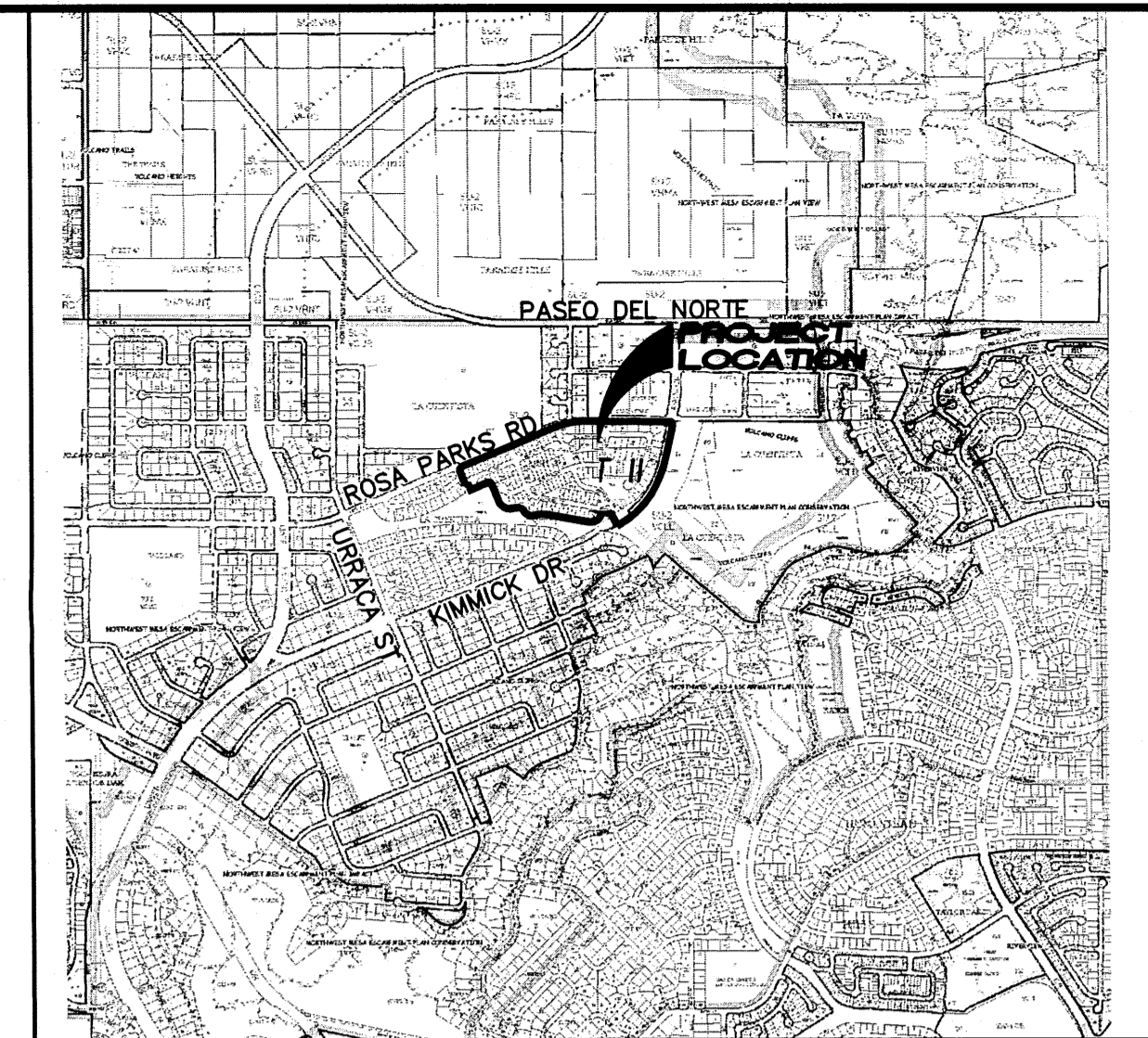


CONSTRUCTION PLANS FOR LA CUENTISTA SUBDIVISION - UNIT II OFFSITE PUBLIC INFRASTRUCTURE IMPROVEMENTS



VICINITY MAP
ZONE ATLAS MAP NO. C-10 & C-11

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NOTES

1. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, 1986 EDITION, AS AMENDED THROUGH UPDATE #9, INCL-AMENDMENT #1.
2. THE CONTRACTOR WILL BE RESPONSIBLE FOR COORDINATING WITH THE WATER AUTHORITY SEVEN (7) DAYS IN ADVANCE OF PERFORMING WORK THAT WILL AFFECT THE PUBLIC WATER AND SANITARY SEWER INFRASTRUCTURE. WORK REQUIRING SHUT-OFF OF FACILITIES DESIGNATED AS MASTER PLAN FACILITIES MUST BE COORDINATED WITH THE WATER AUTHORITY FOURTEEN (14) DAYS IN ADVANCE OF PERFORMING SUCH WORK. ONLY WATER AUTHORITY CREWS ARE AUTHORIZED TO OPERATE PUBLIC VALVES. SHUT OFF REQUESTS MUST BE MADE ONLINE AT http://abcwua.org/Water_Shut_off_and_Turn_on_Procedures.aspx.
3. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, THE CONTRACTOR MUST CONTACT NEW MEXICO ONE CALLS SYSTEM (260-1990) FOR LOCATION OF EXISTING UTILITIES.
4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER OR SURVEYOR IMMEDIATELY SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
5. FIVE (5) WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION, THE CONTRACTOR SHALL SUBMIT TO CONSTRUCTION COORDINATION DIVISION A DETAILED CONSTRUCTION SCHEDULE. TWO (2) WORKING DAYS PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL OBTAIN A BARRICADING PERMIT FROM THE CONST. COORDINATION DIVISION. CONTRACTOR SHALL NOTIFY BARRICADE ENGINEER (924-3400) PRIOR TO OCCUPYING AN INTERSECTION. REFER TO SECTION 19 OF THE GENERAL CONDITIONS OF THE STANDARD SPECIFICATIONS.
6. ALL STREET STRIPING ALTERED OR DESTROYED SHALL BE REPLACED WITH PLASTIC REFLECTORIZED PAVEMENT MARKINGS BY CONTRACTOR TO LOCATION AS EXISTING OR AS INDICATED BY THIS PLAN SET.
7. CONTRACTOR SHALL NOTIFY THE ENGINEER NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT THE ENGINEER MAY TAKE NECESSARY MEASURES TO INSURE THE PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT THE CONSENT OF THE CITY SURVEYOR AND SHALL NOTIFY THE ENGINEER AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED WITHOUT PERMISSION. REPLACEMENT SHALL BE DONE ONLY BY THE CITY SURVEYOR. WHEN A CHANGE IS MADE IN THE FINISHED ELEVATIONS OF THE PAVEMENT OF ANY ROADWAY IN WHICH A PERMANENT SURVEY MONUMENT IS LOCATED, CONTRACTOR SHALL, AT HIS OWN EXPENSE, ADJUST THE MONUMENT COVER TO THE NEW GRADE UNLESS OTHERWISE SPECIFIED. REFER TO SECTION 4.4 OF THE GENERAL CONDITIONS OF THE STANDARD SPECIFICATIONS UPDATE NO. 9, INCLUDING AMENDMENT NO. 1.
8. CONTRACTOR WILL BE RESPONSIBLE FOR PROVIDING AND MAINTAINING ALL SIGNING UNTIL PROJECT HAS BEEN ACCEPTED BY THE CITY.
9. ANY WORK AFFECTING AN ARTERIAL ROADWAY REQUIRES TWENTY-FOUR(24) HOUR CONSTRUCTION.
10. CONTRACTOR SHALL MAINTAIN A GRAFFITI-FREE WORK SITE. CONTRACTOR SHALL PROMPTLY REMOVE ANY GRAFFITI FROM ALL EQUIPMENT, WHETHER PERMANENT OR TEMPORARY.

11. THE FOLLOWING NOTES ALSO APPLY WHEN CHECKED:
- ALL UTILITIES AND UTILITY SERVICE LINES SHALL BE INSTALLED PRIOR TO PAVING.
 - BACKFILL COMPACTION SHALL BE ACCORDING TO SPECIFIED STREET USE.
 - TACK COAT REQUIREMENTS SHALL BE DETERMINED BY THE ENGINEER.
 - SIDEWALKS AND WHEELCHAIR RAMPS WITHIN THE CURB RETURNS SHALL BE CONSTRUCTED WHEREVER A NEW CURB RETURN IS CONSTRUCTED.
 - IF CURB IS DEPRESSED FOR A DRIVEPAD, THE DRIVEPAD SHALL BE CONSTRUCTED PRIOR TO ACCEPTANCE OF CURB AND GUTTER.
 - ALL STORM DRAINAGE FACILITIES SHALL BE COMPLETED PRIOR TO FINAL ACCEPTANCE. RCP PIPE JOINTS SHALL NOT BE GROUTED PRIOR TO FINAL INSPECTION. FINAL INSPECTION WILL DETERMINE JOINTS TO BE GROUTED FOR FINAL ACCEPTANCE.

WILSON & COMPANY

4900 LANG AVE NE
ALBUQUERQUE, NEW MEXICO
87109
(505) 348-4000

REV	SHEETS	CITY ENGINEER	DATE	USER DEPARTMENT	DATE	USER DEPARTMENT	DATE
ENGINEERS STAMP & SIGNATURE APPROVALS ENGINEER DATE *****							
		DRC Chairman	<i>[Signature]</i>	1/8/15	APPROVED FOR CONSTRUCTION <i>[Signature]</i> 3/1/17 City Engineer Date		
		Transportation	<i>[Signature]</i>	04-06-15			
		ABCWUA	<i>[Signature]</i>	2-27-15			
		Hydrology	<i>[Signature]</i>	4-16-15			
		Transportation	<i>[Signature]</i>	2/27/17			
		Constr. Mngmt.					
		Constr. Coord.					
				City Project No.	709786	Drawing	Sheet
						1	2017

DRB# 1005182
WILSON# 1460008900

GENERAL NOTES:

1. THE CONTRACTOR SHALL NOTIFY THE ALBUQUERQUE TRAFFIC ENGINEERING DIVISION THREE (3) WORKING DAYS IN ADVANCE OF ANY WORK REQUIRED REGARDING ALL EXISTING REGULATORY SIGNS AND SIGNALS THAT NEED TO BE REMOVED, RELOCATED, OR REINSTALLED. CALL 857-8680. REFER TO SECTION 18.4.4 OF THE SPECIFICATIONS.
2. THE CONTRACTOR WILL BE REQUIRED TO CONFINED HIS WORK WITHIN THE CONSTRUCTION LIMITS AND/OR R.O.W. TO PRESERVE EXISTING VEGETATION AND PRIVATE PROPERTY. OVERNIGHT PARKING OF CONSTRUCTION EQUIPMENT SHALL NOT OBSTRUCT DRIVEWAY OPENINGS OR DESIGNATED TRAFFIC LANES.
3. THE CONTRACTOR SHALL PROVIDE AND MAINTAIN A SAFE AND ADEQUATE MEANS OF CHANNELING PEDESTRIAN TRAFFIC AROUND ALL WORK AREAS THROUGHOUT THE CONSTRUCTION PERIOD.
4. A DISPOSAL SITE, COMPLYING WITH ALL CITY ORDINANCES, SHALL BE OBTAINED BY THE CONTRACTOR FOR THE DISPOSAL OF ALL EXCESS EXCAVATION MATERIAL, ASPHALTIC PAVEMENT (EXCEPT MILLED BITUMINOUS CONCRETE PAVEMENT) AND OTHER WASTE MATERIALS. THE CONTRACTOR SHALL NOTIFY THE CITY OF ALBUQUERQUE OF THE LOCATION OF THE DISPOSAL SITE PRIOR TO THE REMOVAL AND ACTUAL DISPOSAL OF THE MATERIAL. ALL COSTS IN OBTAINING A DISPOSAL SITE AND HAUL THERETO SHALL BE CONSIDERED INCIDENTAL TO CONSTRUCTION AND NO DIRECT PAYMENT WILL BE MADE THEREFOR.
5. CONTRACTOR TO CAREFULLY FIELD REVIEW SITE TO ASSESS EXTENT OF TRASH REMOVAL. TRASH REMOVAL SHALL BE INCLUDED IN BID ITEM "CLEARING & GRUBBING".
6. ALL VALLEY GUTTERS ARE 6' WIDE UNLESS OTHERWISE NOTED.
7. UNLESS OTHERWISE SHOWN, ALL DIMENSIONS ARE TO FACE OF CURB, INCLUDING RADII OF CURB RETURNS.
8. ALL FINISHED GRADES AND PROFILES SHOWN ARE FLOWLINE GRADES, UNLESS OTHERWISE NOTED.
9. CURB AND GUTTER SHOWN AS EXISTING AND NOT TO BE REMOVED UNDER THIS CONTRACT WHICH IS DAMAGED OR DISPLACED BY THE CONTRACTOR SHALL BE REMOVED AND REPLACED PER STD DWG 2415 BY THE CONTRACTOR AT HIS EXPENSE.
10. WHEN REMOVAL OF EXISTING CURB AND GUTTER OR SIDEWALK IS REQUIRED, REMOVE TO NEAREST JOINT.
11. WHEN ABUTTING NEW PAVEMENT TO EXISTING PAVEMENT, SAW CUT EXISTING PAVEMENT IN A STRAIGHT LINE AS REQUIRED TO REMOVE ANY BROKEN OR CRACKED PAVEMENT. PRIOR TO LAYING NEW PAVEMENT, THE EXPOSED EXISTING ASPHALT EDGE SHALL BE TACK-COATED.
12. WHEELCHAIR RAMPS SHALL BE CONSTRUCTED AT ALL CURB RETURNS CONSTRUCTED WITH THIS PROJECT AT THE LOCATIONS SHOWN ON PLANS, IN ACCORDANCE WITH THE CITY STANDARD DETAILS.
13. ALL WHEELCHAIR RAMP APPROACH SLOPES SHOWN ON STD DWG 2441 SHALL BE INCREASED IN LENGTH TO MAINTAIN 12:1 MAX SLOPE WHEN GRADE IS RISING FROM RAMP. SEE TABLE BELOW.

RISING GRADE	RAMP LENGTH
0%	8.1'
0-1%	9.2'
1-2%	10.6'
2-3%	12.6'
3-4%	15.5'

14. CONTRACTOR IS TO EXERCISE DUE CARE TO AVOID DISTURBING ANY EXISTING UTILITIES. IT SHALL BE HIS RESPONSIBILITY TO COORDINATE WITH THE UTILITY COMPANIES IN ORDER TO PREVENT ANY SERVICE DISRUPTION THAT MIGHT RESULT FROM PROJECT CONSTRUCTION. IT SHALL BE HIS RESPONSIBILITY TO PROTECT AND PRESERVE UTILITY EQUIPMENT AFFECTED BY PROJECT CONSTRUCTION. SHOULD HE BREAK AN EXISTING UTILITY LINE DURING CONSTRUCTION ACTIVITIES HE SHALL BE RESPONSIBLE FOR UTILITY REPAIR COSTS.
15. ALL EXISTING UTILITIES SHOWN HEREIN WERE TAKEN FROM RECORD DRAWINGS, POTHOLES, FIELD SURVEYS, C.O.A. SYSTEMS UTILITIES MAPS AND INFORMATION PROVIDED BY THE UTILITY OWNERS AND ARE APPROXIMATE. IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THEIR HORIZONTAL AND VERTICAL LOCATIONS AND PROVIDE PROTECTION FOR ALL UTILITIES WITHIN THE CONSTRUCTION AREA.
16. CONTRACTOR SHALL FIELD VERIFY LOCATION AND TYPE OF EXISTING UTILITIES TO BE ADJUSTED OR EXTENDED.
17. MANHOLE RIM ELEVATIONS SHOWN ON THESE PLANS ARE APPROXIMATE AND WILL VARY WITH THE FINISHED PAVEMENT ELEVATIONS.
18. CONTRACTOR TO VERIFY ALL EXISTING FIRE HYDRANT FLANGES, PADS, VALVE BOXES, MANHOLE RIMS AND TOP OF PIPE ELEVATIONS IN THE FIELD. ELEVATIONS SHALL BE ADJUSTED TO COMPLY WITH THE REQUIREMENTS OF STANDARD COA/ABCWUA DETAILS.

19. ELEVATIONS FOR PROPOSED WATERLINE CONNECTIONS TO EXISTING WATERLINES SHALL BE VERIFIED BY THE CONTRACTOR IN THE FIELD.
20. ALL WATER VALVE BOXES AND SEWER MANHOLES IN THE CONSTRUCTION AREA ARE TO BE ADJUSTED TO FINISH GRADE UNDER THIS CONTRACT PER STD DWG 2460, AND WILL BE PAID FOR AT CONTRACT UNIT PRICES, PER STD DWG 2460.
21. THE CONTRACTOR SHALL NOTIFY THE LOCAL FIRE DEPARTMENT AT LEAST TWO (2) WORKING DAYS IN ADVANCE OF WHEN FIRE HYDRANTS WILL BE TAKEN OUT OF SERVICE AND RETURNED TO SERVICE.
22. ALL EXISTING GAS VALVE BOXES, GAS MANHOLES, ELECTRICAL MANHOLES, AND TELEPHONE MANHOLES WILL BE ADJUSTED TO GRADE BY OTHERS.
23. WHERE PULL BOXES ABUT BACK OF CURB OR ARE LOCATED IN A CONCRETE PAVED AREA, PROVIDE 3/4" EXPANSION MATERIAL AROUND THE PULL BOX. ALL PULL BOXES TO BE FLUSH WITH SURFACE OF CONCRETE.
24. THE CONTRACTOR SHALL MEET ALL THE REQUIREMENTS OF THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) AS WELL AS LOCAL REGULATIONS.
25. THE CONTRACTOR SHALL COORDINATE WITH PRIVATE UTILITY ENTITIES AS NEEDED FOR RELOCATION AND INSTALLATION OF PRIVATE UTILITIES. ENGINEER HAS NOTIFIED THE PRIVATE UTILITIES OF THIS PROJECT.
26. TEMPORARY EROSION FENCING (OR PERMANENT WALLS OR FENCING) SHALL BE USED TO PROTECT EXISTING RESIDENTIAL PROPERTY, STREETS LANDSCAPING AND THE DRAINAGE DRAINAGE SYSTEMS FROM THE DEPOSITION OF ERODED MATERIAL.
27. CONSTRUCTION AREA SHALL BE CONTROLLED TO PREVENT TRESPASS USE BY 4WD'S, ORV'S AND MOTORCYCLES TO MINIMIZE NOISE, DUST, DESTRUCTION OF VEGETATION, WIND AND WATER EROSION.
28. ALL EXCAVATION, TRENCHING AND SHORING ACTIVITIES MUST BE CARRIED OUT IN ACCORDANCE WITH OSHA 29 CFR 1926.650 SUBPART P.
29. INSTALLATION OF REDROOT ST STREET LIGHTS SHALL BE COORDINATED WITH ENGINEER SO AS NOT CONFLICT WITH LOCATION OF FUTURE DRIVE PADS.


UTILITY CONTACTS

COMPANY	PHONE (505)	CONTACT
ABCWUA	289-3301	KRISTOPHER CADENA
PNM - ELECTRIC	241-0536	ROSIE ROMERO
NEW MEXICO GAS CO	697-6802	JOE HERRERA
CENTURYLINK	767-7446	JAMES ARAGON
COMCAST	761-6252	ROBERT MARTINEZ


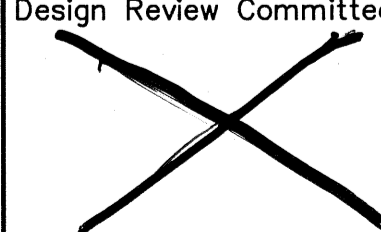
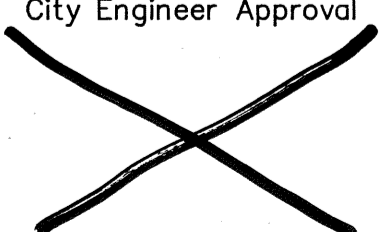
AS-BUILT INFORMATION	
CONTRACTOR	DATE
WORK	DATE
STARTED BY	DATE
ACCEPTANCE BY	DATE
FIELD NOTES BY	DATE
REVISIONS BY	DATE
CHECKED BY	DATE
RECORDED BY	DATE
MICRO-FILM INFORMATION	
NO.	DATE

BENCH MARKS	
ACS 1 3" ALUMINUM DISK STAMPED	ELEVATION = 5270.087 FT.
ACS BM 5-D11, LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EXPOSED TO LAVA ROCK IN OUTCROPPING	NAVD 88 U.S. FEET

SURVEY INFORMATION	
FIELD NOTES	DATE
BY	DATE
AERIAL MAPPING, WCI 05/01	
BOUNDARY, WCI 06/01	
BOUNDARY, WCI 10/01	

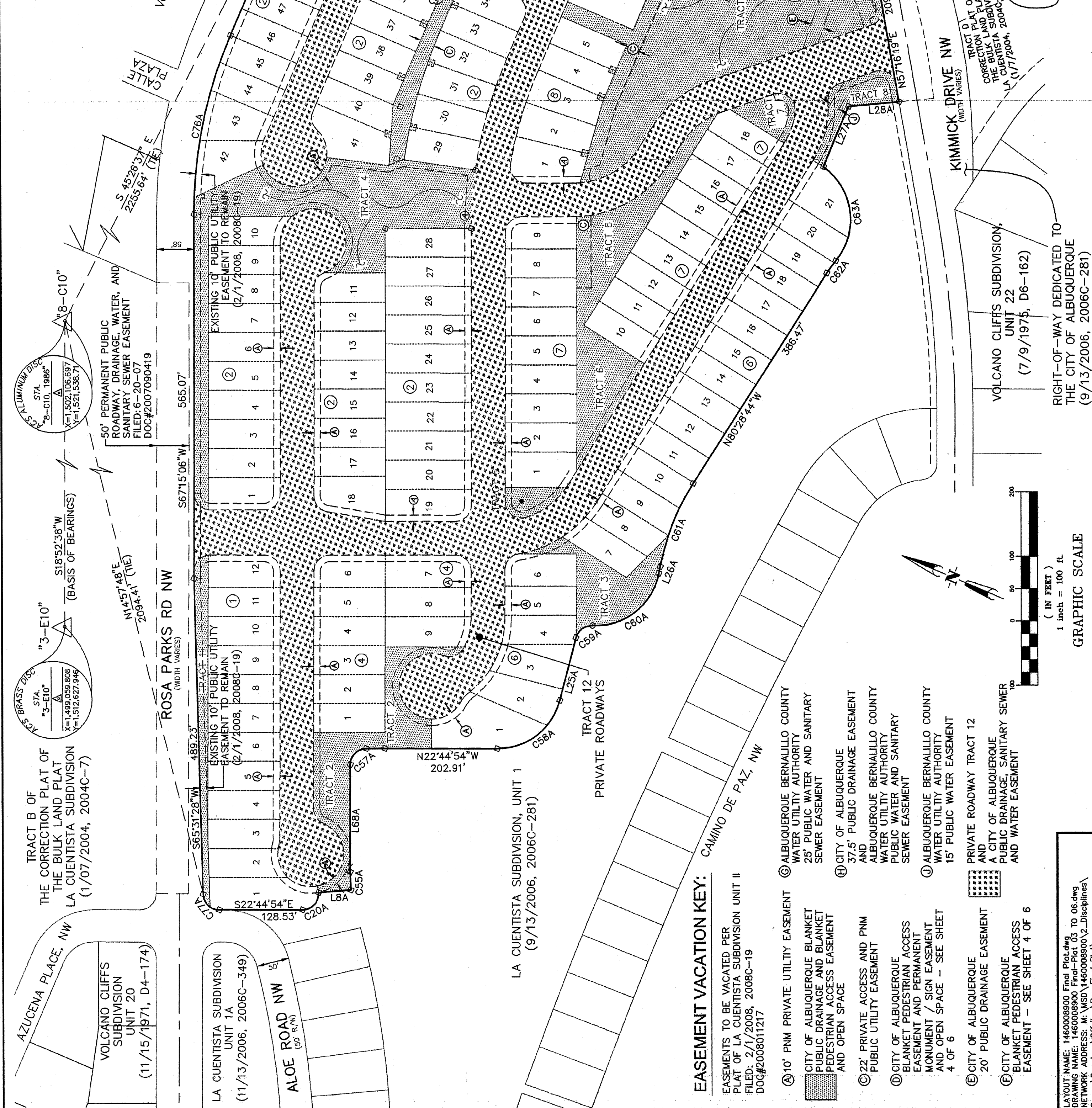
ENGINEER'S SEAL	
	

NO.	DATE	BY
REVISIONS	DATE	BY
WILSON & COMPANY, ENGINEERS & ARCHITECTS	DATE	DATE
DESIGNED BY	DATE	DATE
DRAWN BY	DATE	DATE
CHECKED BY	DATE	DATE

		CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP			
		LA CUENTISTA SUBDIVISION - UNIT II GENERAL NOTES			
Design Review Committee 	City Engineer Approval 	Last Design Update	Mo./Day/Yr.	Mo./Day/Yr.	
City Project No.	Zone Map No.	Drawing	Sheet		
709786	C-10, C-11	2	2017		

WILSON# 1460008900
JANUARY 2015

**VACATION EXHIBIT FOR
REPLAT OF
LA CUENTISTA SUBDIVISION, UNIT II**
WITHIN SECTIONS 15, 22 & 23
T.11N., R.2 E., N.M.P.M.
CITY OF ALBUQUERQUE
BERNALILLO COUNTY, NEW MEXICO
DECEMBER, 2014



**WILSON
& COMPANY**
4900 LANG AVENUE, N.E.
ALBUQUERQUE, NEW MEXICO
87109
(505) 348-4000

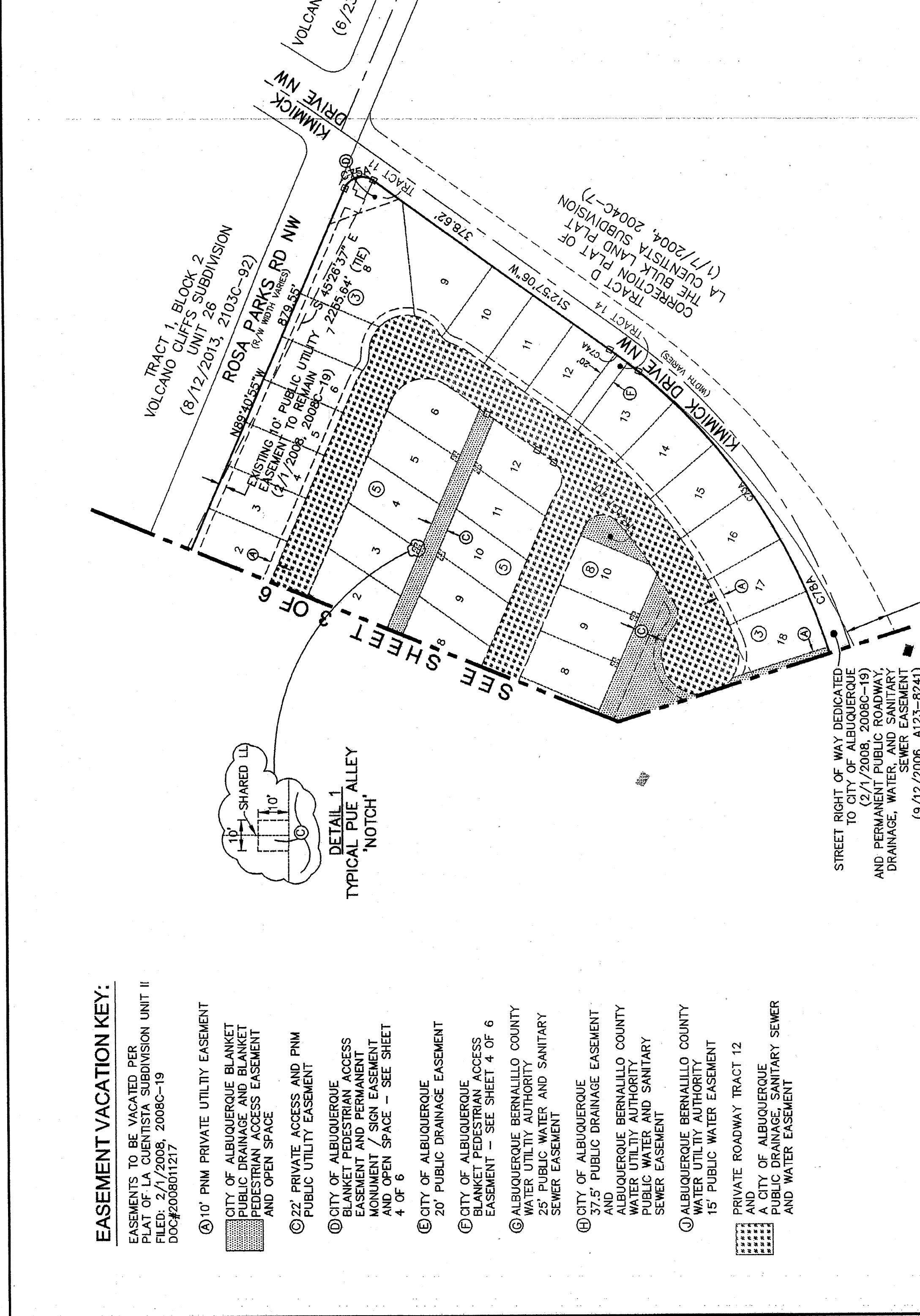
SHEET 3 OF 6
WCI PROJ. NO. 1460008900

EASEMENT VACATION KEY:
EASEMENTS TO BE VACATED PER
PLAT OF LA CUENTISTA SUBDIVISION UNIT II
FILED: 2/1/2008, 2008C-19
DOC#2008011217

- ⑩ 10' PNM PRIVATE UTILITY EASEMENT
- ⑪ CITY OF ALBUQUERQUE BLANKET PEDESTRIAN ACCESS EASEMENT AND OPEN SPACE
- ⑫ 22' PRIVATE ACCESS AND PNM PUBLIC UTILITY EASEMENT
- ⑬ CITY OF ALBUQUERQUE BLANKET PEDESTRIAN ACCESS EASEMENT AND OPEN SPACE - SEE SHEET 4 OF 6
- ⑭ 20' PUBLIC DRAINAGE EASEMENT
- ⑮ CITY OF ALBUQUERQUE BLANKET PEDESTRIAN ACCESS EASEMENT - SEE SHEET 4 OF 6
- ⑯ ALBUQUERQUE BERNALILLO COUNTY WATER UTILITY AUTHORITY 25' PUBLIC WATER AND SANITARY SEWER EASEMENT
- ⑰ CITY OF ALBUQUERQUE AND 37.5' PUBLIC DRAINAGE EASEMENT
- ⑱ ALBUQUERQUE BERNALILLO COUNTY WATER UTILITY AUTHORITY AND SANITARY SEWER EASEMENT
- ⑲ ALBUQUERQUE BERNALILLO COUNTY WATER UTILITY AUTHORITY 15' PUBLIC WATER EASEMENT
- ⑳ PRIVATE ROADWAY TRACT 12
- ㉑ CITY OF ALBUQUERQUE AND A CITY OF ALBUQUERQUE PUBLIC DRAINAGE, SANITARY SEWER AND WATER EASEMENT

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**VACATION EXHIBIT FOR
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LA CUENTISTA SUBDIVISION, UNIT II**
WITHIN SECTIONS 15, 22 & 23
T.11N., R.2 E., N.M.P.M.
CITY OF ALBUQUERQUE
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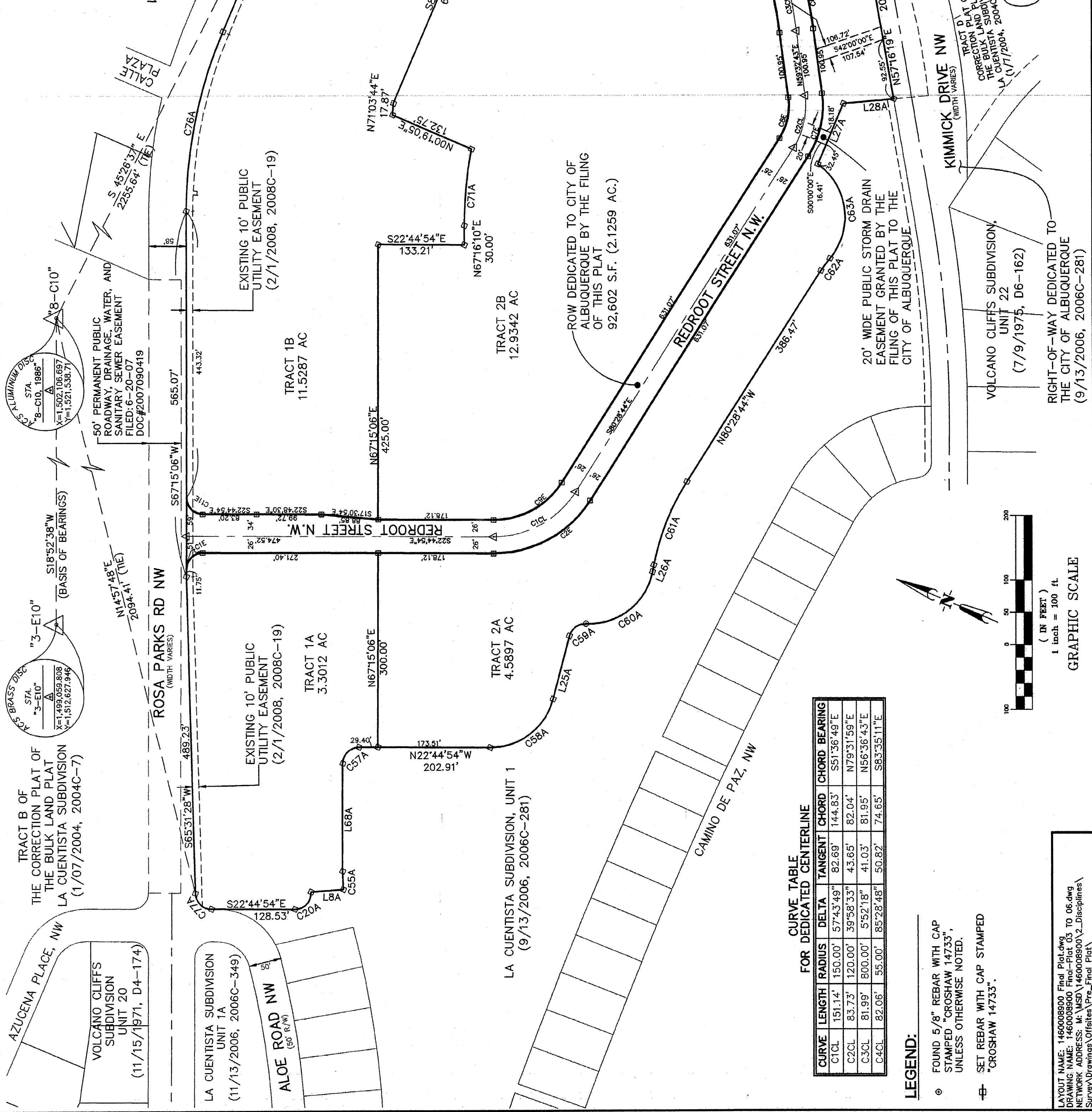
TRACT	SQUARE FEET	ACRES
1	8,051 sq. ft.	0.1857 Ac.
2	19,708 sq. ft.	0.4524 Ac.
3	12,147 sq. ft.	0.2789 Ac.
4	55,625 sq. ft.	1.2770 Ac.
5	3,160 sq. ft.	0.0725 Ac.
6	28,175 sq. ft.	0.6468 Ac.
7	4,276 sq. ft.	0.0982 Ac.
8	6,358 sq. ft.	0.1460 Ac.
9	97,910 sq. ft.	2.2477 Ac.
10	2,342 sq. ft.	0.0538 Ac.
11	2,674 sq. ft.	0.0614 Ac.
12	96,336 sq. ft.	2.2017 Ac.
13	8,784 sq. ft.	0.2017 Ac.
14	2,201 sq. ft.	0.0505 Ac.

**WILSON
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SHEET 4 OF 6
WCI PROJ. NO. 1460008900

FOR INFORMATION ONLY
COA# 709786

REPLAT OF
LA CUENTISTA SUBDIVISION, UNIT II
WITHIN SECTIONS 15, 22 & 23
T.11N., R.2 E., N.M.P.M.
CITY OF ALBUQUERQUE
BERNALILLO COUNTY, NEW MEXICO
DECEMBER, 2014



CURVE TABLE FOR DEDICATED CENTERLINE

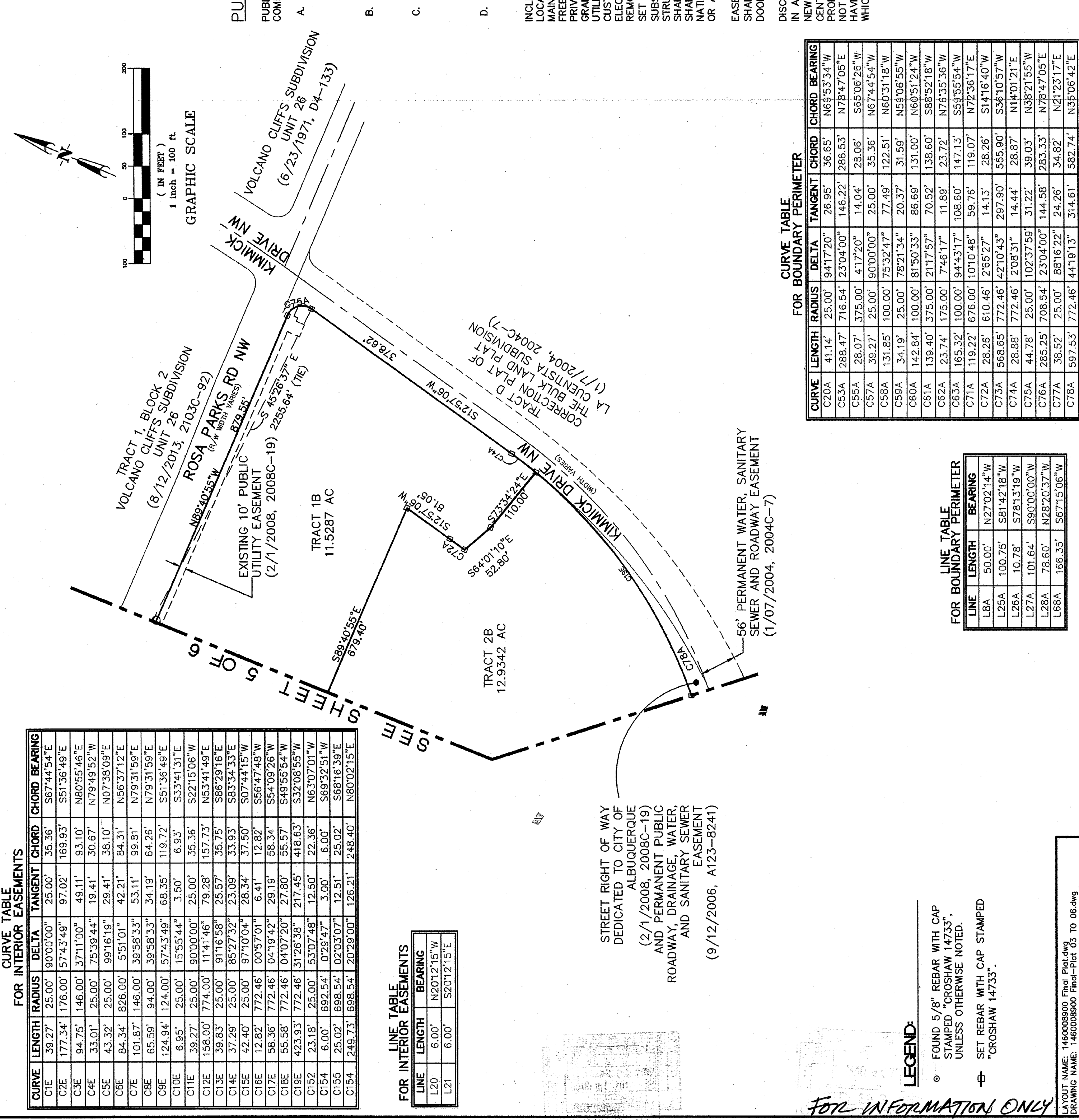
CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD	CHORD BEARING
C1C1	151.14'	150.00'	57°43'49"	82.69'	144.83'	S51°36'49"E
C2C1	81.93'	120.00'	39°58'33"	43.65'	82.04'	N78°31'59"E
C3C1	81.99'	800.00'	5°52'18"	41.03'	81.95'	N66°38'43"E
C4C1	82.06'	55.00'	85°28'48"	50.82'	74.65'	S83°35'11"E

- LEGEND:**
- FOUND 5/8" REBAR WITH CAP UNLESS OTHERWISE NOTED.
 - ⊕ SET REBAR WITH CAP STAMPED "GROSHAW 14733".

LAYOUT NAME: 1460008900 Final Plat.dwg
DRAWING NAME: 1460008900 Final Plat.dwg
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WILSON & COMPANY
4900 LANG AVENUE N.E.
ALBUQUERQUE, NEW MEXICO 87109
(505) 346-4000
SHEET 5 OF 6
WCI PROJ. NO. 1460008900

REPLAT OF
LA CUENTISTA SUBDIVISION, UNIT II
WITHIN SECTIONS 15, 22 & 23
T.11N., R.2 E., N.M.P.M.
CITY OF ALBUQUERQUE
BERNALILLO COUNTY, NEW MEXICO
DECEMBER, 2014



CURVE TABLE FOR INTERIOR EASEMENTS

CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD	CHORD BEARING
C1E	39.27'	25.00'	90°00'00"	25.00'	35.36'	S67°44'54"E
C2E	177.34'	176.00'	57°43'49"	97.02'	169.93'	S51°36'49"E
C3E	94.76'	146.00'	37°11'00"	49.11'	93.10'	N80°55'46"E
C4E	33.01'	25.00'	75°39'44"	19.41'	30.67'	N79°49'52"E
C5E	43.32'	25.00'	99°16'19"	29.41'	38.10'	N07°39'09"E
C6E	84.34'	826.00'	5°51'01"	42.21'	84.31'	N55°37'12"E
C7E	101.87'	146.00'	39°58'33"	53.11'	99.91'	N78°31'59"E
C8E	65.59'	84.00'	39°58'33"	34.19'	64.26'	N78°31'59"E
C9E	124.94'	124.00'	57°43'49"	69.35'	119.72'	S51°36'49"E
C10E	6.95'	25.00'	15°55'44"	3.50'	6.93'	S33°41'31"E
C11E	39.27'	25.00'	90°00'00"	25.00'	35.36'	S22°15'06"W
C12E	158.00'	774.00'	1°41'46"	79.28'	157.73'	N53°41'49"E
C13E	39.83'	25.00'	91°16'58"	25.57'	35.75'	S86°29'16"E
C14E	37.29'	25.00'	85°27'32"	23.09'	33.93'	S63°34'33"E
C15E	42.40'	25.00'	97°10'04"	28.34'	37.50'	S07°44'15"W
C16E	12.82'	772.46'	0°41'42"	29.19'	58.34'	S54°09'26"W
C17E	58.36'	772.46'	0°41'42"	27.80'	55.57'	S49°59'54"W
C18E	423.93'	772.46'	31°26'36"	217.45'	418.63'	S32°08'55"W
C19E	65.59'	25.00'	57°43'49"	32.56'	65.59'	S51°36'49"E
C20E	6.00'	692.54'	0°29'47"	3.00'	6.00'	N63°07'01"W
C21E	25.02'	698.54'	0°20'30"	12.51'	25.02'	S68°16'39"E
C22E	249.73'	698.54'	20°29'00"	126.21'	248.40'	N60°02'15"E

LINE TABLE FOR INTERIOR EASEMENTS

LINE	LENGTH	BEARING
L20	6.00'	N20°12'15"W
L21	6.00'	S20°12'15"E

STREET RIGHT OF WAY DEDICATED TO CITY OF ALBUQUERQUE (2/1/2008, 2008C-19) AND PERMANENT PUBLIC ROADWAY, DRAINAGE, WATER, AND SANITARY SEWER EASEMENT (9/12/2006, A123-8241)

- LEGEND:**
- FOUND 5/8" REBAR WITH CAP STAMPED "GROSHAW 14733". UNLESS OTHERWISE NOTED.
 - ⊕ SET REBAR WITH CAP STAMPED "GROSHAW 14733".

FOR INFORMATION ONLY

PUBLIC UTILITY ACKNOWLEDGMENT:

PUBLIC UTILITY EASEMENTS SHOWN ON THIS PLAT ARE GRANTED FOR THE COMMON AND JOINT USE OF:

- PUBLIC SERVICE COMPANY OF NEW MEXICO ("PNM"), A NEW MEXICO CORPORATION, (PNM ELECTRIC) FOR INSTALLATION, MAINTENANCE, AND SERVICE OF OVERHEAD AND UNDERGROUND ELECTRICAL LINES, TRANSFORMERS, AND OTHER EQUIPMENT AND RELATED FACILITIES REASONABLY NECESSARY TO PROVIDE ELECTRICAL SERVICES.
- NEW MEXICO GAS COMPANY FOR INSTALLATION, MAINTENANCE, AND SERVICE OF NATURAL GAS LINES, VALVES, AND OTHER EQUIPMENT AND FACILITIES REASONABLY NECESSARY TO PROVIDE NATURAL GAS.
- QUEST CORPORATION d/b/a CENTURYLINK, INC. FOR INSTALLATION, MAINTENANCE, AND SERVICE OF SUCH LINES, CABLE, AND OTHER RELATED EQUIPMENT AND FACILITIES REASONABLY NECESSARY TO PROVIDE CABLE TV SERVICE.

INCLUDED, IS THE RIGHT TO BUILD, REBUILD, CONSTRUCT, RECONSTRUCT, LOCATE, RELOCATE, CHANGE, REMOVE, MODIFY, REPAIR, OPERATE, MAINTAIN, AND FREE ACCESS TO FROM, AND OVER SAID EASEMENTS, WITH THE RIGHT AND PRIVILEGE OF GOING UPON, OVER AND ACROSS ADJOINING LANDS OF OTHERS FOR THE PURPOSES SET FORTH HEREIN AND WITH THE RIGHT TO UTILIZE THE RIGHT OF WAY AND EASEMENT TO EXTEND SERVICES TO CUSTOMERS OF GRANTEE, INCLUDING SUFFICIENT WORKING AREA SPACE FOR ELECTRIC TRANSFORMERS, WITH THE RIGHT AND PRIVILEGE TO TRIM AND REMOVE TREES, SHRUBS, BUILDINGS, POOL DECKING, OR OTHER STRUCTURE HOT TUB, CONCRETE OR WOOD POOL DECKING, OR OTHER SURFACE SHALL BE DRILLED OR OPERATED THEREON. PROPERTY OWNERS SHALL BE SOLELY RESPONSIBLE FOR CORRECTING ANY VIOLATIONS OF NATIONAL ELECTRICAL SAFETY CODE BY CONSTRUCTION OF POOLS, DECKING, OR ANY STRUCTURES ADJACENT TO OR NEAR EASEMENTS SHOWN ON PLAT. EASEMENTS FOR ELECTRIC TRANSFORMERS/SWITCHEARS, AS INSTALLED, SHALL EXTEND TEN FEET (10') IN FRONT OF TRANSFORMER/SWITCHEAR DOORS AND FIVE FEET (5') ON EACH SIDE.

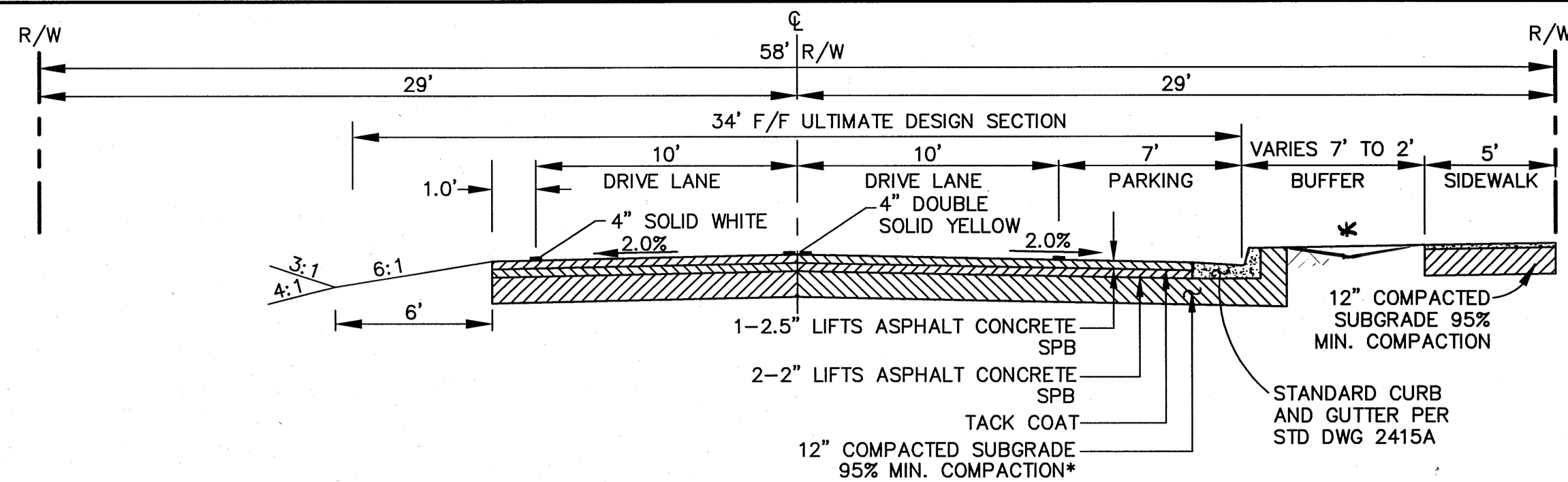
DISCLAIMER
IN APPROVING THIS PLAT, PUBLIC SERVICE COMPANY OF NEW MEXICO (PNM), NEW MEXICO GAS COMPANY (NMGC) AND QUEST CORPORATION d/b/a CENTURYLINK (QUEST) DID NOT CONDUCT A TITLE SEARCH OF THE PROPERTIES SHOWN HEREON. EASEMENT OR PNM, NMGC AND QUEST DO NOT WAIVE OR RELEASE ANY EASEMENT OR RIGHTS WHICH MAY HAVE BEEN GRANTED BY PRIOR PLAT. REPLAT OR OTHER DOCUMENT AND WHICH IS NOT SHOWN ON THIS PLAT.

CURVE TABLE FOR BOUNDARY PERIMETER

CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD	CHORD BEARING
C20A	41.14'	25.00'	94°17'20"	26.95'	36.65'	N69°53'34"W
C53A	288.47'	716.54'	23°04'00"	146.22'	266.53'	N78°47'05"E
C55A	28.07'	375.00'	41°7'20"	14.04'	28.06'	S65°06'26"W
C57A	39.27'	25.00'	90°00'00"	25.00'	35.36'	N67°44'54"W
C59A	131.85'	100.00'	75°32'47"	77.49'	122.51'	N60°31'18"W
C60A	34.19'	25.00'	78°21'34"	20.37'	31.59'	N58°06'55"W
C61A	142.84'	100.00'	81°50'33"	86.69'	131.00'	N60°51'24"W
C62A	139.40'	375.00'	21°17'57"	70.52'	139.60'	S88°52'18"W
C63A	23.74'	175.00'	7°46'17"	11.89'	23.72'	N78°35'36"W
C71A	119.22'	100.00'	94°43'17"	106.60'	147.15'	S59°55'54"W
C72A	28.26'	610.46'	2°55'27"	14.13'	28.26'	S14°16'40"W
C73A	568.65'	772.46'	42°10'43"	297.90'	555.90'	S36°10'57"W
C74A	28.88'	772.46'	2°08'31"	14.44'	28.87'	N18°21'21"E
C75A	44.78'	25.00'	102°37'59"	31.22'	39.03'	N38°21'55"W
C76A	265.25'	708.54'	3°04'00"	144.58'	263.33'	N78°47'05"E
C77A	38.52'	25.00'	88°16'22"	24.26'	34.82'	N21°23'17"E
C78A	597.53'	772.46'	44°19'13"	314.61'	582.74'	N35°06'42"E

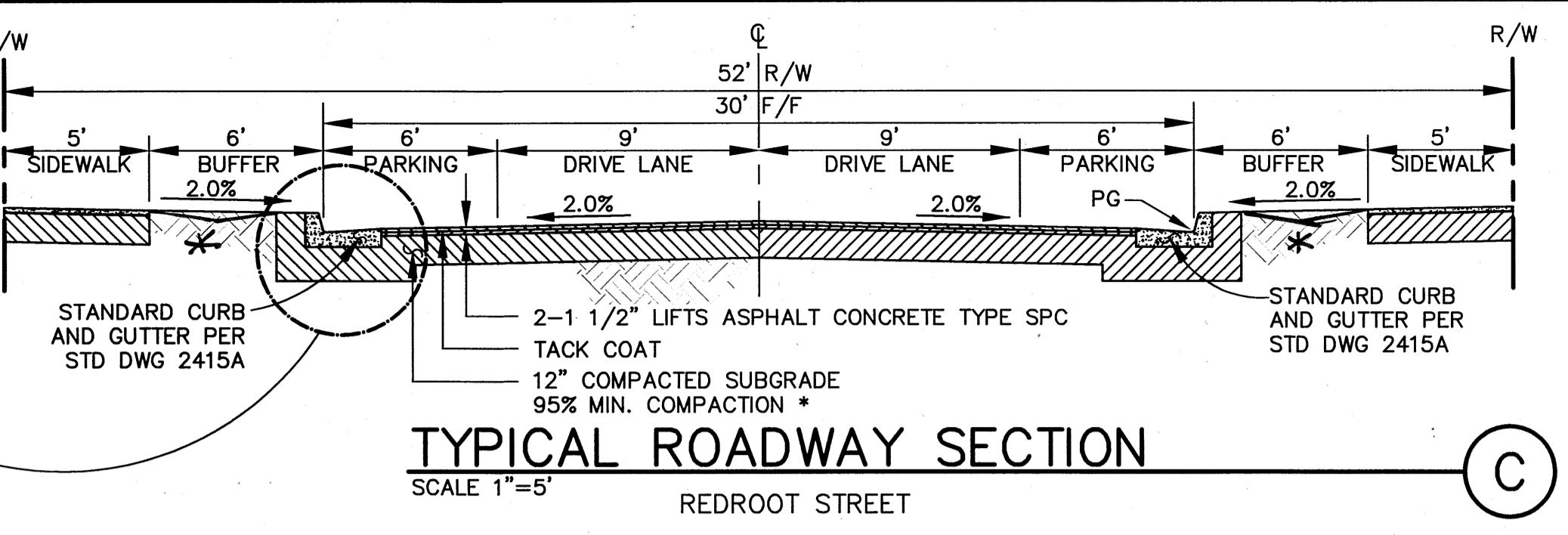
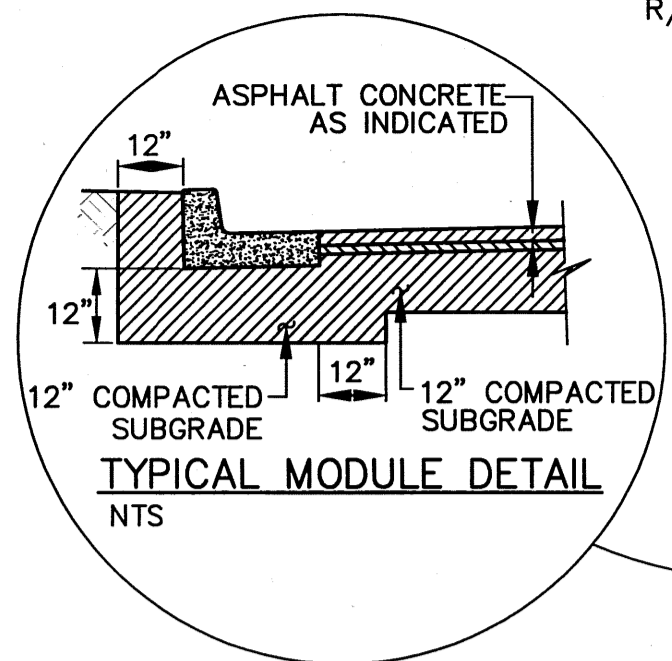
LINE TABLE FOR BOUNDARY PERIMETER

LINE	LENGTH	BEARING
LBA	50.00'	N27°02'14"W
L25A	100.75'	S81°42'18"W
L26A	10.78'	S78°13'19"W
L27A	101.64'	S90°00'00"W
L28A	78.60'	N28°20'37"W
L68A	166.35'	S67°15'06"W



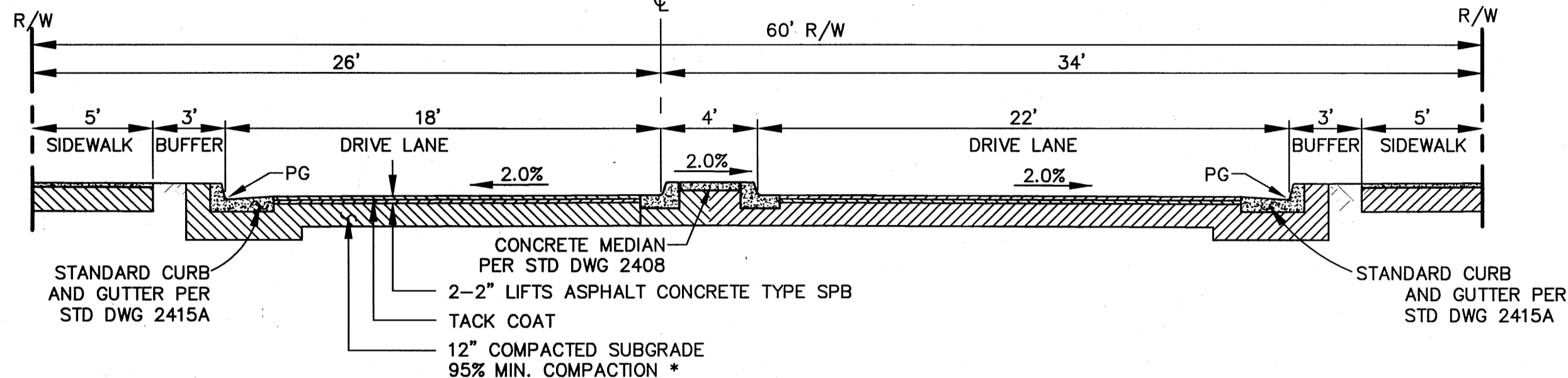
1/2 PERMANENT TYPICAL ROADWAY SECTION

SCALE 1"=5' PER PAVEMENT DESIGN COA #709782
 ROSA PARKS ROAD NW - RESIDENTIAL COLLECTOR
 STA 16+91.52 TO STA 31+35
 TRANSITION STA 16+91.52 25' RT TO STA 21+89.93 17' RT



TYPICAL ROADWAY SECTION

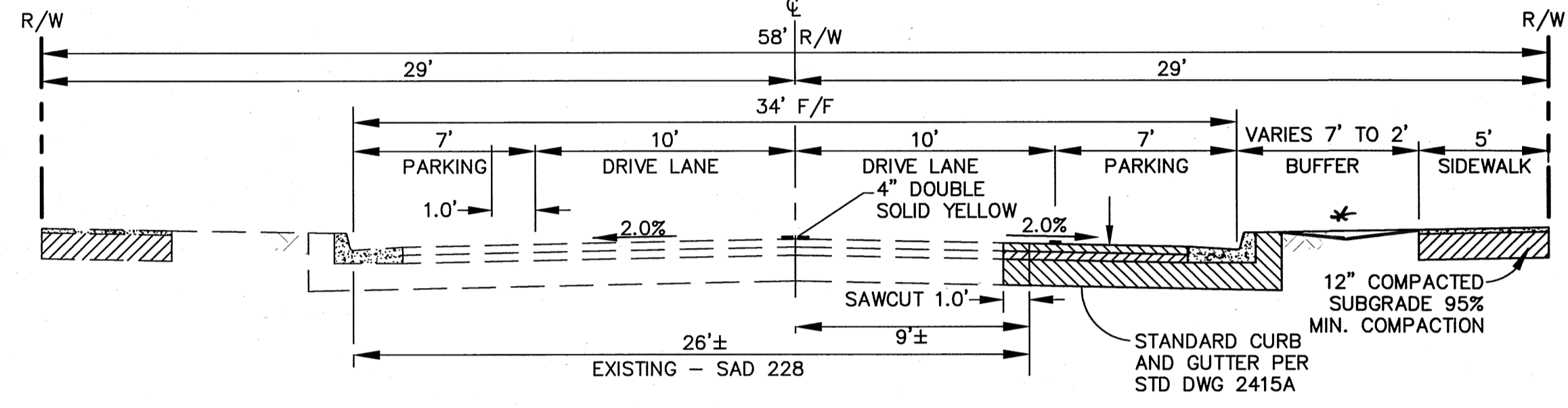
SCALE 1"=5' REDROOT STREET



TYPICAL ENTRANCE SECTION

SCALE 1"=5'

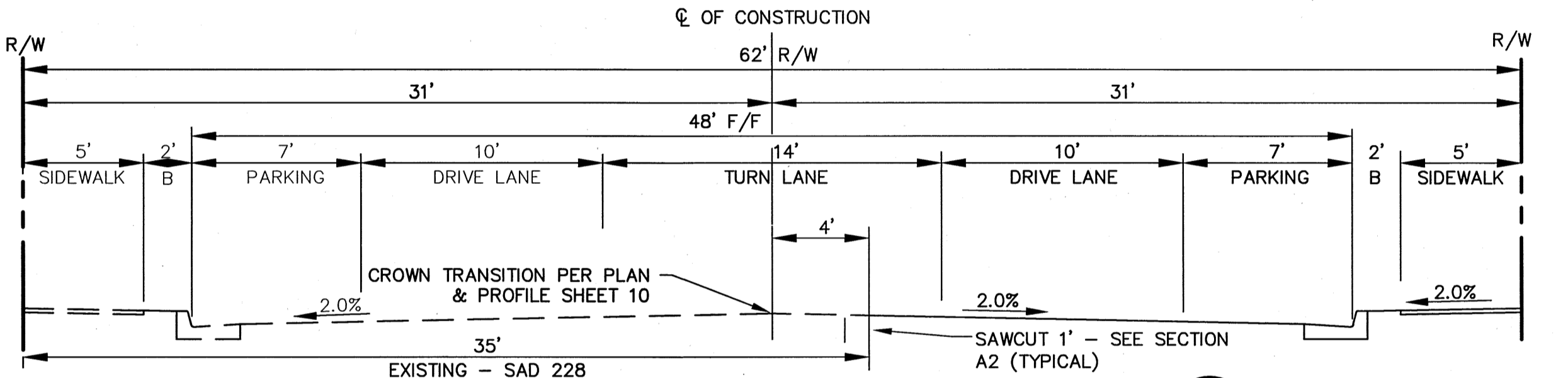
REDROOT STREET - STA 1+20 TO STA 3+37
 TRANSITION STA 2+48.17 22' LT TO STA 4+24.90 15' LT
 REDROOT STREET - STA 17+18 TO STA 18+25



TYPICAL ROADWAY SECTION

SCALE 1"=5' PER PAVEMENT DESIGN COA #709782

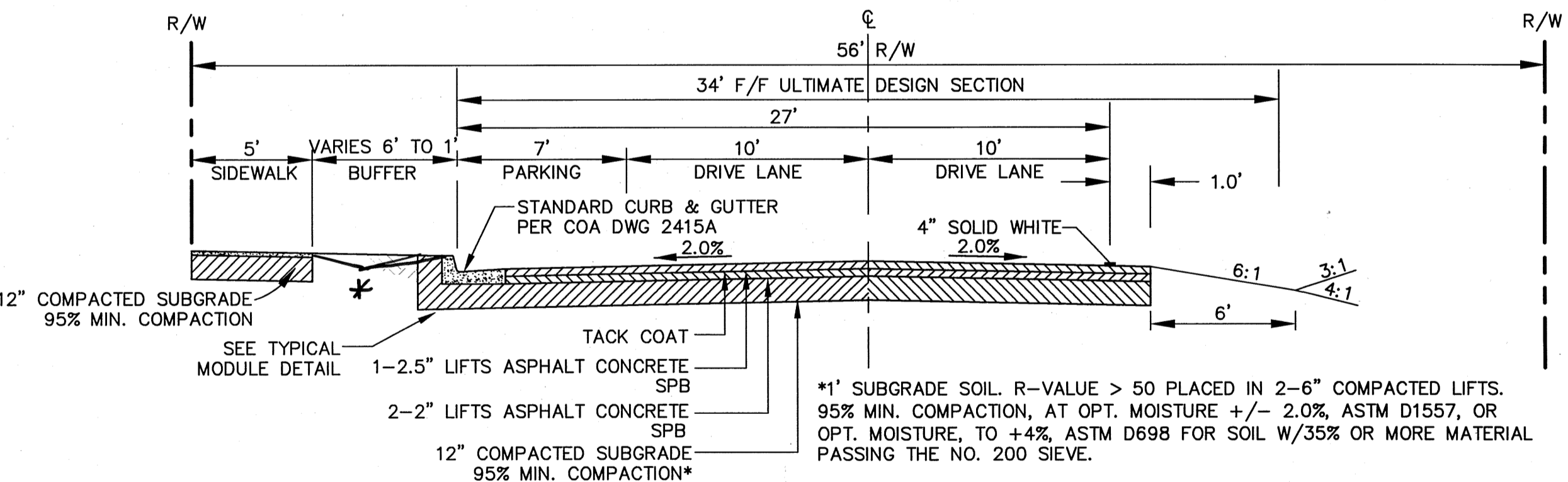
ROSA PARKS ROAD NW - RESIDENTIAL COLLECTOR
 STA 31+35 TO STA 36+40



TYPICAL ROADWAY SECTION

SCALE 1"=5'

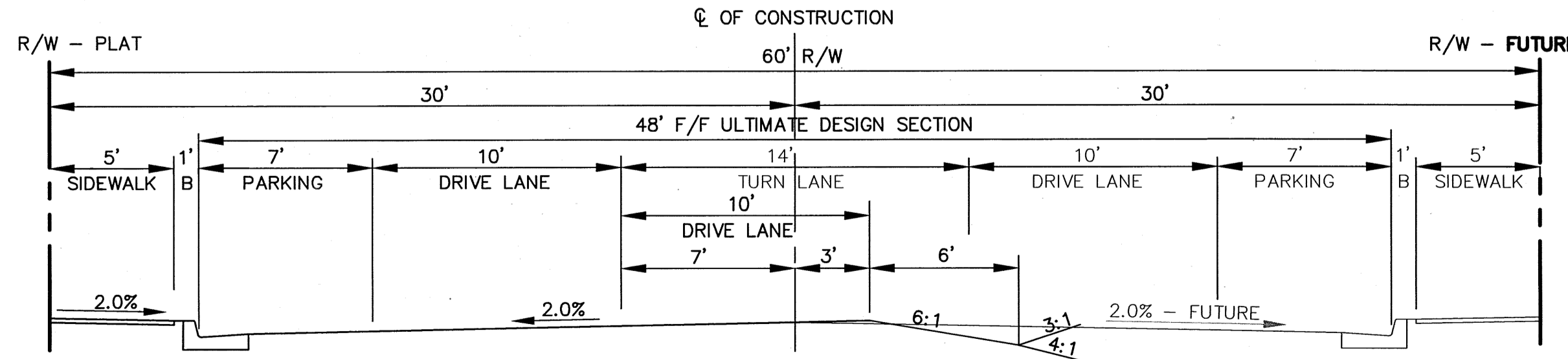
ROSA PARKS RD NW - RESIDENTIAL COLLECTOR
 STA 38+15 TO STA 40+00
 STA 36+40 17' RT TO STA 38+15 22' RT
 175' PAVEMENT TRANSITION @ 35:1 - BUFFER VARIES 7' TO 2'



1/2 PERMANENT TYPICAL ROADWAY SECTION

SCALE 1"=5' PER PAVEMENT DESIGN COA #709782

KIMMICK DRIVE NW - RESIDENTIAL COLLECTOR
 STA 88+80.50 TO STA 98+00
 STA 95+29.80 17' LT TO STA 98+00 22' LT
 270' PVMT TRANSITION TAPER @ 35:1 - BUFFER VARIES 6' TO 1'



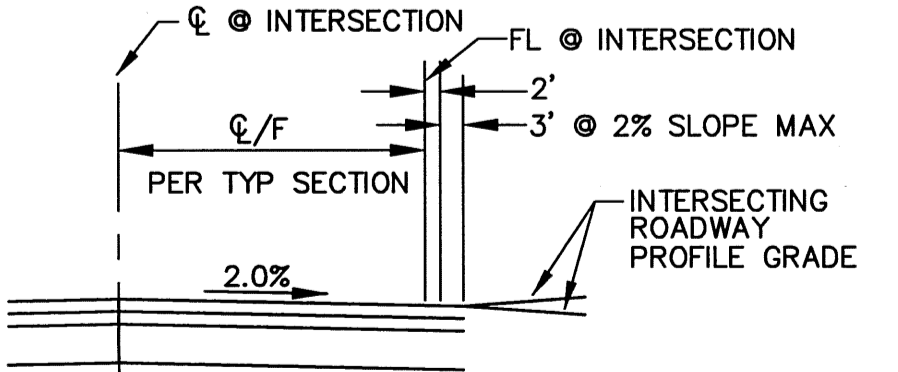
TYPICAL ROADWAY SECTION

SCALE 1"=5'

KIMMICK DRIVE NW - RESIDENTIAL COLLECTOR
 STA 98+00 TO STA 99+23.18

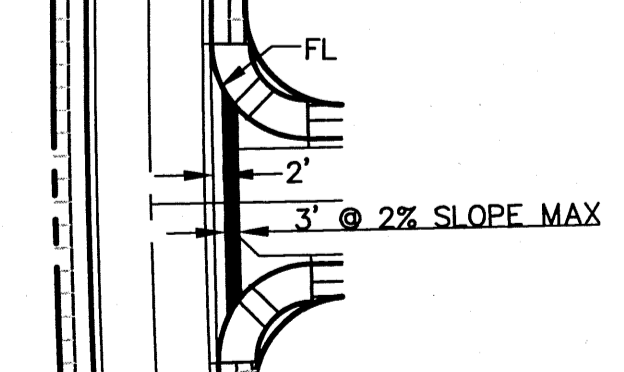
Handwritten note: 3. INSTALL 5:1 SWALE W/ 3/4" GRAVEL MIN IN BUFFERS 7'3" WIDE PER STD DWG 2402A13 BUFFER REVISION

- GENERAL PAVING NOTES:**
- *SUBGRADE SOIL WITH R≤50 SHOULD BE REMOVED TO DEPTH OF 24" AND REPLACED WITH SOIL EXHIBITING R≥50.
 - *1' SUBGRADE SOIL. R-VALUE > 50 PLACED IN 2-6" COMPACTED LIFTS. 95% MIN. COMPACTION, AT OPT. MOISTURE +/- 2.0%, ASTM D1557, OR OPT. MOISTURE, TO +4%, ASTM D698 FOR SOIL W/35% OR MORE MATERIAL PASSING THE NO. 200 SIEVE.



PROFILE @ INTERSECTION DETAIL

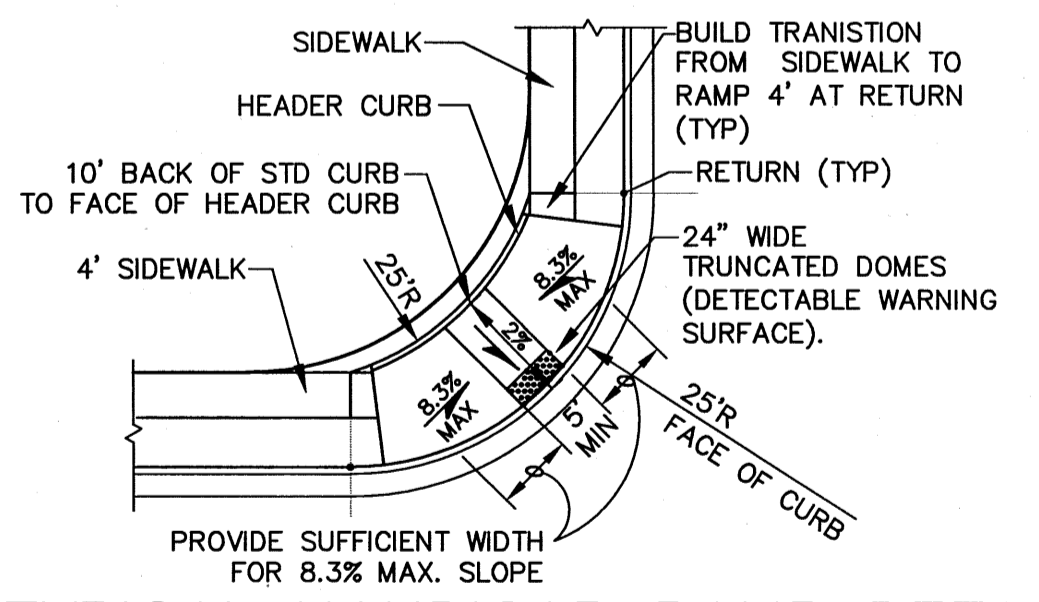
SCALE 1"=5'



PLAN @ INTERSECTION DETAIL

SCALE 1"=50'

(REPLACEABLE) NOTE: DESIGN DETECTABLE WARNING SURFACE SHALL BE CAST-IN-PLACE PER ADA GUIDELINES. SUBMIT SPECS TO CITY CONSTRUCTION ENGINEER FOR EVALUATION PRIOR TO CONSTRUCTION.



TYPICAL HANDICAP RAMP DETAIL

NTS PER STD DWG 2441 CASE II

AS-BUILT INFORMATION	
CONTRACTOR	DATE
WORKING DRAWINGS	DATE
INSPECTOR'S FIELD VERIFICATION	DATE
FIELD VERIFICATION	DATE
CONTRACTOR'S RECORD	DATE
MICRO-FILM INFORMATION	
RECORDED BY	DATE
NO.	

BENCH MARKS	
ACS 1" ALUMINUM DISK STAMPED	DATE
"ACS BM 5-D11" LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING	DATE
ELEVATION = 5270.087 FT.	
NAVD 88 U.S. FEET	

SURVEY INFORMATION	
FIELD NOTES	DATE
BY	
AERIAL MAPPING, WCI 05/01	DATE
BOUNDARY, WCI 06/01	DATE
BOUNDARY, WCI 10/01	DATE

ENGINEER'S SEAL	
NO.	DATE
BY	
REVISIONS	
NO.	DATE
BY	
REVISIONS	
NO.	DATE
BY	
REVISIONS	

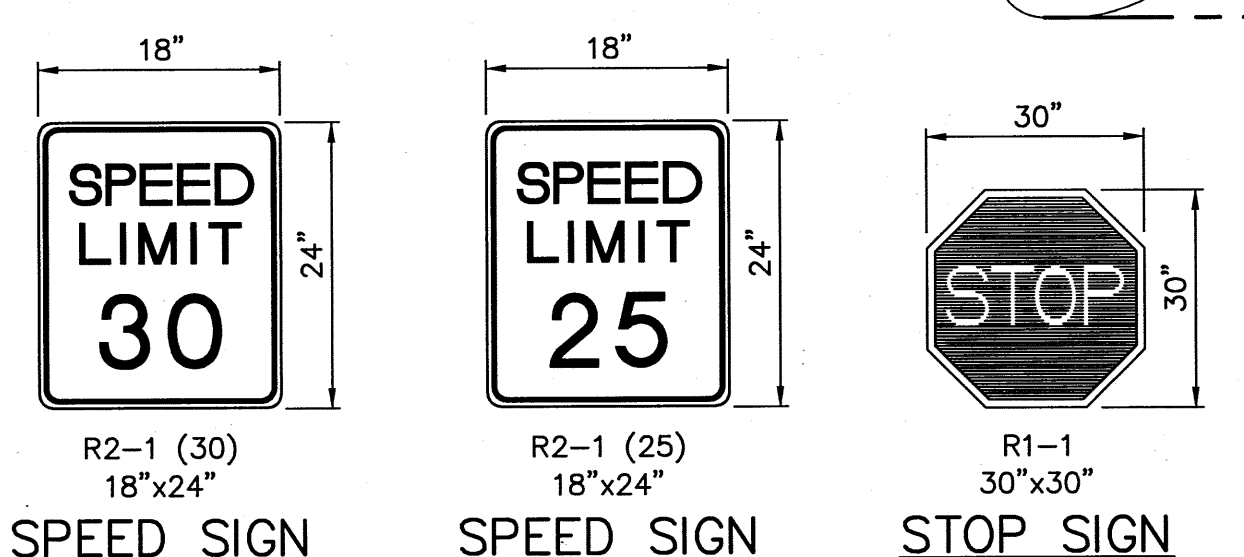
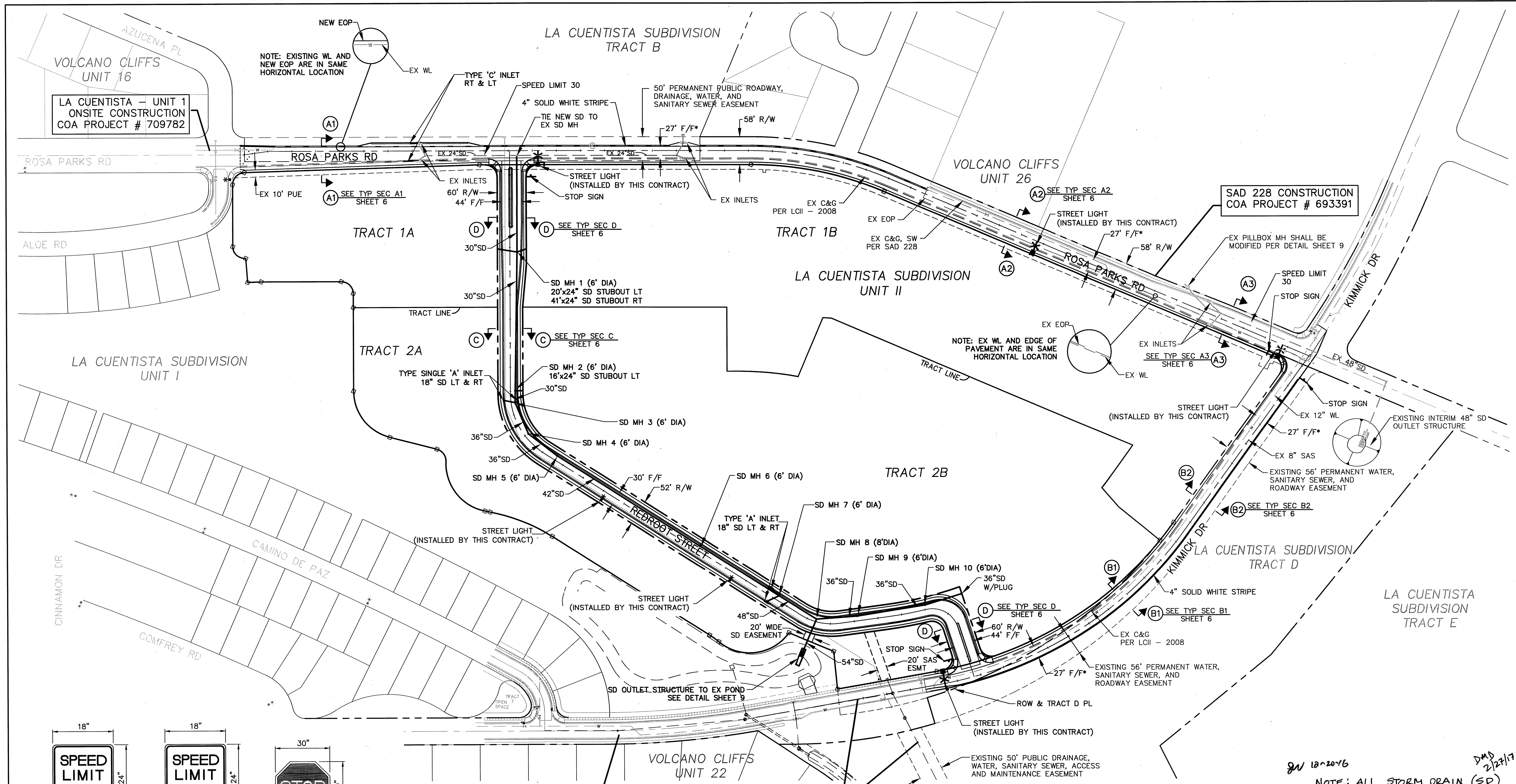
REVISIONS	
NO.	DATE
BY	
REVISIONS	
NO.	DATE
BY	
REVISIONS	

WILSON & COMPANY
 2600 THE AMERICAN ROAD S.E.
 SUITE 100
 RIO RANCHO, NEW MEXICO
 87124
 (505) 898-8021

CITY OF ALBUQUERQUE
 PUBLIC WORKS DEPARTMENT
 ENGINEERING GROUP

LA CUENTISTA SUBDIVISION - UNIT II
 TYPICAL SECTIONS

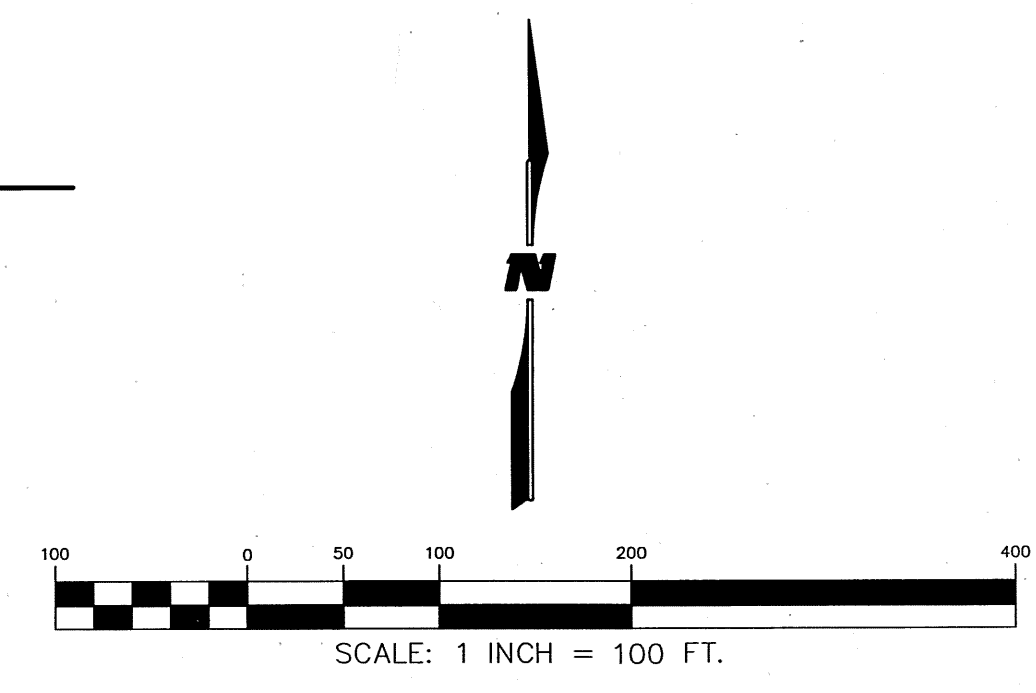
Design Review Committee	City Engineer Approval	Mo./Day/Yr.	Mo./Day/Yr.
APPROVED MAR 01 2017	APPROVED APR 06 2015		
DESIGN REVIEW COMMITTEE	DESIGN REVIEW COMMITTEE	CITY ENGINEER	
City Project No.	Zone Map No.	Drawing	Sheet
709786	C-10, C-11	6	17



NOTE:
ALL SIGNS TO BE MOUNTED WITH A 4#/FT FLANGE
POST WITH BASE POST (3.5' BURY) OR EQUAL.

- LEGEND**
- PROPOSED STOP SIGN
 - CENTER LINE
 - RIGHT OF WAY W/SIDEWALK AND CURB & GUTTER
 - EASEMENT
 - TRACT LINE
 - F/F FACE TO FACE
 - F/F* FACE OF CURB TO EOP STRIPE

- LIGHTING LEGEND**
- * PROPOSED 100 HPS LIGHT STANDARD (TO BE INSTALLED BY THIS CONTRACT)



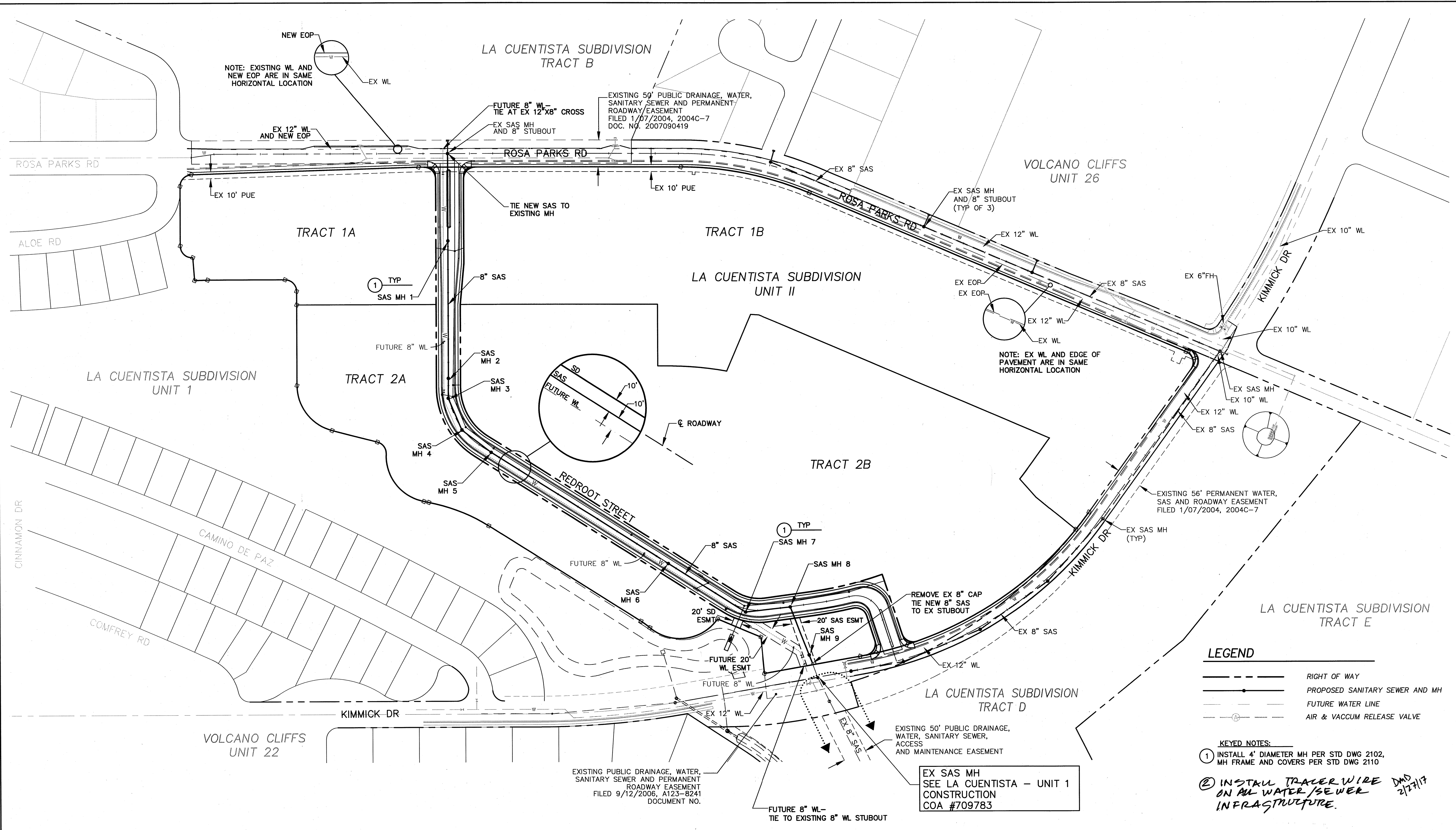
18-20-16
DMD 2/24/17
NOTE: ALL STORM DRAIN (SD) SHALL BE: HIGH PERFORMANCE POLYPROPYLENE PIPE (HPP) PER DETAILS SHEETS 18-20 OR CL III RCP AND CL IV RCP (CL IV TO BE USED FOR ANY SD WITH LESS THAN 24" COVER)

AS-BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL	
CONTRACTOR	DATE	ACS 1 3/4" ALUMINUM DISK STAMPED	DATE	NO.	BY		REVISIONS NO. DATE BY WILSON & COMPANY, ENGINEERS & ARCHITECTS DESIGNED BY KIS, SO DATE JAN 2015 DRAWN BY KIS, SO DATE JAN 2015 CHECKED BY DSA DATE JAN 2015
WORK NO.	DATE	*ACS BM 5-D11" LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING	DATE	NO.	BY		
INSPECTOR'S ACCEPTANCE BY	DATE	ROCK IN OUTCROPPING	DATE	NO.	BY		
VERIFICATION BY	DATE	ELEVATION = 5270.087 FT.	DATE	NO.	BY		
APPROVED BY	DATE	NAVD 88 U.S. FEET	DATE	NO.	BY		

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

**LA CUENTISTA SUBDIVISION - UNIT II
OVERALL PAVING & STORM DRAIN PLAN**

Design Review Committee	City Engineer Approval	Mo./Day/Yr.	Mo./Day/Yr.
MAR 01 2017	MAR 01 2017		
DESIGN REVIEW COMMITTEE	CITY ENGINEER	Last Design Update	
City Project No. 709786	Zone Map No. C-10, C-11	Drawing 7	Sheet 17

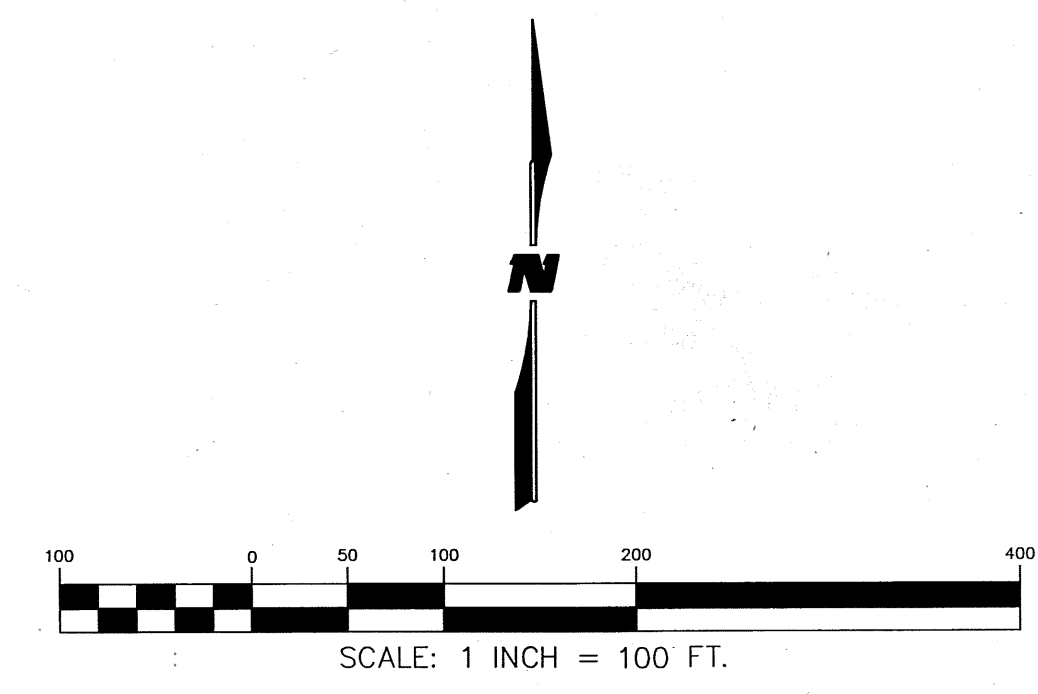


LEGEND

- RIGHT OF WAY
- PROPOSED SANITARY SEWER AND MH
- FUTURE WATER LINE
- AIR & VACCUUM RELEASE VALVE

KEYED NOTES:

- ① INSTALL 4" DIAMETER MH PER STD DWG 2102, MH FRAME AND COVERS PER STD DWG 2110
- ② INSTALL TRACER WIRE ON ALL WATER/SEWER INFRASTRUCTURE.



CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

LA CUENTISTA SUBDIVISION - UNIT II
OVERALL UTILITY PLAN

DESIGNED BY KIS	DATE JAN 2015	DESIGNED BY VKL	DATE JAN 2015
DRAWN BY DSA	DATE JAN 2015	CHECKED BY DSA	DATE JAN 2015

City Project No. **709786** Zone Map No. **C-10, C-11**

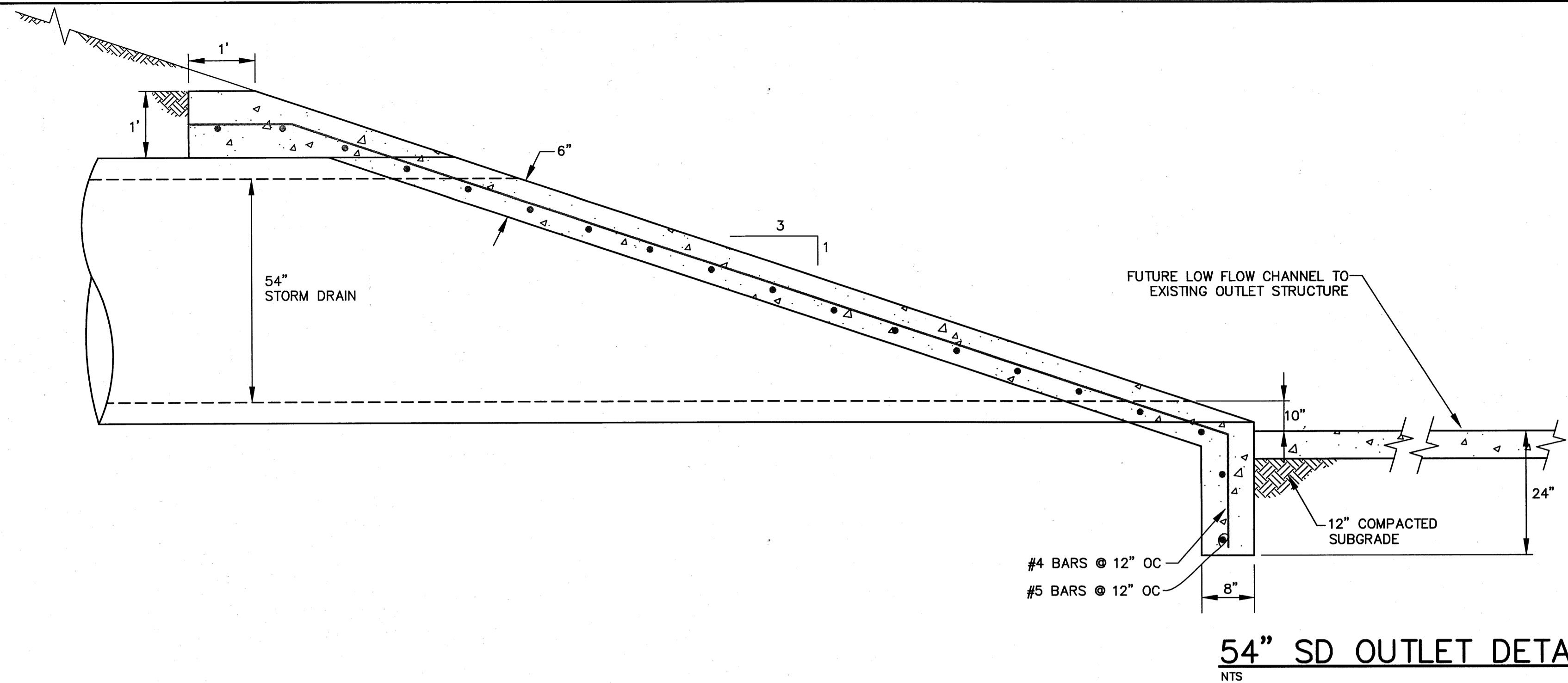
City Engineer Approval: **MAR 01 2017**

Design Review Committee: **MAR 01 2017**

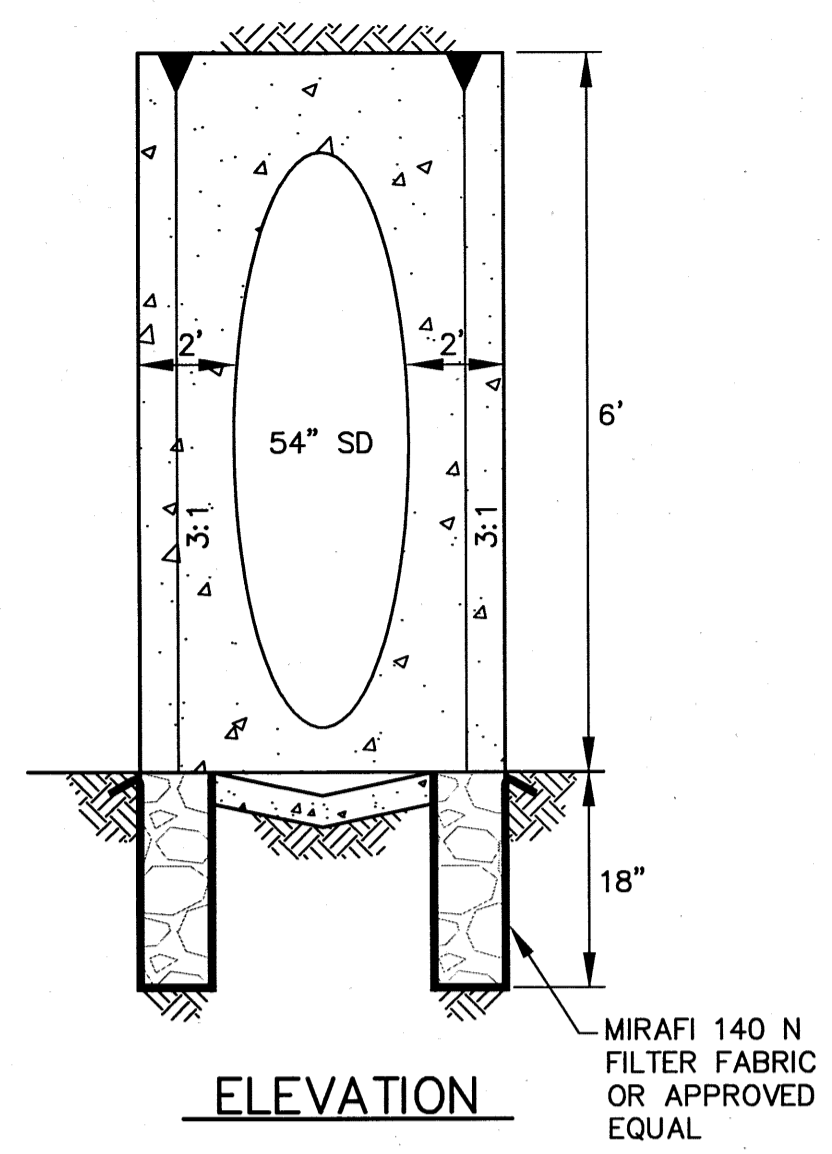
City Project No. **709786** Zone Map No. **C-10, C-11**

Drawing **8** Sheet **17**

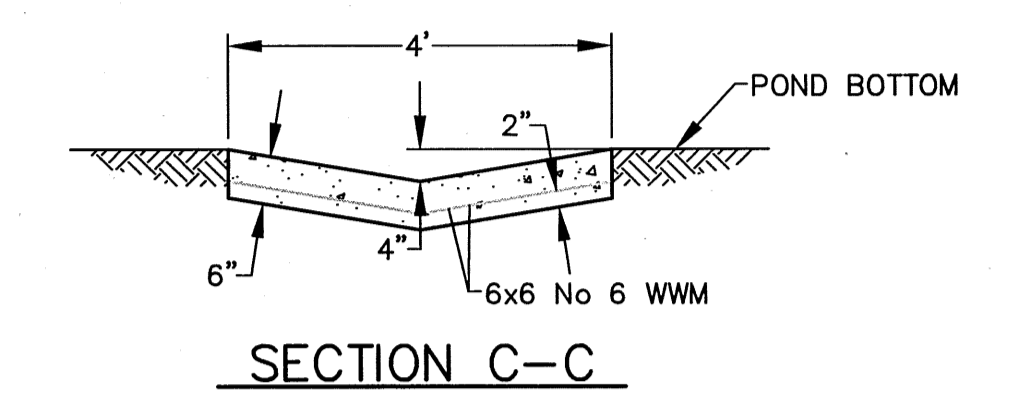
AS-BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL	
CONTRACTOR	DATE	ACS 1 1/2" ALUMINUM DISK STAMPED	NO.	FIELD NOTES	NO.		REVISIONS NO. DATE BY 1 MAR 01 2015 KIS 2 MAR 01 2017 VKL
WORKS BY	DATE	"ACS BM 5-D11", LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING	NO.	AERIAL MAPPING, WCI 05/01 BOUNDARY, WCI 06/01 BOUNDARY, WCI 10/01	NO.		
SURVEYOR'S ACCEPTANCE BY	DATE	ELEVATION = 5270.087 FT. NAVD 88 U.S. FEET	NO.		NO.		
DESIGNED BY	DATE		NO.		NO.		
DRAWN BY	DATE		NO.		NO.		
CHECKED BY	DATE		NO.		NO.		



54" SD OUTLET DETAIL
NTS

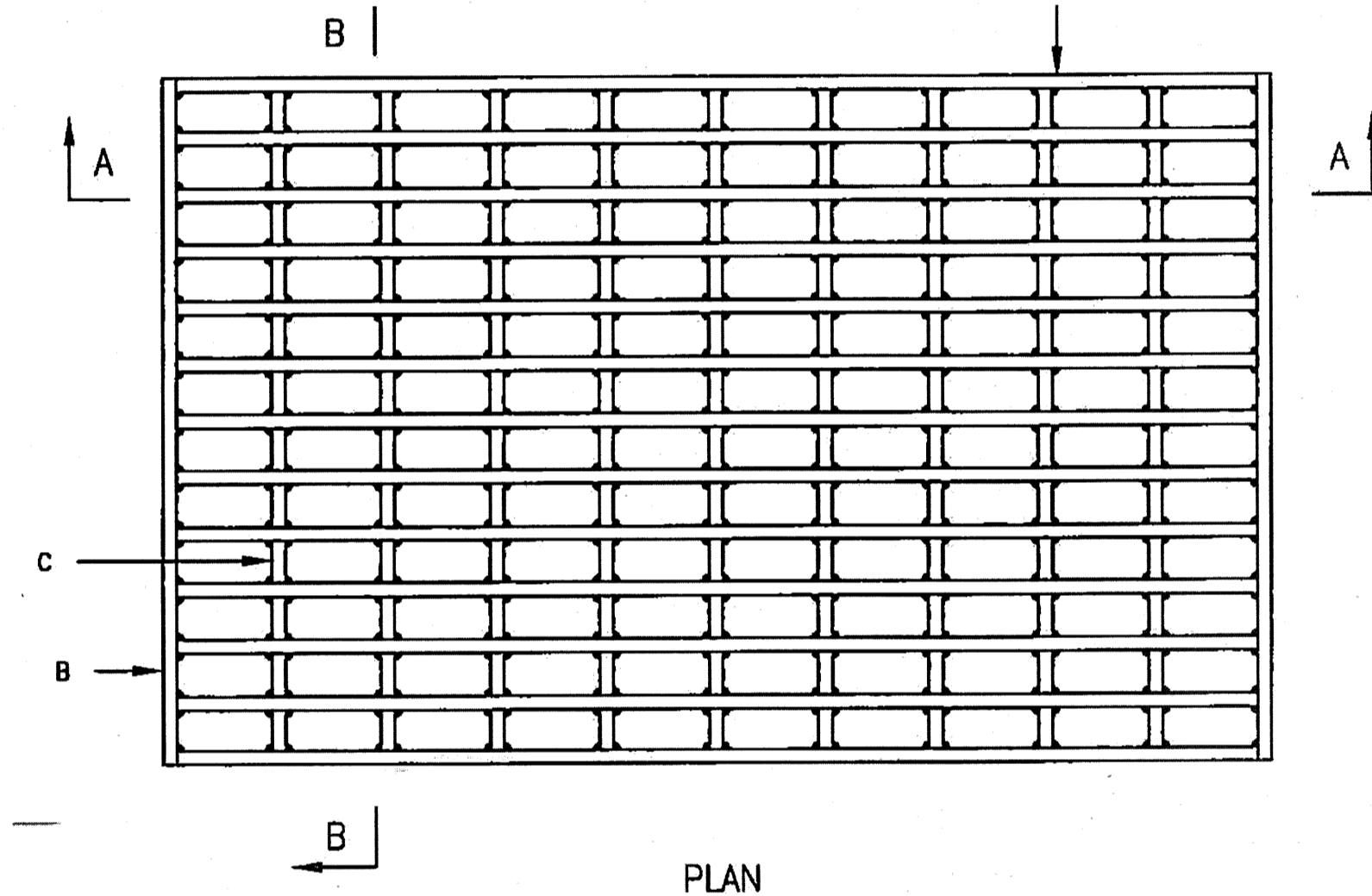


ELEVATION

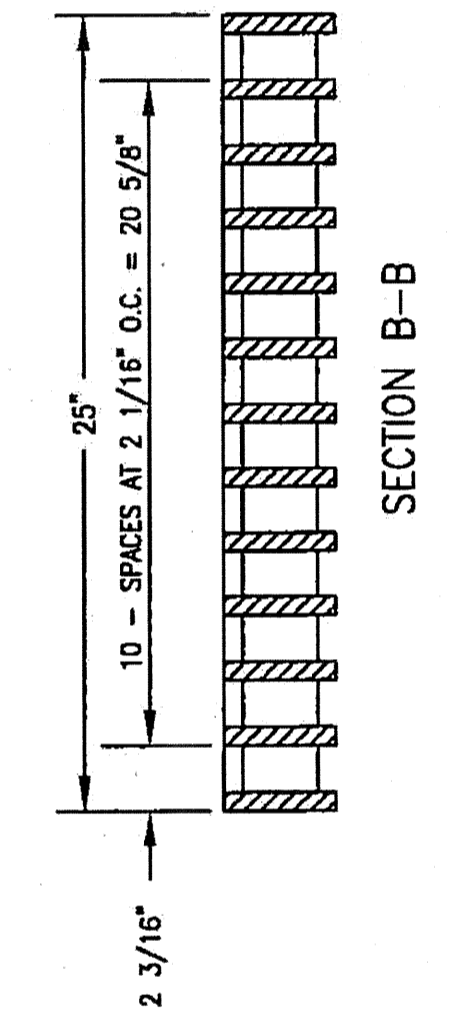


SECTION C-C

FUTURE LOW FLOW CHANNEL DETAIL
SCALE: 1" = 2'



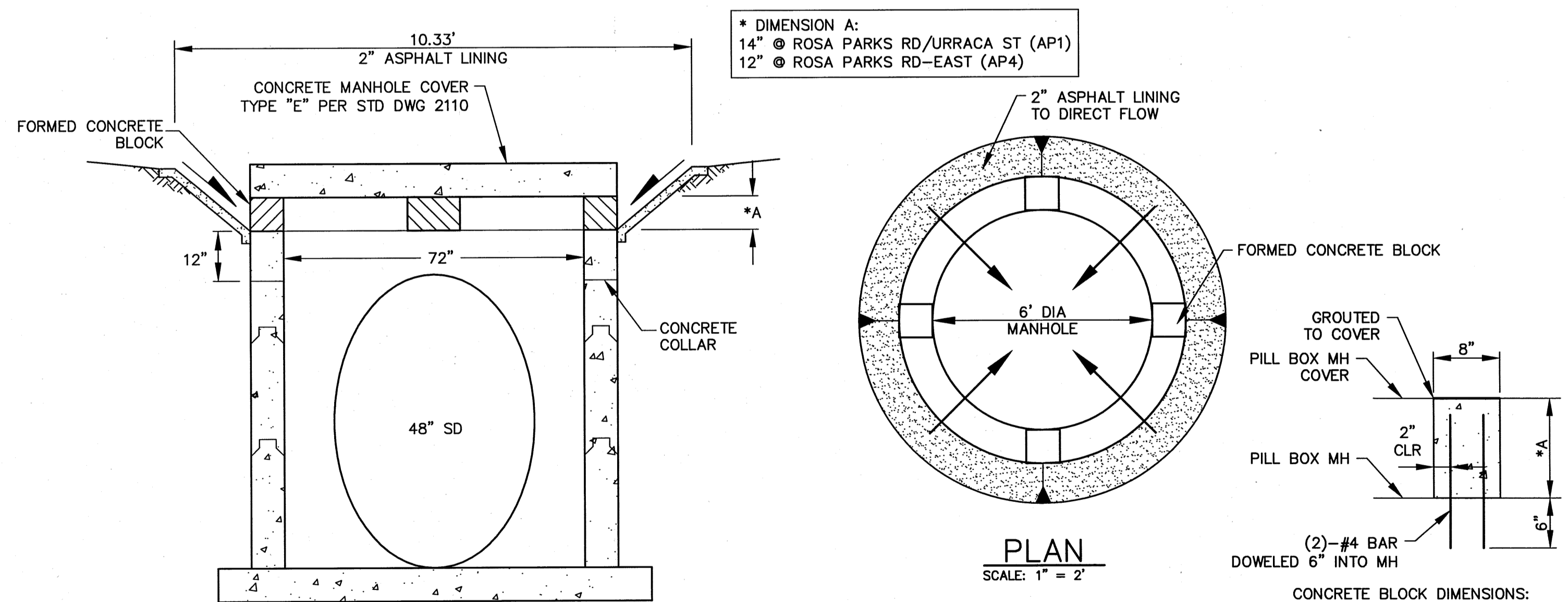
PLAN



SECTION B-B

- GENERAL NOTES:**
1. ALL BARS SHALL BE STRUCTURAL GRADE STEEL, GRADE A36.
 2. THE GRATE SHALL BE WELDED WITH 1/8" FILLET WELD AROUND BOTH SIDES OF CROSS BARS, 1/4". FILLET WELD BOTH SIDES OF BEARING BARS TO END BARS.
 3. AFTER CLEANING SURFACE OF SCALE, RUST, OILS, ETC., PAINT GRATE WITH ONE SHOP COAT RED OXIDE, TWO FINISH COATS ALUMINUM PAINT (AASHTO M 69).
 4. TOP OF CROSS BARS SHALL BE FLUSH WITH TOP OF GRATE.
 5. GRIND WELDS FLUSH WITH BEARING BARS.
 6. WHEN INSTALLED IN FRAME, PUSH TIGHT TO ONE SIDE, OTHER SIDE SHALL HAVE 1/2" MAX. OPENING. SPACERS WELDED TO FRAME MAY BE USED IF REQUIRED TO KEEP 1/2" SPACE OR LESS.

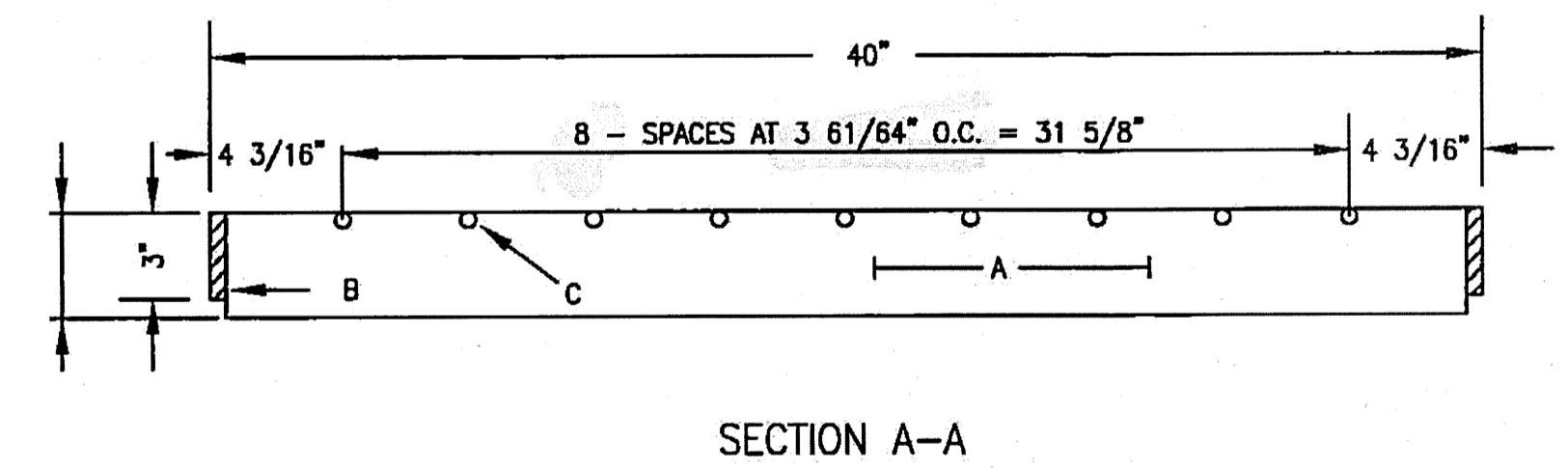
- CONSTRUCTION NOTES:**
- A. BEARING BARS, (13) 1/2" X 3 1/2" X 39".
 - B. END BARS, (2) 1/2" X 3" X 25".
 - C. CROSS BARS, (9) 1/2" DIA. X 24".



PLAN
SCALE: 1" = 2'

CONCRETE BLOCK
SCALE: 1" = 1'

PILL BOX MANHOLE DETAIL (MODIFIED TYPE "E")
SCALE: 1" = 2'



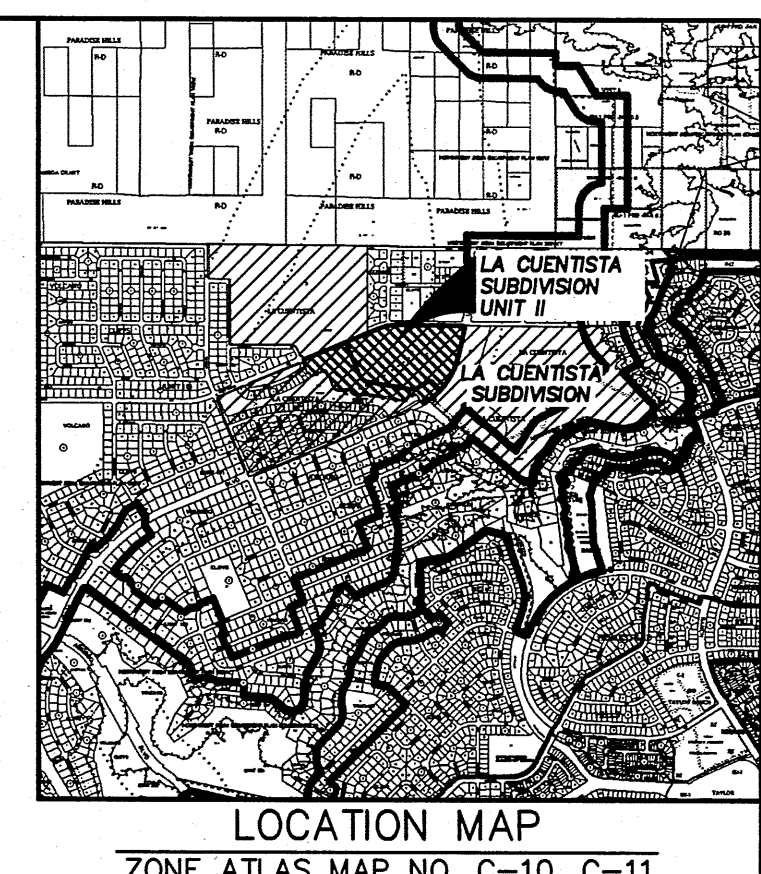
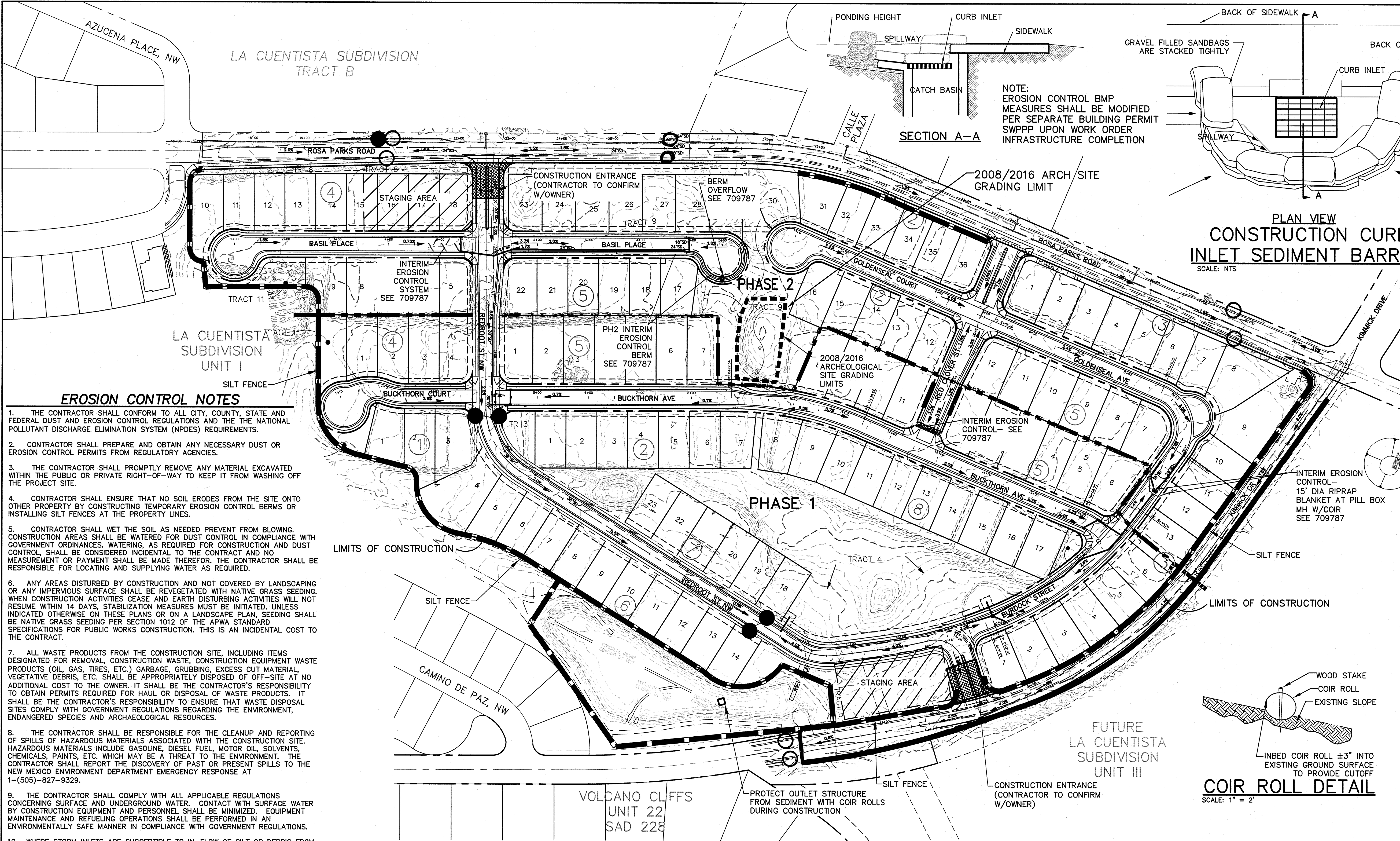
SECTION A-A

CITY OF ALBUQUERQUE
DRAINAGE
STORM INLET
ALBUQUERQUE GRATE
DWG. 2220-MODIFIED FEB. 2014

 WILSON & COMPANY 2600 THE AMERICAN ROAD S.E. SUITE 100 RIO RANCHO, NEW MEXICO 87124 (505) 898-8021	 CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP
	LA CIENTISTA SUBDIVISION-UNIT II DRAINAGE DETAILS
 Design Review Committee	 City Engineer-Approval
City Project No. 709786	Zone Map No. C-10, D-10 C-11, D-11
Drawing 9	Sheet 17

WILSON# 146008900
JANUARY 2015

M:\MSD\14-600-089-00\2_DISCIPLINES\SHEETS\OFFSITES\146089ESC.DWG 2/27/17



PLAN VIEW CONSTRUCTION CURB INLET SEDIMENT BARRIER
SCALE: NTS

LOCATION MAP
ZONE ATLAS MAP NO. C-10, C-11

EROSION CONTROL NOTES

1. THE CONTRACTOR SHALL CONFORM TO ALL CITY, COUNTY, STATE AND FEDERAL DUST AND EROSION CONTROL REGULATIONS AND THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) REQUIREMENTS.
2. CONTRACTOR SHALL PREPARE AND OBTAIN ANY NECESSARY DUST OR EROSION CONTROL PERMITS FROM REGULATORY AGENCIES.
3. THE CONTRACTOR SHALL PROMPTLY REMOVE ANY MATERIAL EXCAVATED WITHIN THE PUBLIC OR PRIVATE RIGHT-OF-WAY TO KEEP IT FROM WASHING OFF THE PROJECT SITE.
4. CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE ONTO OTHER PROPERTY BY CONSTRUCTING TEMPORARY EROSION CONTROL BERMS OR INSTALLING SILT FENCES AT THE PROPERTY LINES.
5. CONTRACTOR SHALL WET THE SOIL AS NEEDED PREVENT FROM BLOWING. CONSTRUCTION AREAS SHALL BE WATERED FOR DUST CONTROL IN COMPLIANCE WITH GOVERNMENT ORDINANCES. WATERING, AS REQUIRED FOR CONSTRUCTION AND DUST CONTROL, SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT AND NO MEASUREMENT OR PAYMENT SHALL BE MADE THEREFOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND SUPPLYING WATER AS REQUIRED.
6. ANY AREAS DISTURBED BY CONSTRUCTION AND NOT COVERED BY LANDSCAPING OR ANY IMPERVIOUS SURFACE SHALL BE REVEGETATED WITH NATIVE GRASS SEEDING. WHEN CONSTRUCTION ACTIVITIES CEASE AND EARTH DISTURBING ACTIVITIES WILL NOT RESUME WITHIN 14 DAYS, STABILIZATION MEASURES MUST BE INITIATED. UNLESS INDICATED OTHERWISE ON THESE PLANS OR ON A LANDSCAPE PLAN, SEEDING SHALL BE NATIVE GRASS SEEDING PER SECTION 1012 OF THE APWA STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION. THIS IS AN INCIDENTAL COST TO THE CONTRACT.
7. ALL WASTE PRODUCTS FROM THE CONSTRUCTION SITE, INCLUDING ITEMS DESIGNATED FOR REMOVAL, CONSTRUCTION WASTE, CONSTRUCTION EQUIPMENT WASTE PRODUCTS (OIL, GAS, TIRES, ETC.) GARBAGE, GRUBBING, EXCESS CUT MATERIAL, VEGETATIVE DEBRIS, ETC. SHALL BE APPROPRIATELY DISPOSED OF OFF-SITE AT NO ADDITIONAL COST TO THE OWNER. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN PERMITS REQUIRED FOR HAUL OR DISPOSAL OF WASTE PRODUCTS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT WASTE DISPOSAL SITES COMPLY WITH GOVERNMENT REGULATIONS REGARDING THE ENVIRONMENT, ENDANGERED SPECIES AND ARCHAEOLOGICAL RESOURCES.
8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CLEANUP AND REPORTING OF SPILLS OF HAZARDOUS MATERIALS ASSOCIATED WITH THE CONSTRUCTION SITE. HAZARDOUS MATERIALS INCLUDE GASOLINE, DIESEL FUEL, MOTOR OIL, SOLVENTS, CHEMICALS, PAINTS, ETC. WHICH MAY BE A THREAT TO THE ENVIRONMENT. THE CONTRACTOR SHALL REPORT THE DISCOVERY OF PAST OR PRESENT SPILLS TO THE NEW MEXICO ENVIRONMENT DEPARTMENT EMERGENCY RESPONSE AT 1-(505)-827-9329.
9. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE REGULATIONS CONCERNING SURFACE AND UNDERGROUND WATER. CONTACT WITH SURFACE WATER BY CONSTRUCTION EQUIPMENT AND PERSONNEL SHALL BE MINIMIZED. EQUIPMENT MAINTENANCE AND REFUELING OPERATIONS SHALL BE PERFORMED IN AN ENVIRONMENTALLY SAFE MANNER IN COMPLIANCE WITH GOVERNMENT REGULATIONS.
10. WHERE STORM INLETS ARE SUSCEPTIBLE TO IN-FLOW OF SILT OR DEBRIS FROM CONSTRUCTION ACTIVITIES, PROTECTION SHALL BE INSTALLED ON INLET UPSTREAM SIDE.
11. DIRT PILE FROM EXCAVATION SHOULD BE PLACED ON THE UPHILL SIDE OF THE UTILITY CUT.
12. INSTALL WATTLES OR J-HOOK SILT FENCE WHERE STREET LONGITUDINAL SLOPE EXCEEDS 2.5%.

AS-BUILT INFORMATION

CONTRACTOR	DATE
WORK STARTED BY	DATE
FIELD ACCEPTANCE BY	DATE
DRAWINGS CORRECTED BY	DATE
MICRO-FILM INFORMATION	DATE
RECORDED BY	DATE
NO.	

BENCH MARKS

CONTRACTOR	DATE
WORK STARTED BY	DATE
FIELD ACCEPTANCE BY	DATE
DRAWINGS CORRECTED BY	DATE
MICRO-FILM INFORMATION	DATE
RECORDED BY	DATE
NO.	

SURVEY INFORMATION

NO.	DATE	BY
1	05/01	AERIAL MAPPING
2	06/01	BOUNDARY
3	10/01	BOUNDARY

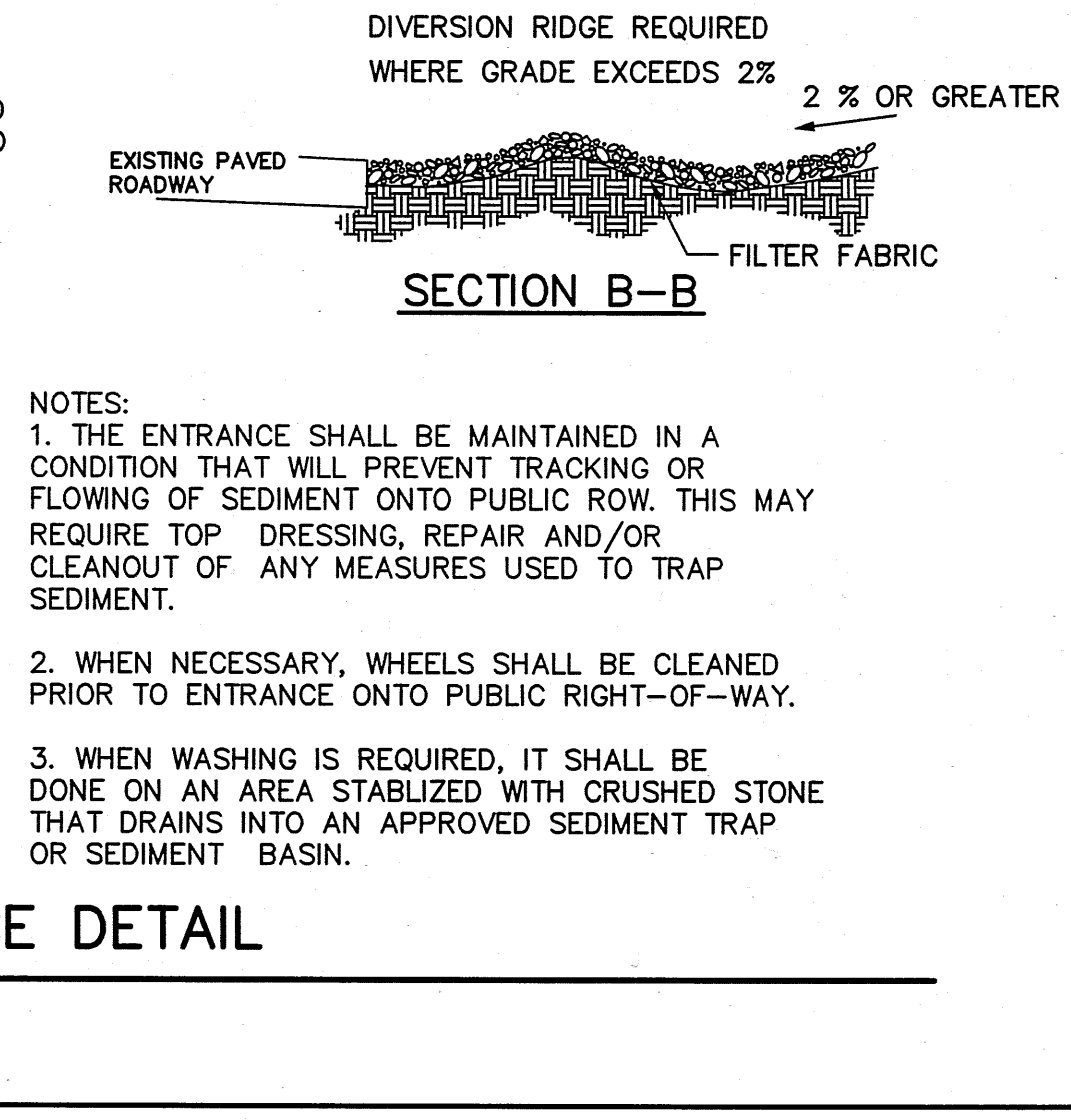
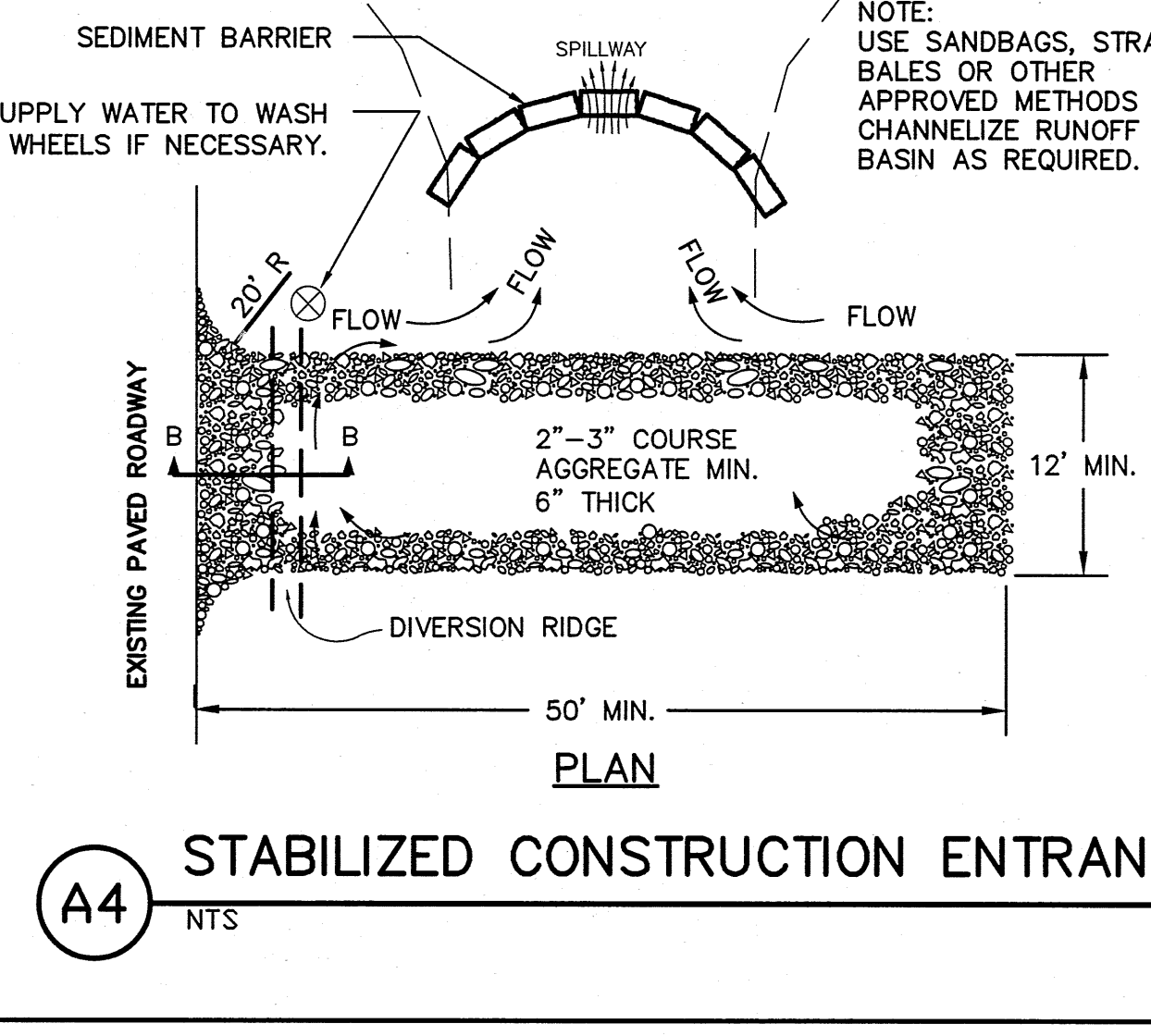
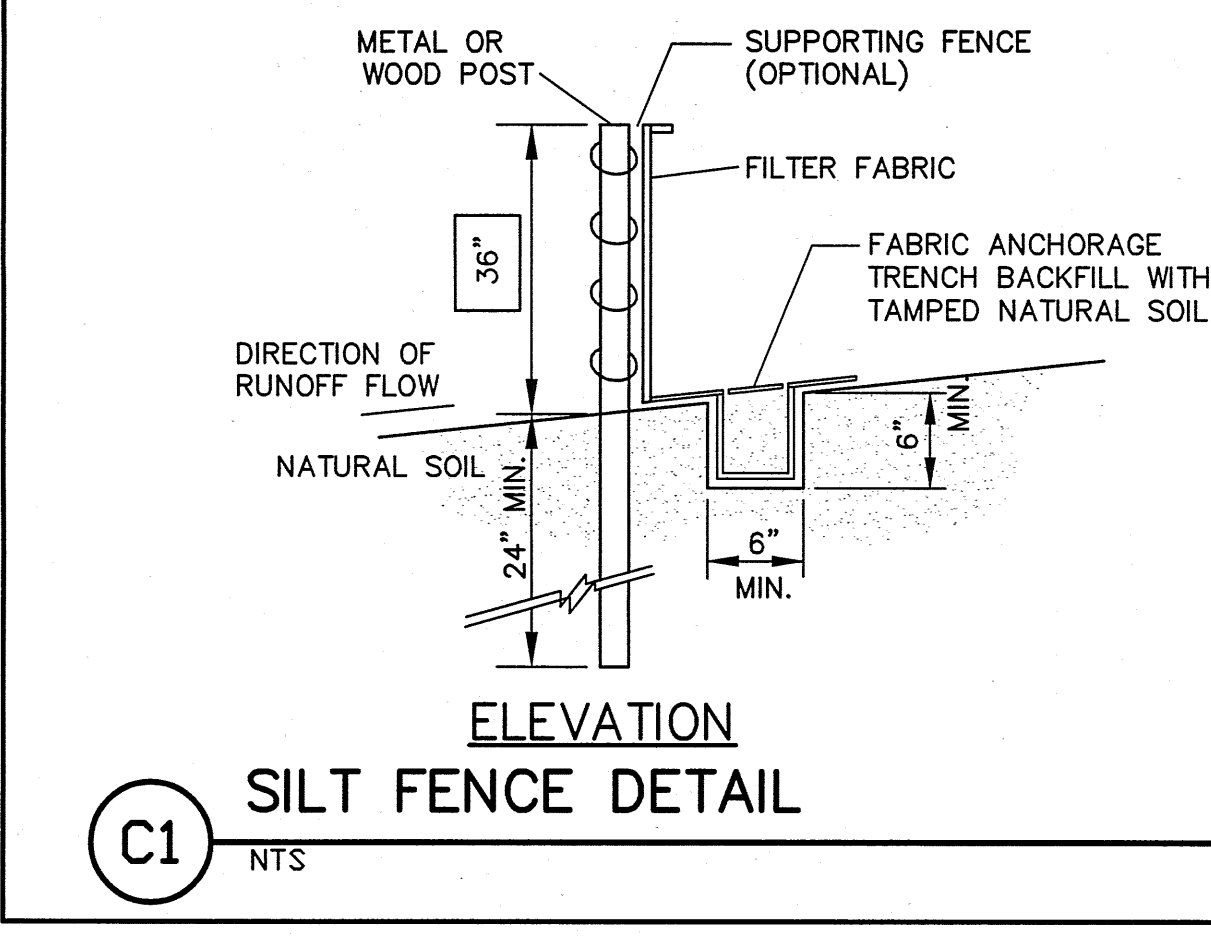
ENGINEER'S SEAL

NO.	DATE	REVISIONS	BY
1		DESIGN REVIEW COMMITTEE	KIS
2		DESIGN REVIEW COMMITTEE	WKL
3		DESIGN REVIEW COMMITTEE	DSA

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

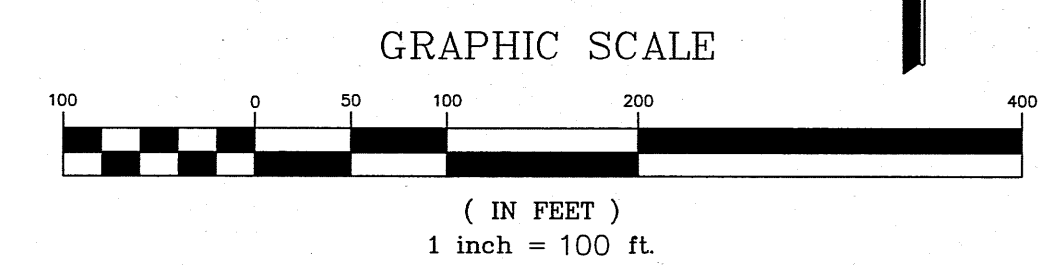
**LA CUENTISTA SUBDIVISION UNIT II
OFFSITE EROSION & SEDIMENT CONTROL PLAN**

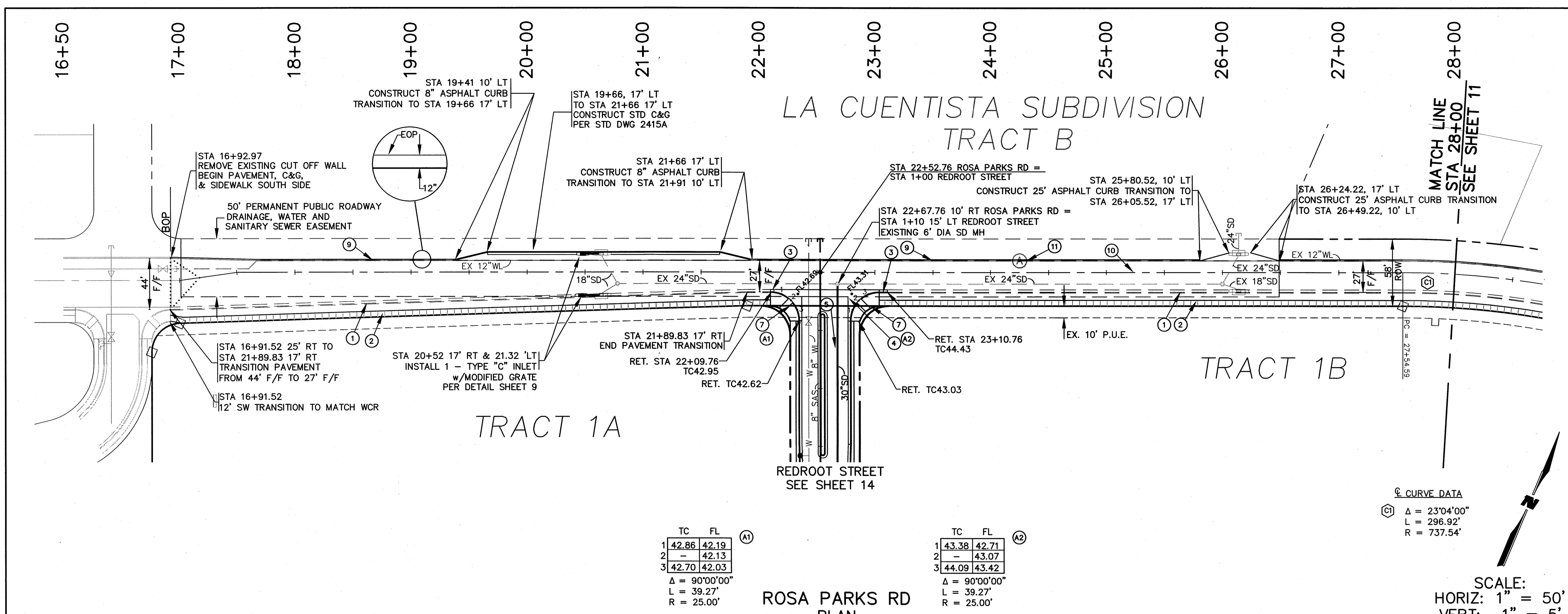
Design Review Committee	City Engineer Approval
APPROVED MAR 01 2017 DESIGN REVIEW COMMITTEE	APPROVED MAR 01 2017 CITY ENGINEER
City Project No. 709786	Zone Map No. C-10, C-11
Drawing 9A	Sheet 17



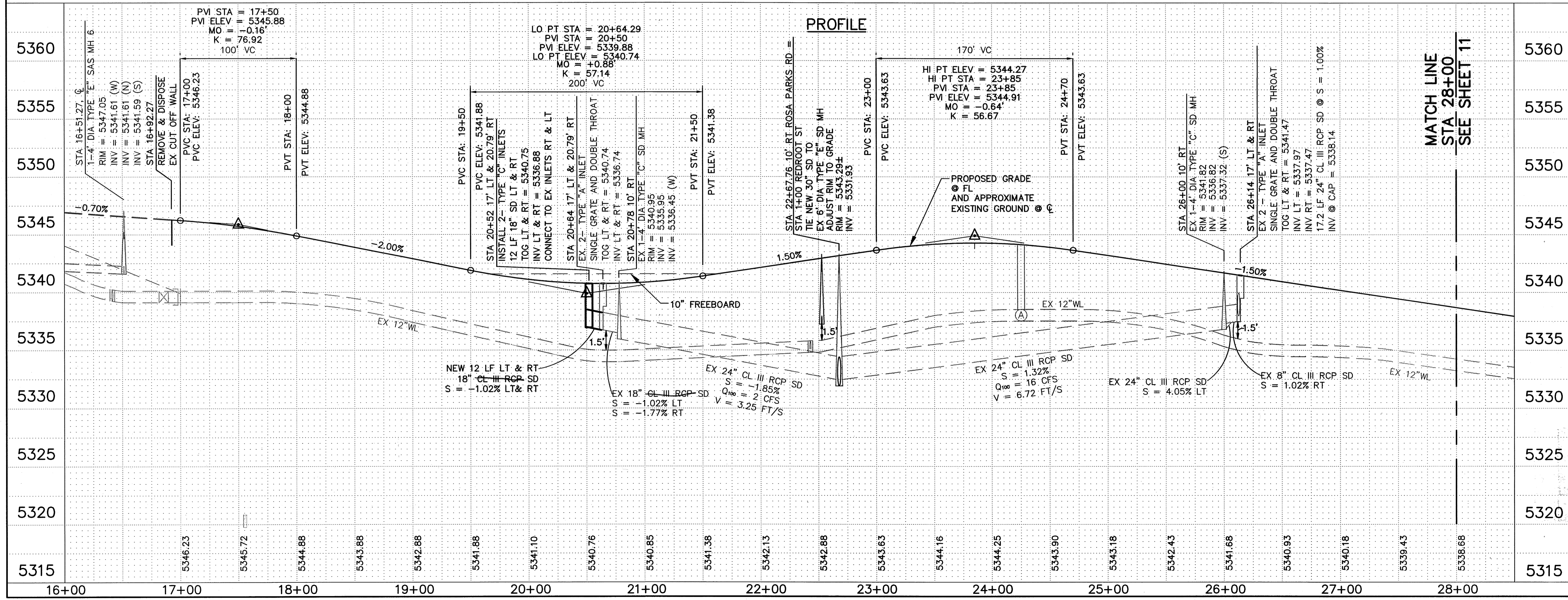
LEGEND

- SILT FENCE
- - - PHASE LINE
- COIR ROLL (WATTLE)
- INLET PROTECTION (PROPOSED INLET)
- INLET PROTECTION (EXISTING INLET)
- DIRECTION OF FLOW
- ▨ CONSTRUCTION ENTRANCE

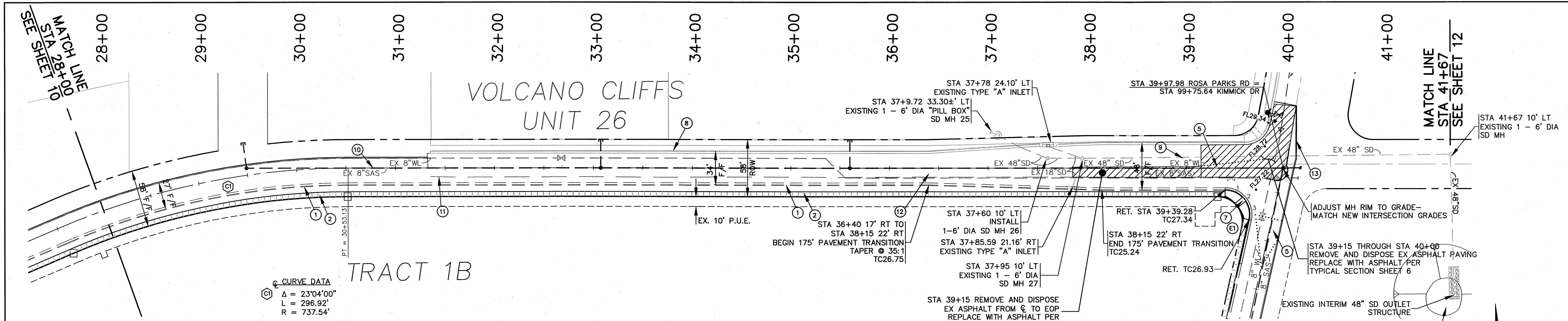




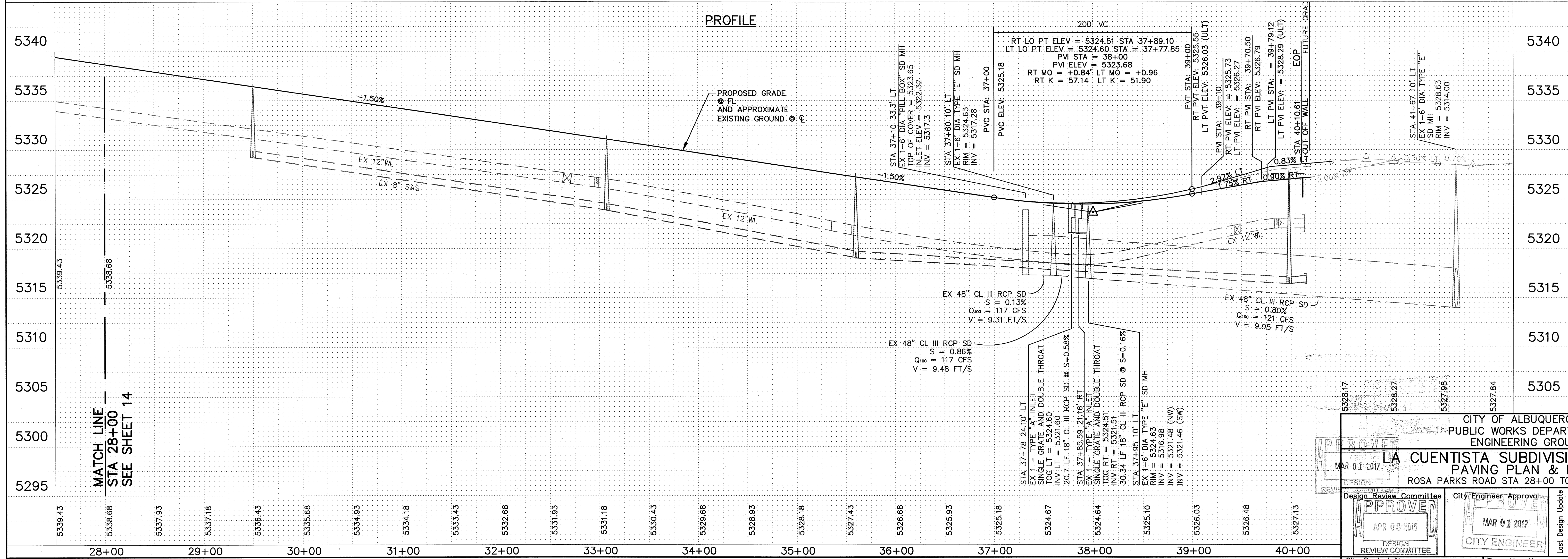
- NOTES**
- SEE TYPICAL ROADWAY SECTION A1 SHEET 6
- ADJUST ALL SD & SAS MH RIMS TO GRADE PER STD DWG 2111
- ① EXISTING STANDARD CURB & GUTTER
 - ② CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (NON-DEFERRED)
 - ③ 22+09.76 17' RT REMOVE AND DISPOSE 3 LF EXISTING C&G INCIDENTAL TO PROJECT
23+03.76 17' RT CONSTRUCT 7 LF OF STD C&G PER STD DWG 2415A
 - ④ CONSTRUCT 6' VALLEY GUTTER PER STD DWG 2420
 - ⑤ 50' CROWN REDUCTION PER STD DWG 2401
 - ⑥ CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (DEFERRED)
 - ⑦ CONSTRUCT WHEELCHAIR RAMP PER STD DWG 2441 CASE II, SEE TYP DETAIL, SHEET 6
 - ⑧ NOT USED
 - ⑨ 4" SOLID WHITE STRIPE SEE SHEET 7
 - ⑩ 4" DOUBLE YELLOW STRIPE
 - ⑪ PAVE AROUND EX AIR RELEASE VALVE ADJUST TO GRADE AS NEEDED
- ALL PROFILE ELEVATIONS ARE FLOWLINE (FL) ELEVATIONS RIGHT (RT) AND LEFT (LT) UNLESS OTHERWISE NOTED
- CONSTRUCT ALL 8" ASPHALT CURB PER STD DWG 2415B
- CONSTRUCT ALL STD C&G PER STD DWG 2415A



AS-BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL	
CONTRACTOR	DATE	ALUMINUM DISK STAMPED	DATE	FIELD NOTES	NO.	REMARKS	BY
ACS 1 3/4"	"ACS BM 5-D11", LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING	NO.	DATE	NO.			
WORKED BY	INSPECTOR'S ACCEPTANCE BY	AERIAL MAPPING, WCI 05/01	BOUNDARY, WCI 06/01	BOUNDARY, WCI 10/01	NO.	NO.	NO.
DATE	DATE	DATE	DATE	DATE	NO.	NO.	NO.
MICRO-FILM INFORMATION		ELEVATION = 5270.087 FT.		NAVD 88 U.S. FEET			



- NOTES**
- SEE TYPICAL ROADWAY SECTION A1-A3 SHEET 6
 - ADJUST ALL SD & SAS MH RIMS TO GRADE PER STD DWG 2111
 - EXISTING STANDARD CURB & GUTTER
 - CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (NON-DEFERRED)
 - CONSTRUCT WHEELCHAIR RAMP PER DTL SHEET 6
 - CONSTRUCT 12' VALLEY GUTTER PER STD DWG 2420
 - 50' CROWN REDUCTION PER STD DWG 2401
 - CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (DEFERRED)
 - CONSTRUCT WHEELCHAIR RAMP PER STD DWG 2441 CASE II, SEE DTL SHEET 6
 - EX C&G, SW, PAVING - SAD 228
 - 4" SOLID WHITE STRIPE SEE SHEET 6
 - 4" DOUBLE YELLOW STRIPE SEE SHEET 6
 - STA 31+35 BEGIN SECTION A2, SAWCUT 1' FROM EX EOP REMOVE AND DISPOSE EX PAVING NEW PAVING TO MATCH EX PAVING ELEVATION
 - STA 36+40 BEGIN SECTION A3 SAWCUT 1' FROM EX EOP REMOVE AND DISPOSE EX PAVING NEW PAVING TO MATCH EX PAVING ELEVATION
 - TRANSITION EOP TO MATCH KIMMICK DR EOP NORTH OF ROSA PARKS RD
 - ALL PROFILE ELEVATIONS ARE FLOWLINE (FL) ELEVATIONS RIGHT (RT) AND LEFT (LT) UNLESS OTHERWISE NOTED
- CURVE DATA**
- ① Δ = 23°04'00"
L = 296.92'
R = 737.54'
- TC FL**
- | | | |
|---|-------|-------|
| 1 | 27.24 | 27.57 |
| 2 | | 26.96 |
| 3 | 27.03 | 26.36 |
- Δ = 102°38'00"
L = 44.78'
R = 25.00'
- NOTE:**
NON-DEFERRED SIDEWALK
STA 33+63.52 RT TO STA 39+45.52 RT



5339.43	5338.68	5337.93	5337.18	5336.43	5335.68	5334.93	5334.18	5333.43	5332.68	5331.93	5331.18	5330.43	5329.68	5328.93	5328.18	5327.43	5326.68	5325.93	5325.18	5324.43	5323.68	5322.93	5322.18	5321.43	5320.68	5319.93	5319.18	5318.43	5317.68	5316.93	5316.18	5315.43	5314.68	5313.93	5313.18	5312.43	5311.68	5310.93	5310.18	5309.43	5308.68	5307.93	5307.18	5306.43	5305.68	5304.93	5304.18	5303.43	5302.68	5301.93	5301.18	5300.43	5299.68	5298.93	5298.18
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AS-BUILT INFORMATION

CONTRACTOR: _____ DATE: _____

WORKED BY: _____ DATE: _____

INSPECTOR'S ACCEPTANCE BY: _____ DATE: _____

REVISION BY: _____ DATE: _____

VERIFICATION BY: _____ DATE: _____

CONNECTED BY: _____ DATE: _____

MICRO-FILM INFORMATION

RECORDED BY: _____ DATE: _____

NO.: _____

BENCH MARKS

ACS 1 3/4" ALUMINUM DISK STAMPED
"ACS BM 5-D11", LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING

ELEVATION = 5270.087 FT.
NAVD 88 U.S. FEET

SURVEY INFORMATION

FIELD NOTES

NO. _____ DATE _____

BY _____

AERIAL MAPPING, W/05/01

BOUNDARY, W/06/01

BOUNDARY, W/10/01

ENGINEER'S SEAL

REVISIONS

NO.	DATE	REMARKS	BY

WILSON & COMPANY, ENGINEERS & ARCHITECTS
DESIGNED BY: KIS DATE: DEC 2004
DRAWN BY: VKL DATE: DEC 2004
CHECKED BY: DSA DATE: DEC 2004

APPROVED

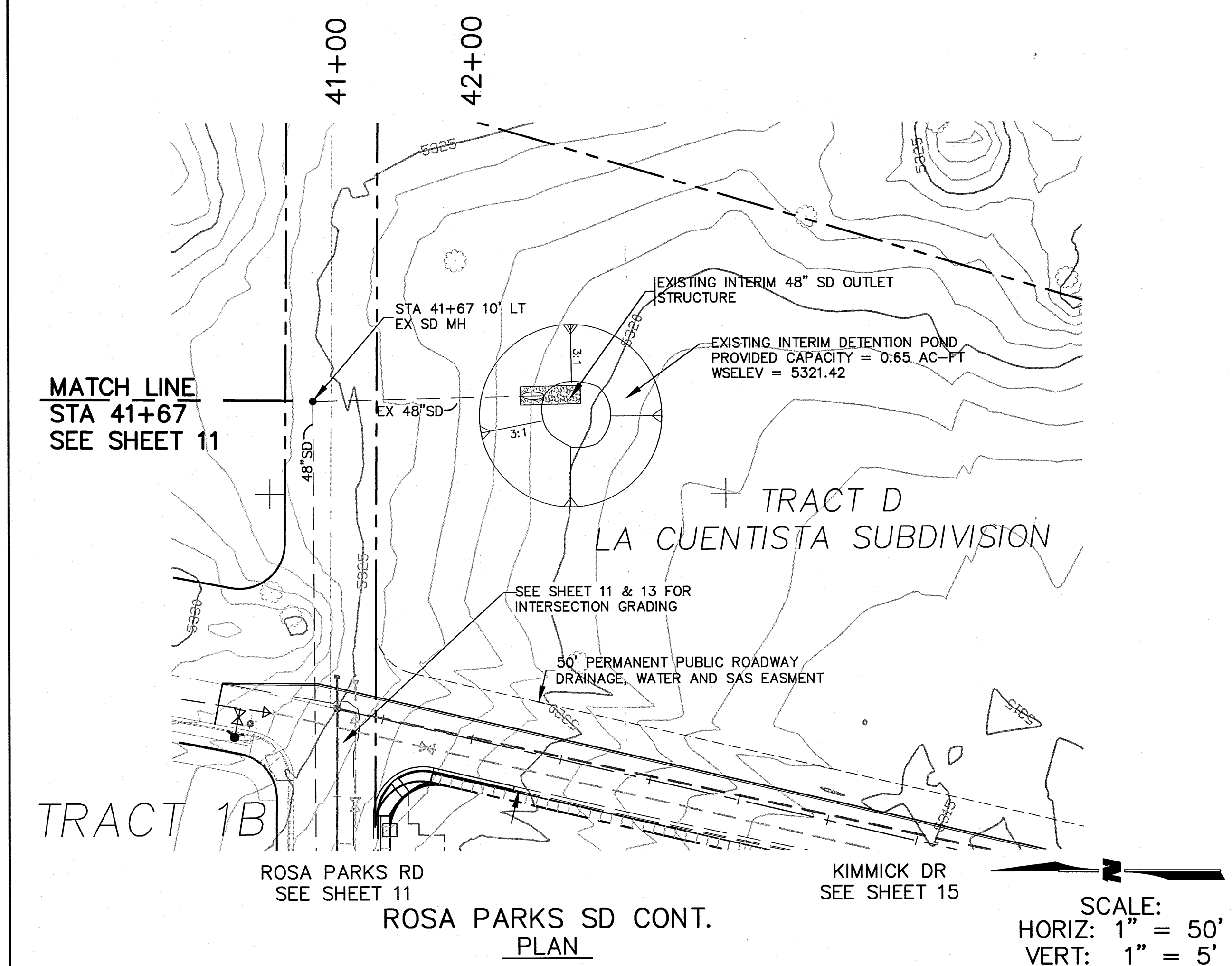
LA CUENTISTA SUBDIVISION - UNIT II
PAVING PLAN & PROFILE
ROSA PARKS ROAD STA 28+00 TO STA 39+97.98

Design Review Committee APPROVED APR 09 2015

City Engineer Approval MAR 01 2017

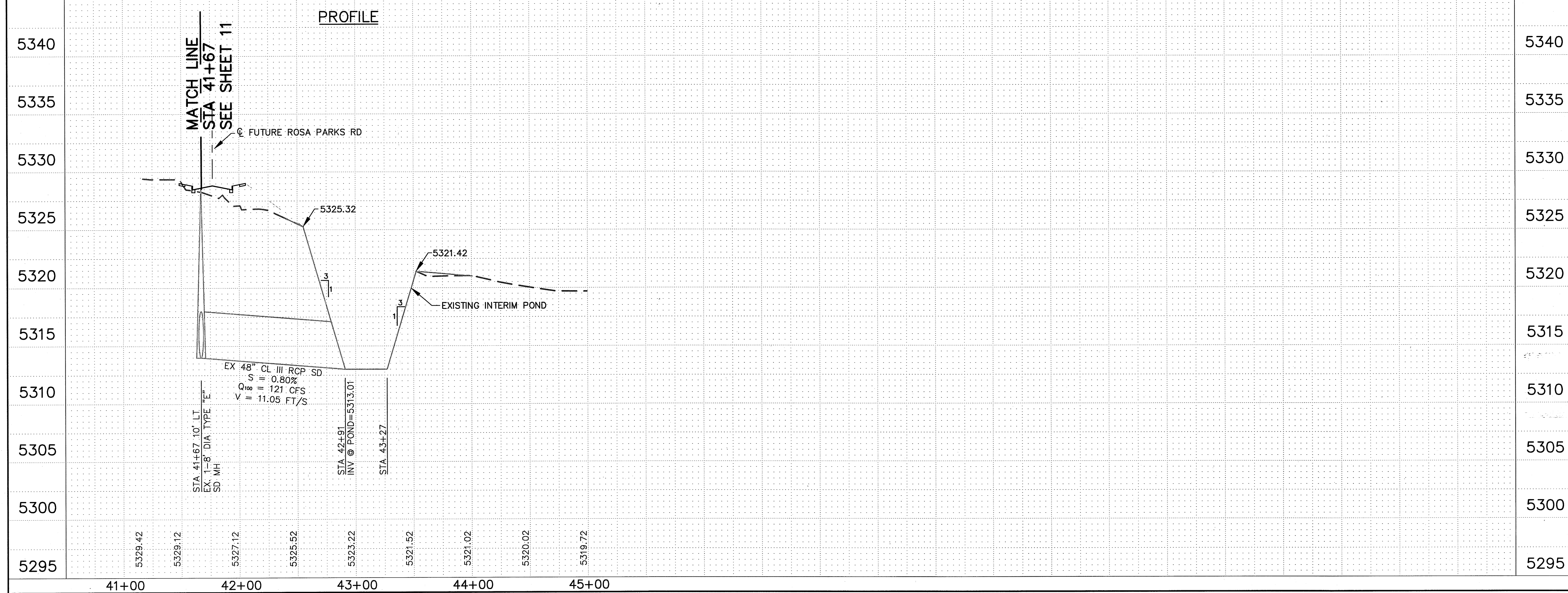
City Project No. 709786 Zone Map No. C-10, C-11 Drawing 11 Sheet 17

WILSON# 1460008900 JANUARY 2015



ROSA PARKS SD CONT. PLAN

SCALE:
HORIZ: 1" = 50'
VERT: 1" = 5'



PROFILE

NOTES - N/A THIS SHEET

ALL PROFILE ELEVATIONS ARE FLOWLINE (FL) ELEVATIONS RIGHT (RT) AND LEFT (LT) UNLESS OTHERWISE NOTED

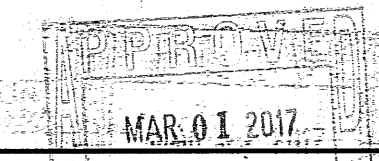
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CONTRACTOR	DATE
WORKED BY	DATE
INSPECTOR'S SIGNATURE	DATE
FIELD VERIFICATION BY	DATE
RECORDED BY	DATE
NO.	

BENCH MARKS	
ACS 1" ALUMINUM DISK STAMPED	
"ACS BM 5-D11", LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING	
ELEVATION = 5270.087 FT.	
NAVD 88 U.S. FEET	

SURVEY INFORMATION	
FIELD NOTES	DATE
BY	NO.
AERIAL MAPPING, WCI 05/01	
BOUNDARY, WCI 06/01	
BOUNDARY, WCI 10/01	

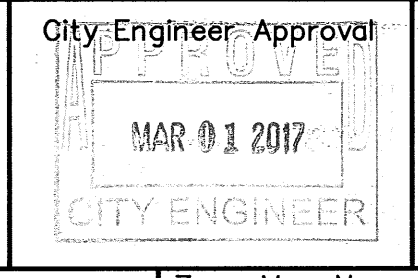


REVISIONS		
NO.	DATE	BY
1		
2		
3		
4		
5		
6		
7		
8		
9		
10		



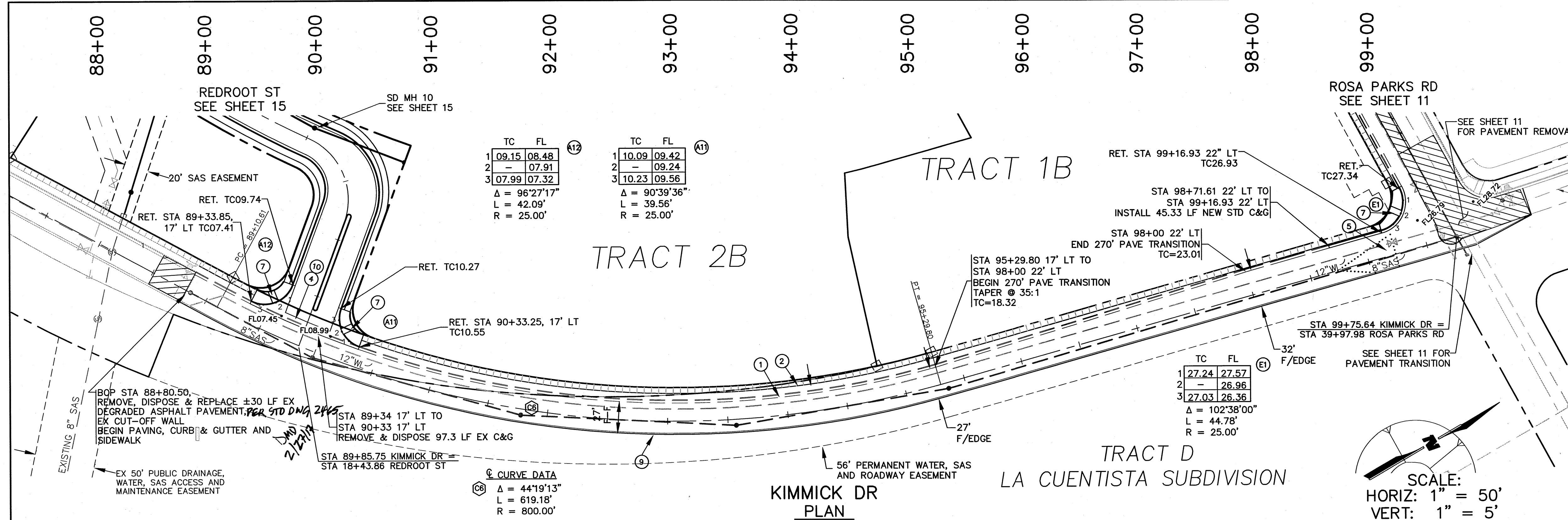
CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

**LA CUENTISTA SUBDIVISION - UNIT II
PAVING PLAN & PROFILE
ROSA PARKS ROAD CONT.**

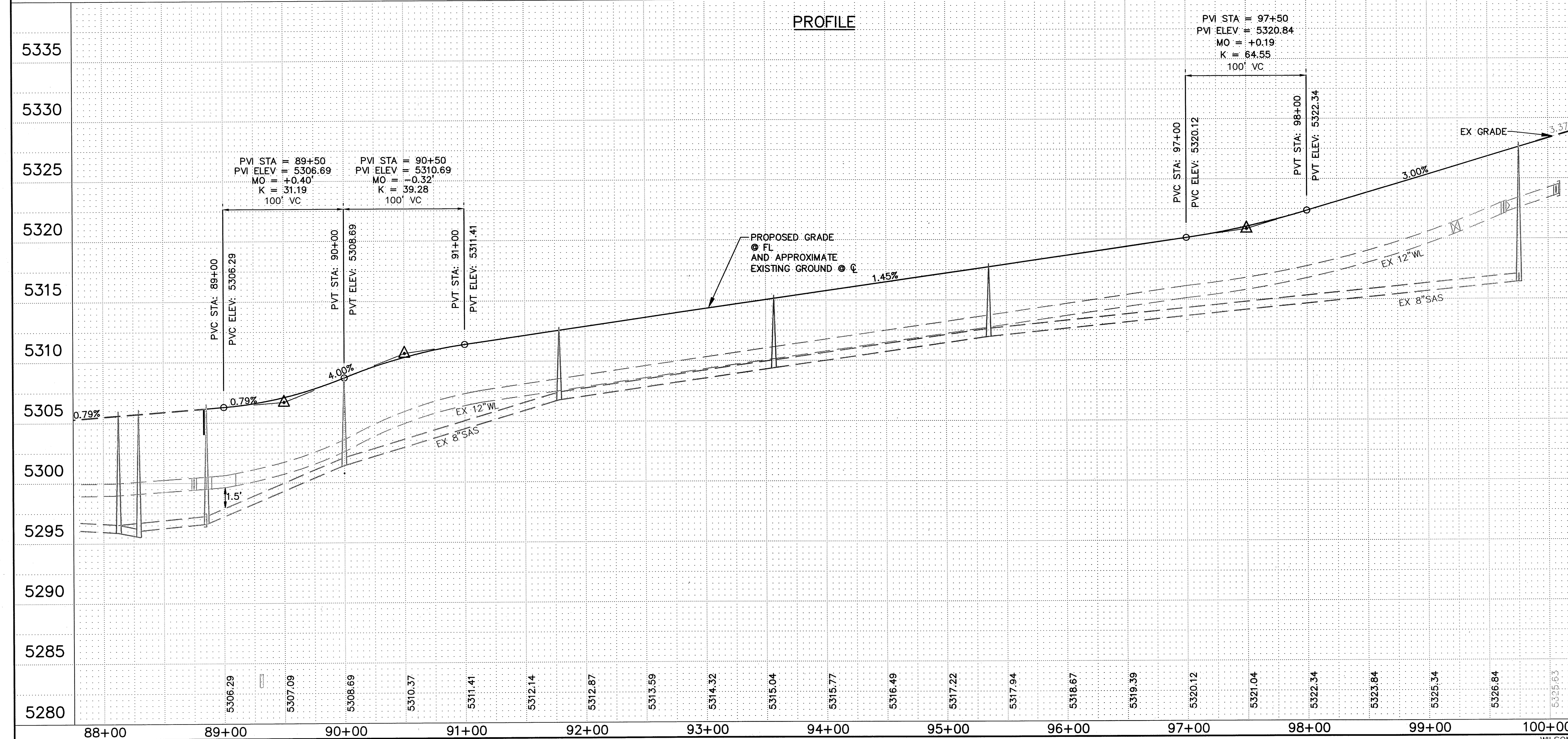


Last Design Update	Mo./Day/Yr.	Mo./Day/Yr.

M:\MSD\1460008900\2_DISCIPLINES\SHEETS\OFFSITES\146089PP03.dwg\03-25-15\SDO



- NOTES**
- SEE TYPICAL ROADWAY SECTION B SHEET 5
- ADJUST ALL SD & SAS MH RIMS TO GRADE PER STD DWG 2111
- EXISTING STANDARD CURB & GUTTER PER STD DWG 2415A
 - CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (NON-DEFERRED)
 - CONSTRUCT WHEELCHAIR RAMP PER DTL SHEET 6
 - CONSTRUCT 6" VALLEY GUTTER PER STD DWG 2420
 - 50' CROWN REDUCTION PER STD DWG 2401
 - CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (DEFERRED)
 - CONSTRUCT WHEELCHAIR RAMP PER STD DWG 2441 CASE II, SEE DTL SHEET 6
 - NOT USED
 - 4" SOLID WHITE STRIPE
 - STA 89+33.85 17' LT TO STA 90+33.25, 17' LT REMOVE AND DISPOSE EX C&G, SW AS REQUIRED FOR INTERSECTION
- ALL PROFILE ELEVATIONS ARE FLOWLINE (FL) ELEVATIONS RIGHT (RT) AND LEFT (LT) UNLESS OTHERWISE NOTED
- NOTE:
NON-DEFERRED SIDEWALK STA 88+40.78 LT TO STA 99+23.18 LT



AS-BUILT INFORMATION

CONTRACTOR	DATE
INSPECTOR'S NAME	DATE
FIELD MADE BY	DATE
VERIFICATION BY	DATE
CONNECTED BY	DATE
RECORDED BY	DATE

BENCH MARKS

ACCS 1" ALUMINUM DISK STAMPED	DATE
"ACS BM 5-D11", LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING	DATE
ELEVATION = 5270.087 FT.	DATE
NAVD 88 U.S. FEET	DATE

SURVEY INFORMATION

NO.	BY	DATE
AERIAL MAPPING	WCI 05/01	
BOUNDARY	WCI 06/01	
BOUNDARY	WCI 10/01	

ENGINEER'S SEAL

REVISIONS

NO.	DATE	REMARKS	BY

DESIGNED BY: KIS DATE: DEC 2004
 DRAWN BY: VKL DATE: DEC 2004
 CHECKED BY: DSA DATE: DEC 2004

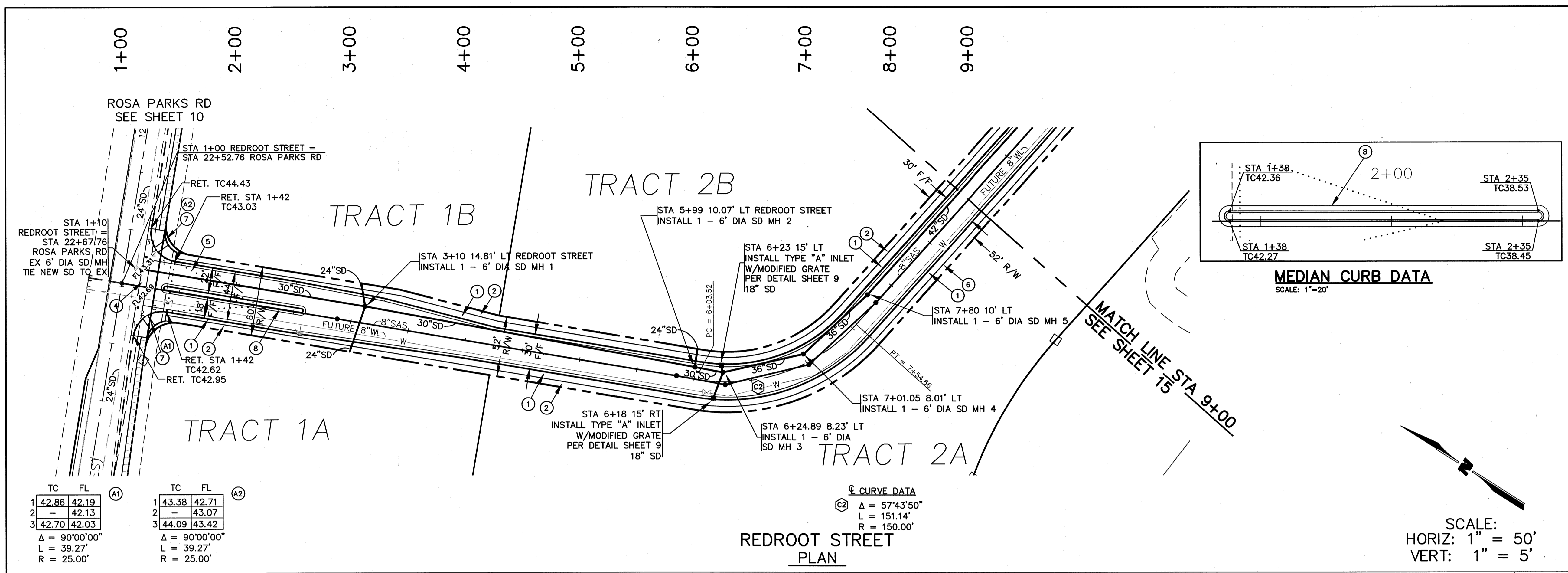
APPROVE
MAR 01 2017
DESIGN REVIEW COMMITTEE

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

LA CUENTISTA SUBDIVISION - UNIT II
PAVING PLAN & PROFILE
KIMMICK DRIVE

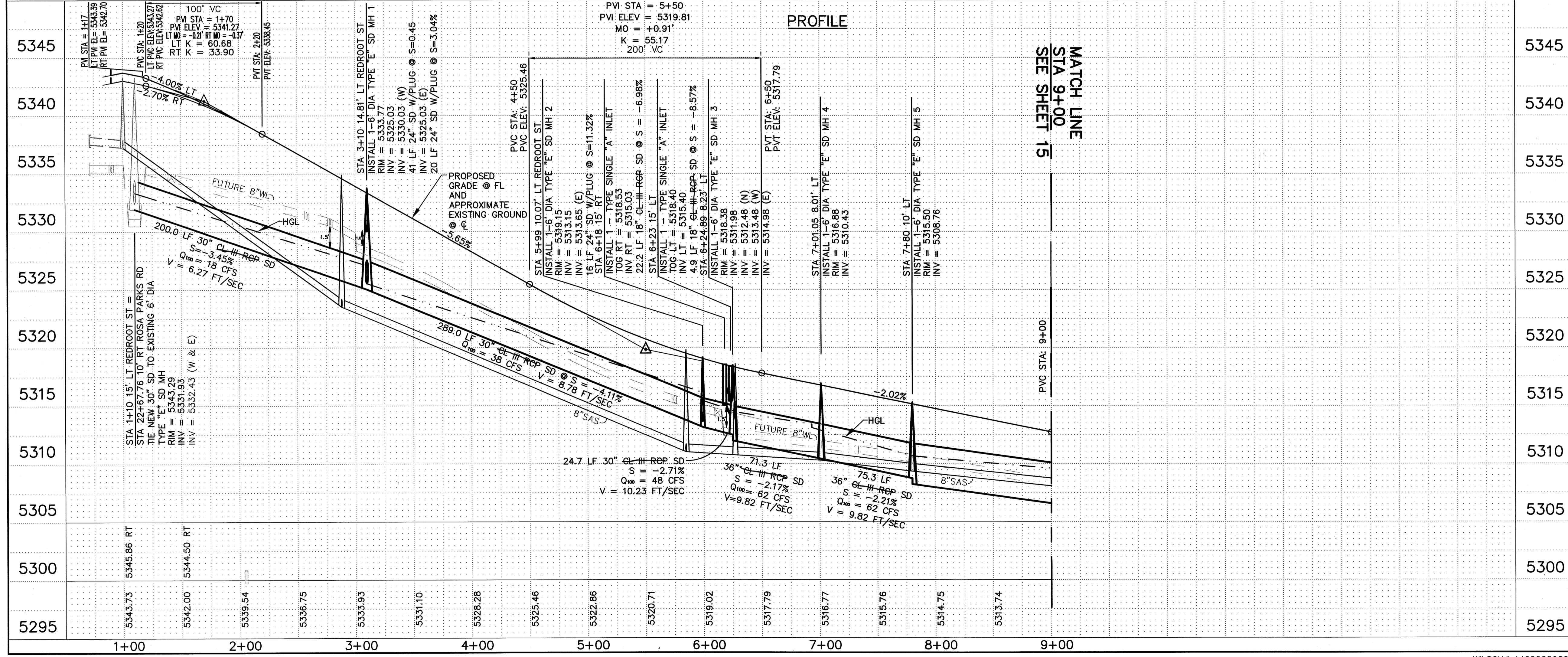
Design Review Committee: APR 03 2015
 City Engineer Approval: MAR 01 2017

City Project No. 709786
 Zone Map No. C-10, C-11
 Drawing 13
 Sheet 17



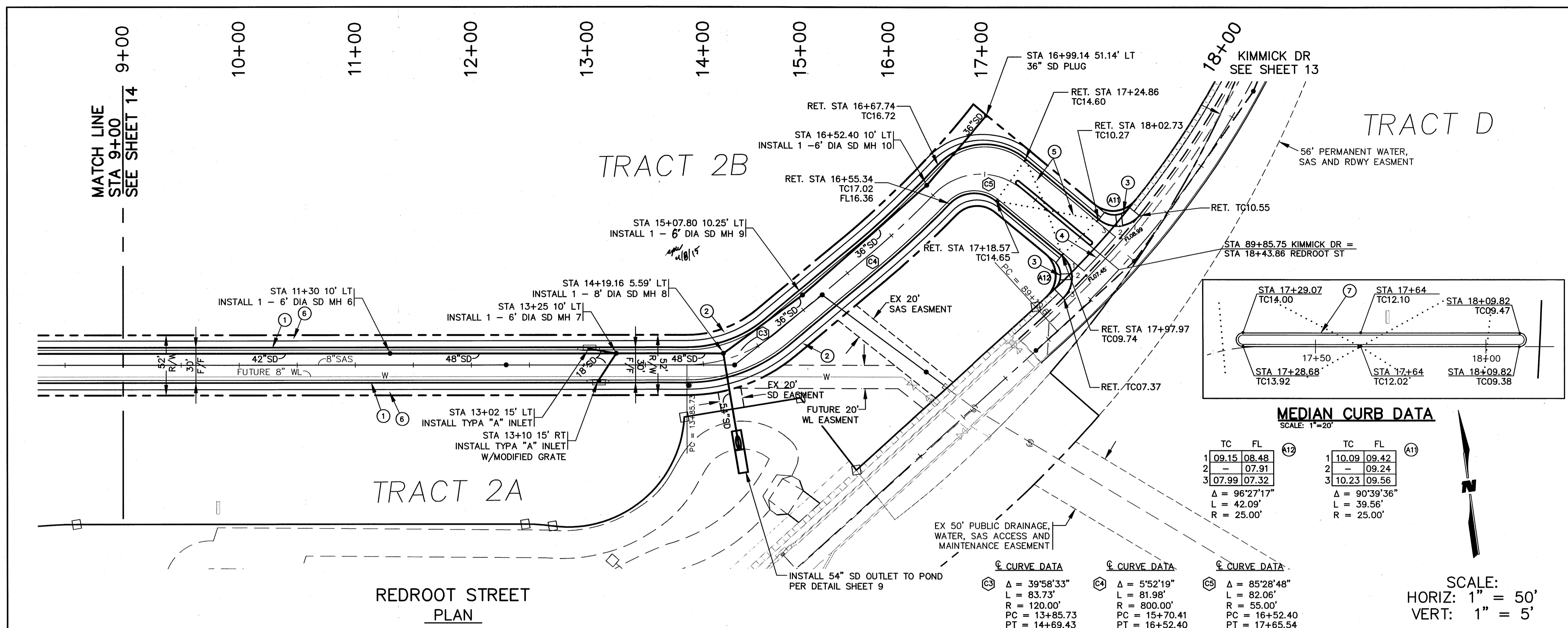
- NOTES**
- SEE TYPICAL ROADWAY SECTION E SHEET 6
- ADJUST ALL SD & SAS MH RIMS TO GRADE PER STD DWG 2111
- CONSTRUCT STANDARD CURB & GUTTER PER STD DWG 2415A
 - CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (NON-DEFERRED)
 - CONSTRUCT WHEELCHAIR RAMP TYPE F PER DTL SHEET 6
 - CONSTRUCT 6" VALLEY GUTTER PER STD DWG 2420
 - 78' CROWN REDUCTION PER STD DWG 2401
 - CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (DEFERRED)
 - CONSTRUCT WHEELCHAIR RAMP PER STD DWG 2441 CASE II, SEE DTL SHEET 6
 - INSTALL MEDIAN C&G PER STD DWG 2415A
- ALL PROFILE ELEVATIONS ARE FLOWLINE (FL) ELEVATIONS RIGHT (RT) AND LEFT (LT) UNLESS OTHERWISE NOTED

AS-BUILT INFORMATION		BENCH MARKS		SURVEY INFORMATION	
CONTRACTOR	DATE	ACS 1" ALUMINUM DISK STAMPED	DATE	NO.	BY
INSPECTOR	DATE	"ACS BM 5-D11", LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EXPOSED TO LAVA ROCK IN OUTCROPPING	AERIAL MAPPING, WCI 05/01	BOUNDARY	BOUNDARY
FIELD VERIFICATION BY	DATE		BOUNDARY	BOUNDARY	BOUNDARY
CONNECTED BY	DATE				
MICRO-FILM INFORMATION					
RECORDED BY	DATE				
NO.					



ENGINEER'S SEAL		REVISIONS	
	NO.	DATE	BY
	WILSON & COMPANY, ENGINEERS & ARCHITECTS DESIGNED BY: KIS DRAWN BY: VKL CHECKED BY: DSA		
MAR 01 2017 CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP		LA CIENTISTA SUBDIVISION - UNIT II PAVING PLAN & PROFILE REDROOT STREET STA 1+00 TO STA 9+00	
DESIGN REVIEW COMMITTEE	CITY ENGINEER APPROVAL	Mo./Day/Yr.	Mo./Day/Yr.
APPROVED APR 03 2015 DESIGN REVIEW COMMITTEE	APPROVED MAR 01 2017 CITY ENGINEER		
City Project No.	Zone Map No.	Drawing	Sheet
709786	C-10, C-11	14	17

12-20-16
SEE SD NOTE SHEET TOP 20



NOTES

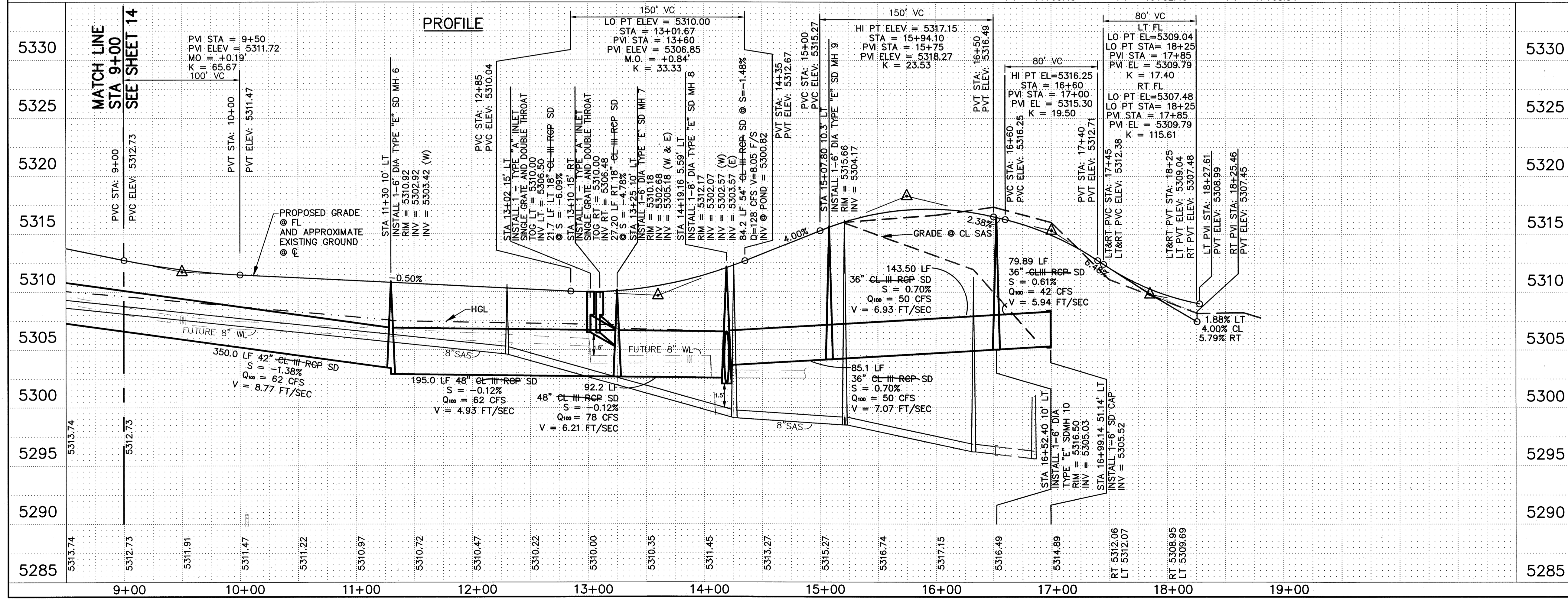
SEE TYPICAL ROADWAY SECTION C AND SECTION D SHEET 6

- CONSTRUCT STANDARD CURB & GUTTER PER STD DWG 2415A
- CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (NON-DEFERRED)
- CONSTRUCT WHEELCHAIR RAMP TYPE F PER DTL SHEET 6
- CONSTRUCT 6' VALLEY GUTTER PER STD DWG 2420
- 50' CROWN REDUCTION PER STD DWG 2401
- CONSTRUCT 5' SIDEWALK PER STD DWG 2430 (DEFERRED)
- INSTALL MEDIAN C&G PER STD DWG 2415A

ALL PROFILE ELEVATIONS ARE FLOWLINE (FL) ELEVATIONS RIGHT (RT) AND LEFT (LT) UNLESS OTHERWISE NOTED

NOTE:
NON-DEFERRED SIDEWALK
STA 13+82.63 RT TO STA 14+70.53
STA 13+08.93 LT TO STA 14+65.19

SCALE:
HORIZ: 1" = 50'
VERT: 1" = 5'



ENGINEER'S SEAL

APPROVED

MAR 01 2017

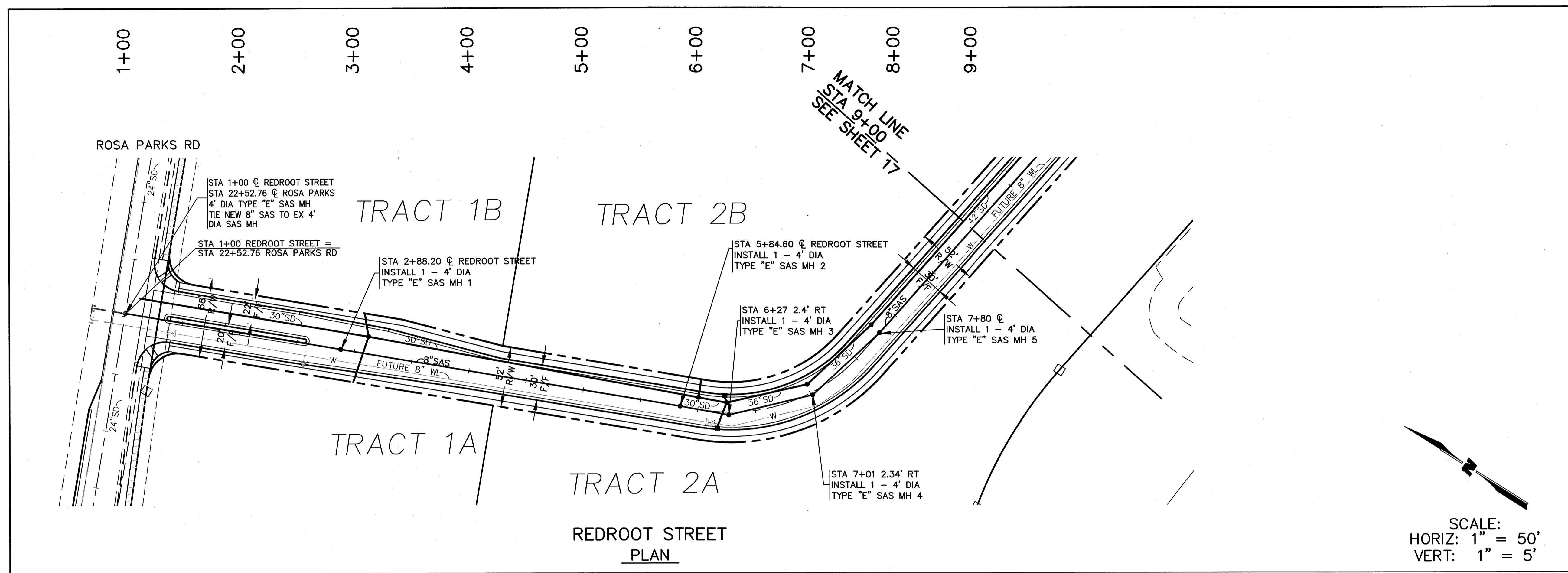
CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

LA CUENTISTA SUBDIVISION - UNIT II
PAVING PLAN & PROFILE
REDROOT STREET STA 9+00 TO STA 15+20.29

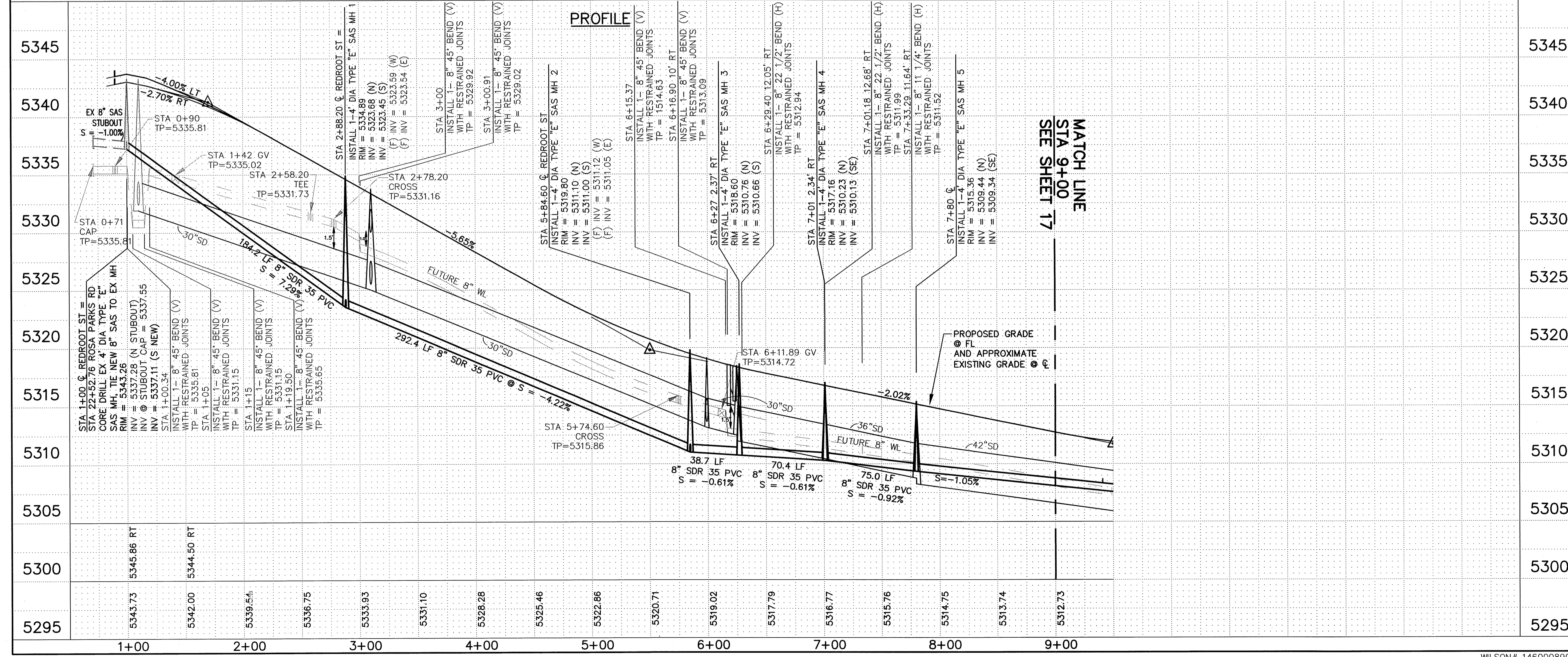
NO.	DATE	REVISIONS	BY

DESIGNED BY: KIS
DRAWN BY: VKL
CHECKED BY: DSA

WILSON & COMPANY, ENGINEERS & ARCHITECTS
DATE DEC 2004
DATE DEC 2004
DATE DEC 2004



- NOTES**
- ADJUST ALL SD & SAS MH RIMS TO GRADE PER STD DWG 2111
 - INSTALL 4' DIAMETER TYPE 'E' MH PER STD DWG 2102, MH FRAME AND COVERS PER STD DWG 2110



AS-BUILT INFORMATION

WORK NO.	DATE
SUPERVISOR'S ACCEPTANCE BY	DATE
VERIFICATION BY	DATE
DRAWINGS BY	DATE
MICRO-FILM INFORMATION	DATE
RECORDED BY	NO.

SURVEY INFORMATION

ACS 1" ALUMINUM DISK STAMPED	DATE
*ACS BM 5-D11", LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING	DATE
AERIAL MAPPING, WCI	DATE
BOUNDARY, WCI	DATE
BOUNDARY, WCI	DATE
ROCK IN OUTCROPPING	DATE
ELEVATION = 5270.087 FT.	
NAVD 88 U.S. FEET	

ENGINEER'S SEAL

NO.	DATE	REVISIONS	BY

WILSON & COMPANY, ENGINEERS & ARCHITECTS
 DESIGNED BY: KIS DATE: DEC 2004
 DRAWN BY: WKL DATE: DEC 2004
 CHECKED BY: DSA DATE: DEC 2004

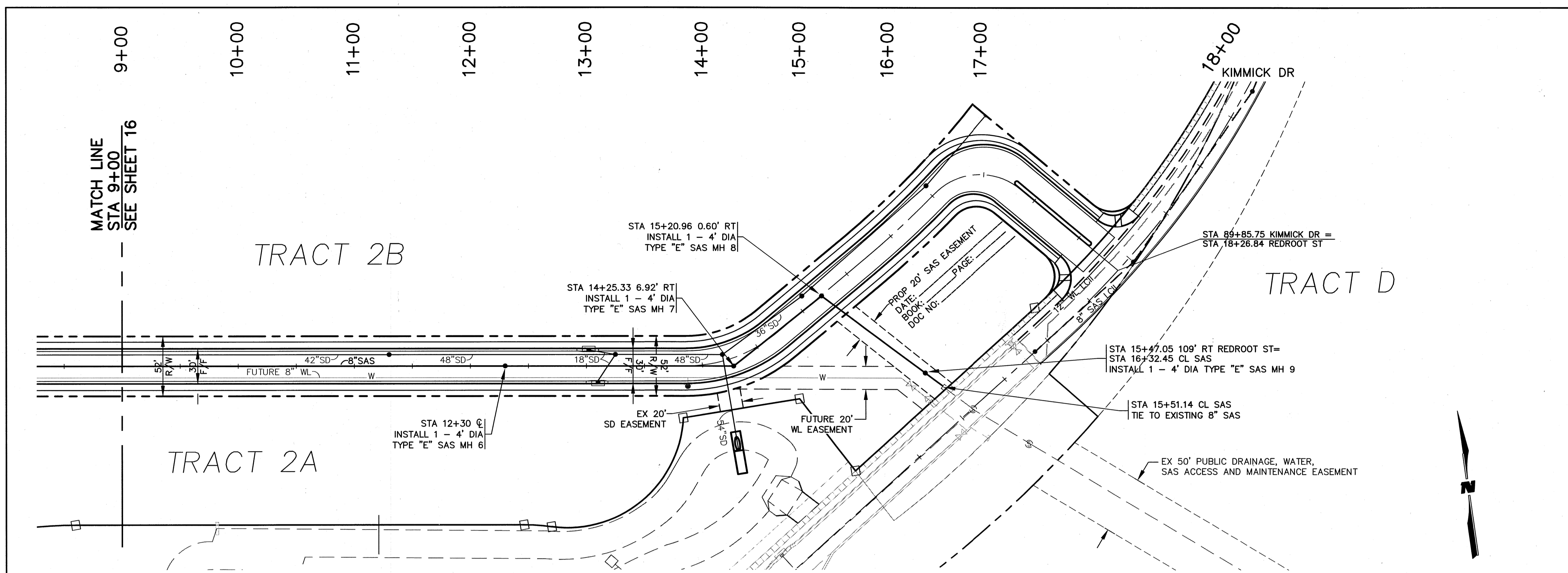
APPROVE
 MAR 03 2017
 DESIGN REVIEW COMMITTEE

CITY OF ALBUQUERQUE
 PUBLIC WORKS DEPARTMENT
 ENGINEERING GROUP

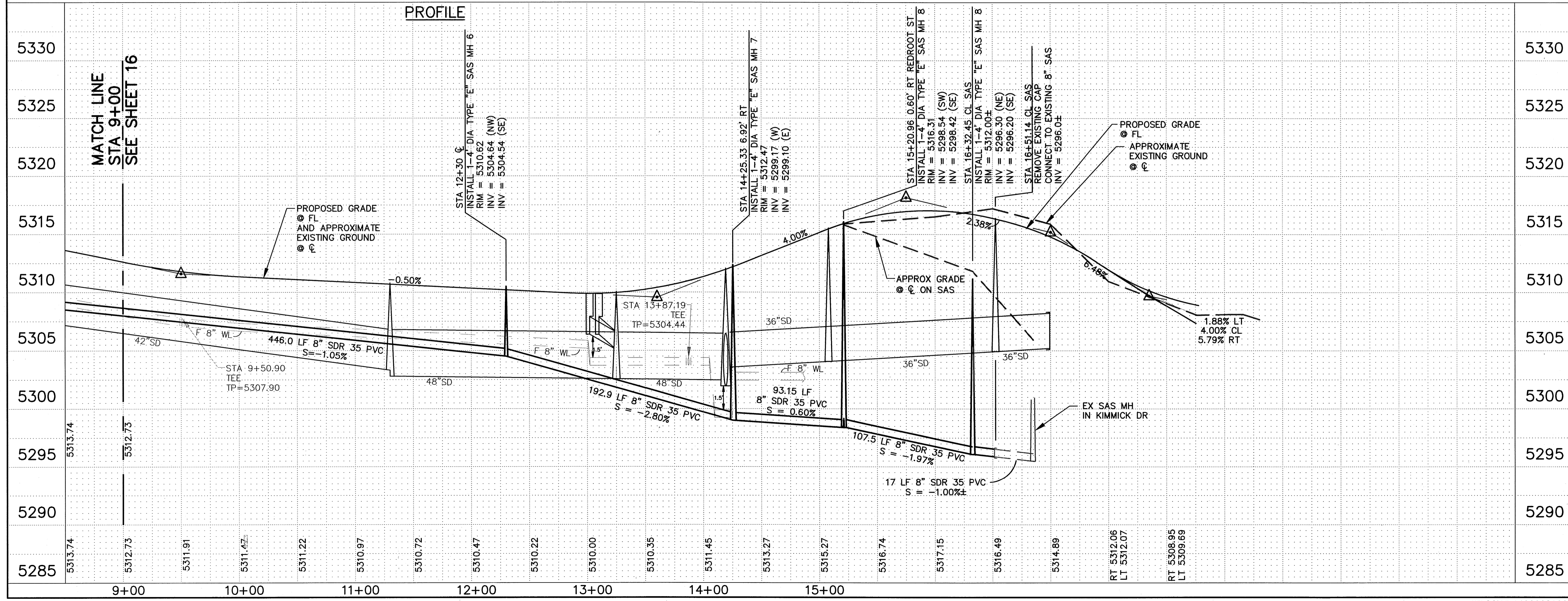
LA CUENTISTA SUBDIVISION - UNIT II
 UTILITY PLAN & PROFILE
 REDROOT STREET STA 1+00 TO STA 9+00

City Project No.	Zone Map No.	Drawing	Sheet
709786	C-10, C-11	16	17

WILSON & COMPANY



REDROOT STREET PLAN
SCALE:
HORIZ: 1" = 50'
VERT: 1" = 5'



- NOTES
- ADJUST ALL SD & SAS MH RIMS TO GRADE PER STD DWG 2111
 - INSTALL 4' DIAMETER TYPE 'E' MH PER STD DWG 2102, MH FRAME AND COVERS PER STD DWG 2110

AS-BUILT INFORMATION	
CONTRACTOR	DATE
WORK ORDER NO.	DATE
INSPECTOR'S RECEIPT NO.	DATE
VERIFICATION BY	DATE
CONNECTED BY	DATE
MICRO-FILM INFORMATION	
RECORDED BY	DATE
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BENCH MARKS	
ACS 1 3/4" ALUMINUM DISK STAMPED	DATE
"ACS BM 5-D11", LOCATED 72' ± ESE OF THE CENTER OF THE CUL-DE-SAC AT THE SOUTH END OF RIDGEWAY DR. NW, EPOXIED TO LAVA ROCK IN OUTCROPPING	DATE
ELEVATION = 5270.087 FT.	DATE
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SURVEY INFORMATION	
FIELD NOTES	DATE
BY	DATE
AERIAL MAPPING, WCI 05/01	DATE
BOUNDARY, WCI 06/01	DATE
BOUNDARY, WCI 10/01	DATE

ENGINEER'S SEAL

NO.	DATE	REVISIONS	BY

WILSON & COMPANY, ENGINEERS & ARCHITECTS
DESIGNED BY: KIS DATE: DEC 2004
DRAWN BY: VKL DATE: DEC 2004
CHECKED BY: DSA DATE: DEC 2004

APPROVED
MAR 01 2017
DESIGN REVIEW COMMITTEE

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

LA CUENTISTA SUBDIVISION - UNIT II
UTILITY PLAN & PROFILE
REDROOT STREET STA 9+00 TO STA 15+20.29

Design Review Committee APPROVED APR 03 2015
City Engineer Approval APPROVED MAR 01 2017
CITY ENGINEER

Last Design Update	Mo./Day/Yr.	Mo./Day/Yr.

STORM DRAINAGE POLYPROPYLENE PIPE TECHNICAL SPECIFICATIONS

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PART 1 — GENERAL

1.1 Related Requirements

A. Section - Submittals: Shop Drawings, Product Data, and Samples
 B. Section - Earth Moving: Excavation and Fill

1.2 Summary

A. This section includes gravity-flow storm drainage outside the building, with the following components:

a. Drainage piping, fittings, and accessories

1.3 Reference Standards

A. American Society for Testing and Materials (ASTM)

a. ASTM D2321 — Underground Installation of Thermoplastic Pipe for Sewers and Other Gravity-Flow Applications
 b. ASTM D3212 — Joints for Drain and Sewer Plastic Pipes Using Flexible Elastomeric Seals
 c. ASTM F477 — Elastomeric Seals (Gaskets) for Joining Plastic Pipe
 d. ASTM F2736 — 6 to 30 in. (152 to 762 mm) Polypropylene (PP) Corrugated Single Wall Pipe And Double Wall Pipe
 e. ASTM F2764 — 30 to 60 in. [300 to 1500 mm] Polypropylene (PP) Triple Wall Pipe and Fittings for Non-Pressure Storm Sewer Applications
 f. ASTM D1557 — Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft³ (2,700 kN-m/m³))

1.4 Definitions

A. PP — Polypropylene Pipe
 B. Piping System: All products associated with the drainage system including but not limited to pipe, fittings, drainage structures, geotextile, best management practice products and storage systems.

1.5 Performance Requirements

All pipe supplied shall meet the minimum joint performance requirements as defined herein and as further defined in the joint performance requirements of this specification.

A. Watertight Gravity-Flow, Non-Pressure, Drainage-Piping shall pass a 10.8 psi (104kPa), laboratory pressure test for 10 minutes with no visible leaks at the

joint or pipe wall. Piping shall pass same tests as above but with an axial joint misalignment of not less than 1 degree.

1.6 Submittals

The following shall be submitted by contractor in accordance with this technical specification.

Submittal Procedures:

A. Product Data for the following:

a. Pipe and Fittings

1. Product specifications
 2. Installation procedures

B. Products submitted as approved equal must be submitted at least 2 weeks prior to project bid opening and must be approved by project engineer. Submittal for approved equal product must contain a signed letter from an executive officer of the manufacturer stating product is equivalent to all applicable requirements of this specification and shall include all items listed in section 1.6 of this specification.

1.7 Delivery, Storage, and Handling

A. All pipe and fittings shall be delivered to the site and unloaded with handling that conforms to the manufacturer's instructions for reasonable care. Pipe shall not be rolled or dragged over gravel or rock during handling. The Contractor shall take necessary precautions to ensure the method used in lifting or placing the pipe does not induce undue stress fatigue in the pipe.

PART 2 — PRODUCTS

2.1 Corrugated Polypropylene (PP) Pipe

2.1.1 General

A. Twelve- through 30-inch (300 through 750 mm) pipe shall be polypropylene or pre-approved equal. Pipe supplied shall be smooth interior and annular exterior corrugated polypropylene (PP) pipe meeting the requirements of ASTM F2736 for respective diameters. The pipe supplied shall be watertight as defined in the joint performance requirements of this specification.

Thirty- through 60-inch (750 through 1500 mm) pipe shall be polypropylene or pre-approved equal. Pipe supplied shall be with annular inner corrugations and smooth interior and exterior surfaces, corrugated polypropylene (PP) pipe meeting the requirements of ASTM F2764 for respective diameters. The pipe supplied shall be watertight as defined in the joint performance requirements of this specification.

B. Virgin material for 12- through 60-inch pipe and fitting production shall be an impact modified copolymer meeting the material requirements of ASTM F2736 and ASTM F2764 for respective pipe diameters.

2.1.2 Joint Performance and Pipe Stiffness

Watertight joints shall be bell-and-spigot meeting the watertight requirements of ASTM F2736 and ASTM F2764. Gaskets shall be made of polyisoprene meeting the requirements of ASTM F477. Gaskets shall be installed by the pipe manufacturer and covered with a removable wrap to ensure the gasket is free from debris. A joint lubricant supplied by the manufacturer shall be used on the gasket and bell during assembly.

Pipe stiffness - Minimum pipe stiffness at 5% deflection shall meet the requirements per ASTM F2736 and ASTM F2764.

2.1.3 Fittings

A. Fittings shall conform to ASTM F2736 or ASTM F2764. Joint shall meet watertight joint performance requirements of ASTM D3212. Bell & spigot connections shall utilize a spun-on, welded or integral bell and spigot with gaskets meeting ASTM F477. Inserta tee type fittings shall be allowed upon approval by the Engineer.

B. Repair couplers may be utilized to connect field-cut pipe.

2.1.4 Installation

Pipe installation shall be in accordance with Section 3 of this specification and the product manufacturer's published installation guides.

PART 3 — EXECUTION

3.1 Earthwork

A. Excavation, trenching, and backfilling shall be as specified in Section titled "Excavation and Fill."

3.2 Identification

A. For all stormwater and subsurface drainage piping, install warning tape directly over pipe and at outside edges of underground structures.

a. Detectable warning tape shall be installed over nonferrous piping and over edges of underground structures.

3.3 Piping Inspection

3.3.1 General

A. Piping, fittings, and drainage structures shall be inspected prior to installation and any defective or damaged product shall be replaced.

3.3.2 Corrugated Polypropylene Pipe and Fittings

A. Any pipe, fittings, or drainage structures with cuts, punctures, or other damage on the interior or exterior shall be rejected and replaced.
 B. Any pipe, fittings or drainage structures with damaged ends or joints, which would prevent proper sealing of the joints, shall be rejected and replaced.

3.4 Piping, Fitting, and Drainage Structure Installation

3.4.1 General

A. General Locations and Arrangements: Drawing plans and details indicate general location and arrangement of underground storm and drainage piping system. Location and arrangement of piping layout take design considerations into account. Install piping system as indicated herein and as directed by the product manufacturer, to extent practical. Where specific installation procedure is not indicated, follow product manufacturer's written instructions.

B. All products shall be inspected for defects and cracks before being lowered into the trench, piece by piece. Any defective, damaged or unsound pipe, fitting or drainage structure or any product that has had its grade disturbed after laying, shall be taken up and replaced. Open ends shall be protected with a pipe plug to prevent earth or other material from entering the pipe during construction. The interior of the pipe shall be free from dirt, excess water and other foreign materials as the pipe laying progresses and left clean at the completion of the installation.

C. Install piping system beginning at low point, true to grades and alignment indicated with unbroken continuity of invert. Place bell ends of piping facing upstream. Install gaskets, seals, sleeves, and couplings according to manufacturer's written instructions. Follow product manufacturer's instructions for the use of lubricants, cements, and other special installation requirements.

D. Use Manholes or Catch Basins for changes in direction, unless fittings are indicated. Use fittings for branch connections, unless direct tap into existing sewer is indicated.

E. Use proper size increasers, reducers, and couplings where different sizes or materials of pipes and fittings are connected. Reducing size of piping in direction of flow is prohibited.

3.4.2 Trench Excavation

3.4.2.1 Excavation

A. Excavate trenches to ensure that sides will be stable under all working conditions. Slope trench walls or provide supports in conformance with all local and national standards for safety. Open only as much trench as can be safely maintained by available equipment. Backfill all trenches as soon as practicable.

B. Where trench walls are stable or supported, provide a width sufficient, but no greater than necessary, to ensure working room to properly and safely place and compact haunching and other embedment materials. The space between the pipe and trench wall must be wider than the compaction equipment used in the pipe zone. Minimum width shall be not less than the greater of either the pipe outside diameter plus 16 in. or the pipe outside diameter times 1.25, plus 12 in. In addition to safety considerations, trench width in unsupported, unstable soils will depend on the size and stiffness of the pipe, stiffness of the embedment and in-situ soil, and depth of cover.

C. When supports such as trench sheeting, trench jacks, trench shields or boxes are used, ensure that support of the pipe and its embedment is maintained throughout installation. Ensure that sheeting is sufficiently tight to prevent washing out of the trench wall from behind the sheeting. Provide tight support of trench walls below viaducts, existing utilities, or other obstructions that restrict driving of sheeting.

3.4.2.2 Dewatering

A. Do not lay or embed pipe fittings or drainage structures in standing or running water. At all times prevent runoff and surface water from entering the trench.

B. When water is present in the work area, dewater to maintain stability of in-situ and imported materials. Maintain water level below pipe bedding and foundation to provide a stable trench bottom. Use, as appropriate, sump pumps, well points, deep wells, geofabrics, perforated underdrains, or stone blankets of sufficient thickness to remove and control water in the trench. When excavating while depressing ground water, ensure the ground water is below the bottom of cut at all times to prevent washout from behind sheeting or sloughing of exposed trench walls. Maintain control of water in the trench before, during, and after pipe system installation and until embedment is installed and sufficient backfill has been placed to prevent flotation of the pipe, fitting, or drainage structures. To preclude loss of soil support, employ dewatering methods

that minimize removal of fines and the creation of voids in in-situ materials.

3.4.2.3 Removal of Rock

A. Rock in either ledge or boulder formation shall be replaced with suitable materials to provide a compacted earth cushion having a thickness between exposed rock and the pipe of at least 12 inches (0.3m). Where Bell-and-Spigot pipe is used, the cushion shall be maintained under the bell as well as under the straight portion of the pipe.

3.4.2.4 Removal of Unstable Material

A. Where wet or otherwise unstable soil incapable of properly supporting the pipe system, as determined by the Engineer, is encountered in the bottom of a trench, such material shall be removed to at least 24 inches below bottom of pipe and replaced to the proper grade with select granular material, compacted as directed by the engineer. When removal of unstable material is due to the fault or neglect of the Contractor while performing shoring and sheeting, water removal, or other specified requirements, such removal and replacement shall be performed at no additional cost to the Owner.

3.4.3 Bedding

A. A stable and uniform bedding shall be provided for the pipe and any protruding features of its joint and/or fittings. The middle of the bedding, equal to one-third of the pipe outside diameter, shall be loosely placed while the remainder shall be compacted to a minimum of 90% of maximum density per ASTM D1557, or as shown in the plans. Pipe bedding shall be a minimum of 4" - 6" in thickness. The bedding surface for the pipe shall provide a firm foundation of uniform density throughout the entire length of the pipe.

3.4.4 Placing Pipe

A. Each pipe shall be thoroughly examined before being laid; defective or damaged pipe shall not be used. Pipelines shall be laid to the grades and alignment indicated. Proper facilities shall be provided for lowering sections of pipe into trenches. Pipe shall not be laid in water, and the pipe shall not be laid when trench conditions or weather are unsuitable for such work. Diversion of drainage or dewatering of trenches shall be provided as directed by the engineer; see dewatering section.

3.4.5 Jointing

A. Joints shall be constructed as described herein and in accordance with manufacturer's installation instructions.

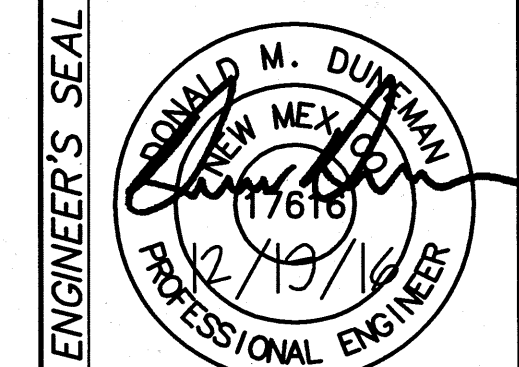
B. All Bell-and-Spigot pipe joints shall be thoroughly cleaned. Joint lubricant, supplied by the manufacturer, shall be liberally applied to entire interior of bell and gasket on spigot prior to assembly.

BENCH MARKS

ACS 1 3" ALUMINUM DISK STAMPED
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 ESE OF THE CENTER OF THE
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 ROCK IN OUTCROPPING
 ELEVATION = 5270.09 FT.
 NAVD 88 U.S. FEET

SURVEY INFORMATION

NO.	FIELD NOTES	
	BY	DATE
	AERIAL MAPPING, WCI	05/01
	BOUNDARY, WCI	06/01
	BOUNDARY, WCI	10/01



NO.	DATE	REMARKS	BY	
			DESIGNED BY	CHECKED BY
			WILSON & COMPANY, ENGINEERS & ARCHITECTS	
			KIS	DATE: JUNE 2016
			ALL	DATE: JUNE 2016
			DSA	DATE: JUNE 2016

<p>2600 THE AMERICAN ROAD S.E. SUITE 100 RIO RANCHO, NEW MEXICO 87124 (505) 898-8021</p>	CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT ENGINEERING GROUP	
	LA CUENTISTA SUBDIVISION UNIT II — OFFSITES HIGH PERFORMANCE POLYPROPYLENE PIPE DETAILS	
Design Review Committee 	City Engineer Approval 	Mo./Day/Yr. _____
City Project No. 709786	Zone Map No. C-10, D-10 C-11, D-11	Drawing 18
		Sheet 20

3.4.6 Backfilling

3.4.6.1 General

Backfill placement and compaction shall be constructed in accordance with the specifications herein and the product manufacturer's published installation guides.

3.4.6.2 Backfilling Pipe in Trenches

After the pipe and pipe system have been properly bedded, selected material from excavation or borrow, at a moisture content that will facilitate compaction, shall be placed along both sides of pipe in layer depths to ensure minimum compaction density is obtained evenly throughout the backfill material. The backfill shall be brought up evenly on both sides of pipe and pipe system for the full length of pipe. The fill shall be thoroughly compacted under the haunches of the pipe. Each layer shall be thoroughly compacted with mechanical tampers or rammers. This method of filling and compacting shall continue until the fill has reached the top of the pipe. The remainder of the trench shall be backfilled and compacted by spreading and rolling or compacted by mechanical rammers or tampers. Tests for density shall be made as necessary to ensure conformance to the compaction requirements specified below.

3.4.6.3 Backfilling Pipe in Fill Sections

For pipe placed in fill sections, fill shall be constructed to at least 6 inches above the top of proposed pipe prior to trench excavation. Fill shall be placed in 12 inch lifts and shall be compacted to achieve 90% of maximum density, or as shown on plans. Once fill is placed and compacted pipe trench shall be constructed in accordance with the Trench Excavation section of this specification.

3.4.6.4 Movement of Construction Machinery

When compacting by rolling or operating heavy equipment parallel with the pipe, displacement or injury to the pipe shall be avoided. Movement of construction machinery over a culvert or storm drain at any stage of construction shall be at the Contractor's risk. Any damaged pipe shall be repaired or replaced.

3.4.6.5 Compaction

3.4.6.5.1 General Requirements

Non-cohesive materials include gravels, gravel-sand mixtures, sands, and gravelly sands. Cohesive materials include clayey and silty gravels, gravel-silt mixtures, clayey and silty sands, sand-clay mixtures, silts, and very fine sands. When results of compaction tests for moisture-density relations are recorded on graphs, non-cohesive soils will show straight lines or reverse-shaped moisture-density curves, and cohesive soils will show normal moisture-density curves.

3.4.6.5.2 Minimum Density

Backfill over and around the pipe and backfill around and adjacent to drainage structures shall be compacted at the approved moisture content to the following applicable minimum density, which will be determined as specified below.

- a. Under airfield and heliport pavements, paved roads, streets, parking areas, and similar-use pavements including adjacent shoulder areas, the density shall be not less than 90 percent of maximum density as determined by ASTM D 1557, up to the elevation where requirements for pavement subgrade materials and compaction shall control.

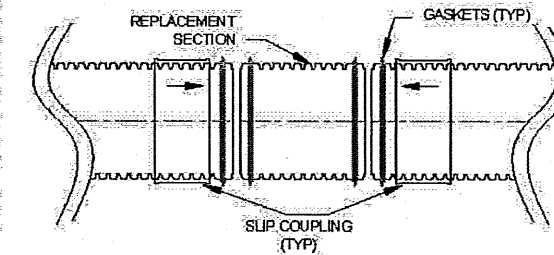
3.4.6.6 Determination of Density

Testing shall be performed by an approved commercial testing laboratory or by the Contractor subject to approval by the Engineer. Tests shall be performed in sufficient number to ensure that specified density is being obtained. Laboratory tests for moisture-density relations shall be made in accordance with ASTM D1557 except that mechanical tampers shall be used provided the results are correlated with those obtained with the specified hand tamper. Field density tests shall be determined in accordance with ASTM D2167 or ASTM D2922. Test results shall be furnished to the Engineer.

Repair Methods

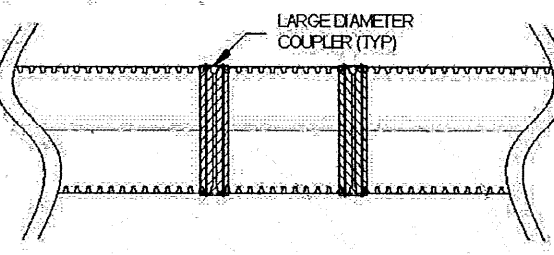
3.5.1 External Methods

Slip Couplings 12- through 30-inch (300 – 750 mm), provides a watertight repair that will meet most pressure testing requirements, when installed correctly. The slip coupling uses PVC bells with gaskets. The gaskets are placed in the valleys on either side of the section to be repaired and slip couplings are then slid over the gaskets. Due to the exterior gasket, the slip coupling can only be used on pipe with a corrugated exterior. PVC slip couplings are most commonly used with watertight smooth interior thermoplastic pipe products. *Note: This repair method cannot be used with the triple wall, smooth exterior profile pipe.*



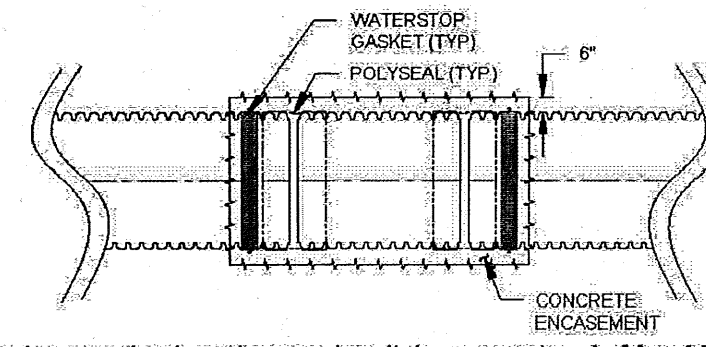
Large Diameter Repair Coupler 12- through 60-inch (300 – 1500 mm) are ideal for repairs and alterations of large diameter sewer pipe. Repair couplers similar to those provided by Mission Rubber Company LLC, Fernco® or equal may be used on HP pipe.

The couplers are used by removing the damaged section of pipe, replacing it with a new section and then sliding the coupler back around the



joint, similar to the slip coupling above. The couplers stainless steel bands are then tightened to the manufacturer's recommendations. These rubber couplings are capable of meeting watertight field test requirements when installed per the manufacturer's recommendations.

Concrete Collar 12- through 60-inch (300 – 1500 mm), provides a water tight repair testable to most hydrostatic test with an appropriate leakage requirement. Installing a concrete collar involves building a form around the area to be repaired and encasing it in concrete. A *Mar Mac Polyseal Pipe Coupler* is wrapped around the repair area or joint prior to pouring the collar to keep the concrete from seeping into the pipe. *WaterStop* gaskets are installed outside of the *Polyseal* coupler towards the outside edge of the concrete collar. Typically, approximately 6" (0.15m) is excavated beneath the pipe to allow for proper application of the *Polyseal* coupler and concrete encasement. If the pipe itself is damaged, the damaged area shall be removed and a replacement pipe section spliced in prior to pouring the collar. This repair option may be employed for either dual wall or triple wall sanitary pipe.



3.5.2 Internal Methods

Internal mechanical repair products generally consist of a flexible cylindrical gasket sleeve, which is expanded to conform to the inner wall of the pipe. The feasibility of this repair method depends on the size of the damaged section or joint and available access into the pipe. Internal mechanical seals slightly restrict the inside diameter of the pipe. This should be considered when assessing the risk of debris obstruction.

NPC Internal Joint Seal, 18- through 60-inch (450 – 1500 mm), consists of an EPDM rubber seal and stainless steel bands. The rubber seal is inserted into the pipe and positioned over the joint. A torque wrench is used to expand the bands against the inner wall of the pipe. The Internal Joint Seal is designed to seal joints – not repair damaged pipe sections. The damaged area of the pipe must be removed and a replacement section spliced in if necessary in order to use the Internal Joint Seal. This system may provide a watertight joint when installed as recommended. The manufacturer should be contacted to verify the product meets the specific application requirements including test requirements, if specified. If pressure tests are required, NPC should be contacted to ensure that the product is suitable for the specific test criteria.

Welding, 36- through 60-inch (900 – 1500 mm), is another method of internal joint repair where personnel use hand-held welding guns to make the needed repair. Extrusion welding techniques are most commonly utilized; however other welding methods may be used depending on the condition of the damage. Clean and dry working conditions and skilled operators are critical to a successful repair. Contact ADS to discuss the type of damage and to assess if a welded repair will be suitable.

Link Pipe Grouting Sleeve™, 12- through 60-inch (100 – 1500 mm), is a stainless steel grouting sleeve that is installed with an inflatable plug. The sleeve may be used to seal a joint or repair short sections of damaged pipe. The manufacturer should be contacted to verify the product meets the specific application requirements including test requirements, if specified.

Internal chemical sealing is another method of internal joint repair using chemically activated gel or grout to minimize joint leakage. The grout is typically applied with specialized remote-controlled equipment. *Test/Seal* packer is used to remotely seal a joint. The grouting chemicals are forced through the joint out into the surrounding soil where they gel with the soil. The gelled mass forms a waterproof collar around the pipe. The result is significantly reduced leakage. There are several types of chemical grouts available and the manufacturer should be contacted to review the specific situation and any joint tightness or pressure test criteria. Companies such as Avanti International, Strata Tech Inc., and Caryl Corporation manufacture and/or install chemical grout. Stephen's Technologies *New Life Coatings* and *NewLife Liner Systems* as well as Avast Hydro-Lining International, are examples of companies that offer cured in place epoxy lining systems that have been effectively used with HDPE pipe. Most pipe diameters can be chemically grouted provided the grouting contractor has the appropriate equipment.

BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL	
ACS 1 3/4" ALUMINUM DISK STAMPED	DATE	NO.	BY		WILSON & COMPANY, ENGINEERS & ARCHITECTS DESIGNED BY: KIS DRAWN BY: ALL CHECKED BY: DSA
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CITY OF ALBUQUERQUE
 PUBLIC WORKS DEPARTMENT
 ENGINEERING GROUP

**LA CUENTISTA SUBDIVISION UNIT II – OFFSITES
 HIGH PERFORMANCE POLYPROPYLENE PIPE DETAILS**

City Project No. **709786**
 Zone Map No. C-10, D-10
 C-11, D-11

Design Review Committee

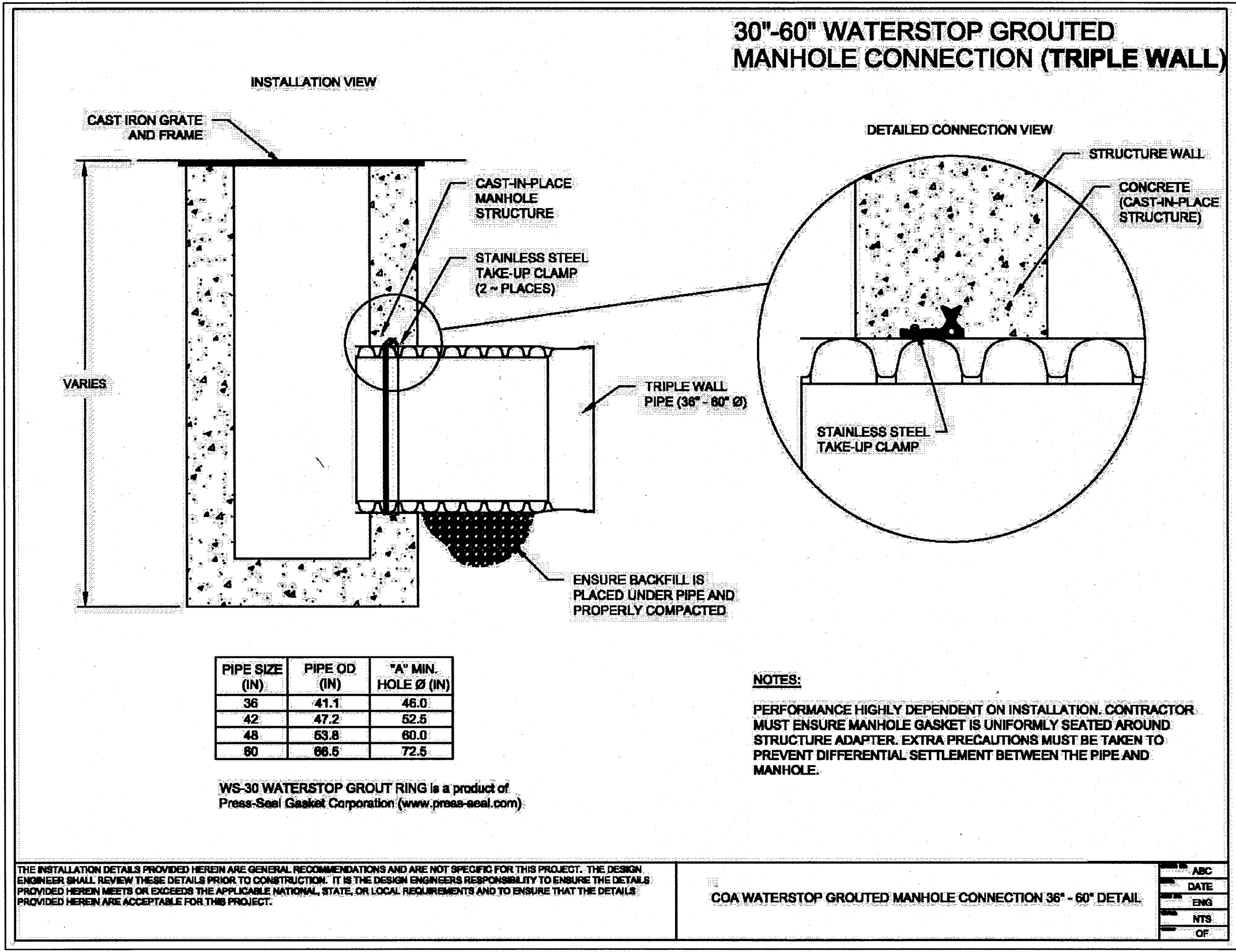
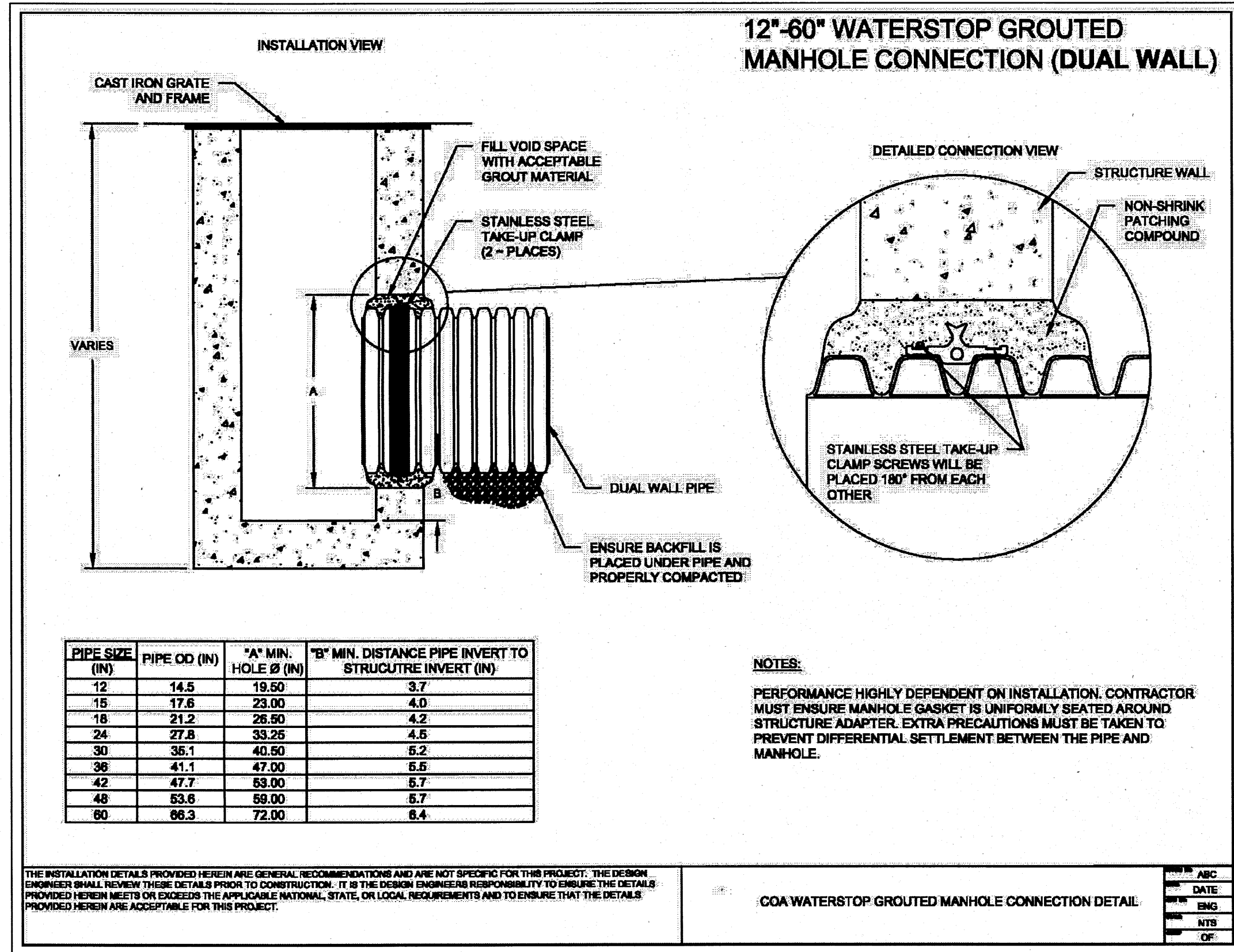
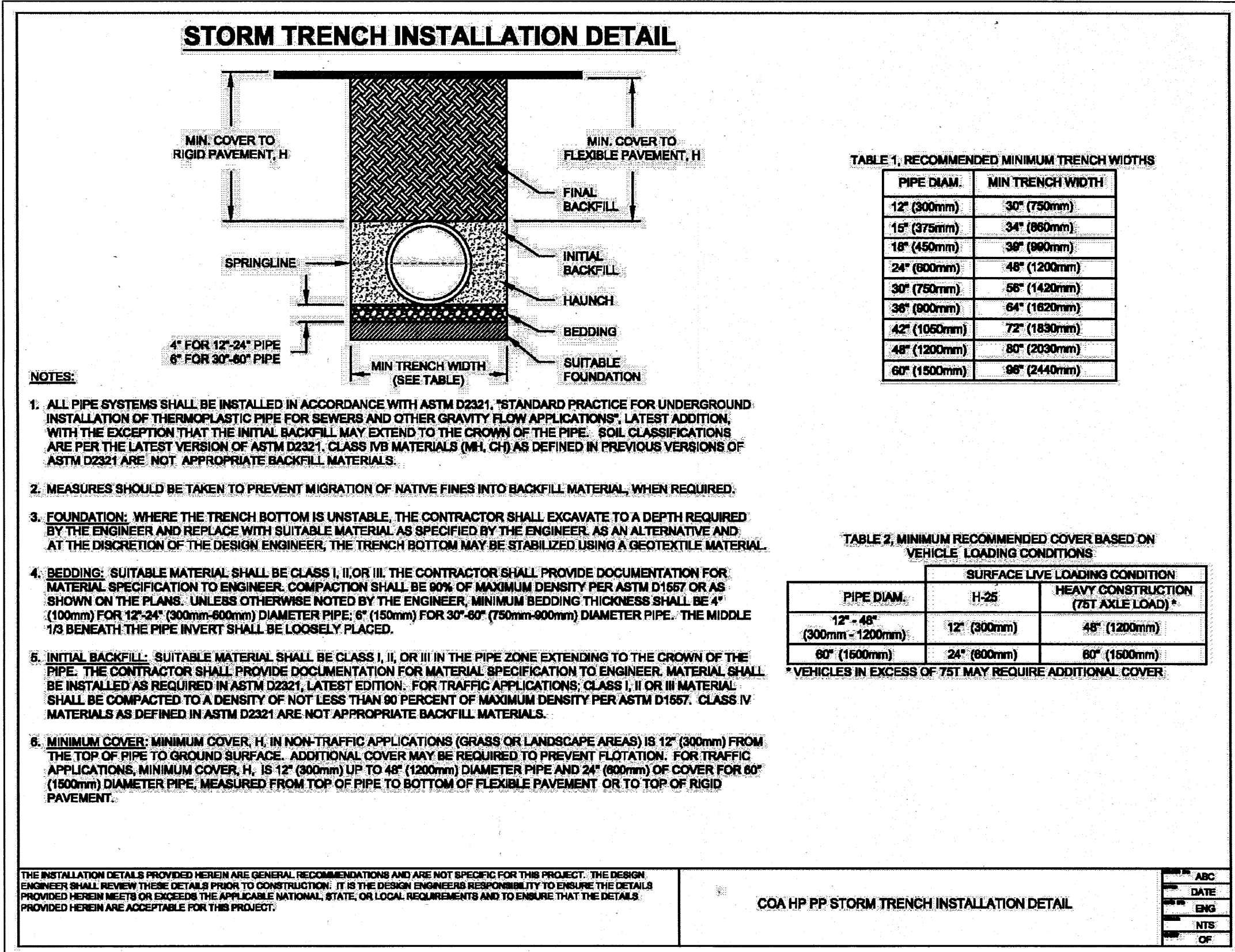
City Engineer Approval

Last Design Update

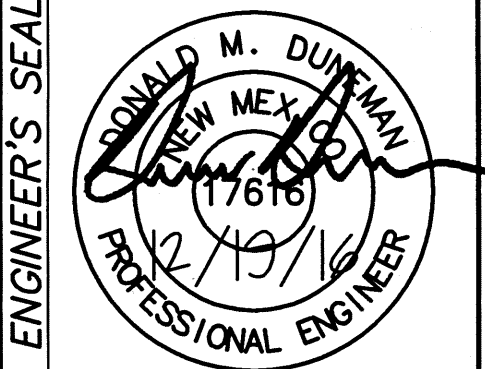
City Project No. **709786**

Zone Map No. C-10, D-10
 C-11, D-11

Drawing **19** Sheet **20**



BENCH MARKS		SURVEY INFORMATION		ENGINEER'S SEAL	
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		DATE	DATE JUNE 2016	DRAWN BY	ALL
		BOUNDARY	DATE JUNE 2016	CHECKED BY	DSA
		BOUNDARY	DATE JUNE 2016		



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CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

**LA CUENTISTA SUBDIVISION UNIT II - OFFSITES
HIGH PERFORMANCE POLYPROPYLENE PIPE DETAILS**

City Project No. **709786**

Design Review Committee
MAR 01 2017
DESIGN REVIEW COMMITTEE

City Engineer Approval
MAR 01 2017
CITY ENGINEER

Mo./Day/Yr. _____

Mo./Day/Yr. _____

Zone Map No. C-10, D-10
C-11, D-11

Drawing **20**

Sheet **20**

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