	0	0	0	0	0	0	0	0	0
Future Vol, veh/h	70	0	0	0	0	0	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	16965	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	73	73	73	73	73	73
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	96	0	0	0	0	0	0	0	0	0	0	0
Major/Minor	Minor2		Minor1		Major1							
Conflicting Flow All	0	0	-	-	0	0	0	0	0			
Stage 1	0	0	-	-	0	-	-	-	-			
Stage 2	0	0	-	-	0	-	-	-	-			
Critical Hdwy	7.15	6.55	-	-	6.55	6.25	4.15	-	-			
Critical Hdwy Stg 1	-	-	-	-	5.55	-	-	-	-			
Critical Hdwy Stg 2	6.15	5.55	-	-	-	-	-	-	-			
Follow-up Hdwy	3.545	4.045	-	-	4.045	3.345	2.245	-	-			
Pot Cap-1 Maneuver	-	-	0	0	-	-	-	-	-			
Stage 1	-	-	0	0	-	-	-	-	-			
Stage 2	-	-	0	0	-	-	-	-	-			
Platoon blocked, %									-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	-	-	-	-			
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	-			
Stage 1	-	-	-	-	-	-	-	-	-			
Stage 2	-	-	-	-	-	-	-	-	-			
Approach	EB		WB		NB							
HCM Control Delay, s					0		0					
HCM LOS	-				A							
Minor Lane/Major Mvmt	NBL		NBT		NBR		EBLn1WBLn1					
Capacity (veh/h)	-		-		-		-					
HCM Lane V/C Ratio	-		-		-		-					
HCM Control Delay (s)	0		-		-		0					
HCM Lane LOS	A		-		-		A					
HCM 95th %tile Q(veh)	-		-		-		-					

HCM 6th TWSC
3: University Blvd SE & Strand Loop SE/Gate A

2026 No-Project
PM Peak Hour

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	64	0	25	1	0	75	12	54	1	34	57	48
Future Vol, veh/h	64	0	25	1	0	75	12	54	1	34	57	48
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	-	-	0	130	-	-	115	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	67	0	26	1	0	79	13	57	1	36	60	51
Major/Minor	Minor2		Minor1		Major1		Major2		Major2		Major2	
Conflicting Flow All	213	242	56	186	267	29	111	0	0	58	0	0
Stage 1	158	158	-	84	84	-	-	-	-	-	-	-
Stage 2	55	84	-	102	183	-	-	-	-	-	-	-
Critical Hdwy	7.6	6.6	7	7.6	6.6	7	4.2	-	-	4.2	-	-
Critical Hdwy Stg 1	6.6	5.6	-	6.6	5.6	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.6	5.6	-	6.6	5.6	-	-	-	-	-	-	-
Follow-up Hdwy	3.55	4.05	3.35	3.55	4.05	3.35	2.25	-	-	2.25	-	-
Pot Cap-1 Maneuver	717	652	989	749	631	1029	1455	-	-	1523	-	-
Stage 1	820	759	-	906	817	-	-	-	-	-	-	-
Stage 2	942	817	-	884	740	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	645	630	989	711	610	1029	1455	-	-	1523	-	-
Mov Cap-2 Maneuver	645	630	-	711	610	-	-	-	-	-	-	-
Stage 1	813	741	-	898	810	-	-	-	-	-	-	-
Stage 2	862	810	-	840	722	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB		SB		SB	
HCM Control Delay, s	10.5		8.8		1.3		1.8		1.8		1.8	
HCM LOS	B		A									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)	1455	-	-	645	989	711	1029	1523	-	-		
HCM Lane V/C Ratio	0.009	-	-	0.104	0.027	0.001	0.077	0.023	-	-		
HCM Control Delay (s)	7.5	-	-	11.2	8.7	10.1	8.8	7.4	-	-		
HCM Lane LOS	A	-	-	B	A	B	A	A	-	-		
HCM 95th %tile Q(veh)	0	-	-	0.3	0.1	0	0.2	0.1	-	-		

HCM 6th TWSC
4: University Blvd SE & Avedon Ave SE

2026 No-Project
PM Peak Hour

Intersection						
Int Delay, s/veh	1.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	30	21	10	209	118	23
Future Vol, veh/h	30	21	10	209	118	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	125	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	31	22	10	215	122	24
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	262	73	146	0	-	0
Stage 1	134	-	-	-	-	-
Stage 2	128	-	-	-	-	-
Critical Hdwy	6.9	7	4.2	-	-	-
Critical Hdwy Stg 1	5.9	-	-	-	-	-
Critical Hdwy Stg 2	5.9	-	-	-	-	-
Follow-up Hdwy	3.55	3.35	2.25	-	-	-
Pot Cap-1 Maneuver	696	964	1412	-	-	-
Stage 1	869	-	-	-	-	-
Stage 2	875	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	691	964	1412	-	-	-
Mov Cap-2 Maneuver	691	-	-	-	-	-
Stage 1	863	-	-	-	-	-
Stage 2	875	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	9.9	0.3		0		
HCM LOS	A					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1412	-	782	-	-	
HCM Lane V/C Ratio	0.007	-	0.067	-	-	
HCM Control Delay (s)	7.6	-	9.9	-	-	
HCM Lane LOS	A	-	A	-	-	
HCM 95th %tile Q(veh)	0	-	0.2	-	-	

Lanes, Volumes, Timings
5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

2026 No-Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	73	75	56	5	20	106	120	117	2	89	105	23
Future Volume (vph)	73	75	56	5	20	106	120	117	2	89	105	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	200		200	325		0	325		325
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	1770	3532	0	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	1770	3532	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			176			176			2			176
Link Speed (mph)		40			35			35			35	
Link Distance (ft)		1786			774			1252			1357	
Travel Time (s)		30.4			15.1			24.4			26.4	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	78	81	60	5	22	114	129	128	0	96	113	25
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8						6
Total Split (s)	9.5	22.5	22.5	9.5	22.5	22.5	10.2	23.3		9.7	22.8	22.8
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	5.1	9.7	9.7	5.1	6.1	6.1	5.8	16.1		5.3	10.0	10.0
Actuated g/C Ratio	0.13	0.25	0.25	0.13	0.16	0.16	0.15	0.42		0.14	0.26	0.26
v/c Ratio	0.34	0.09	0.11	0.02	0.04	0.29	0.49	0.09		0.20	0.12	0.05
Control Delay	21.4	12.2	0.4	17.2	15.7	3.5	25.6	13.7		17.7	15.1	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	21.4	12.2	0.4	17.2	15.7	3.5	25.6	13.7		17.7	15.1	0.2
LOS	C	B	A	B	B	A	C	B		B	B	A
Approach Delay		12.3			5.9			19.6			14.6	
Approach LOS		B			A			B			B	

Intersection Summary

Area Type: Other

Cycle Length: 65

Actuated Cycle Length: 38.7

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.49

Intersection Signal Delay: 14.1

Intersection LOS: B

Intersection Capacity Utilization 31.5%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

			
Ø1	Ø2	Ø3	Ø4
9.7 s	23.3 s	9.5 s	22.5 s
			
Ø5	Ø6	Ø7	Ø8
10.2 s	22.8 s	9.5 s	22.5 s

Synchro 10 Report

HCM 6th TWSC
6: University Blvd SE & Fritts Crossing SE

2026 No-Project
PM Peak Hour

Intersection						
Int Delay, s/veh	2.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	15	82	274	34	97	277
Future Vol, veh/h	15	82	274	34	97	277
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	120	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	15	85	282	35	100	286
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	786	300	0	0	317	0
Stage 1	300	-	-	-	-	-
Stage 2	486	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	357	733	-	-	1226	-
Stage 1	745	-	-	-	-	-
Stage 2	612	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	328	733	-	-	1226	-
Mov Cap-2 Maneuver	328	-	-	-	-	-
Stage 1	745	-	-	-	-	-
Stage 2	562	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	12	0		2.1		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	- 615		1226	-	
HCM Lane V/C Ratio	-	- 0.163		0.082	-	
HCM Control Delay (s)	-	- 12		8.2	-	
HCM Lane LOS	-	- B		A	-	
HCM 95th %tile Q(veh)	-	- 0.6		0.3	-	

HCM 6th TWSC
7: University Blvd SE & Crick Ave SE

2026 No-Project
PM Peak Hour

Intersection						
Int Delay, s/veh	3.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	15	130	399	36	124	350
Future Vol, veh/h	15	130	399	36	124	350
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	180	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	19	163	499	45	155	438
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1270	522	0	0	544	0
Stage 1	522	-	-	-	-	-
Stage 2	748	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	183	549	-	-	1010	-
Stage 1	589	-	-	-	-	-
Stage 2	462	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	155	549	-	-	1010	-
Mov Cap-2 Maneuver	155	-	-	-	-	-
Stage 1	589	-	-	-	-	-
Stage 2	391	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	16.1	0		2.4		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	155	549	1010	-
HCM Lane V/C Ratio	-	-	0.121	0.296	0.153	-
HCM Control Delay (s)	-	-	31.4	14.3	9.2	-
HCM Lane LOS	-	-	D	B	A	-
HCM 95th %tile Q(veh)	-	-	0.4	1.2	0.5	-

2026 With Project Conditions

AM Peak Hour

HCM 6th TWSC
1: University Blvd SE & Stryker Rd

2026 With Project
AM Peak Hour

Intersection												
Int Delay, s/veh	1.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↱			↱					↱↲		
Traffic Vol, veh/h	0	25	0	0	10	0	0	0	0	422	0	45
Future Vol, veh/h	0	25	0	0	10	0	0	0	0	422	0	45
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	0	31	0	0	12	0	0	0	0	521	0	56
Major/Minor	Minor2		Minor1				Major2					
Conflicting Flow All	-	1070	28	1058	1098	-				0	0	0
Stage 1	-	1070	-	0	0	-				-	-	-
Stage 2	-	0	-	1058	1098	-				-	-	-
Critical Hdwy	-	6.6	7	7.6	6.6	-				4.2	-	-
Critical Hdwy Stg 1	-	5.6	-	-	-	-				-	-	-
Critical Hdwy Stg 2	-	-	-	6.6	5.6	-				-	-	-
Follow-up Hdwy	-	4.05	3.35	3.55	4.05	-				2.25	-	-
Pot Cap-1 Maneuver	0	215	1031	175	207	0				-	-	-
Stage 1	0	289	-	-	-	0				-	-	-
Stage 2	0	-	-	235	281	0				-	-	-
Platoon blocked, %											-	-
Mov Cap-1 Maneuver	-	215	1031	156	207	-				-	-	-
Mov Cap-2 Maneuver	-	215	-	156	207	-				-	-	-
Stage 1	-	289	-	-	-	-				-	-	-
Stage 2	-	-	-	210	281	-				-	-	-
Approach	EB		WB				SB					
HCM Control Delay, s	24.5		23.5									
HCM LOS	C		C									
Minor Lane/Major Mvmt	EBLn1WBLn1		SBL	SBT	SBR							
Capacity (veh/h)	215 207		-	-	-							
HCM Lane V/C Ratio	0.144 0.06		-	-	-							
HCM Control Delay (s)	24.5 23.5		-	-	-							
HCM Lane LOS	C C		-	-	-							
HCM 95th %tile Q(veh)	0.5 0.2		-	-	-							

HCM 6th TWSC
2: University Blvd SE & Stryker Rd

2026 With Project
AM Peak Hour

Intersection												
Int Delay, s/veh	2.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↰			↱			↰↱				
Traffic Vol, veh/h	29	417	1	0	10	134	0	1	0	0	0	0
Future Vol, veh/h	29	417	1	0	10	134	0	1	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	16965	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	73	73	73	73	73	73
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	40	571	1	0	14	184	0	1	0	0	0	0
Major/Minor	Minor2			Minor1			Major1					
Conflicting Flow All	100	1	0	-	1	1	0	0	0			
Stage 1	0	0	-	-	1	-	-	-	-			
Stage 2	100	1	-	-	0	-	-	-	-			
Critical Hdwy	7.15	6.55	6.25	-	6.55	6.25	4.15	-	-			
Critical Hdwy Stg 1	-	-	-	-	5.55	-	-	-	-			
Critical Hdwy Stg 2	6.15	5.55	-	-	-	-	-	-	-			
Follow-up Hdwy	3.545	4.045	3.345	-	4.045	3.345	2.245	-	-			
Pot Cap-1 Maneuver	874	889	-	0	889	1075	-	-	-			
Stage 1	-	-	-	0	889	-	-	-	-			
Stage 2	899	889	-	0	-	-	-	-	-			
Platoon blocked, %												
Mov Cap-1 Maneuver	716	889	-	-	889	1075	-	-	-			
Mov Cap-2 Maneuver	716	889	-	-	889	-	-	-	-			
Stage 1	-	-	-	-	889	-	-	-	-			
Stage 2	734	889	-	-	-	-	-	-	-			
Approach	EB			WB			NB					
HCM Control Delay, s				9.2			0					
HCM LOS	-			A								
Minor Lane/Major Mvmt	NBL			NBT			NBR			EBLn1WBLn1		
Capacity (veh/h)	-			-			-			1060		
HCM Lane V/C Ratio	-			-			-			0.186		
HCM Control Delay (s)	0			-			-			9.2		
HCM Lane LOS	A			-			-			A		
HCM 95th %tile Q(veh)	-			-			-			0.7		

HCM 6th TWSC
3: University Blvd SE & Strand Loop SE/Gate A

2026 With Project
AM Peak Hour

Intersection												
Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	31	0	10	3	0	7	19	150	13	20	454	40
Future Vol, veh/h	31	0	10	3	0	7	19	150	13	20	454	40
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	-	-	0	130	-	-	115	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	35	0	11	3	0	8	21	169	15	22	510	45

Major/Minor	Minor2		Minor1		Major1		Major2	
Conflicting Flow All	704	803	278	518	818	92	555	0
Stage 1	577	577	-	219	219	-	-	-
Stage 2	127	226	-	299	599	-	-	-
Critical Hdwy	7.6	6.6	7	7.6	6.6	7	4.2	-
Critical Hdwy Stg 1	6.6	5.6	-	6.6	5.6	-	-	-
Critical Hdwy Stg 2	6.6	5.6	-	6.6	5.6	-	-	-
Follow-up Hdwy	3.55	4.05	3.35	3.55	4.05	3.35	2.25	-
Pot Cap-1 Maneuver	318	310	710	434	304	938	991	-
Stage 1	462	492	-	755	713	-	-	-
Stage 2	855	708	-	677	481	-	-	-
Platoon blocked, %								-
Mov Cap-1 Maneuver	307	299	710	415	293	938	991	-
Mov Cap-2 Maneuver	307	299	-	415	293	-	-	-
Stage 1	452	484	-	739	698	-	-	-
Stage 2	830	693	-	656	473	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	16.2	10.3	0.9	0.3
HCM LOS	C	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	991	-	-	307	710	415	938	1367	-	-
HCM Lane V/C Ratio	0.022	-	-	0.113	0.016	0.008	0.008	0.016	-	-
HCM Control Delay (s)	8.7	-	-	18.2	10.2	13.7	8.9	7.7	-	-
HCM Lane LOS	A	-	-	C	B	B	A	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.4	0	0	0	0.1	-	-

HCM 6th TWSC
4: University Blvd SE & Avedon Ave SE/Gate B

2026 With Project
AM Peak Hour

Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕↕			↕	↗	↖	↕↕		↖	↕↕	
Traffic Vol, veh/h	20	0	11	3	0	7	19	259	13	20	579	25
Future Vol, veh/h	20	0	11	3	0	7	19	259	13	20	579	25
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	125	-	-	115	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	22	0	12	3	0	8	21	291	15	22	651	28
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	897	1057	340	711	1064	153	679	0	0	306	0	0
Stage 1	709	709	-	341	341	-	-	-	-	-	-	-
Stage 2	188	348	-	370	723	-	-	-	-	-	-	-
Critical Hdwy	7.6	6.6	7	7.6	6.6	7	4.2	-	-	4.2	-	-
Critical Hdwy Stg 1	6.6	5.6	-	6.6	5.6	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.6	5.6	-	6.6	5.6	-	-	-	-	-	-	-
Follow-up Hdwy	3.55	4.05	3.35	3.55	4.05	3.35	2.25	-	-	2.25	-	-
Pot Cap-1 Maneuver	230	219	647	314	217	856	889	-	-	1230	-	-
Stage 1	384	428	-	639	630	-	-	-	-	-	-	-
Stage 2	787	625	-	614	422	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	221	210	647	298	208	856	889	-	-	1230	-	-
Mov Cap-2 Maneuver	221	210	-	298	208	-	-	-	-	-	-	-
Stage 1	375	420	-	624	615	-	-	-	-	-	-	-
Stage 2	761	610	-	591	414	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	19.2		11.6		0.6		0.3					
HCM LOS	C		B									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1WBLn2	SBL	SBT	SBR					
Capacity (veh/h)	889	-	-	288	298	856	1230	-	-			
HCM Lane V/C Ratio	0.024	-	-	0.121	0.011	0.009	0.018	-	-			
HCM Control Delay (s)	9.1	-	-	19.2	17.2	9.2	8	-	-			
HCM Lane LOS	A	-	-	C	C	A	A	-	-			
HCM 95th %tile Q(veh)	0.1	-	-	0.4	0	0	0.1	-	-			

Lanes, Volumes, Timings
5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

2026 With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	200	100	25	50	124	99	175	26	147	506	50
Future Volume (vph)	20	200	100	25	50	124	99	175	26	147	506	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	200		200	325		0	325		325
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	1770	3472	0	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	1770	3472	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			153			172			23			153
Link Speed (mph)		40			35			35			35	
Link Distance (ft)		1786			774			1252			1357	
Travel Time (s)		30.4			15.1			24.4			26.4	
Peak Hour Factor	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	28	278	139	35	69	172	138	279	0	204	703	69
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8						6
Total Split (s)	9.9	22.5	22.5	10.0	22.6	22.6	16.0	29.2		13.3	26.5	26.5
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	6.3	10.7	10.7	6.4	10.7	10.7	9.8	17.8		8.8	16.8	16.8
Actuated g/C Ratio	0.12	0.21	0.21	0.12	0.21	0.21	0.19	0.35		0.17	0.33	0.33
v/c Ratio	0.13	0.38	0.31	0.16	0.09	0.37	0.41	0.23		0.35	0.61	0.11
Control Delay	30.1	22.9	6.4	30.2	21.8	7.4	27.8	13.2		25.9	19.2	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	30.1	22.9	6.4	30.2	21.8	7.4	27.8	13.2		25.9	19.2	0.4
LOS	C	C	A	C	C	A	C	B		C	B	A
Approach Delay		18.2			13.9			18.1			19.3	
Approach LOS		B			B			B			B	

Intersection Summary

Area Type: Other

Cycle Length: 75

Actuated Cycle Length: 51.4

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.61

Intersection Signal Delay: 18.1

Intersection LOS: B

Intersection Capacity Utilization 44.2%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

			
Ø1	Ø2	Ø3	Ø4
13.3 s	29.2 s	10 s	22.5 s
			
Ø5	Ø6	Ø7	Ø8
16 s	26.5 s	9.9 s	22.6 s

Synchro 10 Report

HCM 6th TWSC
6: University Blvd SE & Fritts Crossing SE

2026 With Project
AM Peak Hour

Intersection						
Int Delay, s/veh	2.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	15	109	274	10	70	664
Future Vol, veh/h	15	109	274	10	70	664
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	120	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	18	131	330	12	84	800
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	1304	336	0	0	342	0
Stage 1	336	-	-	-	-	-
Stage 2	968	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	174	699	-	-	1200	-
Stage 1	717	-	-	-	-	-
Stage 2	364	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	162	699	-	-	1200	-
Mov Cap-2 Maneuver	162	-	-	-	-	-
Stage 1	717	-	-	-	-	-
Stage 2	339	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	15.3	0		0.8		
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	-	499	1200	-	
HCM Lane V/C Ratio	-	-	0.299	0.07	-	
HCM Control Delay (s)	-	-	15.3	8.2	-	
HCM Lane LOS	-	-	C	A	-	
HCM 95th %tile Q(veh)	-	-	1.2	0.2	-	

HCM 6th TWSC
7: University Blvd SE & Crick Ave SE

2026 With Project
AM Peak Hour

Intersection						
Int Delay, s/veh	2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	10	114	311	30	97	721
Future Vol, veh/h	10	114	311	30	97	721
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	180	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	12	139	379	37	118	879
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	1513	398	0	0	416	0
Stage 1	398	-	-	-	-	-
Stage 2	1115	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	130	645	-	-	1127	-
Stage 1	672	-	-	-	-	-
Stage 2	309	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	116	645	-	-	1127	-
Mov Cap-2 Maneuver	116	-	-	-	-	-
Stage 1	672	-	-	-	-	-
Stage 2	277	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	14.3	0	1			
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 116 645	1127	-		
HCM Lane V/C Ratio	-	- 0.105 0.216	0.105	-		
HCM Control Delay (s)	-	- 39.6 12.1	8.6	-		
HCM Lane LOS	-	- E B	A	-		
HCM 95th %tile Q(veh)	-	- 0.3 0.8	0.4	-		

Intersection	
Intersection Delay, s/veh	10.5
Intersection LOS	B

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	20	397	5	5	139	13
Future Vol, veh/h	20	397	5	5	139	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	22	432	5	5	151	14
Number of Lanes	1	0	1	1	1	0

Approach	EB	WB	NB
Opposing Approach	WB	EB	
Opposing Lanes	2	1	0
Conflicting Approach Left		NB	EB
Conflicting Lanes Left	0	1	1
Conflicting Approach Right	NB		WB
Conflicting Lanes Right	1	0	2
HCM Control Delay	10.9	8.4	9.7
HCM LOS	B	A	A

Lane	NBLn1	EBLn1	WBLn1	WBLn2
Vol Left, %	91%	0%	100%	0%
Vol Thru, %	0%	5%	0%	100%
Vol Right, %	9%	95%	0%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	152	417	5	5
LT Vol	139	0	5	0
Through Vol	0	20	0	5
RT Vol	13	397	0	0
Lane Flow Rate	165	453	5	5
Geometry Grp	2	5	7	7
Degree of Util (X)	0.233	0.5	0.009	0.008
Departure Headway (Hd)	5.071	3.974	5.874	5.369
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	707	911	610	667
Service Time	3.109	1.99	3.607	3.102
HCM Lane V/C Ratio	0.233	0.497	0.008	0.007
HCM Control Delay	9.7	10.9	8.7	8.1
HCM Lane LOS	A	B	A	A
HCM 95th-tile Q	0.9	2.9	0	0

HCM 6th TWSC
13: Gate C & Eastman Crossing/Eastman Crossing/Gate D

2026 With Project
AM Peak Hour

Intersection						
Int Delay, s/veh	0.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↖	↑	↖	↖
Traffic Vol, veh/h	66	33	0	20	10	0
Future Vol, veh/h	66	33	0	20	10	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	0	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	72	36	0	22	11	0
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	108	0	112	54
Stage 1	-	-	-	-	90	-
Stage 2	-	-	-	-	22	-
Critical Hdwy	-	-	4.175	-	6.675	6.975
Critical Hdwy Stg 1	-	-	-	-	5.875	-
Critical Hdwy Stg 2	-	-	-	-	5.475	-
Follow-up Hdwy	-	-	2.2475	-	3.5475	3.3475
Pot Cap-1 Maneuver	-	-	1461	-	871	993
Stage 1	-	-	-	-	916	-
Stage 2	-	-	-	-	992	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1461	-	871	993
Mov Cap-2 Maneuver	-	-	-	-	871	-
Stage 1	-	-	-	-	916	-
Stage 2	-	-	-	-	992	-
Approach	EB	WB	NB			
HCM Control Delay, s	0	0	9.2			
HCM LOS	A					
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	871	-	-	-	1461	-
HCM Lane V/C Ratio	0.012	-	-	-	-	-
HCM Control Delay (s)	9.2	0	-	-	0	-
HCM Lane LOS	A	A	-	-	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0	-

2026 With Project Conditions

PM Peak Hour

HCM 6th TWSC
1: University Blvd SE & Stryker Rd

2026 With Project
PM Peak Hour

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↱			↰					↰↱		
Traffic Vol, veh/h	0	23	0	0	30	0	0	0	0	435	1	10
Future Vol, veh/h	0	23	0	0	30	0	0	0	0	435	1	10
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	16974	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	0	26	0	0	33	0	0	0	0	483	1	11
Major/Minor	Minor2		Minor1				Major2					
Conflicting Flow All	-	973	6	980	978	-				0	0	0
Stage 1	-	973	-	0	0	-				-	-	-
Stage 2	-	0	-	980	978	-				-	-	-
Critical Hdwy	-	6.6	7	7.6	6.6	-				4.2	-	-
Critical Hdwy Stg 1	-	5.6	-	-	-	-				-	-	-
Critical Hdwy Stg 2	-	-	-	6.6	5.6	-				-	-	-
Follow-up Hdwy	-	4.05	3.35	3.55	4.05	-				2.25	-	-
Pot Cap-1 Maneuver	0	246	1065	200	244	0				-	-	-
Stage 1	0	322	-	-	-	0				-	-	-
Stage 2	0	-	-	262	320	0				-	-	-
Platoon blocked, %											-	-
Mov Cap-1 Maneuver	-	246	1065	184	244	-				-	-	-
Mov Cap-2 Maneuver	-	246	-	184	244	-				-	-	-
Stage 1	-	322	-	-	-	-				-	-	-
Stage 2	-	-	-	241	320	-				-	-	-
Approach	EB		WB				SB					
HCM Control Delay, s	21.3		22.1									
HCM LOS	C		C									
Minor Lane/Major Mvmt	EBLn1WBLn1		SBL	SBT	SBR							
Capacity (veh/h)	246 244		-	-	-							
HCM Lane V/C Ratio	0.104 0.137		-	-	-							
HCM Control Delay (s)	21.3 22.1		-	-	-							
HCM Lane LOS	C C		-	-	-							
HCM 95th %tile Q(veh)	0.3 0.5		-	-	-							

HCM 6th TWSC
2: University Blvd SE & Stryker Rd

2026 With Project
PM Peak Hour

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕				
Traffic Vol, veh/h	45	413	0	0	30	542	0	0	0	0	0	0
Future Vol, veh/h	45	413	0	0	30	542	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	16965	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	73	73	73	73	73	73
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	62	566	0	0	41	742	0	0	0	0	0	0
Major/Minor	Minor2		Minor1		Major1							
Conflicting Flow All	392	0	-	-	0	0	0	0	0			
Stage 1	0	0	-	-	0	-	-	-	-			
Stage 2	392	0	-	-	0	-	-	-	-			
Critical Hdwy	7.15	6.55	-	-	6.55	6.25	4.15	-	-			
Critical Hdwy Stg 1	-	-	-	-	5.55	-	-	-	-			
Critical Hdwy Stg 2	6.15	5.55	-	-	-	-	-	-	-			
Follow-up Hdwy	3.545	4.045	-	-	4.045	3.345	2.245	-	-			
Pot Cap-1 Maneuver	562	-	0	0	-	-	-	-	-			
Stage 1	-	-	0	0	-	-	-	-	-			
Stage 2	627	-	0	0	-	-	-	-	-			
Platoon blocked, %									-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	-	-	-	-			
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	-			
Stage 1	-	-	-	-	-	-	-	-	-			
Stage 2	627	-	-	-	-	-	-	-	-			
Approach	EB		WB		NB							
HCM Control Delay, s	0											
HCM LOS	-		-									
Minor Lane/Major Mvmt	NBL		NBT		NBR		EBLn1WBLn1					
Capacity (veh/h)	-		-		-		-					
HCM Lane V/C Ratio	-		-		-		-					
HCM Control Delay (s)	0		-		-		-					
HCM Lane LOS	A		-		-		-					
HCM 95th %tile Q(veh)	-		-		-		-					

HCM 6th TWSC
3: University Blvd SE & Strand Loop SE/Gate A

2026 With Project
PM Peak Hour

Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	64	0	25	1	0	29	12	574	1	12	420	48
Future Vol, veh/h	64	0	25	1	0	29	12	574	1	12	420	48
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	-	-	0	130	-	-	115	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	67	0	26	1	0	31	13	604	1	13	442	51

Major/Minor	Minor2		Minor1		Major1		Major2	
Conflicting Flow All	822	1125	247	878	1150	303	493	0
Stage 1	494	494	-	631	631	-	-	-
Stage 2	328	631	-	247	519	-	-	-
Critical Hdwy	7.6	6.6	7	7.6	6.6	7	4.2	-
Critical Hdwy Stg 1	6.6	5.6	-	6.6	5.6	-	-	-
Critical Hdwy Stg 2	6.6	5.6	-	6.6	5.6	-	-	-
Follow-up Hdwy	3.55	4.05	3.35	3.55	4.05	3.35	2.25	-
Pot Cap-1 Maneuver	261	199	744	237	192	684	1046	-
Stage 1	518	537	-	428	465	-	-	-
Stage 2	651	465	-	726	523	-	-	-
Platoon blocked, %								-
Mov Cap-1 Maneuver	244	194	744	224	187	684	1046	-
Mov Cap-2 Maneuver	244	194	-	224	187	-	-	-
Stage 1	512	529	-	423	459	-	-	-
Stage 2	614	459	-	691	516	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	21	10.9	0.2	0.2
HCM LOS	C	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1046	-	-	244	744	224	684	949	-	-
HCM Lane V/C Ratio	0.012	-	-	0.276	0.035	0.005	0.045	0.013	-	-
HCM Control Delay (s)	8.5	-	-	25.3	10	21.1	10.5	8.8	-	-
HCM Lane LOS	A	-	-	D	B	C	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	1.1	0.1	0	0.1	0	-	-

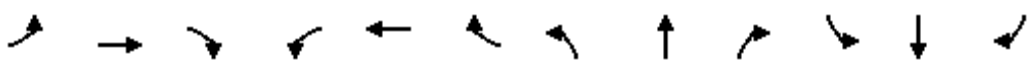
HCM 6th TWSC
4: University Blvd SE & Avedon Ave SE/Gate B

2026 With Project
PM Peak Hour

Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕↕			↕↕	↕↕	↕↕	↕↕		↕↕	↕↕	
Traffic Vol, veh/h	30	0	21	1	0	29	10	685	1	12	468	23
Future Vol, veh/h	30	0	21	1	0	29	10	685	1	12	468	23
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	125	-	-	115	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	5	5	5	5	5	5	5	5	5	5	5	5
Mvmt Flow	31	0	22	1	0	30	10	706	1	12	482	24
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	891	1245	253	992	1257	354	506	0	0	707	0	0
Stage 1	518	518	-	727	727	-	-	-	-	-	-	-
Stage 2	373	727	-	265	530	-	-	-	-	-	-	-
Critical Hdwy	7.6	6.6	7	7.6	6.6	7	4.2	-	-	4.2	-	-
Critical Hdwy Stg 1	6.6	5.6	-	6.6	5.6	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.6	5.6	-	6.6	5.6	-	-	-	-	-	-	-
Follow-up Hdwy	3.55	4.05	3.35	3.55	4.05	3.35	2.25	-	-	2.25	-	-
Pot Cap-1 Maneuver	232	169	737	196	166	634	1034	-	-	868	-	-
Stage 1	501	524	-	375	420	-	-	-	-	-	-	-
Stage 2	612	420	-	709	517	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	217	165	737	187	162	634	1034	-	-	868	-	-
Mov Cap-2 Maneuver	217	165	-	187	162	-	-	-	-	-	-	-
Stage 1	496	517	-	371	416	-	-	-	-	-	-	-
Stage 2	578	416	-	679	510	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	19.2		11.4		0.1		0.2					
HCM LOS	C		B									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1WBLn2	SBL	SBT	SBR					
Capacity (veh/h)	1034	-	-	306	187	634	868	-	-			
HCM Lane V/C Ratio	0.01	-	-	0.172	0.006	0.047	0.014	-	-			
HCM Control Delay (s)	8.5	-	-	19.2	24.4	11	9.2	-	-			
HCM Lane LOS	A	-	-	C	C	B	A	-	-			
HCM 95th %tile Q(veh)	0	-	-	0.6	0	0.1	0	-	-			

Lanes, Volumes, Timings
5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

2026 With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	73	75	115	25	125	156	250	495	2	126	357	40
Future Volume (vph)	73	75	115	25	125	156	250	495	2	126	357	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	200		200	325		0	325		325
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	1770	3536	0	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	1770	3536	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			218			218		1				218
Link Speed (mph)		40			35			35			35	
Link Distance (ft)		1786			774			1252			1357	
Travel Time (s)		30.4			15.1			24.4			26.4	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	78	81	124	27	134	168	269	534	0	135	384	43
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8						6
Total Split (s)	10.0	22.6	22.6	9.9	22.5	22.5	19.0	31.2		11.3	23.5	23.5
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	5.7	11.8	11.8	5.6	7.9	7.9	12.9	20.9		6.8	11.6	11.6
Actuated g/C Ratio	0.11	0.22	0.22	0.10	0.15	0.15	0.24	0.39		0.13	0.22	0.22
v/c Ratio	0.41	0.10	0.24	0.15	0.26	0.40	0.63	0.39		0.31	0.50	0.08
Control Delay	34.4	20.6	1.6	28.1	24.3	5.2	28.6	15.1		26.9	22.3	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	34.4	20.6	1.6	28.1	24.3	5.2	28.6	15.1		26.9	22.3	0.3
LOS	C	C	A	C	C	A	C	B		C	C	A
Approach Delay		16.1			14.9			19.6			21.7	
Approach LOS		B			B			B			C	

Intersection Summary

Area Type: Other

Cycle Length: 75

Actuated Cycle Length: 53.9

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.63

Intersection Signal Delay: 18.9

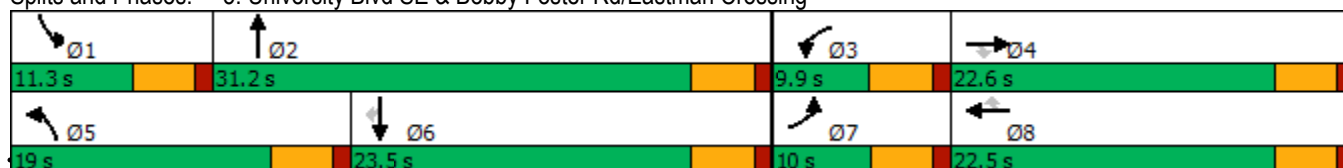
Intersection LOS: B

Intersection Capacity Utilization 47.1%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 5: University Blvd SE & Bobby Foster Rd/Eastman Crossing



Synchro 10 Report

HCM 6th TWSC
6: University Blvd SE & Fritts Crossing SE

2026 With Project
PM Peak Hour

Intersection						
Int Delay, s/veh	2.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	15	82	635	34	97	529
Future Vol, veh/h	15	82	635	34	97	529
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	120	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	15	85	655	35	100	545
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	1418	673	0	0	690	0
Stage 1	673	-	-	-	-	-
Stage 2	745	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	149	450	-	-	891	-
Stage 1	501	-	-	-	-	-
Stage 2	464	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	132	450	-	-	891	-
Mov Cap-2 Maneuver	132	-	-	-	-	-
Stage 1	501	-	-	-	-	-
Stage 2	412	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	20.7	0	1.5			
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	328	891	-	
HCM Lane V/C Ratio	-	-	0.305	0.112	-	
HCM Control Delay (s)	-	-	20.7	9.6	-	
HCM Lane LOS	-	-	C	A	-	
HCM 95th %tile Q(veh)	-	-	1.3	0.4	-	

HCM 6th TWSC
7: University Blvd SE & Crick Ave SE

2026 With Project
PM Peak Hour

Intersection						
Int Delay, s/veh	3.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	15	130	695	36	124	602
Future Vol, veh/h	15	130	695	36	124	602
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	180	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	19	163	869	45	155	753
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	1955	892	0	0	914	0
Stage 1	892	-	-	-	-	-
Stage 2	1063	-	-	-	-	-
Critical Hdwy	6.45	6.25	-	-	4.15	-
Critical Hdwy Stg 1	5.45	-	-	-	-	-
Critical Hdwy Stg 2	5.45	-	-	-	-	-
Follow-up Hdwy	3.545	3.345	-	-	2.245	-
Pot Cap-1 Maneuver	69	336	-	-	734	-
Stage 1	395	-	-	-	-	-
Stage 2	328	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	54	336	-	-	734	-
Mov Cap-2 Maneuver	54	-	-	-	-	-
Stage 1	395	-	-	-	-	-
Stage 2	259	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	33.5	0	1.9			
HCM LOS	D					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 54 336	734	-		
HCM Lane V/C Ratio	-	- 0.347 0.484	0.211	-		
HCM Control Delay (s)	-	- 103.6 25.4	11.2	-		
HCM Lane LOS	-	- F D	B	-		
HCM 95th %tile Q(veh)	-	- 1.2 2.5	0.8	-		

Intersection	
Intersection Delay, s/veh	31.6
Intersection LOS	D

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	13	400	10	20	552	0
Future Vol, veh/h	13	400	10	20	552	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	14	435	11	22	600	0
Number of Lanes	1	0	1	1	1	0

Approach	EB	WB	NB
Opposing Approach	WB	EB	
Opposing Lanes	2	1	0
Conflicting Approach Left		NB	EB
Conflicting Lanes Left	0	1	1
Conflicting Approach Right	NB		WB
Conflicting Lanes Right	1	0	2
HCM Control Delay	18.6	10.4	42.4
HCM LOS	C	B	E

Lane	NBLn1	EBLn1	WBLn1	WBLn2
Vol Left, %	100%	0%	100%	0%
Vol Thru, %	0%	3%	0%	100%
Vol Right, %	0%	97%	0%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	552	413	10	20
LT Vol	552	0	10	0
Through Vol	0	13	0	20
RT Vol	0	400	0	0
Lane Flow Rate	600	449	11	22
Geometry Grp	2	5	7	7
Degree of Util (X)	0.923	0.667	0.023	0.043
Departure Headway (Hd)	5.538	5.352	7.653	7.14
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	654	673	465	498
Service Time	3.572	3.408	5.439	4.926
HCM Lane V/C Ratio	0.917	0.667	0.024	0.044
HCM Control Delay	42.4	18.6	10.6	10.3
HCM Lane LOS	E	C	B	B
HCM 95th-tile Q	12.2	5.1	0.1	0.1

HCM 6th TWSC
13: Gate C & Eastman Crossing/Eastman Crossing/Gate D

2026 With Project
PM Peak Hour

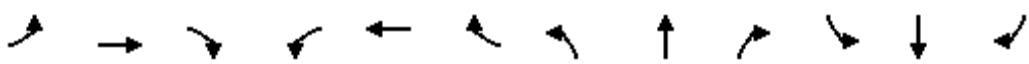
Intersection						
Int Delay, s/veh	2.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↖	↑	↖	↖
Traffic Vol, veh/h	26	13	0	60	30	0
Future Vol, veh/h	26	13	0	60	30	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	0	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	28	14	0	65	33	0
Major/Minor	Major1	Major2		Minor1		
Conflicting Flow All	0	0	42	0	100	21
Stage 1	-	-	-	-	35	-
Stage 2	-	-	-	-	65	-
Critical Hdwy	-	-	4.175	-	6.675	6.975
Critical Hdwy Stg 1	-	-	-	-	5.875	-
Critical Hdwy Stg 2	-	-	-	-	5.475	-
Follow-up Hdwy	-	-2.2475		-3.54753.3475		
Pot Cap-1 Maneuver	-	-	1546	-	885	1043
Stage 1	-	-	-	-	975	-
Stage 2	-	-	-	-	949	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1546	-	885	1043
Mov Cap-2 Maneuver	-	-	-	-	885	-
Stage 1	-	-	-	-	975	-
Stage 2	-	-	-	-	949	-
Approach	EB	WB		NB		
HCM Control Delay, s	0	0		9.2		
HCM LOS	A					
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	885	-	-	-	1546	-
HCM Lane V/C Ratio	0.037	-	-	-	-	-
HCM Control Delay (s)	9.2	0	-	-	0	-
HCM Lane LOS	A	A	-	-	A	-
HCM 95th %tile Q(veh)	0.1	-	-	-	0	-

Buildout With Project Conditions

AM Peak Hour

Lanes, Volumes, Timings
1: University Blvd SE & Stryker Rd

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↰			↱						↱↰	
Traffic Volume (vph)	0	0	51	0	0	0	0	0	0	66	349	0
Future Volume (vph)	0	0	51	0	0	0	0	0	0	66	349	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	0	1611	0	0	1863	0	0	0	0	0	3511	0
Flt Permitted											0.992	
Satd. Flow (perm)	0	1611	0	0	1863	0	0	0	0	0	3511	0
Right Turn on Red			Yes			Yes				Yes		Yes
Satd. Flow (RTOR)		435										
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		518			474			764			758	
Travel Time (s)		10.1			9.2			14.9			14.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	55	0	0	0	0	0	0	0	0	451	0
Turn Type		NA								Split	NA	
Protected Phases		4			8					6	6	
Permitted Phases				8								
Total Split (s)		26.0		26.0	26.0					39.0	39.0	
Total Lost Time (s)		4.5			4.5						4.5	
Act Effct Green (s)		5.6									25.3	
Actuated g/C Ratio		0.19									0.84	
v/c Ratio		0.08									0.15	
Control Delay		0.3									2.1	
Queue Delay		0.0									0.0	
Total Delay		0.3									2.1	
LOS		A									A	
Approach Delay		0.3									2.1	
Approach LOS		A									A	

Intersection Summary

Area Type: Other

Cycle Length: 65

Actuated Cycle Length: 30.2

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.15

Intersection Signal Delay: 1.9

Intersection LOS: A

Intersection Capacity Utilization 51.5%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: University Blvd SE & Stryker Rd



Lanes, Volumes, Timings
2: University Blvd SE & Stryker Rd

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	46	0	0	0	113	0	1200	120	0	0	0
Future Volume (vph)	0	46	0	0	0	113	0	1200	120	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	0	1863	0	0	1611	0	0	3490	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	1863	0	0	1611	0	0	3490	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					45			28				
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		474			880			736			768	
Travel Time (s)		9.2			17.1			14.3			15.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	50	0	0	123	0	0	1434	0	0	0	0
Turn Type		NA			NA			NA				
Protected Phases		4			8			2				
Permitted Phases	4						2					
Total Split (s)	22.6	22.6			22.6		37.4	37.4				
Total Lost Time (s)		4.5			4.5			4.5				
Act Effct Green (s)		8.1			8.1			34.0				
Actuated g/C Ratio		0.17			0.17			0.72				
v/c Ratio		0.16			0.39			0.57				
Control Delay		18.8			16.7			6.0				
Queue Delay		0.0			0.0			0.0				
Total Delay		18.8			16.7			6.0				
LOS		B			B			A				
Approach Delay		18.8			16.7			6.0				
Approach LOS		B			B			A				

Intersection Summary

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 47.2

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.57

Intersection Signal Delay: 7.2

Intersection LOS: A

Intersection Capacity Utilization 51.5%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: University Blvd SE & Stryker Rd



Lanes, Volumes, Timings

3: University Blvd SE & Strand Loop SE/Gate A

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	132	0	136	3	0	7	109	1109	13	20	261	10
Future Volume (vph)	132	0	136	3	0	7	109	1109	13	20	261	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	130		0	115		0
Storage Lanes	1		0	0		1	1		0	1		0
Taper Length (ft)	25			25			80			80		
Satd. Flow (prot)	1770	1583	0	0	1770	1583	1770	3532	0	1770	3518	0
Flt Permitted	0.756				0.663		0.950			0.950		
Satd. Flow (perm)	1408	1583	0	0	1235	1583	1770	3532	0	1770	3518	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		605				127		2			6	
Link Speed (mph)		30			30			35			35	
Link Distance (ft)		485			304			432			470	
Travel Time (s)		11.0			6.9			8.4			9.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	143	148	0	0	3	8	118	1219	0	22	295	0
Turn Type	Perm	NA		Perm	NA	Perm	Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8						
Total Split (s)	26.0	26.0		26.0	26.0	26.0	19.0	53.0		11.0	45.0	
Total Lost Time (s)	4.5	4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Act Effct Green (s)	12.3	12.3			12.3	12.3	10.0	31.2		6.6	22.4	
Actuated g/C Ratio	0.22	0.22			0.22	0.22	0.18	0.55		0.12	0.40	
v/c Ratio	0.47	0.18			0.01	0.02	0.38	0.63		0.11	0.21	
Control Delay	28.5	0.5			23.7	0.1	29.4	11.8		33.1	13.1	
Queue Delay	0.0	0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	28.5	0.5			23.7	0.1	29.4	11.8		33.1	13.1	
LOS	C	A			C	A	C	B		C	B	
Approach Delay		14.3			6.6			13.3			14.5	
Approach LOS		B			A			B			B	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 56.7

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.63

Intersection Signal Delay: 13.6

Intersection LOS: B

Intersection Capacity Utilization 60.5%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 3: University Blvd SE & Strand Loop SE/Gate A

		
Ø1	Ø2	Ø4
11 s	53 s	26 s
		
Ø5	Ø6	Ø8
19 s	45 s	26 s

Lanes, Volumes, Timings
4: University Blvd SE & Avedon Ave SE/Gate B

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	132	0	78	3	0	7	73	1300	13	20	284	11
Future Volume (vph)	132	0	78	3	0	7	73	1300	13	20	284	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	125		0	115		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	25			25			80			25		
Satd. Flow (prot)	0	1717	0	0	1770	1583	1770	3536	0	1770	3518	0
Flt Permitted		0.808			0.635		0.557			0.126		
Satd. Flow (perm)	0	1430	0	0	1183	1583	1038	3536	0	235	3518	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		34				18		2			8	
Link Speed (mph)		30			30			35			35	
Link Distance (ft)		260			279			470			1252	
Travel Time (s)		5.9			6.3			9.2			24.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	228	0	0	3	8	79	1427	0	22	321	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Total Split (s)	31.0	31.0		31.0	31.0	31.0	59.0	59.0		59.0	59.0	
Total Lost Time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Act Effct Green (s)		14.7			14.7	14.7	38.3	38.3		38.3	38.3	
Actuated g/C Ratio		0.24			0.24	0.24	0.61	0.61		0.61	0.61	
v/c Ratio		0.63			0.01	0.02	0.12	0.66		0.15	0.15	
Control Delay		27.7			21.3	5.6	6.6	10.2		9.5	5.7	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		27.7			21.3	5.6	6.6	10.2		9.5	5.7	
LOS		C			C	A	A	B		A	A	
Approach Delay		27.7			9.9			10.0			6.0	
Approach LOS		C			A			B			A	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 62.5

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.66

Intersection Signal Delay: 11.3

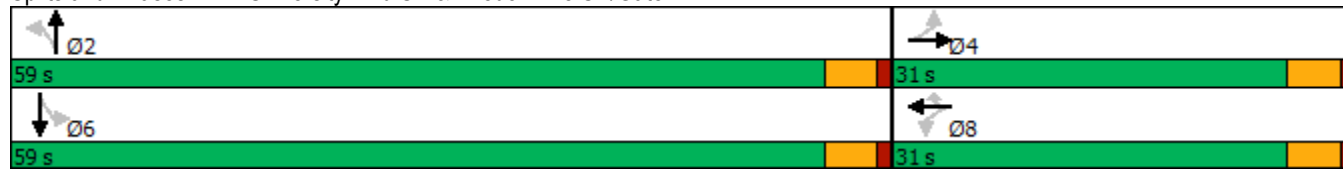
Intersection LOS: B

Intersection Capacity Utilization 70.5%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 4: University Blvd SE & Avedon Ave SE/Gate B



Lanes, Volumes, Timings
5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	155	909	89	106	292	103	169	1340	129	139	129	19
Future Volume (vph)	155	909	89	106	292	103	169	1340	129	139	129	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		300	200		200	325		0	325		325
Storage Lanes	2		1	2		1	1		0	2		1
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	3433	3539	1583	3433	3539	1583	1770	3493	0	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3539	1583	3433	3539	1583	1770	3493	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			149			193			12			193
Link Speed (mph)		40			35			35			35	
Link Distance (ft)		1786			774			1252			1357	
Travel Time (s)		30.4			15.1			24.4			26.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	168	988	97	115	317	112	184	1597	0	151	140	21
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8						6
Total Split (s)	14.6	35.4	35.4	9.6	30.4	30.4	24.8	54.8		10.2	40.2	40.2
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	9.5	30.9	30.9	5.1	26.5	26.5	16.1	50.3		5.7	39.9	39.9
Actuated g/C Ratio	0.09	0.28	0.28	0.05	0.24	0.24	0.15	0.46		0.05	0.36	0.36
v/c Ratio	0.57	0.99	0.18	0.72	0.37	0.21	0.71	1.00		0.85	0.11	0.03
Control Delay	56.0	67.2	1.9	77.1	36.5	0.9	59.4	51.7		90.1	24.7	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	56.0	67.2	1.9	77.1	36.5	0.9	59.4	51.7		90.1	24.7	0.1
LOS	E	E	A	E	D	A	E	D		F	C	A
Approach Delay		60.6			37.8			52.5			54.7	
Approach LOS		E			D			D			D	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.00

Intersection Signal Delay: 53.2

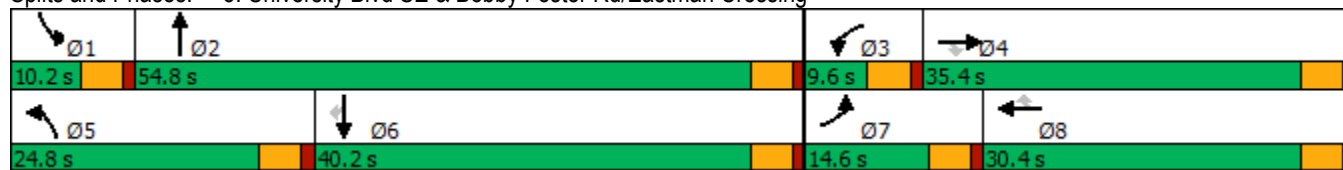
Intersection LOS: D

Intersection Capacity Utilization 89.6%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 5: University Blvd SE & Bobby Foster Rd/Eastman Crossing



Lanes, Volumes, Timings
6: University Blvd SE & Fritts Crossing SE

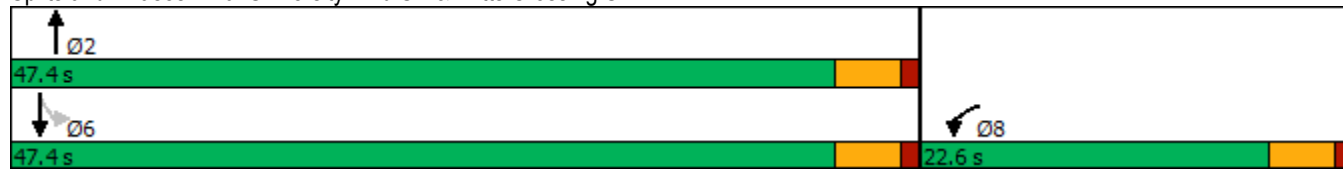
Buildout With Project
AM Peak Hour

	↖	↗	↑	↘	↙	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		T		T	T
Traffic Volume (vph)	35	99	1347	429	48	380
Future Volume (vph)	35	99	1347	429	48	380
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	120	
Storage Lanes	1	0		0	1	
Taper Length (ft)	25				25	
Satd. Flow (prot)	1655	0	3412	0	1770	3539
Flt Permitted	0.987				0.085	
Satd. Flow (perm)	1655	0	3412	0	158	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	44		113			
Link Speed (mph)	25		35			35
Link Distance (ft)	832		739			284
Travel Time (s)	22.7		14.4			5.5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)						
Lane Group Flow (vph)	146	0	1930	0	52	413
Turn Type	Prot		NA		Perm	NA
Protected Phases	8		2			6
Permitted Phases					6	
Total Split (s)	22.6		47.4		47.4	47.4
Total Lost Time (s)	4.5		4.5		4.5	4.5
Act Effct Green (s)	9.6		48.7		48.7	48.7
Actuated g/C Ratio	0.15		0.77		0.77	0.77
v/c Ratio	0.51		0.73		0.43	0.15
Control Delay	25.3		8.2		21.3	3.3
Queue Delay	0.0		0.0		0.0	0.0
Total Delay	25.3		8.2		21.3	3.3
LOS	C		A		C	A
Approach Delay	25.3		8.2			5.4
Approach LOS	C		A			A

Intersection Summary

Area Type: Other
 Cycle Length: 70
 Actuated Cycle Length: 63.3
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.73
 Intersection Signal Delay: 8.7
 Intersection Capacity Utilization 66.5%
 Analysis Period (min) 15
 Intersection LOS: A
 ICU Level of Service C

Splits and Phases: 6: University Blvd SE & Fritts Crossing SE



Lanes, Volumes, Timings
7: University Blvd SE & Crick Ave SE

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	100	100	70	400	99	443	88	727	650	934	143	210
Future Volume (vph)	100	100	70	400	99	443	88	727	650	934	143	210
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	350		0	180		500	600		0
Storage Lanes	1		0	2		1	1		1	2		0
Taper Length (ft)	25			25			25			120		
Satd. Flow (prot)	1770	1747	0	3433	1863	1583	1770	3539	1583	3433	3224	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	1747	0	3433	1863	1583	1770	3539	1583	3433	3224	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		25				482			336		228	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		276			576			790			926	
Travel Time (s)		5.4			11.2			15.4			18.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	109	185	0	435	108	482	96	790	707	1015	383	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			
Total Split (s)	15.3	22.5		20.0	27.2	27.2	17.6	38.1	38.1	39.4	59.9	
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	
Act Effct Green (s)	10.2	14.9		15.5	20.2	20.2	10.9	33.6	33.6	34.9	57.6	
Actuated g/C Ratio	0.09	0.13		0.13	0.17	0.17	0.09	0.29	0.29	0.30	0.49	
v/c Ratio	0.71	0.76		0.96	0.34	0.72	0.58	0.78	1.02	0.99	0.22	
Control Delay	77.0	62.2		83.7	45.7	10.5	65.5	44.9	61.6	67.5	7.5	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	77.0	62.2		83.7	45.7	10.5	65.5	44.9	61.6	67.5	7.5	
LOS	E	E		F	D	B	E	D	E	E	A	
Approach Delay		67.7			45.3			53.5			51.0	
Approach LOS		E			D			D			D	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 117

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.02

Intersection Signal Delay: 51.7

Intersection LOS: D

Intersection Capacity Utilization 87.7%

ICU Level of Service E

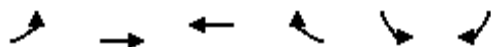
Analysis Period (min) 15

Splits and Phases: 7: University Blvd SE & Crick Ave SE



Lanes, Volumes, Timings
8: Eastman Crossing/Gate D & Watson Dr SE

Buildout With Project
AM Peak Hour



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	850	33	10	0	0	350
Future Volume (vph)	850	33	10	0	0	350
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1681	1692	3539	0	1863	1583
Flt Permitted	0.950	0.956				
Satd. Flow (perm)	1681	1692	3539	0	1863	1583
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)						1058
Link Speed (mph)		35	30		25	
Link Distance (ft)		411	915		1159	
Travel Time (s)		8.0	20.8		31.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)	48%					
Lane Group Flow (vph)	480	480	11	0	0	380
Turn Type	Split	NA	NA		Perm	Over
Protected Phases	4	4	8			4
Permitted Phases					6	
Total Split (s)	35.0	35.0	22.5		22.5	35.0
Total Lost Time (s)	4.5	4.5	4.5		4.5	4.5
Act Effct Green (s)	20.3	20.3	6.0			20.3
Actuated g/C Ratio	0.55	0.55	0.16			0.55
v/c Ratio	0.52	0.52	0.02			0.28
Control Delay	8.0	8.0	18.5			0.5
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	8.0	8.0	18.5			0.5
LOS	A	A	B			A
Approach Delay		8.0	18.5		0.5	
Approach LOS		A	B		A	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 37.2

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 6.0

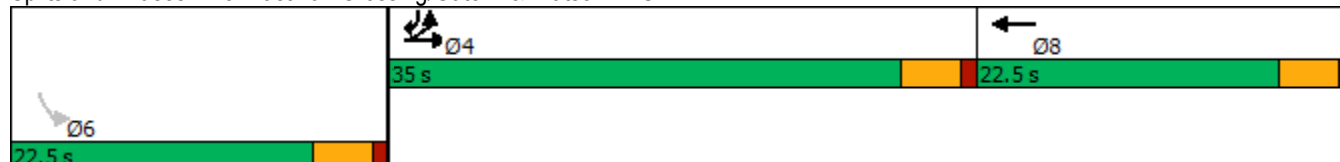
Intersection LOS: A

Intersection Capacity Utilization 34.8%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 8: Eastman Crossing/Gate D & Watson Dr SE



Lanes, Volumes, Timings
9: Mesa Del Sol Blvd & Gate E/Eastman Crossing

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	1	4	22	4	39	21	1742	188	105	675	8
Future Volume (vph)	4	1	4	22	4	39	21	1742	188	105	675	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	125		0	200		0	200		200
Storage Lanes	1		0	1		0	1		0	1		1
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	1639	0	1770	1608	0	1770	3486	0	1770	3539	1583
Flt Permitted	0.870			0.870			0.950			0.950		
Satd. Flow (perm)	1621	1639	0	1621	1608	0	1770	3486	0	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4			42			21				73
Link Speed (mph)		30			35			35				35
Link Distance (ft)		419			491			470				342
Travel Time (s)		9.5			9.6			9.2				6.7
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	4	5	0	24	46	0	23	2097	0	114	734	9
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								6
Total Split (s)	22.6	22.6		22.6	22.6		10.0	56.2		11.2	57.4	57.4
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	6.9	6.9		6.9	6.9		5.5	56.5		6.7	65.5	65.5
Actuated g/C Ratio	0.09	0.09		0.09	0.09		0.07	0.71		0.08	0.82	0.82
v/c Ratio	0.03	0.03		0.17	0.26		0.19	0.85		0.77	0.25	0.01
Control Delay	33.0	23.6		35.9	16.4		39.0	14.6		69.2	3.7	0.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	33.0	23.6		35.9	16.4		39.0	14.6		69.2	3.7	0.0
LOS	C	C		D	B		D	B		E	A	A
Approach Delay		27.8			23.1			14.9			12.4	
Approach LOS		C			C			B			B	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 79.5

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 14.4

Intersection LOS: B

Intersection Capacity Utilization 79.1%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 9: Mesa Del Sol Blvd & Gate E/Eastman Crossing

		
Ø1	Ø2	Ø4
11.2 s	56.2 s	22.6 s
		
Ø5	Ø6	Ø8
10 s	57.4 s	22.6 s

Intersection	
Intersection Delay, s/veh	8.1
Intersection LOS	A

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	20	50	5	5	150	13
Future Vol, veh/h	20	50	5	5	150	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	22	54	5	5	163	14
Number of Lanes	1	0	1	1	1	0

Approach	EB	WB	NB
Opposing Approach	WB	EB	
Opposing Lanes	2	1	0
Conflicting Approach Left		NB	EB
Conflicting Lanes Left	0	1	1
Conflicting Approach Right	NB		WB
Conflicting Lanes Right	1	0	2
HCM Control Delay	7.4	8	8.4
HCM LOS	A	A	A

Lane	NBLn1	EBLn1	WBLn1	WBLn2
Vol Left, %	92%	0%	100%	0%
Vol Thru, %	0%	29%	0%	100%
Vol Right, %	8%	71%	0%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	163	70	5	5
LT Vol	150	0	5	0
Through Vol	0	20	0	5
RT Vol	13	50	0	0
Lane Flow Rate	177	76	5	5
Geometry Grp	2	5	7	7
Degree of Util (X)	0.208	0.085	0.008	0.008
Departure Headway (Hd)	4.22	4.031	5.506	5.004
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	847	894	654	719
Service Time	2.264	2.032	3.208	2.705
HCM Lane V/C Ratio	0.209	0.085	0.008	0.007
HCM Control Delay	8.4	7.4	8.3	7.7
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.8	0.3	0	0

Lanes, Volumes, Timings
11: Gate G/Hawking Dr SE & Crick Ave SE

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	156	1495	15	10	404	575	3	2	5	150	8	37
Future Volume (vph)	156	1495	15	10	404	575	3	2	5	150	8	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300		0	150		0	0		0	0		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	3536	0	1770	3228	0	1770	1663	0	1770	1635	0
Flt Permitted	0.950			0.950			0.725			0.753		
Satd. Flow (perm)	1770	3536	0	1770	3228	0	1350	1663	0	1403	1635	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2			547			5			40	
Link Speed (mph)		35			35			30			35	
Link Distance (ft)		2075			1380			306			1115	
Travel Time (s)		40.4			26.9			7.0			21.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	170	1641	0	11	1064	0	3	7	0	163	49	0
Turn Type	Prot	NA		Prot	NA		Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases							2			6		
Total Split (s)	19.4	42.0		9.6	32.2		23.4	23.4		23.4	23.4	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Act Effct Green (s)	10.9	36.1		5.3	22.0		12.3	12.3		12.3	12.3	
Actuated g/C Ratio	0.18	0.61		0.09	0.37		0.21	0.21		0.21	0.21	
v/c Ratio	0.52	0.76		0.07	0.69		0.01	0.02		0.56	0.13	
Control Delay	30.3	13.6		31.5	10.5		20.7	15.2		30.7	10.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	30.3	13.6		31.5	10.5		20.7	15.2		30.7	10.4	
LOS	C	B		C	B		C	B		C	B	
Approach Delay		15.1			10.7			16.8			26.0	
Approach LOS		B			B			B			C	

Intersection Summary

Area Type: Other

Cycle Length: 75

Actuated Cycle Length: 59.3

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.76

Intersection Signal Delay: 14.3

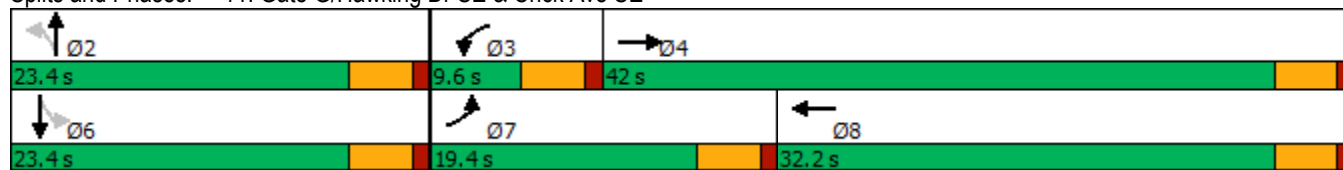
Intersection LOS: B

Intersection Capacity Utilization 72.2%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 11: Gate G/Hawking Dr SE & Crick Ave SE



Lanes, Volumes, Timings
12: Gate H/Molina Rd & Crick Ave SE

Buildout With Project
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	14	1550	20	13	1030	104	3	2	5	23	0	32
Future Volume (vph)	14	1550	20	13	1030	104	3	2	5	23	0	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	150		0	0		0	0		0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	3532	0	1770	3490	0	1770	1663	0	0	1681	0
Flt Permitted	0.195			0.132			0.718				0.862	
Satd. Flow (perm)	363	3532	0	246	3490	0	1337	1663	0	0	1479	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3			28			5			35	
Link Speed (mph)		35			35			30			25	
Link Distance (ft)		1380			1210			256			1120	
Travel Time (s)		26.9			23.6			5.8			30.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	15	1707	0	14	1233	0	3	7	0	0	60	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Total Split (s)	37.4	37.4		37.4	37.4		22.6	22.6		22.6	22.6	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5			4.5	
Act Effct Green (s)	30.3	30.3		30.3	30.3		6.6	6.6			6.6	
Actuated g/C Ratio	0.66	0.66		0.66	0.66		0.14	0.14			0.14	
v/c Ratio	0.06	0.73		0.09	0.53		0.02	0.03			0.25	
Control Delay	3.7	7.6		4.5	5.1		18.0	13.7			13.8	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Total Delay	3.7	7.6		4.5	5.1		18.0	13.7			13.8	
LOS	A	A		A	A		B	B			B	
Approach Delay		7.6			5.1			15.0			13.8	
Approach LOS		A			A			B			B	

Intersection Summary

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 46

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 6.7

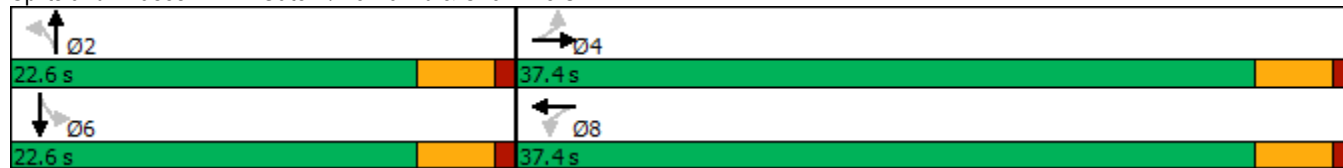
Intersection LOS: A

Intersection Capacity Utilization 60.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 12: Gate H/Molina Rd & Crick Ave SE



HCM 6th TWSC
13: Gate C & Eastman Crossing

Buildout With Project
AM Peak Hour

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑	↑	↑
Traffic Vol, veh/h	930	33	0	400	7	3
Future Vol, veh/h	930	33	0	400	7	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1011	36	0	435	8	3
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1047	0	1247	524
Stage 1	-	-	-	-	1029	-
Stage 2	-	-	-	-	218	-
Critical Hdwy	-	-	4.14	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	-	-	2.22	-	3.52	3.32
Pot Cap-1 Maneuver	-	-	660	-	166	498
Stage 1	-	-	-	-	306	-
Stage 2	-	-	-	-	797	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	660	-	166	498
Mov Cap-2 Maneuver	-	-	-	-	166	-
Stage 1	-	-	-	-	306	-
Stage 2	-	-	-	-	797	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		23.1	
HCM LOS					C	
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	166	498	-	-	660	-
HCM Lane V/C Ratio	0.046	0.007	-	-	-	-
HCM Control Delay (s)	27.7	12.3	-	-	0	-
HCM Lane LOS	D	B	-	-	A	-
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-

Buildout With Project Conditions

PM Peak Hour

Lanes, Volumes, Timings
1: University Blvd SE & Stryker Rd

Buildout With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	15	6	200	200	0	0	0	0	137	1213	137
Future Volume (vph)	0	15	6	200	200	0	0	0	0	137	1213	137
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	0	1786	0	0	1818	0	0	0	0	0	3472	0
Flt Permitted					0.830						0.995	
Satd. Flow (perm)	0	1786	0	0	1546	0	0	0	0	0	3472	0
Right Turn on Red			Yes			Yes				Yes		Yes
Satd. Flow (RTOR)		7									25	
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		518			474			764			758	
Travel Time (s)		10.1			9.2			14.9			14.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	23	0	0	434	0	0	0	0	0	1616	0
Turn Type		NA		Perm	NA					Split	NA	
Protected Phases		4			8					6	6	
Permitted Phases				8								
Total Split (s)		26.0		26.0	26.0					39.0	39.0	
Total Lost Time (s)		4.5			4.5						4.5	
Act Effct Green (s)		20.1			20.1						33.0	
Actuated g/C Ratio		0.32			0.32						0.53	
v/c Ratio		0.04			0.87						0.87	
Control Delay		12.3			40.9						19.7	
Queue Delay		0.0			0.0						0.0	
Total Delay		12.3			40.9						19.7	
LOS		B			D						B	
Approach Delay		12.3			40.9						19.7	
Approach LOS		B			D						B	

Intersection Summary

Area Type: Other

Cycle Length: 65

Actuated Cycle Length: 62.2

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.87

Intersection Signal Delay: 24.0

Intersection LOS: C

Intersection Capacity Utilization 77.6%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: University Blvd SE & Stryker Rd



Lanes, Volumes, Timings
2: University Blvd SE & Stryker Rd

Buildout With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	108	0	0	400	368	0	261	40	0	0	0
Future Volume (vph)	0	108	0	0	400	368	0	261	40	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	0	1863	0	0	1742	0	0	3468	0	0	0	0
Flt Permitted												
Satd. Flow (perm)	0	1863	0	0	1742	0	0	3468	0	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					121			29				
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		474			880			736			768	
Travel Time (s)		9.2			17.1			14.3			15.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	117	0	0	835	0	0	327	0	0	0	0
Turn Type		NA			NA			NA				
Protected Phases		4			8			2				
Permitted Phases	4						2					
Total Split (s)	37.1	37.1			37.1		22.9	22.9				
Total Lost Time (s)		4.5			4.5			4.5				
Act Effct Green (s)		25.0			25.0			9.7				
Actuated g/C Ratio		0.57			0.57			0.22				
v/c Ratio		0.11			0.80			0.42				
Control Delay		4.8			14.1			16.5				
Queue Delay		0.0			0.0			0.0				
Total Delay		4.8			14.1			16.5				
LOS		A			B			B				
Approach Delay		4.8			14.1			16.5				
Approach LOS		A			B			B				

Intersection Summary

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 44.1

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.80

Intersection Signal Delay: 13.9

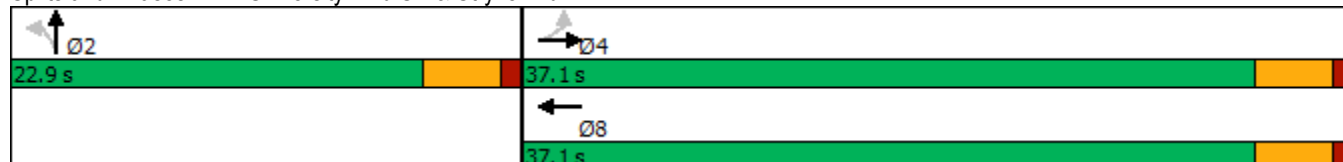
Intersection LOS: B

Intersection Capacity Utilization 59.5%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: University Blvd SE & Stryker Rd



Lanes, Volumes, Timings

3: University Blvd SE & Strand Loop SE/Gate A

Buildout With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	40	0	40	1	0	29	65	628	7	6	1563	164
Future Volume (vph)	40	0	40	1	0	29	65	628	7	6	1563	164
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	130		0	115		0
Storage Lanes	1		0	0		1	1		0	1		0
Taper Length (ft)	25			25			80			80		
Satd. Flow (prot)	1770	1583	0	0	1770	1583	1770	3532	0	1770	3490	0
Flt Permitted	0.757				0.729		0.950			0.950		
Satd. Flow (perm)	1410	1583	0	0	1358	1583	1770	3532	0	1770	3490	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		115				73		2			21	
Link Speed (mph)		30			30			35			35	
Link Distance (ft)		485			304			432			470	
Travel Time (s)		11.0			6.9			8.4			9.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	43	43	0	0	1	32	71	691	0	7	1877	0
Turn Type	Perm	NA		Perm	NA	Perm	Prot	NA		Prot	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8						
Total Split (s)	22.6	22.6		22.6	22.6	22.6	10.0	57.8		9.6	57.4	
Total Lost Time (s)	4.5	4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Act Effct Green (s)	8.1	8.1			8.1	8.1	5.9	59.6		5.4	54.1	
Actuated g/C Ratio	0.11	0.11			0.11	0.11	0.08	0.80		0.07	0.73	
v/c Ratio	0.28	0.16			0.01	0.13	0.51	0.24		0.05	0.74	
Control Delay	38.2	1.2			32.0	2.4	51.6	3.5		37.2	11.4	
Queue Delay	0.0	0.0			0.0	0.0	0.0	0.0		0.0	1.1	
Total Delay	38.2	1.2			32.0	2.4	51.6	3.5		37.2	12.5	
LOS	D	A			C	A	D	A		D	B	
Approach Delay		19.7			3.3			8.0			12.6	
Approach LOS		B			A			A			B	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 74.1

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 11.4

Intersection LOS: B

Intersection Capacity Utilization 70.4%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 3: University Blvd SE & Strand Loop SE/Gate A

		
Ø1	Ø2	Ø4
9.6 s	57.8 s	22.6 s
		
Ø5	Ø6	Ø8
10 s	57.4 s	22.6 s

Lanes, Volumes, Timings
4: University Blvd SE & Avedon Ave SE/Gate B

Buildout With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	40	0	40	1	0	29	80	611	6	9	1682	7
Future Volume (vph)	40	0	40	1	0	29	80	611	6	9	1682	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	125		0	115		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	25			25			80			25		
Satd. Flow (prot)	0	1694	0	0	1770	1583	1770	3532	0	1770	3536	0
Flt Permitted		0.842			0.715		0.097			0.397		
Satd. Flow (perm)	0	1462	0	0	1332	1583	181	3532	0	740	3536	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		32				32		3				1
Link Speed (mph)		30			30			35				35
Link Distance (ft)		260			279			470				1252
Travel Time (s)		5.9			6.3			9.2				24.4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	86	0	0	1	32	87	671	0	10	1836	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		
Total Split (s)	22.6	22.6		22.6	22.6	22.6	67.4	67.4		67.4	67.4	
Total Lost Time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Act Effect Green (s)		8.8			8.8	8.8	71.0	71.0		71.0	71.0	
Actuated g/C Ratio		0.10			0.10	0.10	0.83	0.83		0.83	0.83	
v/c Ratio		0.48			0.01	0.17	0.58	0.23		0.02	0.63	
Control Delay		33.7			32.0	13.9	25.6	2.5		2.6	5.0	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		33.7			32.0	13.9	25.6	2.5		2.6	5.0	
LOS		C			C	B	C	A		A	A	
Approach Delay		33.7			14.5			5.1			5.0	
Approach LOS		C			B			A			A	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 85.5

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.63

Intersection Signal Delay: 6.1

Intersection LOS: A

Intersection Capacity Utilization 73.7%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 4: University Blvd SE & Avedon Ave SE/Gate B



Lanes, Volumes, Timings
5: University Blvd SE & Bobby Foster Rd/Eastman Crossing

Buildout With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	112	737	26	393	1208	86	40	375	74	32	1207	438
Future Volume (vph)	112	737	26	393	1208	86	40	375	74	32	1207	438
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		300	200		200	325		0	325		325
Storage Lanes	2		1	2		1	1		0	2		1
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	3433	3539	1583	3433	3539	1583	1770	3451	0	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3539	1583	3433	3539	1583	1770	3451	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			149			104			24			159
Link Speed (mph)		40			35			35			35	
Link Distance (ft)		1786			774			1252			1357	
Travel Time (s)		30.4			15.1			24.4			26.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	122	801	28	427	1313	93	43	488	0	35	1312	476
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8						6
Total Split (s)	9.5	35.3	35.3	19.2	45.0	45.0	9.5	46.0		9.5	46.0	46.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	5.0	30.8	30.8	14.7	40.6	40.6	5.0	43.4		5.0	41.6	41.6
Actuated g/C Ratio	0.05	0.28	0.28	0.14	0.38	0.38	0.05	0.40		0.05	0.38	0.38
v/c Ratio	0.77	0.79	0.05	0.91	0.99	0.14	0.52	0.35		0.22	0.96	0.67
Control Delay	81.7	43.0	0.2	72.2	56.9	4.3	74.5	22.8		54.1	50.8	23.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	81.7	43.0	0.2	72.2	56.9	4.3	74.5	22.8		54.1	50.8	23.9
LOS	F	D	A	E	E	A	E	C		D	D	C
Approach Delay		46.7			57.8			27.0			43.9	
Approach LOS		D			E			C			D	

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 108.1

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 47.6

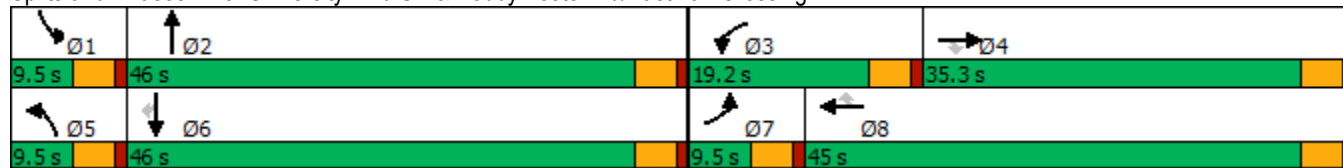
Intersection LOS: D

Intersection Capacity Utilization 82.2%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 5: University Blvd SE & Bobby Foster Rd/Eastman Crossing



Lanes, Volumes, Timings
6: University Blvd SE & Fritts Crossing SE

Buildout With Project
PM Peak Hour

	↖	↗	↑	↘	↙	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		T		T	T
Traffic Volume (vph)	227	321	430	22	21	1481
Future Volume (vph)	227	321	430	22	21	1481
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	120	
Storage Lanes	1	0		0	1	
Taper Length (ft)	25				25	
Satd. Flow (prot)	1681	0	3514	0	1770	3539
Flt Permitted	0.980				0.461	
Satd. Flow (perm)	1681	0	3514	0	859	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	112		11			
Link Speed (mph)	25		35			35
Link Distance (ft)	832		739			284
Travel Time (s)	22.7		14.4			5.5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)						
Lane Group Flow (vph)	596	0	491	0	23	1610
Turn Type	Prot		NA		Perm	NA
Protected Phases	8		2			6
Permitted Phases					6	
Total Split (s)	29.0		41.0		41.0	41.0
Total Lost Time (s)	4.5		4.5		4.5	4.5
Act Effct Green (s)	23.0		34.9		34.9	34.9
Actuated g/C Ratio	0.34		0.52		0.52	0.52
v/c Ratio	0.92		0.27		0.05	0.87
Control Delay	39.4		9.4		8.7	21.3
Queue Delay	0.0		0.0		0.0	0.0
Total Delay	39.4		9.4		8.7	21.3
LOS	D		A		A	C
Approach Delay	39.4		9.4			21.1
Approach LOS	D		A			C

Intersection Summary

Area Type: Other

Cycle Length: 70

Actuated Cycle Length: 67

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 23.0

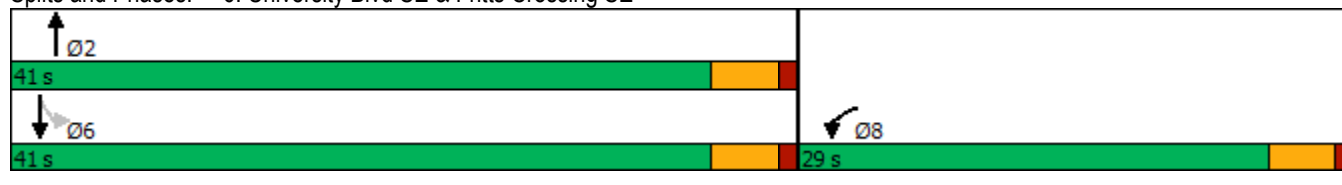
Intersection LOS: C

Intersection Capacity Utilization 80.7%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 6: University Blvd SE & Fritts Crossing SE



Lanes, Volumes, Timings
7: University Blvd SE & Crick Ave SE

Buildout With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	183	25	477	400	855	67	384	480	469	852	44
Future Volume (vph)	25	183	25	477	400	855	67	384	480	469	852	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	350		0	180		500	600		0
Storage Lanes	1		0	2		1	1		1	2		0
Taper Length (ft)	25			25			25			120		
Satd. Flow (prot)	1770	1829	0	3433	1863	1583	1770	3539	1583	3433	3514	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	1829	0	3433	1863	1583	1770	3539	1583	3433	3514	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		6				458			522			5
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		276			576			790			926	
Travel Time (s)		5.4			11.2			15.4			18.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	27	226	0	518	435	929	73	417	522	510	974	0
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			
Total Split (s)	9.5	36.0		30.5	57.0	57.0	11.0	29.5	29.5	24.0	42.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	
Act Effct Green (s)	5.1	29.9		21.7	50.9	50.9	6.6	22.7	22.7	19.0	35.1	
Actuated g/C Ratio	0.05	0.27		0.19	0.46	0.46	0.06	0.20	0.20	0.17	0.31	
v/c Ratio	0.34	0.46		0.78	0.51	0.96	0.70	0.58	0.71	0.87	0.88	
Control Delay	67.2	37.8		52.0	25.4	36.5	88.8	44.6	9.4	63.1	46.8	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	67.2	37.8		52.0	25.4	36.5	88.8	44.6	9.4	63.1	46.8	
LOS	E	D		D	C	D	F	D	A	E	D	
Approach Delay		40.9			38.2			29.7			52.4	
Approach LOS		D			D			C			D	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 111.6

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.96

Intersection Signal Delay: 41.1

Intersection LOS: D

Intersection Capacity Utilization 79.0%

ICU Level of Service D

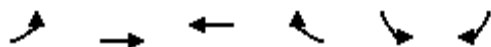
Analysis Period (min) 15

Splits and Phases: 7: University Blvd SE & Crick Ave SE

			
Ø1	Ø2	Ø3	Ø4
24 s	29.5 s	30.5 s	36 s
			
Ø5	Ø6	Ø7	Ø8
11 s	42.5 s	9.5 s	57 s

Lanes, Volumes, Timings
8: Eastman Crossing/Gate D & Watson Dr SE

Buildout With Project
PM Peak Hour



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	523	4	25	5	9	975
Future Volume (vph)	523	4	25	5	9	975
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1681	1686	3458	0	1770	1583
Flt Permitted	0.950	0.953			0.950	
Satd. Flow (perm)	1681	1686	3458	0	1770	1583
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			5			1014
Link Speed (mph)		35	30		25	
Link Distance (ft)		433	915		1159	
Travel Time (s)		8.4	20.8		31.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)	50%					
Lane Group Flow (vph)	284	288	32	0	10	1060
Turn Type	Split	NA	NA		Perm	Over
Protected Phases	4	4	8			4
Permitted Phases					6	
Total Split (s)	35.0	35.0	22.5		22.5	35.0
Total Lost Time (s)	4.5	4.5	4.5		4.5	4.5
Act Effct Green (s)	22.5	22.5	6.4		6.3	22.5
Actuated g/C Ratio	0.54	0.54	0.15		0.15	0.54
v/c Ratio	0.32	0.32	0.06		0.04	0.80
Control Delay	7.2	7.2	19.3		21.9	7.2
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	7.2	7.2	19.3		21.9	7.2
LOS	A	A	B		C	A
Approach Delay		7.2	19.3		7.4	
Approach LOS		A	B		A	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 41.9

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.80

Intersection Signal Delay: 7.5

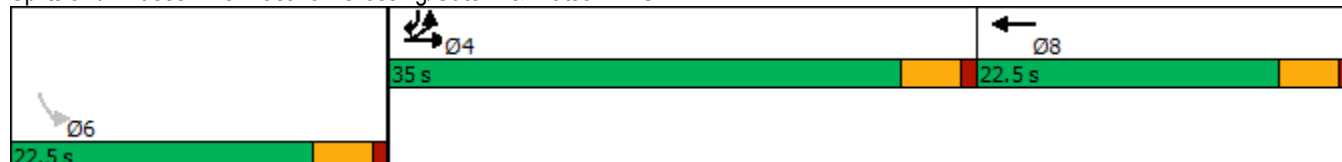
Intersection LOS: A

Intersection Capacity Utilization 72.0%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 8: Eastman Crossing/Gate D & Watson Dr SE



Lanes, Volumes, Timings
9: Mesa Del Sol Blvd & Gate E/Eastman Crossing

Buildout With Project
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	13	4	13	340	3	85	5	1053	200	25	1588	5
Future Volume (vph)	13	4	13	340	3	85	5	1053	200	25	1588	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	125		0	200		0	200		200
Storage Lanes	1		0	1		0	1		0	1		1
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	1645	0	1770	1593	0	1770	3454	0	1770	3539	1583
Flt Permitted	0.695			0.746			0.950			0.950		
Satd. Flow (perm)	1295	1645	0	1390	1593	0	1770	3454	0	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14			92			36				73
Link Speed (mph)		30			35			35				35
Link Distance (ft)		419			491			470				342
Travel Time (s)		9.5			9.6			9.2				6.7
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	14	18	0	370	95	0	5	1362	0	27	1726	5
Turn Type	Perm	NA		Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8								6
Total Split (s)	29.4	29.4		29.4	29.4		9.5	51.0		9.6	51.1	51.1
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	24.8	24.8		24.8	24.8		5.0	45.6		5.1	47.4	47.4
Actuated g/C Ratio	0.30	0.30		0.30	0.30		0.06	0.55		0.06	0.57	0.57
v/c Ratio	0.04	0.04		0.89	0.18		0.05	0.71		0.25	0.85	0.01
Control Delay	23.5	13.6		55.5	7.0		40.8	16.7		45.4	21.0	0.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	23.5	13.6		55.5	7.0		40.8	16.7		45.4	21.0	0.0
LOS	C	B		E	A		D	B		D	C	A
Approach Delay		18.0			45.6			16.8			21.4	
Approach LOS		B			D			B			C	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 83

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 22.7

Intersection LOS: C

Intersection Capacity Utilization 76.9%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 9: Mesa Del Sol Blvd & Gate E/Eastman Crossing

9.6 s	51 s	29.4 s	29.4 s
9.5 s	51.1 s	29.4 s	29.4 s

Intersection	
Intersection Delay, s/veh	10
Intersection LOS	A

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	13	0	10	20	322	0
Future Vol, veh/h	13	0	10	20	322	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	14	0	11	22	350	0
Number of Lanes	1	0	1	1	1	0

Approach	EB	WB	NB
Opposing Approach	WB	EB	
Opposing Lanes	2	1	0
Conflicting Approach Left		NB	EB
Conflicting Lanes Left	0	1	1
Conflicting Approach Right	NB		WB
Conflicting Lanes Right	1	0	2
HCM Control Delay	8	8.4	10.2
HCM LOS	A	A	B

Lane	NBLn1	EBLn1	WBLn1	WBLn2
Vol Left, %	100%	0%	100%	0%
Vol Thru, %	0%	100%	0%	100%
Vol Right, %	0%	0%	0%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	322	13	10	20
LT Vol	322	0	10	0
Through Vol	0	13	0	20
RT Vol	0	0	0	0
Lane Flow Rate	350	14	11	22
Geometry Grp	2	5	7	7
Degree of Util (X)	0.41	0.019	0.018	0.032
Departure Headway (Hd)	4.214	4.897	5.867	5.363
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	851	735	614	672
Service Time	2.266	2.899	3.567	3.063
HCM Lane V/C Ratio	0.411	0.019	0.018	0.033
HCM Control Delay	10.2	8	8.7	8.2
HCM Lane LOS	B	A	A	A
HCM 95th-tile Q	2	0.1	0.1	0.1

Lanes, Volumes, Timings
11: Gate G/Hawking Dr SE & Crick Ave SE

Buildout With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	51	900	8	2	576	30	20	3	7	243	3	750
Future Volume (vph)	51	900	8	2	576	30	20	3	7	243	3	750
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	300		0	150		0	0		0	0		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	3536	0	1770	3511	0	1770	1660	0	1770	1585	0
Flt Permitted	0.950			0.950			0.123			0.750		
Satd. Flow (perm)	1770	3536	0	1770	3511	0	229	1660	0	1397	1585	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			7			8			256	
Link Speed (mph)		35			35			30			35	
Link Distance (ft)		2075			1380			306			1115	
Travel Time (s)		40.4			26.9			7.0			21.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	55	987	0	2	659	0	22	11	0	264	818	0
Turn Type	Prot	NA		Prot	NA		Perm	NA		Perm	NA	
Protected Phases	7	4		3	8			2			6	
Permitted Phases							2			6		
Total Split (s)	9.6	26.3		9.6	26.3		39.1	39.1		39.1	39.1	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Act Effct Green (s)	5.2	24.9		5.2	21.4		32.5	32.5		32.5	32.5	
Actuated g/C Ratio	0.08	0.36		0.08	0.31		0.48	0.48		0.48	0.48	
v/c Ratio	0.41	0.76		0.01	0.60		0.20	0.01		0.40	0.92	
Control Delay	42.7	25.8		33.0	23.8		17.7	7.4		14.6	30.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	42.7	25.8		33.0	23.8		17.7	7.4		14.6	30.5	
LOS	D	C		C	C		B	A		B	C	
Approach Delay		26.7			23.8			14.3			26.6	
Approach LOS		C			C			B			C	

Intersection Summary

Area Type: Other

Cycle Length: 75

Actuated Cycle Length: 68.3

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 25.8

Intersection LOS: C

Intersection Capacity Utilization 87.1%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 11: Gate G/Hawking Dr SE & Crick Ave SE

		
Ø2	Ø3	Ø4
39.1 s	9.6 s	26.3 s
		
Ø6	Ø7	Ø8
39.1 s	9.6 s	26.3 s

Lanes, Volumes, Timings
12: Gate H/Molina Rd & Crick Ave SE

Buildout With Project
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	109	1190	8	3	301	15	20	3	7	15	2	114
Future Volume (vph)	109	1190	8	3	301	15	20	3	7	15	2	114
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	150		0	0		0	0		0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1770	3536	0	1770	3514	0	1770	1660	0	0	1633	0
Flt Permitted	0.546			0.165			0.827				0.962	
Satd. Flow (perm)	1017	3536	0	307	3514	0	1540	1660	0	0	1581	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2			12			8			124	
Link Speed (mph)		35			35			30			25	
Link Distance (ft)		1380			1210			256			1120	
Travel Time (s)		26.9			23.6			5.8			30.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Lane Group Flow (vph)	118	1302	0	3	343	0	22	11	0	0	142	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Total Split (s)	40.0	40.0		40.0	40.0		25.0	25.0		25.0	25.0	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5			4.5	
Act Effct Green (s)	24.3	24.3		24.3	24.3		7.0	7.0			7.0	
Actuated g/C Ratio	0.60	0.60		0.60	0.60		0.17	0.17			0.17	
v/c Ratio	0.19	0.62		0.02	0.16		0.08	0.04			0.38	
Control Delay	4.5	6.5		3.7	3.6		18.1	12.8			9.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Total Delay	4.5	6.5		3.7	3.6		18.1	12.8			9.2	
LOS	A	A		A	A		B	B			A	
Approach Delay		6.4			3.6			16.3			9.2	
Approach LOS		A			A			B			A	

Intersection Summary

Area Type: Other

Cycle Length: 65

Actuated Cycle Length: 40.7

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.62

Intersection Signal Delay: 6.2

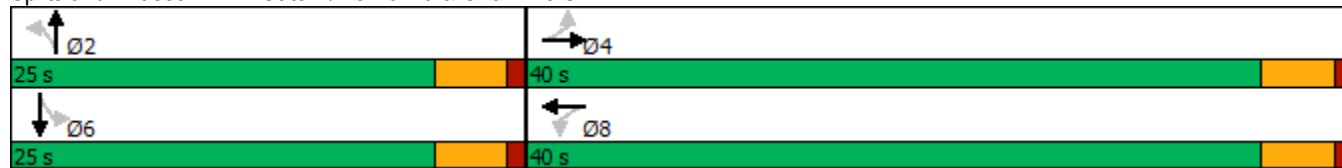
Intersection LOS: A

Intersection Capacity Utilization 63.2%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 12: Gate H/Molina Rd & Crick Ave SE



HCM 6th TWSC
13: Gate C & Eastman Crossing

Builldout With Project
PM Peak Hour

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↔↑	↔	↔
Traffic Vol, veh/h	625	13	0	1050	20	10
Future Vol, veh/h	625	13	0	1050	20	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	679	14	0	1141	22	11
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	693	0	1257	347
Stage 1	-	-	-	-	686	-
Stage 2	-	-	-	-	571	-
Critical Hdwy	-	-	4.14	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	-	-	2.22	-	3.52	3.32
Pot Cap-1 Maneuver	-	-	898	-	163	649
Stage 1	-	-	-	-	461	-
Stage 2	-	-	-	-	529	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	898	-	163	649
Mov Cap-2 Maneuver	-	-	-	-	163	-
Stage 1	-	-	-	-	461	-
Stage 2	-	-	-	-	529	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		23.9	
HCM LOS					C	
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	163	649	-	-	898	-
HCM Lane V/C Ratio	0.133	0.017	-	-	-	-
HCM Control Delay (s)	30.5	10.6	-	-	0	-
HCM Lane LOS	D	B	-	-	A	-
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0	-